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Preliminary Servicing & Stormwater Management Design Report for:

Ainley Farm Subdivision  
Township of Centre Wellington (Elora)

**GMBP File: 411009**  
**Revised April 2023**

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**PRELIMINARY SERVICING &  
STORMWATER MANAGEMENT REPORT  
AINLEY FARM SUBDIVISION  
TOWNSHIP OF CENTRE WELLINGTON (ELORA)  
Revised April 2023  
Our File: 411009**

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## **1.0 INTRODUCTION**

In support of the Draft Plan of Subdivision Application for Part of Lots 17 and 18, Concession 12 in the Township of Centre Wellington (Geographic Township of Nichol) herein after referred to as the Ainley Farm Subdivision, GM BluePlan Engineering Limited have prepared this report to address the preliminary servicing and stormwater management requirements for the site and to address the comments received from Grand River Conservation Authority (dated July 31, 2018) and from the Township of Centre Wellington (dated March 20, 2020).

The servicing and stormwater management techniques were derived from the recommendations presented in the following reports:

- Stormwater Management Plan for The North Valley Subdivision (Cambridge Engineering and Planning Consultants Limited, January 1994),
- Design Report, Ville Lora Downs North Subdivision, Phase III (Gamsby and Mannerow Limited, July 2004),
- Design Report, Ville Lora Downs Subdivision, Stage VI (Gamsby and Mannerow Limited, April 1998),
- Preliminary Geotechnical Investigation completed CMT Engineering Inc. (March 29, 2006), and
- Environmental Impact Study completed by North-South Environmental Inc. (July 24, 2019).

Together, these reports form the overview for the development of these lands while maintaining the adjacent natural features.

## **2.0 SITE INFORMATION**

Figure 1 shows the location of the Ainley Farm Subdivision and the surrounding area. The 21.46-hectare site is bound by existing agricultural and future development lands to the north, Gerrie Road to the east, existing residential lands to the south (Ville Lora Downs Subdivision, Phase V and Phase VI) and existing wetland and residential lands to the west (Ville Lora Downs North Subdivision, Phase III).

## **3.0 EXISTING CONDITIONS**

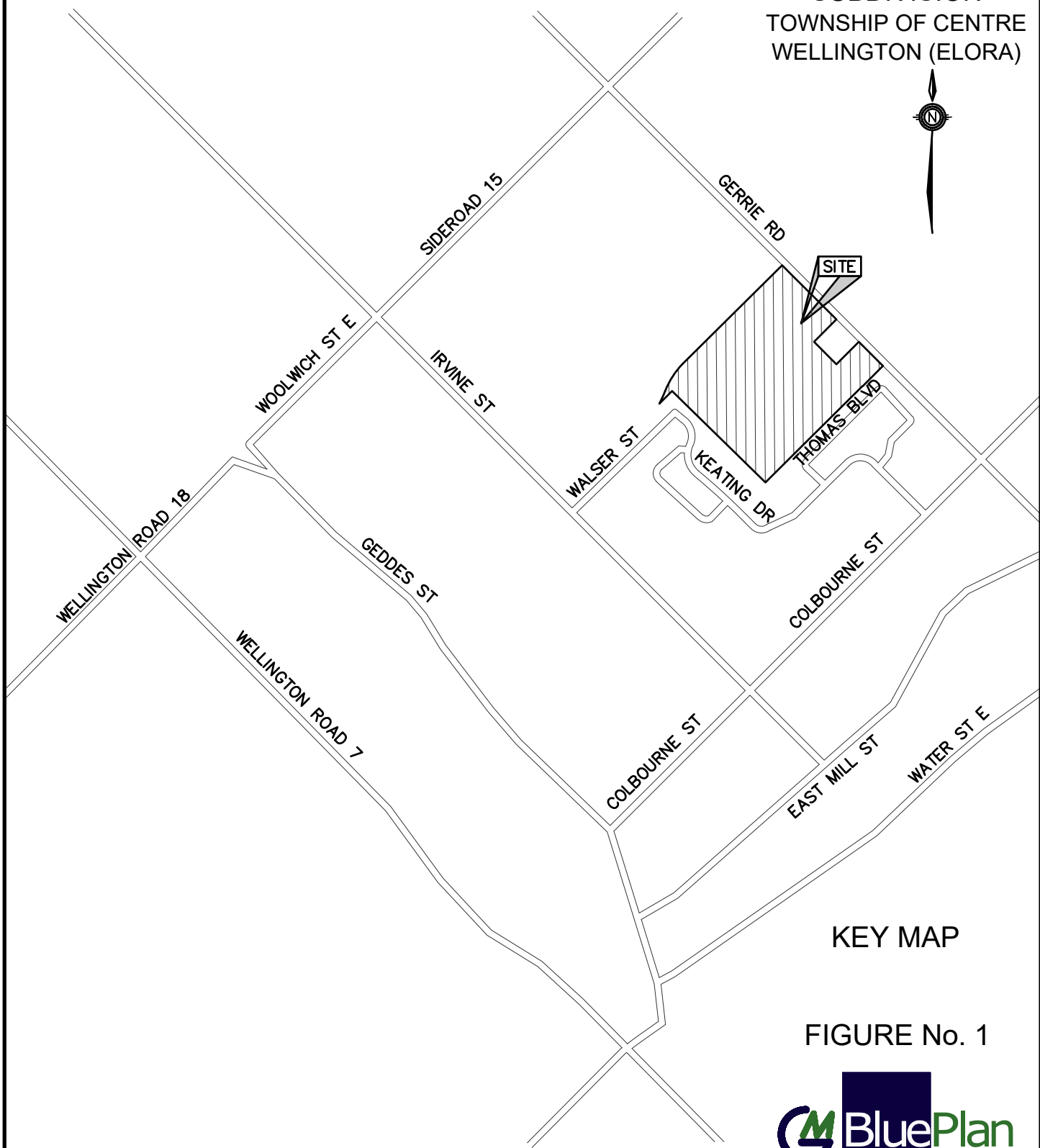
### **3.1 LAND USE**

The site is currently comprised of agricultural fields and a natural heritage feature consisting of a woodlot and wetland. The existing site features are shown on the General Plans (GM BluePlan Engineering Limited Drawing No. 1 to 4).

### **3.2 TOPOGRAPHY**

The topography throughout the Ainley Farm Subdivision is undulating and consists of rolling slopes with gradients ranging from 0.5% to 20%. Original ground elevations on site range from approximately 410.0m to approximately 416.0m. The northeastern portion of the site generally drains in a northeast direction towards Gerrie Road. The remainder of the site generally drains in a southwest direction towards the existing wetland, ultimately discharging to the existing channel located immediately south of the wetland. The northwestern

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KEY MAP

FIGURE No. 1



JOB NUMBER 411009  
FEBRUARY 2019  
NOT TO SCALE



portion of the site, adjacent to the existing Walser Street right-of-way, drains in a southerly direction towards Walser Street.

### **3.3 SOILS**

The predominant surface soil type on the site is Harriston Loam (Soil Survey of Wellington County Report No. 35). Harriston Loam has a hydrologic soil classification of BC and generally has good drainage characteristics.

The Preliminary Geotechnical Investigation by CMT Engineering Inc. (March 2006) established the characteristics of the underlying soils. The boreholes identified the underlying soils as topsoil overlying organic silt, silt or sandy silt, silt till or sandy silt till, sand or silty sand and clayey silt. The results of the geotechnical investigation are included in Appendix 'A'.

### **4.0 PROPOSED DEVELOPMENT**

The Draft Plan of Subdivision, prepared by J.D. Barnes Limited (March 23, 2023) (Figure 2), illustrates the proposed lot fabric, internal roads, park block, and open space areas and stormwater management blocks.

Access to the 21.46-hectare development will be provided via Gerrie Road and the extension of Walser Street.

Within the development, there are 101 single family lots, three (3) on-street townhouse blocks, one (1) apartment block, one (1) cluster townhouse block, one (1) open space block, one (1) park block and two (2) stormwater management blocks.

In addition, four (4) future single detached lots will be created on the north side of Walser Avenue through the extension of Walser Avenue into the Ainley Farm property (Future Development Block 120).

### **4.1 SITE GRADING**

The site layout and internal road network for the Ainley Farm Subdivision are shown on the General Plans (GM BluePlan Engineering Limited Drawing No. 1 to 4). The grade and elevation of the internal streets are controlled by the existing centre line elevations of Walser Street and Gerrie Road, the major overland flow route to the stormwater management facilities, existing groundwater elevations and the elevation of the existing sanitary sewers on Walser Street and Keating Drive.

The site has been graded to match the existing elevations along the property boundary of the adjacent lands. Minor grading on the adjacent lands located along the north boundary of the site is required. The adjacent lands along the north boundary of the site are owned by the Developer (James Keating Construction (2004) Limited).

### **4.2 STREETS**

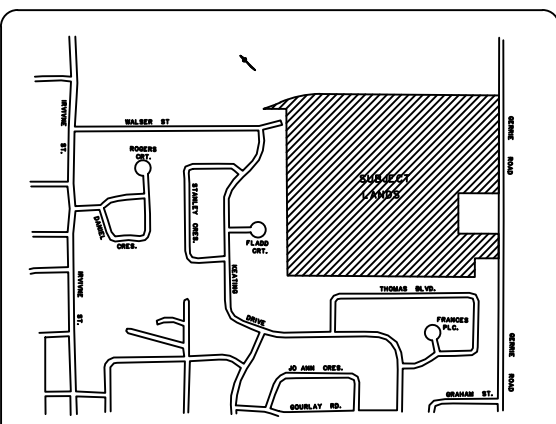
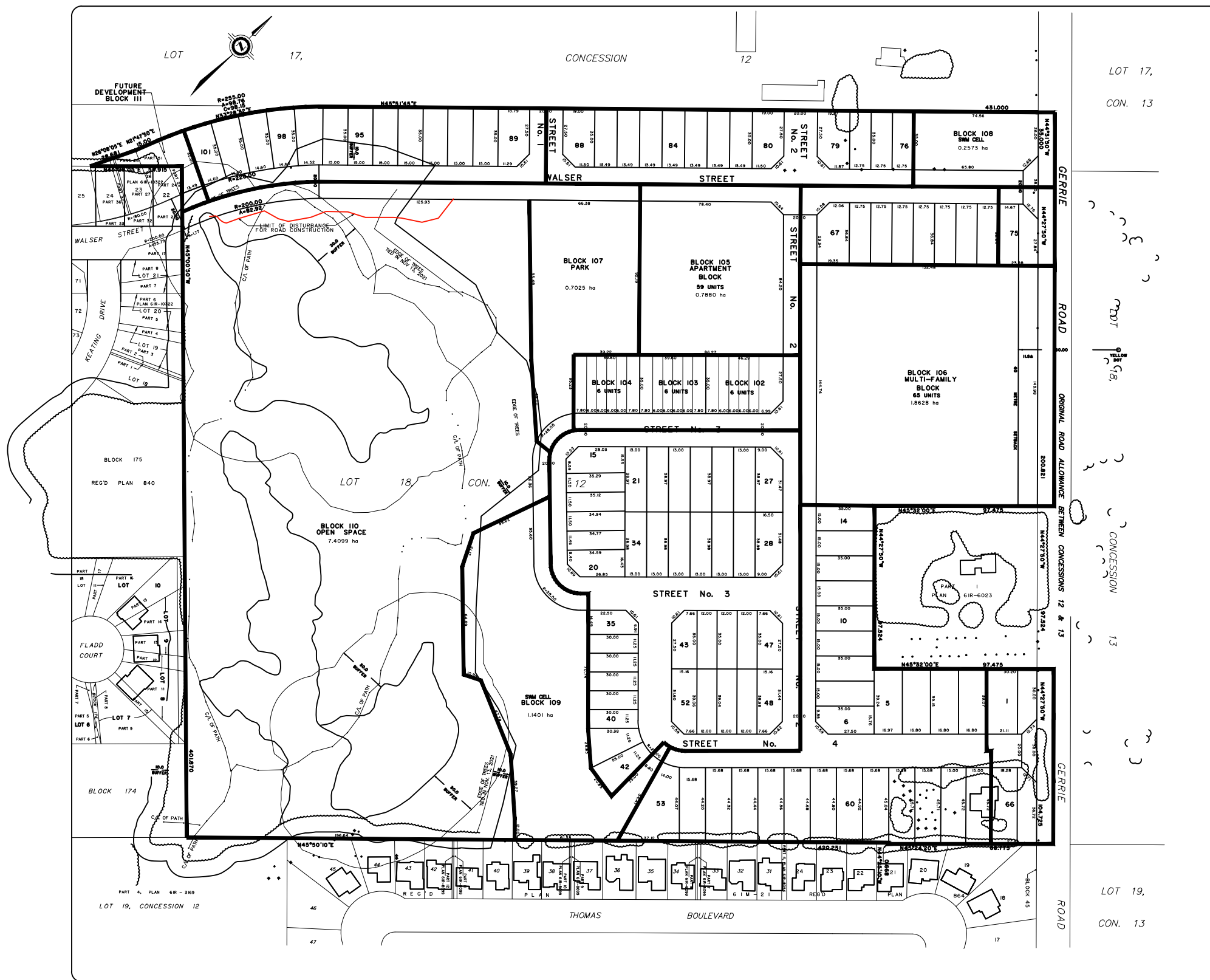
All streets will be constructed with a minimum grade of 0.5% and a maximum grade of 8.0% as per Township of Centre Wellington standards. An urban road cross-section (20 m right-of-way width), with concrete curb and gutter will be provided for Street No.1, 2, 3, 4 and the extension of Walser Street, as per Township of Centre Wellington Standard Drawing STD R1.

Concrete sidewalks (1.5 metre wide) will be constructed along both sides of the Walser Street extension and Street No. 1, 2, 3 and 4.

### **4.3 WATER SUPPLY**

As part of the Ville Lora Downs North Subdivision Phase III, a 200mm diameter watermain was terminated at the easterly limit of Walser Street. There is currently no watermain on Gerrie Road across the frontage of the Ainley Farm Subdivision.

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KEY PLAN n.t.s.

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51 (17) OF THE PLANNING ACT

- 51 (17) (a) - (c) AS SHOWN SEE SCHEDULE
- (d) - (g) AS SHOWN MUNICIPAL WATER SUPPLY HARRISTON LOAM
- (h) AS SHOWN MUNICIPAL STORM AND SANITARY SEWERS
- (i) NONE KNOWN

WE HEREBY AUTHORIZE BLACK, SHOEMAKER, ROBINSON, AND DONALDSON LIMITED, ONTARIO LAND SURVEYORS, URBAN AND RURAL PLANNERS TO SUBMIT THIS DRAFT PLAN OF PROPOSED SUBDIVISION.

DATE: JAMES KEATING CONSTRUCTION (2004) LTD.

I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LANDS TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

DATE: KERRY F. HALLIS ONTARIO LAND SURVEYOR

DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

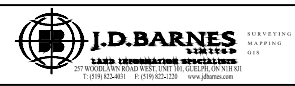
RELEVANT INFORMATION

LOTS/BLOCKS	LAND USE	UNITS	AREAS (ha)
LOTS 1 TO 101	SINGLE-DETACHED RESIDENTIAL	101	5.8078
BLOCKS 102 TO 104	STREET TOWNHOUSES	18	0.4384
BLOCK 105	APARTMENT SITE	59	0.7880
BLOCK 106	CLUSTER TOWNHOUSES	65	1.0626
BLOCK 107	PARK		0.7029
BLOCKS 108 & 109	STORMWATER MANAGEMENT		1.3974
BLOCK 110	OPEN SPACE		7.3820
STREETS	ROADS	4	3.2004
BLOCK III	FUTURE DEVELOPMENT		0.0816
<b>TOTAL</b>		<b>247</b>	<b>21.4603</b>

NOTES:  
ELEVATIONS AND RELEVANT INFORMATION TAKEN FROM

**DRAFT PLAN OF SUBDIVISION**  
OF PART OF  
LOTS 17 & 18, CONCESSION 12  
TOWNSHIP OF CENTRE WELLINGTON  
(GEOGRAPHIC TOWNSHIP OF NICHOL)  
COUNTY OF WELLINGTON

SCALE: 1:1000



DATE: MARCH 23, 2023  
DRAWN BY: KJ, MA  
PROJECT: 04-5865-18  
6-14-086-01

DRAFT PLAN OF SUBDIVISION

FIGURE No. 2



Water supply for the Ainley Farm Subdivision will be provided via the extension of a 200mm diameter watermain, along the Walser Street extension, Street No. 2 and a portion of Street No. 1. A 150mm diameter watermain will also be extended along the remainder of Street No. 1, Street No. 3, and Street No. 4.

#### **4.4 SANITARY SEWER**

During the municipal servicing of the Ville Lora Downs North Subdivision Phase III, a 200mm diameter sanitary sewer was designed, approved and constructed on Walser Street. The existing 200mm diameter sanitary sewer is currently terminated at the easterly limit of Walser Street. As part of the Ville Lora Downs Subdivision, Phase VI, a 200mm diameter sanitary sewer was also designed, approved and constructed on Keating Drive. There are currently no sanitary sewers on Gerrie Road across the frontage of the Ainley Farm.

Sanitary service for the Ainley Farm Subdivision will be provided via connections to both the existing 200mm diameter sanitary sewer on Walser Street and the existing 200mm diameter sanitary sewer on Keating Drive.

The extension of a 200mm diameter sanitary sewer along the Walser Street extension will service the lots fronting on to Walser Street, as well as a portion of the lots fronting onto Street No. 2. The extension of a 200mm diameter sanitary sewer on easement through Drimmie Park to Street No. 1, from the existing 200mm diameter sanitary sewer on Keating Drive, will service the remainder of the subdivision (Street No. 2, Street No. 3 and Street No. 4).

Sanitary sewer design sheets have been included in Appendix C.

#### **4.5 STORM SEWER**

The storm sewer system for the Ainley Farm Subdivision will be sized to convey the 5-year design storm event and the storm sewer system will discharge to the two (2) proposed stormwater management facilities or to the existing storm sewer on Walser Street.

The storm sewers on Street No. 1, Street No. 3, Street No. 4, a portion of Street No. 2 and a portion of the Walser Street extension will discharge to the proposed Stormwater Management Facility No. 1 located to the east of the existing wetland.

The storm sewers on the remainder of Street No. 2, along with a portion of the Walser Street extension, will discharge to the proposed Stormwater Management Facility No. 2 located to the west of Gerrie Road.

The storm sewers on the remainder of the Walser Street extension will discharge directly to the existing storm sewer system on Walser Street, ultimately discharging to the existing storm sewers on Keating Drive.

Storm sewer design sheets have been included in Appendix C.

#### **4.6 DEWATERING**

Dewatering may be required during the installation of sanitary sewer, storm sewer and watermain. A Permit to Take Water (PTTW) or an Environmental Activity and Sector Registry (EASR) from the Ministry of Environment, Conservation and Parks (MECP) will be required if dewatering activities will involve the removal of more than 400,000 litres of groundwater per day from the site.

If dewatering activities are required during the installation of sewers and watermain, all discharge will be directed to the interim sediment control pond prior to discharge from the site.

As part of the area grading of the site, the interim stormwater management facility will be constructed and will act as an interim sedimentation control pond for the remainder of the municipal servicing and home building construction. This will prevent sediment from being discharged to the wetland. Upon build-out, accumulated

sediment will be collected and removed from the interim sediment control pond before it is constructed on Stormwater Management Facility No. 1.

#### 4.7 FOUNDATION DRAINAGE

As per the Township of Centre Wellington municipal standards, foundation drainage will be provided via sump pits and sump pumps in each residential unit, ultimately discharging via individual storm sewer lateral connections to the storm sewer system located within the municipal right-of-way.

#### 5.0 STORMWATER MANAGEMENT

##### 5.1 DESIGN CRITERIA

The studies, policies and guidelines used to develop the stormwater management plan for this development were as follows:

- 1) Stormwater Management Planning and Design Manual, 2003
- 2) Design Principles for Stormwater Management Facilities, 1996
- 3) The Interim Stormwater Quality Control Guidelines, 1991
- 4) The Stormwater Quality Best Management Practices Manual, 1991
- 5) The MTO Drainage Management Technical Guidelines, 1989
- 6) The Ontario Urban Design Guidelines, 1987

The objectives of the stormwater management plan are as follows:

- a) Provide Enhanced (80% Total Suspended Solids) water quality control prior to discharge to the existing wetland and to an existing tributary of the Grand River.
- b) Provide quantity control for the full range of design storms to attenuate post-development runoff to the existing condition level.
- c) Match pre- and post-development infiltration rates.
- d) Route the Regional Storm to minimize flood damage.

A three-hour duration rainfall event was used to generate the mass rainfall data required for the 2, 5, 10, 25, 50 and 100-year design storms. The Fergus Shand Dam Chicago parameters and the total depth of rainfall for each storm are as follows:

**Table No. 1: Chicago Rainfall Distribution Parameters**

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
a =	695.047	1459.072	2327.596	3701.648	5089.418	6933.019
b =	6.387	13.690	19.500	25.500	30.000	34.699
c =	0.793	0.850	0.894	0.937	0.967	0.998
r =	0.38	0.38	0.38	0.38	0.38	0.38
Duration = (minutes)	180	180	180	180	180	180
Rainfall Depth = (mm)	33.014	49.792	61.359	75.581	86.737	97.921

The SCS infiltration method was used in the runoff calculations. The CN parameters used in the MIDUSS modelling are as follows:

**Table No. 2: SCS Curve Number Parameters**

	IMPERVIOUS AREAS	PERVIOUS AREAS
Residential	98	78
Agricultural	98	74
Wetland/Forest	98	50

The hydrologic model MIDUSS was used to create the runoff hydrographs and to route the flows through the storage structures.

## 5.2 STORMWATER MANAGEMENT APPROACH

In line with current practices and guidelines, the stormwater management plan for the Ainley Farm Subdivision is a “treatment train” to attenuate post-development flows and to provide Enhanced (80% total suspended solids removal) water quality control treatment prior to discharge from the site. The “treatment train” will include a combination of lot level, conveyance and end-of-pipe best management practices.

Lot level controls will simply consist of directing roof leaders to grassed areas and grassed swales.

Conveyance controls will include the use of storm sewers, grassed swales, four (4) oil/grit separator structures for Stormwater Management Facility No.1 and Stormwater Management Facility No.2.

End-of-pipe controls will be provided by two (2) extended detention stormwater management facilities designed to attenuate post-development runoff prior to discharge from the site. Runoff generated from Stormwater Management Facility No.1 will discharge to the existing wetland, ultimately discharging to the existing swale in Drimmie Park and the existing storm sewers on Keating Drive. The stormwater management facility has been designed as a wetland with 5,464m<sup>3</sup> of storage, discharging via a multi-stage outlet consisting of a minor outlet with a 300mm diameter orifice plate and a major outlet with a 350mm diameter orifice place, as well as a 20m wide overflow weir.

Runoff generated from Stormwater Management Facility No.2 will discharge to the roadside ditch along Gerrie Road, ultimately discharging to a tributary of the Grand River. The stormwater management facility has been designed as a wetland with 1,195m<sup>3</sup> of storage, discharging via a multi-stage outlet consisting of a 120mm knockout for minor storms and a 260mm orifice plate for major storms, as well as a 10m wide overflow weir.

A small portion of runoff from the westerly portion of Walser Street will discharge uncontrolled to the existing storm sewer system on Walser Street.

Major storm flows from the development will sheetflow overland via the municipal right-of-ways to either Stormwater Management Facility No.1 or Stormwater Management Facility No. 2.

This combination of lot-level, conveyance and end-of-pipe controls will control the release of the runoff from the site.

### **5.3 STORMWATER MANAGEMENT PLAN**

The best management practices (BMP's) in the Stormwater Management Planning and Design Manual (2003) were screened. Those found to be applicable to this development are discussed in the following sections.

#### **5.3.1 LOT LEVEL CONTROLS**

Stormwater management practices recommended to provide lot level control on this site are as follows:

##### **a) Roof Drainage to Ground Surface**

The driveways and front yards will drain to the street. The roof and rear yard will generally drain to the rear of the lot with exception for lots with back to front drainage.

The roof runoff will be filtered across the grassed surface and some will infiltrate. The runoff for any event large enough to generate flow to the swale system will be adequately filtered by the grass en route.

##### **b) Rear Yard Swales**

The lots will be graded to current Township of Centre Wellington Standards. Where practical, the length of the rear lot swales between catch basins will be increased to extend the contact time with the grassed surfaces.

To promote infiltration on the lots and in the swales, it is recommended that the average depth of graded topsoil be 300 mm.

#### **5.3.2 CONVEYANCE CONTROLS**

The storm conveyance system for the development will consist of grassed swales, storm sewers, major overland channel and four (4) oil/grit separator structures. Conveyance controls will be achieved through the regular maintenance of the grassed swales, storm sewers, major overland channel, and oil/grit separator structures as part of the Township's annual maintenance program. Maintenance requirements will include the annual removal of accumulated sediments and debris from manholes, catch basins, and oil/grit separator structures.



### 5.3.3 END-OF-PIPE CONTROLS

#### a) Existing Conditions

Under existing conditions, the majority of the site is utilized for agricultural purposes. For hydrologic modelling purposes, the 21.42-hectare site and 1.20 hectares of external areas was modelled as seven (7) catchments. These catchments are shown on the Existing Conditions Storm Drainage Area Plan (Figure 3).

**Catchment 10 (7.66 hectares, 0% impervious)** consists primarily of agricultural lands and an existing residential lot.

**Catchment 11 (0.13 hectares, 0% impervious)** represents the external lands, which consists primarily of agricultural lands of an existing residential lot.

Runoff generated from Catchment 10 and 11 currently sheetflows overland in an east to west direction, ultimately discharging to the existing wetland.

**Catchment 20 (6.66 hectares, 0% impervious)** consists primarily of agricultural lands and an existing residential lot.

**Catchment 21 (0.83 hectares, 10% impervious)** represents external lands consisting of an existing residential dwelling.

Runoff generated from Catchment 20 and 21 currently sheetflows overland to the existing roadside ditch along Gerrie Road and ultimately to a tributary of the Grand River.

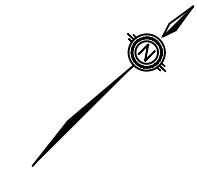
**Catchment 30 (0.24 hectares, 0% impervious)** represents the external lands, which consists primarily of an existing wetland and agricultural lands.

Runoff generated from Catchment 30 currently sheetflows overland, ultimately discharging to the existing wetland.


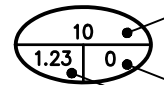
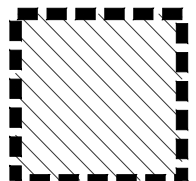
**Catchment 40 (7.12 hectares, 0% impervious)** represents the south-westerly portion of the site, consisting of a natural heritage feature (wetland and woodlot).

Runoff generated from Catchment 40 currently sheetflows overland in an east to west direction, discharging to an existing swale in Drimmie Park and ultimately the existing storm sewer system on Keating Drive.

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LEGEND

-  DRAINAGE AREA BOUNDARY
-  CATCHMENT NUMBER  
% IMPERVIOUS AREA IN HECTARES
-  EXTERNAL LANDS

EXISTING  
 CONDITIONS STORM  
 DRAINAGE AREA  
 PLAN

FIGURE No. 3



JOB NUMBER 411009  
 FEBRUARY 2019  
 Scale: 1:2500

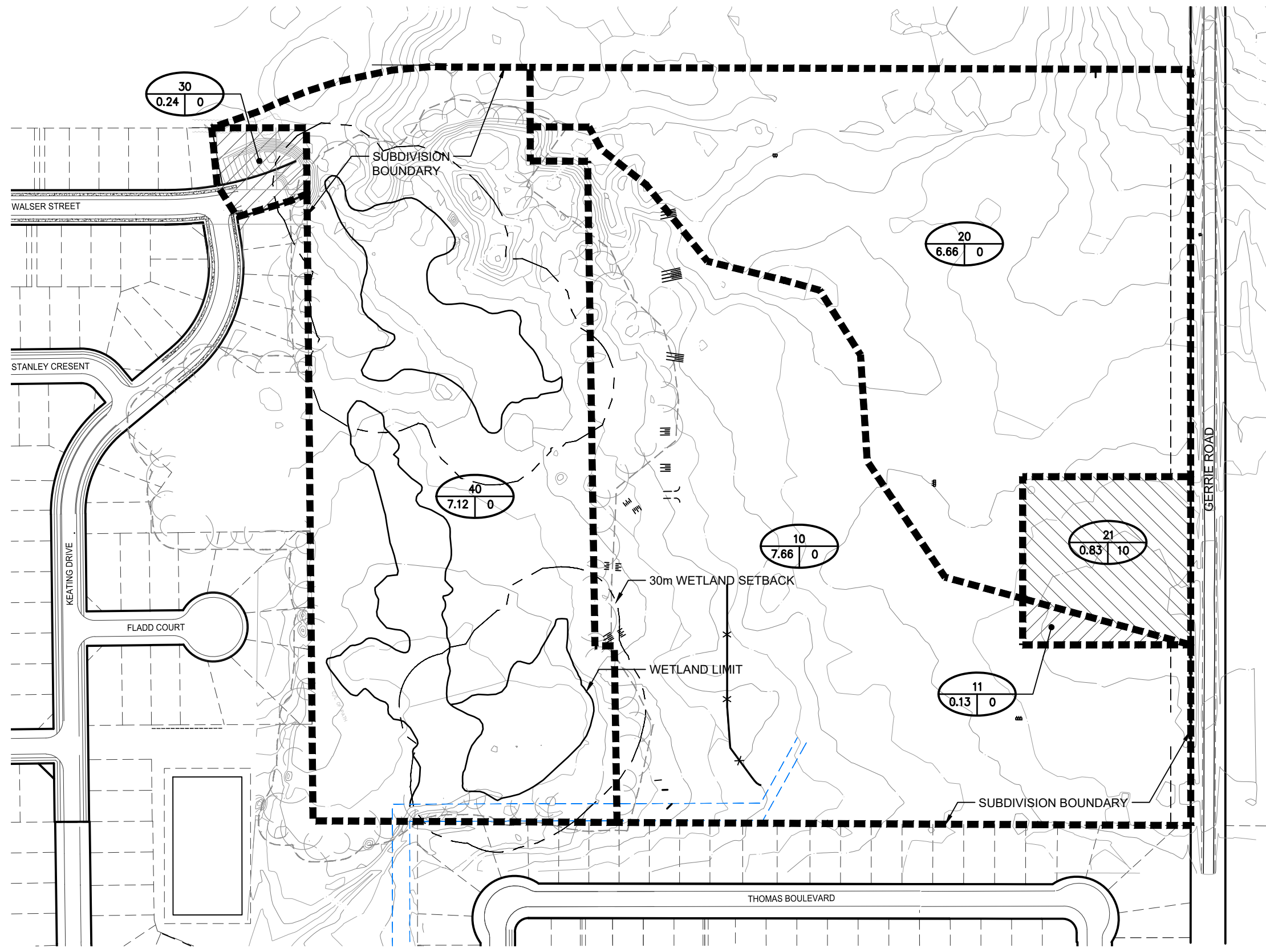




Table No. 3 summarizes the existing condition flow rates and runoff volumes from the site for the full range of design storm events.

**Table No. 3: Existing Condition Flow Rates and Runoff Volumes**

	CATCHMENTS									
	30	Total to Walsler	10	11	40	Total to Ex. Wetland	20	21	Total to Tributary of Grand River	Total from Site
<b>2-Year</b>										
Flow Rate (m <sup>3</sup> /s)	0.003	0.003	0.044	0.001	0.060	0.099	0.038	0.016	0.043	<b>0.138</b>
Runoff Volume (m <sup>3</sup> )	12.3	12.3	397.3	6.7	364.5	768.4	340.5	61.1	401.6	<b>1,170.0</b>
<b>5-Year</b>										
Flow Rate (m <sup>3</sup> /s)	0.010	0.010	0.157	0.004	0.209	0.353	0.135	0.030	0.153	<b>0.472</b>
Runoff Volume (m <sup>3</sup> )	30.7	30.7	995.9	16.7	913.0	1,925.6	853.4	132.1	985.5	<b>2,911.1</b>
<b>10-Year</b>										
Flow Rate (m <sup>3</sup> /s)	0.017	0.017	0.273	0.008	0.359	0.608	0.234	0.050	0.264	<b>0.806</b>
Runoff Volume (m <sup>3</sup> )	46.5	46.5	1,505.1	25.2	1,380.7	2,911.0	1,289.8	190.6	1,480.4	<b>4,391.4</b>
<b>25-Year</b>										
Flow Rate (m <sup>3</sup> /s)	0.026	0.026	0.454	0.012	0.584	1.001	0.389	0.079	0.436	<b>1.308</b>
Runoff Volume (m <sup>3</sup> )	68.1	68.1	2,210.7	37.0	2,027.3	4,274.9	1,894.4	269.8	2,164.2	<b>6,439.2</b>
<b>50-Year</b>										
Flow Rate (m <sup>3</sup> /s)	0.035	0.035	0.618	0.016	0.789	1.341	0.530	0.104	0.593	<b>1.755</b>
Runoff Volume (m <sup>3</sup> )	86.8	86.8	2,811.2	47.1	2,577.4	5,435.7	2,409.1	336.8	2,745.9	<b>8,181.5</b>
<b>100-Year</b>										
Flow Rate (m <sup>3</sup> /s)	0.043	0.043	0.801	0.020	0.985	1.721	0.687	0.135	0.765	<b>2.244</b>
Runoff Volume (m <sup>3</sup> )	106.3	106.3	3,447.0	57.6	3,160.5	6,665.1	2,953.9	406.6	3,360.5	<b>10,025.6</b>
<b>Regional</b>										
Flow Rate (m <sup>3</sup> /s)	0.027	0.027	0.881	0.014	0.772	1.667	0.755	0.088	0.841	<b>2.444</b>
Runoff Volume (m <sup>3</sup> )	487.6	487.6	15,780.0	267.9	14,536.0	30,583.9	13,523.0	1,740.4	15,263.6	<b>45,846.2</b>

Table No. 4 gives the results of the ponding in the existing wetland.

**Table No. 4: Wetland (Stage/Storage/Discharge)**

	Available Capacity			Actual Capacity Used			Drawdown Time (hr)**
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	
Wetland Bottom	0.000	0.0	409.63	---	---	---	---
2-Year	---	---	---	0.095	57	409.65	9.3
5-Year	---	---	---	0.322	195	409.69	5.6
10-Year	---	---	---	0.546	330	409.73	5.6
25-Year	---	---	---	0.881	534	409.77	5.5
50-Year	---	---	---	1.176	715	409.79	5.4
100-Year	---	---	---	1.499	912	409.82	5.4
Regional Storm	---	---	---	1.601	971	409.83	52.0
Overflow	18.965	15,227.7	410.75	---	---	---	---

\*\*Drawdown time obtained from the hydrologic modelling software MIDUSS

Table No. 5 gives the results of the existing condition drainage channel routing downstream of the existing wetland.

**Table No. 5: Wetland (Existing Condition Drainage Channel Downstream of Wetland – Section 1 of 2)**

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.095	0.162	0.541
5-Year	---	---	---	0.322	0.256	0.734
10-Year	---	---	---	0.546	0.312	0.838
25-Year	---	---	---	0.881	0.373	0.944
50-Year	---	---	---	1.176	0.416	1.015
100-Year	---	---	---	1.499	0.455	1.078
Regional Storm	---	---	---	1.601	0.467	1.096
Top of Bank	10.655	0.95	1.602	---	---	---

Table No. 6 gives the results of the existing condition drainage channel routing downstream of the existing wetland.

**Table No. 6: Wetland (Existing Condition Drainage Channel Downstream of Wetland – Section 2 of 2)**

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.094	0.082	0.510
5-Year	---	---	---	0.321	0.167	0.772
10-Year	---	---	---	0.544	0.224	0.912
25-Year	---	---	---	0.876	0.290	1.054
50-Year	---	---	---	1.174	0.340	1.148
100-Year	---	---	---	1.490	0.385	1.230
Regional Storm	---	---	---	1.595	0.399	1.254
Top of Bank	9.246	0.95	1.966	---	---	---

**b) Proposed Release Rates**

In order to maintain the existing condition drainage pattern to the existing wetland and Grand River tributary, the release rates have been determined by the existing condition release rates. Under post-development conditions, runoff generated from the site will be attenuated to the existing condition levels conveyed to the existing wetland and the Grand River tributary.

The release rate to Walser Street under minor storm design events is determined by the capacity of the existing storm sewers in Ville Lora Downs North Subdivision Phase III, which were designed to incorporate a small contributing area from Walser Street. The capacity of the existing 300mm diameter storm sewer conveying a portion of the proposed development is approximately 0.110 m<sup>3</sup>/s, based on a grade of 1.34%. Excluding the existing contributing area to this storm sewer (0.05ha), the proposed development's allotment of the pipe's capacity is 0.106 m<sup>3</sup>/s. The allowable to Walser Street under major storm events has been determined based on the allotted area of 0.65 ha, as per the Villa Lora Downs North Phase III storm drainage area plans.

Therefore, the proposed release rates from the site under post-development conditions are outlined in Table No. 7.

**Table No. 7: Proposed Release Rates**

Release Route	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	Regional
To Ex. Wetland	0.099 m <sup>3</sup> /s	0.353 m <sup>3</sup> /s	0.608 m <sup>3</sup> /s	1.001 m <sup>3</sup> /s	1.341 m <sup>3</sup> /s	1.721 m <sup>3</sup> /s	1.667 m <sup>3</sup> /s
To Tributary of Grand River	0.043 m <sup>3</sup> /s	0.153 m <sup>3</sup> /s	0.264 m <sup>3</sup> /s	0.436 m <sup>3</sup> /s	0.593 m <sup>3</sup> /s	0.765 m <sup>3</sup> /s	0.841 m <sup>3</sup> /s
To Walser Street	0.106 m <sup>3</sup> /s		0.122 m <sup>3</sup> /s	0.150m <sup>3</sup> /s	0.173m <sup>3</sup> /s	0.196m <sup>3</sup> /s	0.080m <sup>3</sup> /s

### c) Post-Development Conditions

Under post-development conditions, the existing drainage patterns of the site will be maintained. Post-development flows from the site will be attenuated to existing condition levels through the use of two (2) stormwater management facilities. Stormwater Management Facility No. 1 will outlet to the existing wetland. Stormwater Management Facility No. 2 will outlet to the existing roadside ditch along Gerrie Road and ultimately a tributary of the Grand River.

For the post-development condition analysis, the 21.42-hectare site and 1.20 hectares of external areas was modelled as fifteen (15) drainage catchments. These catchments are shown on the Post-Development Storm Drainage Area Plan (Figure 4).

**Catchment 1000 (6.98-hectares, 50% Impervious)** represents a central portion of the development, including Street No. 3, Street No. 4, and a portion of Street No. 2. Major and minor storm runoff generated from Catchment 1000 will be directed to Stormwater Management Facility No. 1.

**Catchment 1100 (0.48-hectares, 0% Impervious)** represents a portion of external lands including existing residential lot. Major and minor storm runoff generated from Catchment 1100 will be directed to Stormwater Management Facility No. 1.

**Catchment 1200 (0.22-hectares, 50% Impervious)** represents the rear yards of lots 35-42. Runoff generated from Catchment 1200 will be directed to Infiltration Gallery No. 1.

**Catchment 1300 (0.70-hectares, 50% Impervious)** represents the rear yards of lots 53-64. Runoff generated from Catchment 1300 will be directed to Infiltration Gallery No. 1.

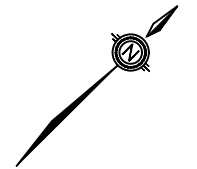
**Catchment 1400 (0.62-hectares, 20% Impervious)** represents the park block. Runoff generated from Catchment 1400 will be directed to Infiltration Gallery No. 2. The clear stone infiltration gallery (80m L x 10m W x 0.70m D) with two (2) 300mm diameter perforated pipes which run the length of the gallery will provide approximately 192 m<sup>3</sup> of storage. The clear stone infiltration gallery has been designed with sufficient capacity to infiltrate minor storm runoff generated by Catchment 1400. Flows exceeding the capacity of the clear stone infiltration gallery will be directed to Stormwater Management Facility No. 1.

**Catchment 1500 (1.11-hectares, 50% Impervious)** represents a portion of Walser Street and Street No. 1. Minor runoff generated from Catchment 1500 will be conveyed via storm sewers to Stormwater Management Facility No. 1, ultimately discharging to the existing wetland. Major runoff generated from Catchment 1500 will sheetflow uncontrolled to the existing wetland.


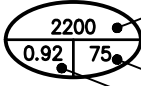
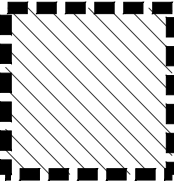
Quality control treatment (80% TSS removal) for runoff generated from Catchment 1000, 1100, 1400, and 1500 will be provided by three (3) oil/grit separator structures. The first oil/grit separator structure (Stormceptor EF8 or approved equivalent) will be located north of the northerly inlet to Stormwater Management Facility No. 1 (Street 3). The second oil/grit separator structure (Stormceptor STC EF6 or approved equivalent) will be located south of the northerly inlet to Stormwater Management Facility No. 1 (Street 4). The third oil/grit separator structure (Stormceptor STC EF6 or approved equivalent) will be located at the southerly inlet to Stormwater Management Facility No. 1 (Street 1).

**Catchment 1600 (0.22-hectares, 50% Impervious)** represents the rear yards of lots 43-45 and 50-52. Runoff generated from Catchment 1600 will be directed to Infiltration Gallery No. 1. The clear stone infiltration gallery (145m L x 3.5-5m W x 0.70m D), receiving flows from Catchments 1200, 1300, and 1600 with four to six (4 to 6) 450mm diameter perforated pipes which run the length of the gallery will provide approximately 192.1 m<sup>3</sup> of storage. Flows exceeding the capacity of the clear stone infiltration gallery will be directed to Stormwater Management Facility No. 1.

411009  
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 SUBDIVISION  
 TOWNSHIP OF CENTRE  
 WELLINGTON (ELORA)

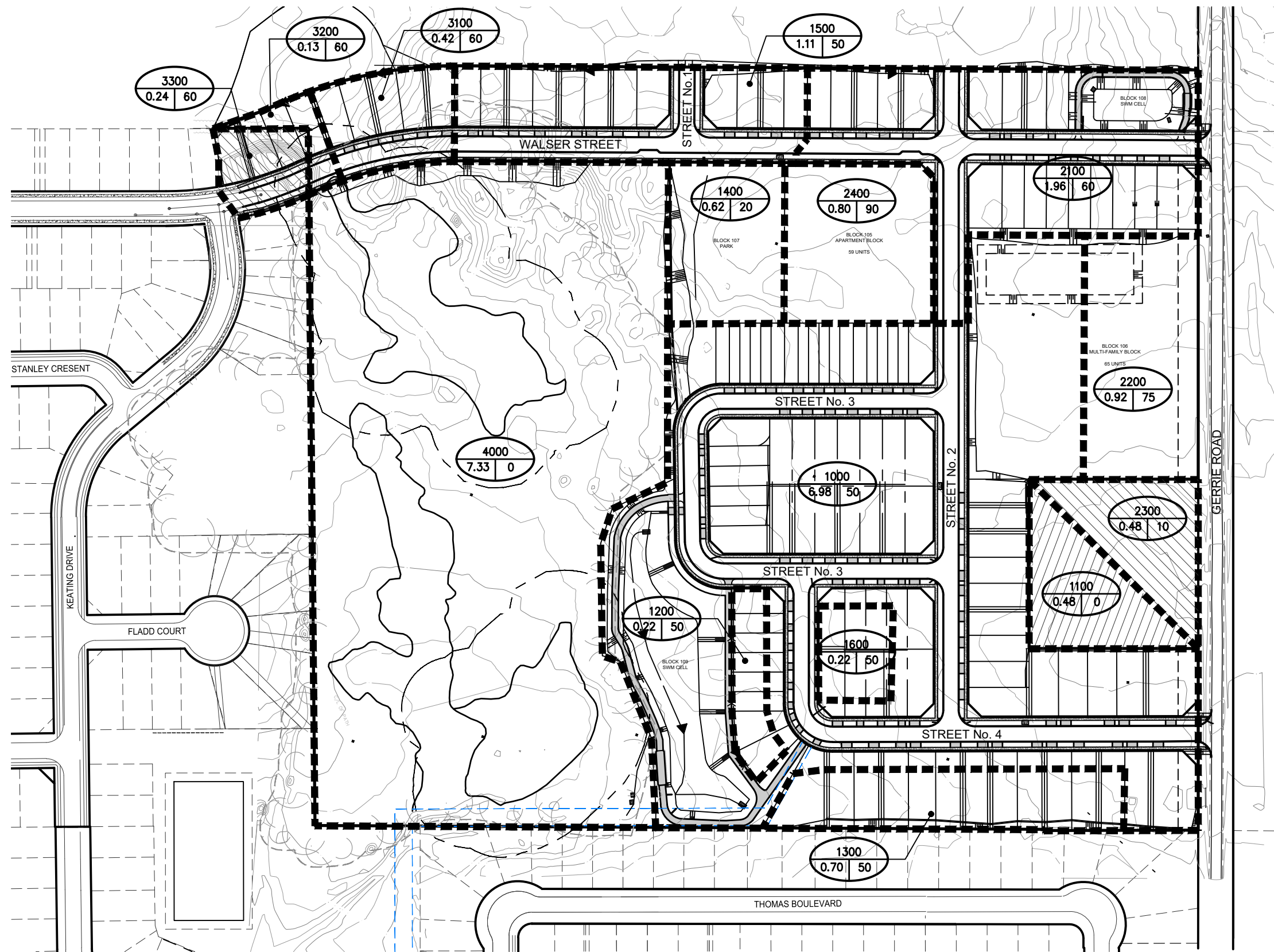


LEGEND

-  DRAINAGE AREA BOUNDARY
-  CATCHMENT NUMBER  
% IMPERVIOUS AREA IN HECTARES
-  EXTERNAL LANDS

POST DEVELOPMENT  
 STORM DRAINAGE  
 AREA PLAN

FIGURE No. 4





Stormwater Management Facility No. 1 has been designed as a dry pond with approximately 3,030 m<sup>3</sup> of storage. Discharging from this pond will be via a multi-stage outlet consisting of a double inlet catchbasin with a 250 mm orifice for minor storms and a double inlet catchbasin with a 350mm diameter orifice for major storms, as well as a 20 m wide overflow weir.

**Catchment 2100 (1.96-hectares, 60% Impervious)** represents the north portion of development, including the remainder of Street 2, and a portion of Walser Street. Major and minor storm runoff generated from Catchment 2100 will be directed to Stormwater Management Facility No. 2, ultimately discharging to a tributary of the Grand River via the existing roadside ditch along Gerrie Road.

**Catchment 2200 (0.92-hectares, 75% Impervious)** represents a portion of the multi-family residential block. Runoff generated from Catchment 2200 will discharge to the roadside ditch along Gerrie Road, and ultimately a tributary of the Grand River. At such time as development of Catchment 2200 proceeds, a privately owned and operated on-site quality and quantity control stormwater management facility will be required to attenuate stormwater runoff to the existing condition level, prior to discharge to the existing roadside ditch along Gerrie Road.

The privately owned and operated on-site stormwater management facility will be designed, reviewed and approved as part of the site plan approval process for the development block. The on-site stormwater management controls which may be utilized include, but are not limited to, a stormwater management facility (i.e. SWM pond), rooftop storage, parking lot ponding (to a maximum depth of 0.3m), below grade storage (i.e. clear stone storage, superpipe storage, etc.) and oil/grit separators. A preliminary stormwater management facility with approximately 2,085 m<sup>3</sup> of storage has been modelled to provide attenuation levels for the post-development runoff generated by Catchment 2200.

**Catchment 2300 (0.48-hectares, 10% Impervious)** represents the remainder of the existing residential lot on Gerrie Road. Major and minor storm runoff generated from Catchment 2300 will be directed to the existing roadside ditch on Gerrie Road, ultimately discharging to a tributary of the Grand River.

**Catchment 2400 (0.80-hectares, 90% Impervious)** represents the apartment block. Runoff generated from Catchment 2400 will be directed to Stormwater Management Facility No. 2, ultimately discharging to a tributary of the Grand River via the existing roadside ditch along Gerrie Road.

Stormwater Management Facility No. 2 has been designed as a dry pond with approximately 1,741 m<sup>3</sup> of storage. Discharging from this pond will be via a multi-stage outlet consisting of a 150 mm knockout for minor storms and a 300mm diameter orifice for major storms, as well as a 10 m wide overflow weir.

Quality control treatment (80% TSS removal) for runoff generated from Catchment 2100 and 2400 will be provided by one (1) oil/grit separator structure. The oil/grit separator structure (Stormceptor EF8 or approved equivalent) will be located at the inlet to Stormwater Management Facility No. 2.

**Catchment 3100 (0.42-hectares, 60% Impervious)** represents five (5) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3100 will be captured and attenuated via a 1350mm diameter super-pipe system which is controlled by a 120 mm diameter orifice plate prior to discharging to on-site storm sewers. The super-pipe system has approximately 75.2 m<sup>3</sup> of storage when combined with storage generated from the storm sewer structures. Runoff from the super-pipe system will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3100 will sheetflow overland to the existing Walser Street right-of-way.

**Catchment 3200 (0.13-hectares, 60% Impervious)** represents five (5) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3200 will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3200 will sheetflow overland to the existing Walser Street right-of-way.

**Catchment 3300 (0.24-hectares, 60% Impervious)** represents three (3) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3300 will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3300 will discharge overland to the existing Walser Street right-of-way.

Quality and quantity control for stormwater runoff generated from Catchments 3200 and 3300 will be provided by the existing stormwater management facilities approved and constructed as part of the Villa Lora Downs North Phase II development.

**Catchment 4000 (7.33 hectares, 0% impervious)** represents the remainder of the site, which is a natural heritage feature consisting of a woodlot and wetland area. Runoff generated from Catchment 4000 will continue to sheetflow overland, ultimately discharging to the existing swale in Drimmie Park and the existing storm sewers on Keating Drive.

Table No. 8 lists the uncontrolled flow rate and runoff volumes generated from each catchment area shown on Figure No. 4, for the 2, 5, 10, 25, 50 and 100-year design storm events and the Regional storm.

**Table No. 8: Post-Development Uncontrolled Flow Rate and Runoff Volume**

	CATCHMENTS																	
	1000	1100	1200	1300	1400	1500	1600	4000	To Ex. Wetland	2100	2200	2300	2400	To Roadside Ditch	3100	3200	3300	To Walser Street
<b>2-Year</b>																		
Flow Rate (m <sup>3</sup> /s)	0.662	0.009	0.021	0.063	0.024	0.105	0.020	0.001	<b>0.895</b>	0.223	0.130	0.011	0.129	<b>0.489</b>	0.049	0.014	0.026	<b>0.088</b>
Runoff Volume (m <sup>3</sup> )	1,213.7	32.9	37.6	120.7	68.2	190.9	37.9	16.2	<b>1,718.2</b>	377.7	205.8	42.8	204.6	<b>830.9</b>	80.6	25.1	46.4	<b>152.1</b>
<b>5-Year</b>																		
Flow Rate (m <sup>3</sup> /s)	0.935	0.028	0.029	0.093	0.038	0.146	0.029	0.019	<b>1.280</b>	0.304	0.176	0.030	0.177	<b>0.678</b>	0.067	0.020	0.037	<b>0.124</b>
Runoff Volume (m <sup>3</sup> )	2,097.7	76.2	65.1	209.5	133.2	330.9	65.7	156.6	<b>3,134.8</b>	638.8	338.3	89.7	329.4	<b>1,396.1</b>	136.7	42.5	78.6	<b>257.8</b>
<b>10-Year</b>																		
Flow Rate (m <sup>3</sup> /s)	1.094	0.043	0.037	0.114	0.056	0.174	0.036	0.050	<b>1.526</b>	0.359	0.206	0.045	0.208	<b>0.805</b>	0.078	0.024	0.045	<b>0.147</b>
Runoff Volume (m <sup>3</sup> )	2,751.8	111.7	85.4	275.0	184.0	434.9	86.2	326.8	<b>4,255.8</b>	830.1	433.4	127.0	416.9	<b>1,807.5</b>	177.6	55.2	102.0	<b>334.8</b>
<b>25-Year</b>																		
Flow Rate (m <sup>3</sup> /s)	1.292	0.065	0.047	0.144	0.082	0.213	0.046	0.114	<b>1.852</b>	0.435	0.245	0.067	0.246	<b>0.975</b>	0.091	0.029	0.055	<b>0.176</b>
Runoff Volume (m <sup>3</sup> )	3,583.9	160.0	111.3	359.0	251.3	568.2	112.3	606.6	<b>5,752.7</b>	1,073.0	552.8	177.2	525.3	<b>2,328.3</b>	228.9	71.3	131.7	<b>432.0</b>
<b>50-Year</b>																		
Flow Rate (m <sup>3</sup> /s)	1.452	0.082	0.055	0.169	0.105	0.242	0.054	0.183	<b>2.124</b>	0.491	0.275	0.086	0.274	<b>1.107</b>	0.102	0.034	0.064	<b>0.200</b>
Runoff Volume (m <sup>3</sup> )	4,253.0	200.4	132.0	426.7	306.8	675.6	133.5	874.1	<b>7,002.0</b>	1,267.8	647.8	218.8	610.7	<b>2,745.1</b>	270.7	84.3	155.5	<b>510.5</b>
<b>100-Year</b>																		
Flow Rate (m <sup>3</sup> /s)	1.610	0.101	0.063	0.194	0.129	0.277	0.062	0.268	<b>2.406</b>	0.554	0.306	0.105	0.301	<b>1.245</b>	0.114	0.038	0.072	<b>0.225</b>
Runoff Volume (m <sup>3</sup> )	4,934.5	242.1	152.9	495.5	363.6	785.9	155.1	1,180.1	<b>8,309.9</b>	1,466.7	744.1	261.6	696.5	<b>3,169.0</b>	313.1	97.5	179.7	<b>590.2</b>
<b>Regional Storm</b>																		
Flow Rate (m <sup>3</sup> /s)	0.768	0.055	0.027	0.085	0.071	0.129	0.027	0.619	<b>1.666</b>	0.232	0.112	0.055	0.102	<b>0.501</b>	0.049	0.016	0.029	<b>0.094</b>
Runoff Volume (m <sup>3</sup> )	16,390.0	1031.7	490.7	1,595.8	1,379.9	2,555.4	496.2	9,470.4	<b>33,410.0</b>	4,559.9	2,173.8	1,044.2	1,907.3	<b>9,685.2</b>	981.6	301.7	553.4	<b>1,836.7</b>



**d) Routing**

Table No. 9 compares the routing results through the proposed Infiltration Gallery No. 1.

**Table No. 9: Catchment 1200, 1300 & 1600 – Infiltration Gallery No. 1 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m
Bottom of Stone	0.000	0.0	412.00	---	---	---
2-Year	---	---	---	0.000	104.0	412.30
Top of Stone	0.000	192.1	412.70	---	---	---
CB Lip (1)	0.000	193.0	413.23	---	---	---
5-Year	---	---	---	0.074	194.3	413.51
Regional Storm	---	---	---	0.127	195.4	413.64
10-Year	---	---	---	0.130	195.6	413.66
25-Year	---	---	---	0.213	197.2	413.79
50-Year	---	---	---	0.270	198.4	413.87
100-Year	---	---	---	0.316	199.4	413.93
CB Lip (2)	0.437	202.6	414.09	---	---	---
Overflow	0.928	214.6	414.49	---	---	---

Table No. 10 compares the routing results through the proposed Infiltration Gallery No. 2.

**Table No. 10: Catchment 1400 – Infiltration Gallery No. 2 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m
Bottom of Stone	0.000	0.0	413.92	---	---	---
2-Year	---	---	---	0.001	55.2	414.12
5-Year	---	---	---	0.001	123.6	414.36
10-Year	---	---	---	0.001	139.6	414.42
Top of Stone	0.001	191.8	414.62	---	---	---
Pipe Invert	0.001	191.8	414.72	---	---	---
25-Year	---	---	---	0.017	191.9	414.83
50-Year	---	---	---	0.046	192.3	415.12
Top of Grate	0.053	192.1	415.12	---	---	---
Regional Storm	---	---	---	0.070	192.9	415.13
100-Year	---	---	---	0.090	193.7	415.14
Weir	1.244	304.7	415.42	---	---	---

Table No. 11 compares the routing results through the proposed Stormwater Management Facility No. 1.

**Table No. 11: Catchment 1000, 1100, 1200, 1300, 1400, 1500 (minor) & 1600 – Stormwater Management Facility No. 1 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used			Drawdown Time (hr)
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	
CB Lip 1 Elevation	0.00	0.0	411.00	---	---	---	---
2-Year	---	---	---	0.097	726.7	411.51	3
CB Lip 2 Elevation	0.105	1,027.6	411.60	---	---	---	---
5-Year	---	---	---	0.234	1,286.5	411.66	4
10-Year	---	---	---	0.323	1,623.0	411.74	4
Weir	0.349	2,195.7	411.85	---	---	---	---
25-Year	---	---	---	0.398	2,237.9	411.86	5
50-Year	---	---	---	0.658	2,458.1	411.90	5
Regional Storm	---	---	---	0.875	2,573.8	411.92	5
100-Year	---	---	---	0.970	2,593.1	411.92	5
Top of Bank	2.018	3,030.1	412.00	---	---	---	---

Table No. 12 gives the results of the post-development condition of the existing wetland.

**Table No. 12: Wetland Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used			Drawdown Time (hr)
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	
Wetland Bottom	0.000	0.0	409.63	---	---	---	---
2-Year	---	---	---	0.097	58.6	409.65	6.8
5-Year	---	---	---	0.237	143.1	409.67	8.0
10-Year	---	---	---	0.367	222.3	409.70	8.1
25-Year	---	---	---	0.485	293.3	409.72	8.5
50-Year	---	---	---	0.768	466.3	409.76	8.8
100-Year	---	---	---	1.083	656.4	409.79	8.8
Regional Storm	---	---	---	1.444	875.7	409.82	55.0
Overflow	18.965	15,227.7	410.75	---	---	---	---

Table No. 13 gives the results of the post-development condition drainage channel routing downstream of the existing wetland.

**Table No. 13: Wetland (Post-Development Condition Drainage Channel Downstream of Wetland – Section 1 of 2)**

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.097	0.163	0.544
5-Year	---	---	---	0.237	0.228	0.680
10-Year	---	---	---	0.367	0.269	0.759
25-Year	---	---	---	0.485	0.298	0.813
50-Year	---	---	---	0.768	0.354	0.912
100-Year	---	---	---	1.083	0.403	0.994
Regional Storm	---	---	---	1.444	0.449	1.068
Top of Bank	10.655	0.95	1.602	---	---	---

Table No. 14 gives the results of the post-development condition drainage channel routing downstream of the existing wetland.

**Table No. 14: Wetland (Post-Development Condition Drainage Channel Downstream of Wetland – Section 2 of 2)**

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s	Peak Flow m <sup>3</sup> /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.097	0.084	0.516
5-Year	---	---	---	0.236	0.140	0.699
10-Year	---	---	---	0.367	0.180	0.806
25-Year	---	---	---	0.484	0.210	0.880
50-Year	---	---	---	0.767	0.270	1.013
100-Year	---	---	---	1.078	0.325	1.120
Regional Storm	---	---	---	1.438	0.378	1.217
Top of Bank	9.246	0.950	1.966	---	---	---

Table No. 15 compares the routing results through the proposed Stormwater Management Facility No. 2.

**Table No. 15: Catchment 2100, 2400 – Stormwater Management Facility No. 2  
 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used			Drawdown Time (hr)
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	
150mm Knockout	0.00	0.0	410.65	---	---	---	---
2-Year	---	---	---	0.027	400.2	411.06	7
CB Lip Elevation	0.029	442.0	411.10	---	---	---	---
5-Year	---	---	---	0.121	550.4	411.20	8
10-Year	---	---	---	0.141	693.3	411.31	9
25-Year	---	---	---	0.161	911.7	411.48	12
50-Year	---	---	---	0.175	1,094.9	411.62	13
Weir	0.187	1,143.0	411.65	---	---	---	---
Regional Storm	---	---	---	0.268	1,184.5	411.68	13
100-Year	---	---	---	0.306	1,201.9	411.69	13
Top of bank	1.015	1,367.8	411.80	---	---	---	---

Table No. 16 compares the routing results through the proposed super-pipe storage system on Walser Street.

**Table No. 16: Catchment 3100 – Super-Pipe Storage System  
 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m	Peak Flow m <sup>3</sup> /s	Storage Volume m <sup>3</sup>	Storage Elevation m
Orifice Invert	0.00	0.0	410.62	---	---	---
2-Year	---	---	---	0.022	19.0	411.24
5-Year	---	---	---	0.029	34.3	411.63
10-Year	---	---	---	0.033	45.2	411.91
25-Year	---	---	---	0.038	59.4	412.27
Regional Storm	---	---	---	0.040	67.2	412.47
50-Year	---	---	---	0.041	70.1	412.54
100-Year	---	---	---	0.050	76.2	413.47
CBMH.43 T/G Weir	0.055	77.9	413.92	---	---	---
Overflow	0.160	78.5	414.23	---	---	---

Table No. 17 summarizes the post-development flow rates from the site.

**Table No. 17: Summary of Post-Development Flow Rates and Runoff Volumes from the Site**

	CATCHMENTS												
	1000, 1100, 1200, 1300, 1400, 1500 (minor), 1600 (controlled)	1500 (major) (uncontrolled)	4000 (uncontrolled)	To Ex. Wetland	2100, 2400 (controlled)	2200 (controlled)	2300 (uncontrolled)	To Roadside Ditch	3100 (minor) (controlled)	3100 (major) (controlled)	3200 (uncontrolled)	3300 (uncontrolled)	To Walser Street
<b>2-Year</b>													
Flow Rate (m <sup>3</sup> /s)	0.097	0.000	0.001	<b>0.097</b>	0.027	0.008	0.011	<b>0.043</b>	0.022	0.000	0.014	0.026	<b>0.060</b>
Runoff Volume (m <sup>3</sup> )	1,437.5	0.0	16.2	<b>1,454.3</b>	582.2	205.8	42.8	<b>830.9</b>	80.6	0	25.1	46.4	<b>152.2</b>
<b>5-Year</b>													
Flow Rate (m <sup>3</sup> /s)	0.234	0.000	0.019	<b>0.244</b>	0.121	0.012	0.030	<b>0.151</b>	0.029	0.000	0.020	0.037	<b>0.082</b>
Runoff Volume (m <sup>3</sup> )	2,504.7	0.0	156.6	<b>2,662.1</b>	968.1	338.3	89.7	<b>1,395.2</b>	136.7	0	42.5	78.6	<b>257.9</b>
<b>10-Year</b>													
Flow Rate (m <sup>3</sup> /s)	0.323	0.028	0.050	<b>0.372</b>	0.141	0.014	0.045	<b>0.190</b>	0.033	0.011	0.024	0.045	<b>0.107</b>
Runoff Volume (m <sup>3</sup> )	3,287.6	10.7	326.8	<b>3,625.2</b>	1,247.0	433.4	127.0	<b>1807.2</b>	174.4	3.2	55.2	102.0	<b>334.7</b>
<b>25-Year</b>													
Flow Rate (m <sup>3</sup> /s)	0.398	0.067	0.114	<b>0.512</b>	0.161	0.027	0.067	<b>0.227</b>	0.038	0.024	0.029	0.055	<b>0.137</b>
Runoff Volume (m <sup>3</sup> )	4,266.0	46.1	606.6	<b>4,920.7</b>	1,598.3	552.8	177.2	<b>2,329.8</b>	216.2	12.8	71.3	131.7	<b>431.9</b>
<b>50-Year</b>													
Flow Rate (m <sup>3</sup> /s)	0.658	0.096	0.183	<b>0.841</b>	0.175	0.061	0.086	<b>0.267</b>	0.041	0.035	0.034	0.064	<b>0.163</b>
Runoff Volume (m <sup>3</sup> )	5,038.2	90.8	874.1	<b>6,000.2</b>	1,878.4	647.8	218.8	<b>2,744.6</b>	246.4	24.2	84.3	155.5	<b>510.6</b>
<b>100-Year</b>													
Flow Rate (m <sup>3</sup> /s)	0.970	0.131	0.268	<b>1.235</b>	0.306	0.099	0.105	<b>0.454</b>	0.050	0.047	0.038	0.072	<b>0.190</b>
Runoff Volume (m <sup>3</sup> )	5,817.0	145.5	1,180.1	<b>7,146.6</b>	2,163.2	744.1	261.6	<b>3,165.4</b>	273.1	40.0	97.5	179.7	<b>590.0</b>
<b>Regional Storm</b>													
Flow Rate (m <sup>3</sup> /s)	0.875	0.000	0.619	<b>1.494</b>	0.268	0.099	0.055	<b>0.406</b>	0.040	0.000	0.016	0.029	<b>0.077</b>
Runoff Volume (m <sup>3</sup> )	19,977.0	0.0	9,470.4	<b>29,575.5</b>	6,467.2	2,173.8	1,044.2	<b>9,743.4</b>	981.6	0.0	301.7	553.4	<b>1,827.1</b>

The following table compares the proposed release rates to the post-development flow rates for the site.

**Table No. 18: Comparison of Release Rates and Post-Development Conditions Flow Rates**

DESIGN STORM	To Ex. Wetland		To Tributary of Grand River		To Walser Street	
	Proposed Release Rate (m <sup>3</sup> /s)	Post Flow Rate (m <sup>3</sup> /s)	Proposed Release Rate (m <sup>3</sup> /s)	Post Flow Rate (m <sup>3</sup> /s)	Proposed Release Rate (m <sup>3</sup> /s)	Post Flow Rate (m <sup>3</sup> /s)
2 Year	0.099	0.097	0.043	0.043	0.106	0.060
5 Year	0.353	0.244	0.153	0.151		0.082
10 Year	0.608	0.372	0.264	0.190	0.122	0.107
25 Year	1.001	0.512	0.436	0.227	0.150	0.137
50 Year	1.341	0.841	0.593	0.267	0.173	0.163
100 Year	1.721	1.235	0.765	0.454	0.196	0.190
Regional	1.667	1.494	0.841	0.406	0.080	0.077

Therefore, the post-development runoff generated from the site will be attenuated to the less than the proposed release rates to all outlets.

#### 5.3.4 MINOR / MAJOR DRAINAGE SYSTEM

Minor storm drainage will be conveyed to the proposed stormwater management facilities and the existing storm sewers on Walser Street via storm sewers with the capacity to convey the 5-year design storm event.

The major storm runoff generated from Street 1, and a portion of Street 2, Street 3 and Street 4 will discharge to the proposed stormwater management facility located east of the existing wetland (Stormwater Management Facility No. 1), which outlets to the existing wetland, ultimately discharging to the existing storm sewers on Keating Drive.

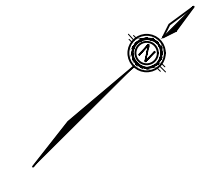
The major storm runoff generated from the remainder of Street 2 and a portion of the Walser Street extension will discharge to the proposed stormwater management facility (Stormwater Management Facility No. 2), ultimately discharging to a tributary of the Grand River.

The major storm runoff generated from the remainder of the Walser Street extension will discharge directly to the existing Walser Street Right-of-Way, ultimately discharging to the Keating Drive Right-of-Way.

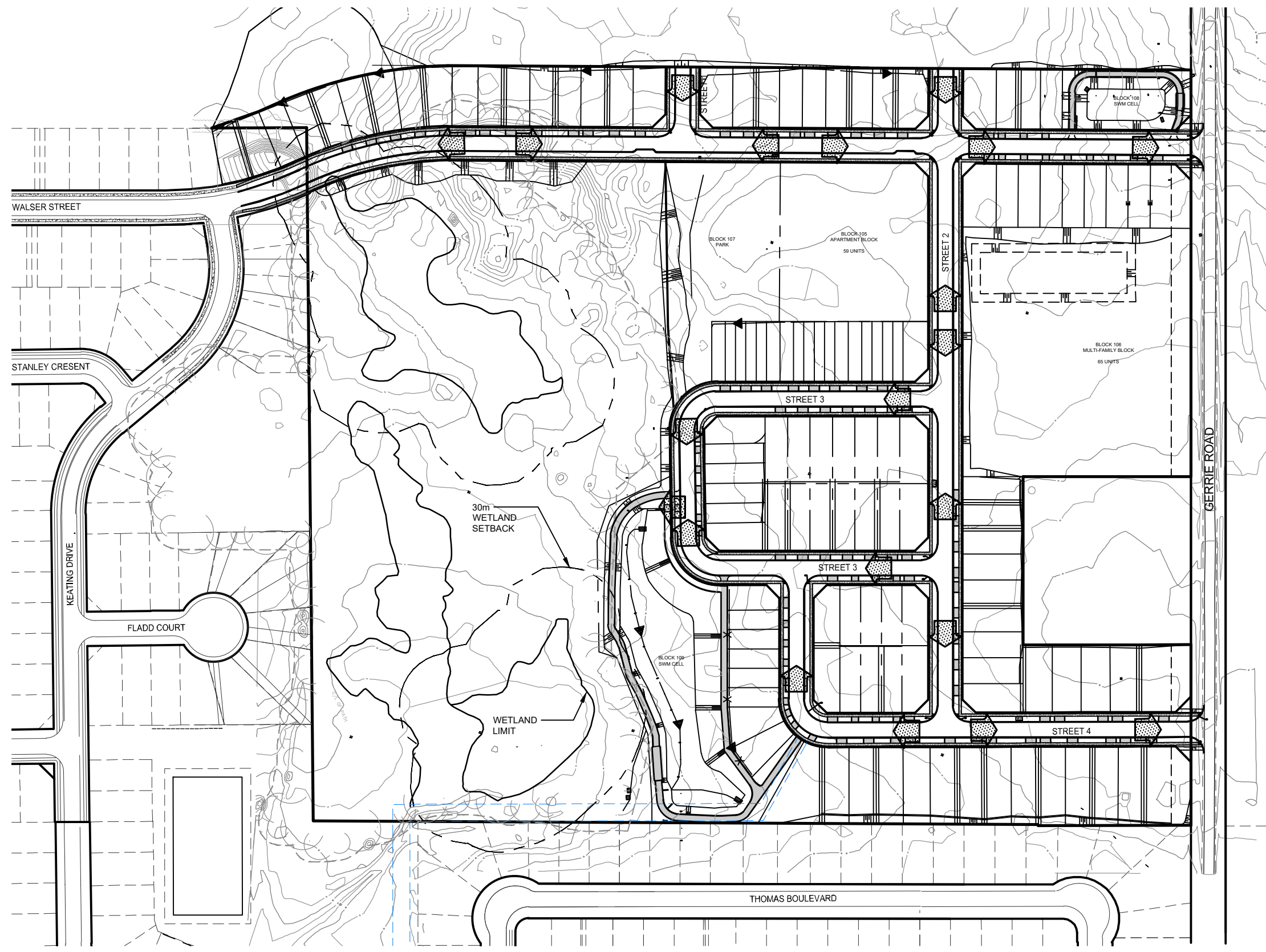
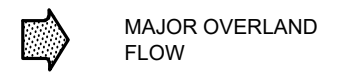
Preliminary analysis indicates that the municipal right-of-way has the capacity to convey the runoff from a major design storm event.

The major design storm drainage patterns expected for the Ainley Farm Subdivision are shown on Figure 5.

411009  
AINLEY FARM  
SUBDIVISION  
TOWNSHIP OF CENTRE  
WELLINGTON (ELORA)



LEGEND



MAJOR STORM  
DRAINAGE PATTERN  
PLAN

FIGURE No. 5





## 5.4 WATER BUDGET

The average annual precipitation for the site is estimated to be 945.9 mm. This amount is based on precipitation data recorded at the Fergus Shand Dam meteorological station for the period from 1981 to 2010.

From the Preliminary Geotechnical Investigation (CMT Engineering Inc., March 26, 2006), the surficial deposits across the majority of the site are described as native silt tills, with some sandy silt tills. As there are no areas of consistent sandy soils across the site, the characteristics of the silt tills will be used to develop the water budget analysis across the site.

The potential for evapotranspiration for this area is estimated to be 557.8 mm for the pervious surfaces. Therefore, 388.1 mm remain available for infiltration and runoff from the silt till.

Per Table 3.1 of the Stormwater Management Planning and Design Manual (Ministry of Environment, dated 2003), a silt till, which acts similar to clay (hence the low conductivity values), in flat cultivated land has an approximate infiltration rate of 87 mm/yr. Therefore, the runoff is estimated to be 301.1 mm/yr.

Based on the annual infiltration rates, the existing annual average groundwater recharge occurring within the 21.42-hectare site, and 1.20 hectares of external areas discharging to the site, is estimated to be 12,674 m<sup>3</sup>. Under post-development conditions, the annual average groundwater recharge occurring on-site and within the external areas naturally is estimated to be 5,473 m<sup>3</sup>. The additional annual recharge that will occur on-site via the two (2) proposed infiltration galleries is estimated to be 6,502 m<sup>3</sup>, resulting in a total post-development annual recharge rate of 11,974 m<sup>3</sup>.

Under existing conditions, the annual average runoff from the site and external areas is estimated to be 68,109 m<sup>3</sup>. As a result of the proposed development the impervious area (rooftop and paved surfaces) of the site increases, the annual potential evapotranspiration for impervious surfaces decreases to 200 mm and the runoff from the site increases. The runoff from the site and external areas under post-development conditions is estimated to be 97,828 m<sup>3</sup> per year.

The estimated existing and post-development recharge and runoff volumes for the Ainley Farm Subdivision are detailed in Table No. 19. The estimations take into account the surficial geology, which is comprised mainly of glacial tills. The net recharge values are for the uppermost overburden aquifer. The water budget analysis has been included in Appendix D.

**Table No. 19: Summary of Recharge and Runoff Volume**

	<b>Existing Condition</b>	<b>Post-Development Condition</b>	<b>Percent Change</b>
Total Estimated Recharge	12,674 m <sup>3</sup>	11,974 m <sup>3</sup>	-5.5%
Total Estimated Runoff	68,109 m <sup>3</sup>	98,828 m <sup>3</sup>	+45.1%

The opportunities for further recharge are limited by the high groundwater elevations across the site, as noted in CMT's groundwater monitoring across the site from 2006 to 2016.



## **6.0 SEDIMENT AND EROSION CONTROL PLAN**

A silt fence will be installed along the property boundary. The silt fence will serve to minimize the opportunity for water borne sediments to be transported from the site to the adjacent properties.

Temporary straw bale check dams will be installed in rear yard swales after the initial grading has been completed to slow the flow rates and promote the settlement of water borne sediments before they reach the silt fences and stormwater management facilities.

Upon completion of the grading, any area not subject to active construction within 30 days will be top soiled and seeded as per OPSS 572.

Once catch basins have been installed, the grates will be wrapped in filter cloth. This feature will be maintained until all building and landscaping has been completed.

Inspection and maintenance of all silt fencing and sediment and erosion controls will start after installation is complete. These features will be inspected on a weekly basis or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the controls found to need repair.

Once construction and landscaping within the limits of the subdivision has been substantially completed (75% house building construction is complete), the silt fence will be removed, any accumulated sediment will be collected, and the area will be restored.

After construction of the subdivision, erosion and sediment transport will be minimal.

## **7.0 MAINTENANCE PLAN**

A two-phase maintenance plan is recommended. Phase I will address the short-term more intensive maintenance necessary during and immediately after construction. Once all landscaping has been completed, maintenance will shift to Phase II.

As outlined in the section on Sediment and Erosion Control, Phase I will include weekly inspection of all sediment and erosion control devices plus “as needed” inspection after significant rainfall, with the repair of any damaged works and collection of captured sediment.

Phase II will be the maintenance carried out by the Township of Centre Wellington after all construction has been completed. This work will involve a yearly visual inspection of the stormwater management facilities and catch basins to determine the amount of sediment accumulation. Sediment should be removed as required and the recommended vegetation replanted.

## 8.0 CONCLUSIONS

From the foregoing analysis, the following conclusions are drawn:

- Water supply for the Ainley Farm Subdivision will be provided via the extension of a 200 mm diameter watermain along the Walsler Street extension, Street No. 2 and a portion of Street No. 1. A 150 mm diameter watermain will be extended along the remainder of Street No. 1, Street No. 3, and Street No. 4.
- Sanitary service for the proposed lots along the Walsler Street extension and a portion of Street No. 2 will be provided by the extension of a 200 mm diameter sanitary sewer from the existing 200 mm diameter sanitary sewer on Walsler Street. Sanitary service for the remainder of the site will be provided by the extension of a 200 mm diameter sanitary sewer on easement from the existing 200 mm diameter sanitary sewer on Keating Drive.
- Storm sewers will be designed to convey the 5-year design storm event and will discharge to the two (2) stormwater management facilities and the existing storm sewer on Walsler Street.
- Major storm runoff will be conveyed within the limits of the street right-of-ways to the two (2) stormwater management facilities and the existing Walsler Street right-of-way.
- As per the Township of Centre Wellington municipal standards, foundation drainage will be collected in sump pits in each residential unit and pumped to the storm sewer system located within the municipal right-of-way.
- Quantity control for runoff generated from the development will be provided by two (2) stormwater management facilities,
- Quality control for runoff generated from the development will be provided by four (4) oil/grit separators (Stormceptor or approved equivalent).
- The post-development runoff generated from the site will be attenuated to the less than the proposed release rates to all outlets.
- Infiltration rates under post-development conditions are 5.5% less than existing conditions with two (2) infiltration galleries to provide additional recharge. The opportunities for further recharge are limited by the high groundwater elevations across the site, as noted in CMT's groundwater monitoring across the site from 2006 to 2016.
- During the construction phase, the erosion control measures will minimize the transport of sediment off-site during the construction period.

All of which is respectfully submitted.

**GM BLUEPLAN ENGINEERING LIMITED**

Per:



Patrick Grier, P.Eng.

PG/





**APPENDIX A**  
**PRELIMINARY GEOTECHNICAL INVESTIGATION**  
**CMT ENGINEERING INC.**  
**MARCH 29, 2006**

**PRELIMINARY GEOTECHNICAL  
INVESTIGATION**

**AINLEY SUBDIVISION  
TOWNSHIP OF CENTRE WELLINGTON  
VILLAGE OF ELORA, ONTARIO**

**CMT Project 06-004**

**Prepared For:**

**Gamsby and Mannerow Limited**

**March 29, 2006**





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March 29, 2006

06-004.R01

Gamsby and Mannerow Limited  
255 Woodlawn Road West, Suite 210  
Guelph, Ontario  
N1H 8J1

Attention: Mr. Glenn Anderson, C.E.T.

Dear Sir:

**Re: Preliminary Geotechnical Investigation  
Ainley Subdivision  
Township of Centre Wellington  
Village of Elora, Ontario**

As requested, CMT Engineering Inc. conducted a subsoil investigation at the above-referenced site, and we are pleased to present the enclosed report.

We trust that this information meets your present requirements and we thank you for this opportunity to have been of service. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

A handwritten signature in black ink, appearing to read 'Robert Koopmans', written over a series of horizontal, wavy lines that serve as a background for the signature.

Robert Koopmans, P.Eng.

ks

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Drawing 1 - Site Plan Showing Borehole Locations

Appendix A - Borehole Logs 101 to 108

Appendix B - Cross-Sections

Appendix C - Grain Size Analyses

Appendix D - Laboratory Proctor Test

Appendix E - Well Record

## **1.0 INTRODUCTION**

The services of CMT Engineering Inc. were retained by Mr. Glenn Anderson of Gamsby and Mannerow Limited to carry out a subsurface investigation for the proposed Ainley Subdivision in the Township of Centre Wellington (Village of Elora).

It is our understanding that single-family and multi-family residences are proposed to be built on the eastern two-thirds of the property. The western one-third of the property is proposed for open space and may contain a storm water management pond.

The purpose of the investigation was to determine the subsurface soil profile, the water levels in the boreholes and provide recommendations with respect to site grading, bearing capacity for house foundations, trench excavations, bedding and backfilling for service pipes, site dewatering, road construction, pavement design recommendations and soil hydraulic conductivity for storm water management design.

## **2.0 SITE CONDITIONS**

The geotechnical investigation was conducted on Part Lot 18, Concession 12 of the Township of Centre Wellington. In general, the eastern two-thirds of the property is currently farm land, while the western one-third is treed. The site topography undulates slightly and the ground surface elevation drops towards the southwest corner of the property.

## **3.0 FIELD AND LABORATORY PROCEDURES**

On January 25, 2006, a track-mount CME 55 drillrig operated by Aardvark Drilling Inc. was used to drill eight (8) boreholes (referenced as Boreholes 101 to 108) to depths of between 3.5 m (11.5 ft) and 5.0 m (16.4 ft) below the existing ground surface elevation. Standard penetration tests were conducted at 0.76 m (2.5 ft) intervals to depths of 3.0 m (10.0 ft) and at 1.5 m (5.0 ft) intervals below 3.0 m (10.0 ft) in all boreholes. Monitoring wells were installed in all eight boreholes to determine the presence and depth of the groundwater table.

Technical staff from CMT Engineering Inc. observed the drilling operation and collected and logged the recovered soil samples. Soil samples taken from Borehole 102 (3.05 to 3.51 m), Borehole 103 (4.57 to 5.03 m), Borehole 105 (2.29 to 2.74 m) and Borehole 107 (2.29 to 2.74 m) were placed in marked sample bags for grain size analyses (refer to Appendix C for laboratory test results). A bulk sample from Borehole 103 (1.5 to 2.0 m) was submitted for laboratory Proctor testing (refer to Appendix D for the laboratory test results). A small portion of each sample was placed in a sealed marked jar for moisture content determinations.

Gamsby and Mannerow Limited surveyed the ground surface elevations for all boreholes, as well as the tops of the monitoring wells.

Drawing 1 shows the site plan with all of the borehole locations.

#### **4.0 SUBSOIL CONDITIONS**

The soil conditions at the borehole locations are summarized briefly below, while a more detailed stratigraphic description is provided in the borehole logs in Appendix A. Cross-section profiles through Boreholes 101-103-102-104 and Boreholes 105-106-107-108 are provided in Appendix B.

##### **4.1 Topsoil**

Dark brown silt topsoil was found at the top of all eight boreholes. The topsoil was frozen at the time of the investigation. The thickness of the topsoil ranged from 30 mm to 60 mm (average 42 mm).

##### **4.2 Organic Silt**

Organic silt was found underlying the topsoil in Borehole 103. The organic silt was saturated, loose and brown with some sand and occasional topsoil nodules. The moisture content of the organic silt was 83.5% and the N-count was 8 blows per 0.30 m.

##### **4.3 Silt or Sandy Silt**

Silt was found underlying the sandy silt in Borehole 105. The silt was very moist, loose and brown with some clay, trace sand and trace gravel. The moisture content of the silt was 18.5% and the N-count was 8 blows per 0.30 m.

Sandy silt was found underlying the topsoil in Borehole 102. In general, the sandy silt was moist, compact and brown with a trace of clay and a trace of gravel. The moisture content was 10.4% and the N-count was 12 blows per 0.30 m.

##### **4.4 Silt Till or Sandy Silt Till**

Glacial till comprising silt or sandy silt was found in all boreholes (101 to 108). In general, the silt till was moist, compact to very dense and brown with trace to some sand,



gravel and clay. The moisture content ranged from 6.8% to 20.4% (average 11.7%) and the N-count ranged from 14 to 100 blows per 0.30 m (average 37 blows per 0.30 m). The sandy silt fill was generally moist, compact to very dense and brown with trace to some sand, gravel and clay. The moisture content ranged from 7.6% to 26.7% (average 13.4%) and the N-count ranged from 7 to 100 blows per 0.30 m (average 36 blows per 0.30 m).

#### 4.5 Sand or Silty Sand

Sand was found in Boreholes 101, 102, 105 and 107. In general, the sand was very moist to wet, compact and brown with trace silt and/or trace gravel. The moisture content ranged from 14.3% to 20.0% (average 17.2%) and the N-count ranged from 7 to 22 blows per 0.30 m (average 15 blows per 0.30 m).

Silty sand was found in Boreholes 101, 103, 104, 105, 106 and 108. In general, the silty sand was wet to saturated, compact and brown with occasional trace gravel. The moisture content ranged from 10.5% to 27.7% (average 20.8%) and the N-count ranged from 1 to 25 blows per 0.30 m (average 10 blows per 0.30 m).

#### 4.6 Clayey Silt

A localized layer of clayey silt was found in Borehole 107. The clayey silt was moist, compact and brown with trace sand and trace gravel. The moisture content was 13.6% and the N-count was 11 blows per 0.30 m.

#### 4.7 Groundwater Conditions

Monitoring wells were installed in all boreholes. The monitoring wells were constructed utilizing 50 mm Schedule 40 PVC pipe with a 3 m long slot 10 screen surrounded by the sand filter comprising #3 industrial sand. The boreholes were backfilled with 3/8" bentonite holeplug from the top of the sand filter to the existing ground surface. For protection and security purposes, locking steel protective covers were installed on all of the monitoring wells.

A copy of the well record has been included in Appendix E. It is a requirement of Regulation 903 of the Ontario Water Resources Act that the monitoring well installations be abandoned within 180 days after they are no longer in use.

At the time of writing, the static water levels in the monitoring wells had been read on February 8, 2006, February 20, 2006, March 9, 2006, March 25, 2006 and March 29, 2006. A summary showing the ground surface, borehole bottom and water level elevations for Boreholes 101 to 108 are provided below:

Borehole No.	Ground Surface Elevation (m)	Elevation of Borehole Bottom (m)	Elevation of Water Table (m)				
			(F) - Frozen				
			Feb 8, 2006	Feb 20, 2006	Mar 9, 2006	Mar 25, 2006	Mar 29, 2006
101	413.64	408.64	413.07	413.11	412.83	412.96	--
102	414.37	409.37	411.57	411.96	411.91	412.48	--
103	414.89	409.89	412.65	412.98	412.88	412.77	--
104	410.93	407.43	410.36	410.60	410.17	410.66	--
105	414.05	409.28	414.05	414.07 (F)	414.15 (F)	414.15 (F)	414.68
106	410.91	405.94	410.67	410.86 (F)	410.93 (F)	410.75	--
107	409.58	406.08	409.43	409.06 (F)	409.12 (F)	409.41	--
108	410.32	406.82	409.06	409.21	408.82	409.01	--

Due to the close proximity of the groundwater to the ground surface, some of the monitoring wells were frozen at the time of the water level readings.

The groundwater levels will be measured on a monthly basis in an effort to try and establish extreme (high and low) groundwater elevations.

## 5.0 DISCUSSION

It is our understanding that the property owner is proposing to develop a residential subdivision on the property investigated. The subdivision will be fully serviced with municipal sewers and water supply. A storm water management facility is proposed to be constructed in the western portion of the site.

### 5.1 Site Grading

Prior to the commencement of any site grading, all topsoil and organic silt soils (Borehole 103) must be removed from the proposed building envelopes (including extended zone of influence areas), road allowance and driveways.

Due to the high water table and isolated wet surface conditions, it may be necessary to utilize an excavator during topsoil stripping to minimize over-excavation as a result of soil disturbance from heavy construction traffic.

At this time, the proposed founding elevations for the residences are not available. However, it would appear that some cut and fill operations will be required to level the building site.

Prior to any placement of structural fill, the subgrade for the building envelope must be prepared large enough to accommodate a 1:1 slope commencing at a distance of 1.0 m beyond the outside edge of the proposed foundation down to approved native founding soils.

Soils approved for use as structural fill must be placed in loose lifts not exceeding 0.3 m (1 ft) in depth for granular soils and 0.2 m (8") in depth for fine grained (silt and clay) soils and compacted using adequate heavy vibratory padfoot compaction equipment to a minimum of 98% standard Proctor maximum dry density (SPMDD). The approved structural fill materials must be free of frozen materials, organics or other deleterious materials and must not contain particles exceeding 150 mm (6") in diameter. The soils must be at moisture contents suitable to achieve the specified compaction.

A laboratory Proctor moisture-density test was performed on a bulk sample of the silt till from Borehole 103 (depth 1.5 to 2.0 m). The results of the laboratory Proctor test indicate that the optimum moisture content of the sample is 8.3%. Since the insitu moisture contents of the split spoon sample of silt till ranged from 6.8% to 20.4% (average 11.7%), it should be anticipated that the majority of the silt till will require air-drying in order to achieve the specified compaction during construction.

The fine grained soils encountered in the geotechnical investigation are highly susceptible to strength losses if subjected to frequent disturbance by construction traffic. Therefore, it is recommended to minimize construction traffic on subgrade soils.

It would be recommended that the site grading and underground service installation be undertaken during drier warm weather conditions in order to minimize dewatering operations, eliminate frost problems and most importantly improve the placement and compaction of structural fill and backfill materials. Proper compaction and backfilling operations are imperative in order to provide adequate support for structures, service pipes, driveway and roadways.

If site grading and site servicing is undertaken during cold or wet weather conditions, projected overall costs would be anticipated to be higher and the project would be expected to take longer to complete.

## 5.2 Site Dewatering

Based on this geotechnical investigation and similar high water tables encountered during the construction of the neighbouring Ville Lora Downs Subdivision, water concerns should be anticipated for this project. Static water levels measured in the monitoring wells suggest that perched groundwater can be expected at the locations of Boreholes 101, 102, 103, 105 and 106 which were advanced within the proposed residential development area. The water appears to be surface water that has perched on top of the relatively impermeable sandy silt till, sandy silt and silt till soils. Furthermore, artesian conditions can also be expected at the locations of Boreholes 101, 102 and 103. The artesian water appears to be located between the upper sandy silt till and lower silt till in Borehole 101, between the upper and lower silt tills in Borehole 102 and below the silt till layer in Borehole 103.

Provisions for site dewatering should be part of the site development and construction process. Normally, it would be recommended that well points be installed in order to dewater the site so that site services and residential foundations could be installed. However, based on past experience, the installation of a well point dewatering system by qualified contractors can be very expensive and not necessarily guaranteed. It is probably most cost-effective to install a series of inverted drainage pipes in advance of the service (sanitary, storm and water) trench excavations and also at the locations of the manholes. Water pumps should be utilized to pump water from the inverted pipes on a continuous basis in order to keep the water table drawn down below the excavation level. Temporary drainage trenches should be constructed to remove the site water to a storm water retention pond (or reasonable alternative). The removal of considerable amounts of fine soil particles from the pumping operation can be anticipated. As such, the drainage trenches, storm water pond, pumps and hoses will most likely require regular cleanout. It might be cost-effective in regard to road construction and house construction to investigate the possibility of installing a permanent deep drainage system to lower the water table in the immediate area. Caution would be necessary with this option, since it could affect wells and building structures on adjacent properties.

The dewatering conditions may improve if work is conducted during the drier summer months as well as following the installation of the services.

## 5.3 Excavations

The anticipated sanitary, storm and water pipe invert elevations are all expected to be well below the water table and therefore site dewatering will be required (see Section 5.2 above). Based on observations from the neighbouring Ville Lora Downs Subdivision, the water levels in the summer are generally lower and therefore dewatering requirements

may be less. However, the anticipated effects of the artesian water conditions are still expected to be of concern.

All excavations must be carried out in accordance with Ontario Regulation 213/91 (Reg 213/91) of the Occupational Health and Safety Act and Regulations for Construction Projects.

**Type 2 Soils:** The native glacial till soils would be classified as Type 2 soils under Reg 213/91 and must be sloped to within 1.2 m of the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical. Where excavations expose glacial till soils underlain by wet sand or silt soils, the recommendations for Type 4 soils below must be adhered to.

**Type 3 Soils:** The native sand and silt soils in an unsaturated condition (above the water table) would be classified as Type 3 soils under Reg 213/91 and must be sloped from the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical.

**Type 4 Soils:** All native sand or silt soils in a saturated condition (below the water table) would be classified as Type 4 soils under Reg 213/91. Excavations that expose the Type 2 and Type 3 soils noted above but are underlain by saturated sand or silt soils must be treated as Type 4 soils as well. Type 4 soils must be sloped at a minimum gradient of 3 horizontal to 1 vertical. The loose wet condition of the Type 4 soils makes them very susceptible to sloughing and slope failure during excavation.

If it is not practical to excavate according to the above requirements, then a trench box system (designed in accordance with the Ontario Health and Safety Act Regulations) may be utilized.

It should be noted that some of the native glacial till soils become very dense with depth (N-values in excess of 100 blows per 0.30 m) and may prove difficult to excavate with conventional excavating equipment. It is also imperative that when the very dense soils are utilized for backfilling of service trenches, the material must be broken down (pulverized) to minimize voids and reduce the potential for settlement.

#### **5.4 Service Pipe Bedding**

The native soils are generally considered to be suitable for indirect support of the proposed service pipes. Where water inflow is a concern and the soil conditions are not suitable to support the pipe, then 80 mm to 120 mm (3" to 5") river stone (or equivalent) with a 150 mm (6") layer of 19 mm clear stone should be used to create an adequate supporting base for the pipe.

Pipe embedment and backfill for flexible pipes should be undertaken in accordance with OPSD-802.010. Pipe embedment, cover and backfill for rigid pipes should be undertaken in accordance with OPSD-802.030 or OPSD-802.031. Trenching, backfilling and compaction with respect to storm sewer pipe installations should comply with OPSS 514.

**Flexible Pipes:** The pipe bedding should be shaped to receive the bottom of the pipe. If necessary, pipe culvert frost treatment should be undertaken in accordance with OPSD-803.030 and OPSD-803.031. The trench excavations should be symmetrical with respect to the centreline of the pipe. The granular material placed under the haunches of the pipe must be compacted to 95% SPMDD prior to the continued placement and compaction of the embedment material. The homogeneous granular material used for embedment should be placed and compacted uniformly around the pipe. Should wet conditions be encountered at the base of the trench, then the pipe should consist of 19 mm clear stone (meeting OPS Specifications). Normally, it would be advisable to wrap the clear stone with geotextile to prevent fine soils from entering the clear stone and thereby creating voids around the pipe. In wet conditions, this is not possible to do and generally not necessary since most of the void spaces are quickly filled with fine soils as water (with suspended fine soils) rapidly enters the excavation. It is imperative that the newly installed pipe be backfilled as soon as possible in order to prevent the potential for pipe uplift. This can occur due to buoyancy, as water enters the excavation. It is also advisable to check the elevation of the installed pipe at regular intervals to ensure that uplift has not occurred. Protection against heavy construction equipment should be undertaken in accordance with OPSD-808.010.

**Rigid Pipes:** In general, the pipe installation recommendations for rigid pipes are the same as those for flexible pipes except that the minimum depth of bedding below a rigid pipe should be  $0.15 D$  (where  $D$  is the pipe diameter). In no case should this dimension be less than 150 mm or greater than 300 mm.

### 5.5 Trench Backfill

Native backfill material can be used to fill the trench from 12" (30 cm) above the pipe to the subgrade elevation provided that the material is free of organics, not frozen and is not overly wet (above the optimum moisture).

Based on the existing water table, the moisture contents determined from soil samples that were taken during the geotechnical investigation, and the laboratory Proctor test (see Appendix C), it can be assumed that most soils will be too wet to enable proper compaction. As such, these soils should be allowed to drain and air-dry as long as possible before backfilling.

If wet or frozen soils are used for backfill purposes, proper compaction of the backfill will not be possible and settlement of the trenches can be expected. Site assessments will be required to determine what options can be undertaken to construct a suitable road base. These options may include subexcavating and increasing the thickness of the granular subbase, the possible use of high strength geotextiles, or a combination of both.

#### **5.6 Sensitivity of Subsoils**

The silty nature of many of the soils encountered in the boreholes can make them highly susceptible to strength losses and will prove difficult to place and compact if they become overly wet as a result of inclement weather or water seepage. If the soils become overly wet and disturbed, they may become unsuitable for reuse and require subexcavation. As such, the following is recommended:

- provide proper measures for adequate drainage during construction
- use a smooth-lipped bucket while excavating to the subgrade elevation to reduce disturbance
- minimize construction traffic traveling over the subgrade soils

#### **5.7 Road Construction and Pavement Design**

In order to achieve a suitable subgrade for the construction of the pavement structure, the following recommendations are provided:

- a) If necessary, maintain the site dewatering system during preparation of the road subgrade. Once the road subgrade is completed, the drainage pipes should be removed or cut off at the subgrade elevation and infilled with lean concrete or a bentonite slurry.
- b) The design subgrade for the road should be proof-rolled using heavy rubber-tire equipment, such as a grader. Compactive effort should be applied and compaction tests should be undertaken. Areas requiring fill to achieve the subgrade elevation should be treated as indicated above prior to placement of any additional fill. The subgrade should be evaluated to determine if subexcavation and additional Granular 'B' will be required or if the installation of a reinforcing geotextile will be necessary.
- c) The road subgrade should be cut to grade using a smooth-lipped bucket. The subgrade should be graded smooth (with no depressions) and sloped at a minimum of 2%. Construction traffic should not be allowed onto the prepared road subgrade. Construction traffic should travel only on the Granular 'B' subbase. It may be necessary to temporarily



increase the thickness of the Granular 'B' during road construction to accommodate the truck traffic.

d) It is recommended that 100 mm diameter perforated subdrains fitted with a filter sock be installed along each curb line to collect and redirect water beneath the pavement surface. It is suggested that the subdrains be installed in a 0.3 m (1 ft) by 0.3 m (1 ft) trench and placed approximately 50 mm (2") from the trench bottom. In drier conditions, the perforated subdrain with a factory-installed filter sock can be installed in Granular 'A' bedding. In wet conditions, 19 mm clear stone wrapped completely in non-woven geotextile (such as Terrafix 270R or equivalent) is recommended. Rapid drainage of the pavement structure is critical to ensure long-term performance of the road.

Based on the anticipated loading and considering that the subsoils contain frost-susceptible soils, the following pavement design is recommended for the proposed roads:

Material	Recommended Thickness
Asphaltic Concrete	HL3 - 40 mm (1.5") HL4 or HL8 - 50 mm (2.0")
Granular 'A' Base	150 mm (6.0")
Granular 'B' Subbase	450 mm (18.0")

The granular subbase materials should be compacted to 100% SPMDD. Asphaltic concrete should be supplied, placed and compacted to 97% Marshall bulk relative density in accordance with OPSS 1150 and OPSS 310.

The pavement should be designed to ensure that water will not pond on the pavement surface. If the surface asphalt is not placed in a reasonable time following the placement of the binder asphalt, it is recommended that the catch basin lids be lowered or apertures provided to allow the surface water to drain rather than accumulating around the catch basins.

### 5.8 Bearing Capacity / Settlement

The proposed residential buildings may be supported on conventional spread and pier footings provided they are founded on undisturbed native soils at or below the elevations listed in the following table or structural fill prepared as detailed in Section 5.1 of this report:

<b>Borehole No.</b>	<b>Existing Ground Surface Elevation (m)</b>	<b>Highest Recommended Footing Elevation (m)</b>	<b>Soil Type</b>
101	413.64	413.01	sandy silt till
102	414.37	412.70	silt till
103	414.89	413.59	silt till
105	414.05	411.65	sandy silt till
106	410.91	408.51	silt till

It is ideally recommended that foundations be constructed above the water table. The native founding soils and structural fill in a drained condition would be considered suitable to support foundations designed with a safe net allowable bearing capacity of 150 kPa. It is anticipated that the water table may be within one footing width below the founding elevation. Therefore, a safe net allowable bearing capacity of 75 kPa should be used for design purposes.

With respect to the bearing capacities as determined above, total and differential settlements are estimated to be within the generally acceptable limits of 25 mm (1") and 19 mm (3/4") respectively.

A minimum of 1.2 m (4 ft) of soil cover above the footing grade must be provided for frost protection.

### 5.9 Residential Drainage Considerations

If high water conditions continue to exist during the construction of the residential foundations, and the foundations are constructed near or below the water table, then the following will be required:

- a granular drainage layer and sump pump will be required as per Section 9.14.4 of the current Ontario Building Code

- slab-on-grades constructed where groundwater levels may cause hydrostatic pressure must be designed to resist such pressures
- slab-on-grade and exterior walls must be waterproofed

If foundation construction occurs above the high water table, then conventional construction methods can be utilized.

#### **5.10 Potential Storm Water Management Facility**

Boreholes 104, 107 and 108 were all drilled in the open space area (west side of property) where a storm water management facility is proposed. In general, Borehole 104 has silt till underlain by silty sand. It would appear that artesian conditions are present in the silty sand layer below the more impermeable silt till layer. Based on the monitoring well readings, the water level fluctuates to just below the ground surface elevation.

In general, Borehole 107 has sand underlain by clayey silt and lower sandy silt till. Artesian conditions may be present in the sandy silt till below the more impermeable clayey silt layer. Based on the monitoring well readings, the water level was just below the ground surface elevation. The upper sand layer has a high moisture content due to the infiltration of surface water, which is in turn impeded by the lower clayey silt layer.

In general, Borehole 108 has silty sand underlain by sandy silt till. Artesian conditions may be present in the lower portion of the sandy silt layer below the more impermeable silty sand layer (higher density, lower moisture). Based on the monitoring well readings, the water level has fluctuated to within approximately 1.0 m of the ground surface elevation.

Based on the results of the geotechnical investigation, it can be concluded that the soil and groundwater conditions in the area of Boreholes 104, 107 and 108 are unsuitable for an inground storm water management facility.

### **6.0 SITE INSPECTIONS**

Site grading, dewatering, trench excavations, backfilling and compaction of the service pipes should be supervised by qualified geotechnical personnel to ensure that a suitable subbase is prepared, proper backfill materials are used and that the specified compaction is achieved.

The construction of the pavement structure should also be supervised by qualified personnel to ensure that suitable materials are used and that the specified compaction is achieved. It is also

recommended that the residential foundation excavations be examined to ensure that the bearing capacity of the soil is suitable to support the structures.

CMT Engineering Inc. would be pleased to provide inspection, testing and consulting services for this project.

#### **7.0 LIMITATIONS OF THE INVESTIGATION**

This investigation was conducted to determine the subsurface conditions for this project and the comments are based on the information gathered at the borehole locations only. It is therefore assumed that the borehole information is representative of the subsoil conditions across the site. Should any conditions at the site be encountered which differ from those found at the borehole locations, we request that we be notified immediately.

This report is intended solely for the client named. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties.

We trust that this report meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Respectfully submitted,

Robert Koopmans, P.Eng.  
Consulting Engineer



Tim Salter, C.E.T.



ks

Base plan provided by:



QUALITY AND PROFESSIONALISM  
CONSULTING ENGINEERS AND ARCHITECTS



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BOREHOLE LOCATION

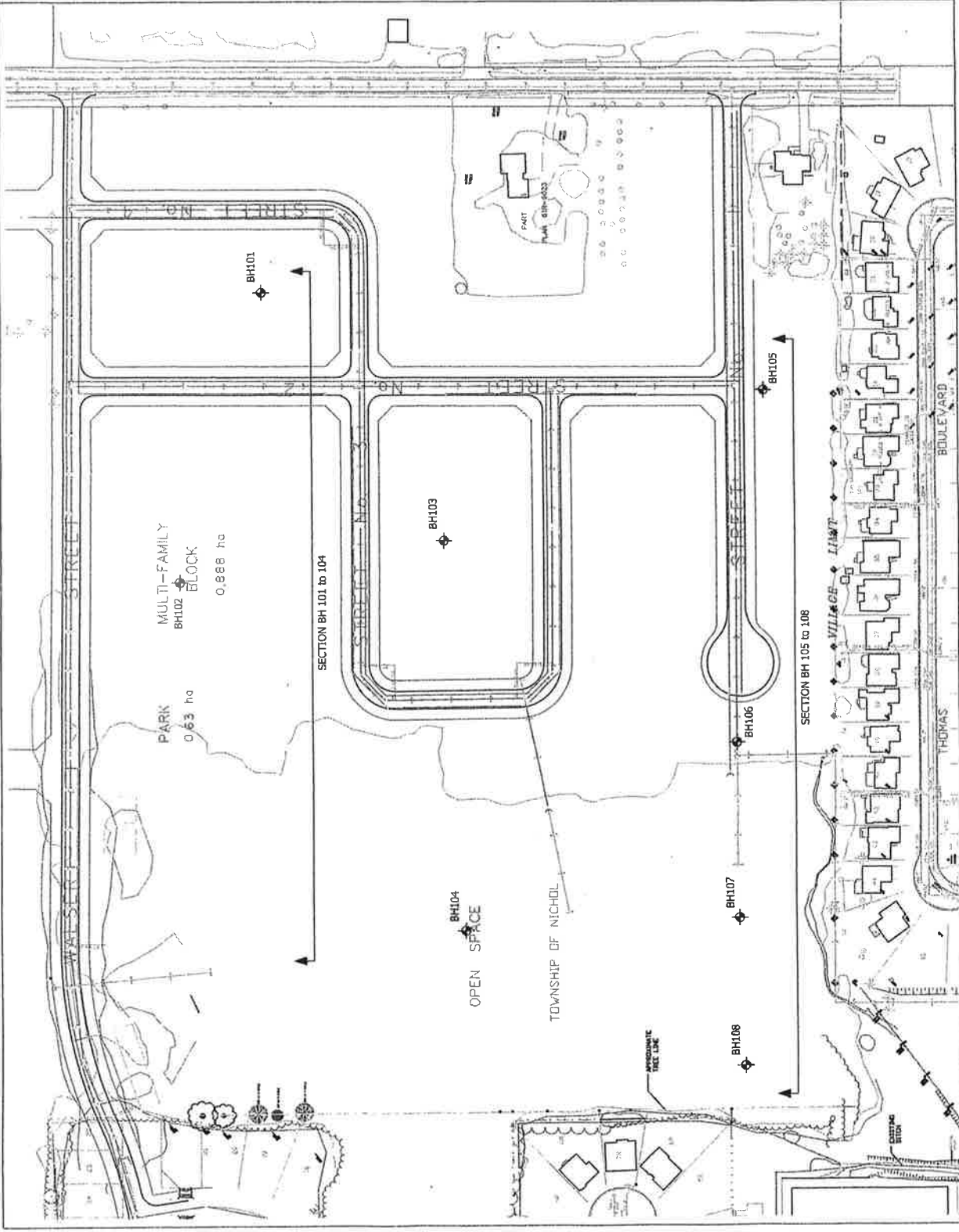
PLAN

Ainley Subdivision  
Township of Centre  
Wellington  
Village of Elora, ON

Project: 06-004 Drawing: 2

Date: Mar. 2006 Sheet: 1

Scale: 1:2000



**APPENDIX A**

**BOREHOLE LOGS  
Boreholes 101 to 108**

# BOREHOLE 101

**Date Drilled:** Jan. 25, 2006  
**Rig:** CME 55  
**Contractor:** Aardvark  
**Drilling Method:** HSA

**Elevation:** 413.64m  
**Logged by:** CD

**Project No.:** 06-004  
**Project:** Ainley Subdivision  
 Township of Centre Wellington  
**Location:** Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content %		Pocket Penetrometer	
							Wp [---X---] Wl	SPT (N)	kPa	Blows/0.3 m
0					Ground Surface (m) 413.64					
0					<i>Topsoil</i> Dark brown silt, frozen 0.00					
1					<i>Sandy Silt Till</i> Compact brown sandy silt till, some clay, some gravel, moist 413.31					
2					0.33					
3	SS		1					9.7		22
4										
5										
6	SS		2		<i>Sand</i> Compact brown sand, very moist to wet 411.84			20.7		20
7					1.80					
8					<i>Silty Sand</i> Compact brown silty sand, wet 411.54					
9	SS		3					24.9		14
10										
11	SS		4		spoon sank through silty sand under weight of rods 2.10			27.7		1
12										
13										
14					<i>Sand</i> Dense brown sand, trace silt, wet 409.64					
15					4.00					
16	SS		5		<i>Silt Till</i> Dense brown silt till, some sand, gravel and clay, moist 408.94			20.4		40
17					4.70					
18					408.64					
19					5.00					
					End of Borehole					

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# BOREHOLE 102

**Date Drilled:** Jan. 25, 2006  
**Rig:** CME 55  
**Contractor:** Aardvark  
**Drilling Method:** HSA

**Elevation:** 414.37m  
**Logged by:** CD

**Project No.:** 06-004  
**Project:** Ainley Subdivision  
 Township of Centre Wellington  
**Location:** Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m	
0					Ground Surface (m) 414.37	<p style="font-size: small;">50mm schedule 40 PVC pipe with slot 10 screen #3 Sand Filter Bentonite Seal Stick up well with protective metal casing 411.57m (Feb 8/06)</p>			
0					<i>Topsoil</i> Dark brown silt, frozen				
1					414.07				
1	SS		1		<i>Sandy Silt</i> Compact mottled brown sandy silt till, trace clay, trace gravel, moist			10.4	12
2					413.00				
2					<i>Silt Till</i> Compact brown silt till, trace sand, trace gravel, moist			17.6	15
3	SS		2		413.00				
4					411.77				
4					<i>Sand</i> Compact brown sand, trace silt, trace gravel, moist to very moist			4.8	30
5	SS		3		411.77				
6					410.37				
6	SS		4		<i>Silt Till</i> Dense grey silt till, trace sand gravel and clay, moist			20.0	22
7					410.37				
8					410.37				
9					410.37				
10					410.37				
11	SS		5		410.37			7.7	53
12					409.37				
13					409.37				
14					409.37				
15					409.37				
16					409.37				
17					End of Borehole 5.00				
18									
19									

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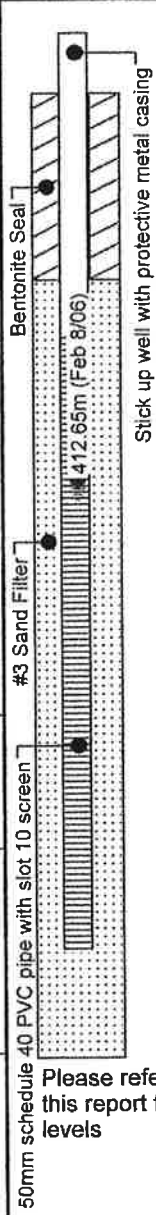
# BOREHOLE 103

**Date Drilled:** Jan. 24, 2006  
**Rig:** CME 55  
**Contractor:** Aardvark  
**Drilling Method:** HSA

**Elevation:** 414.89m  
**Logged by:** CD

**Project No.:** 06-004  
**Project:** Ainley Subdivision  
 Township of Centre Wellington  
**Location:** Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 414.89			
0					Topsoil Dark brown silt, frozen 0.00			
1					414.34			
2					Organic Silt Loose brown organic silt, some sand, occasional topsoil nodule, very moist 0.55			
3	SS		1		413.89			
4					Silt Till Compact brown silt till, trace sand, trace gravel, moist, becoming dense with depth 1.00			
5							13.6	20
6	AS		2					
7								
8	SS		3				9.2	32
9								
10								
11	SS		4		411.54		13.1	37
12					Silty Sand Dense brown silty sand, wet 3.35			
13					410.89			
14					Becoming loose, trace clay, saturated 4.00			
15								
16	SS		5		409.89		23.1	6
17					End of Borehole 5.00			
18								
19								



Please refer to section 4.7 of this report for additional water levels

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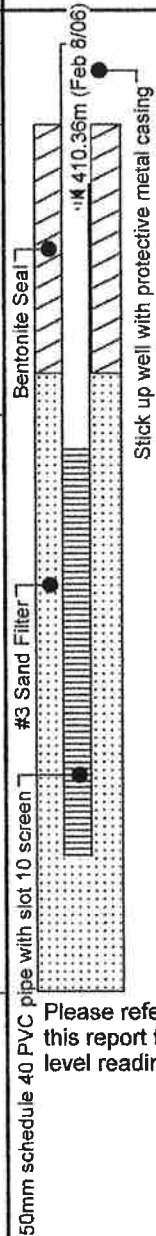
# BOREHOLE 104

**Date Drilled:** Jan. 24, 2006  
**Rig:** CME 55  
**Contractor:** Aardvark  
**Drilling Method:** HSA

**Elevation:** 410.93m  
**Logged by:** CD

**Project No.:** 06-004  
**Project:** Ainley Subdivision  
 Township of Centre Wellington  
**Location:** Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 410.93			
0					<i>Topsail</i> Dark brown silt, frozen			
1					410.63			
1					<i>Silt Till</i> Compact dark brown silt till, some clay, trace sand, trace gravel, moist			
3	SS		1				10.2	14
4					409.56			
5					<i>Silty Sand</i> Compact brown silty sand, saturated			
5	SS		2				23.0	14
6								
7								
8	SS		3				17.8	8
9								
10								
10	SS		4				25.9	10
11					407.43			
11					End of Borehole 3.50			
12								
13								
14								



Please refer to section 4.7 of this report for additional water level readings



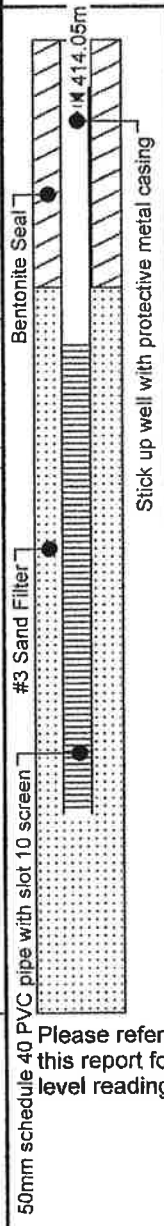
# BOREHOLE 105

Date Drilled: Jan. 25, 2006  
 Rig: CME 55  
 Contractor: Aardvark  
 Drilling Method: HSA

Elevation: 414.05m  
 Logged by: CD

Project No.: 06-004  
 Project: Ainley Subdivision  
 Township of Centre Wellington  
 Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Inst'n ation (Feb 8/06)	Moisture Content %		Pocket Penetrometer	
							Wp [---X---] Wl	SPT (N)	kPa	Blows/0.3 m
							10 20 30 40	10 30 50 70 90	100 300	
0					Ground Surface (m) 414.05					
0					Topsoil 0.00					
1					Dark brown silt, frozen 413.67					
2					Silty Sand 0.38					
2					Loose red-brown silty sand, moist					
3	SS		1					18.5		4
4										
5					Silt 1.37					
5					Loose brown silt, some clay, trace sand, trace gravel, very moist			18.5		8
6	SS		2							
7					Sand 2.00					
7					wet sand seam at 2.0m depth					
8					Sandy Silt Till			7.7		26
8	SS		3		Very dense brown sandy silt till, some clay, trace gravel, moist					
9										
10								8.1		100
10	SS		4							
11										
12										
13										
14										
15								7.6		100
15	SS		5							
16					End of Borehole 4.77					
16										
17										
18										
19										



Please refer to section 4.7 of this report for additional water level readings

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# BOREHOLE 106

**Date Drilled:** Jan. 25, 2006  
**Rig:** CME 55  
**Contractor:** Aardvark  
**Drilling Method:** HSA

**Elevation:** 410.91m  
**Logged by:** CD

**Project No.:** 06-004  
**Project:** Ainley Subdivision  
 Township of Centre Wellington  
**Location:** Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation 50mm schedule 40 PVC pipe with slot 10 screen #3 Sand Filter Bentonite Seal Stick up well with protective metal casing (Feb 8/06)	Moisture Content %		Pocket Penetrometer		
							Wp [---X---] Wl	SPT (N)	kPa	Blows/0.3 m	
10	20	30	40	10	30	50	70	90			
0					Ground Surface (m) 410.91						
0					Topsoil Dark brown silt, frozen 410.59						
0.32					Silty Sand Loose mottled grey silty sand, trace gravel, moist						
1	SS		1					24.0		9	
2											
2.10	SS		2					12.1		9	
3											
408.81					Sandy Silt Till Dense to very dense grey sandy silt till, trace sand, trace gravel, moist						
2.10	SS		3					6.8		33	
3											
3	SS		4					8.3		28	
4											
405.94	SS		5					9.0		100	
4.97					End of Borehole						

**CMT ENGINEERING INC.**  
 1011 Industrial Crescent, Unit 1  
 St. Clements, Ontario N0B 2M0  
 phone 519-699-5775 fax 519-699-4664  
 www.cmtinc.net



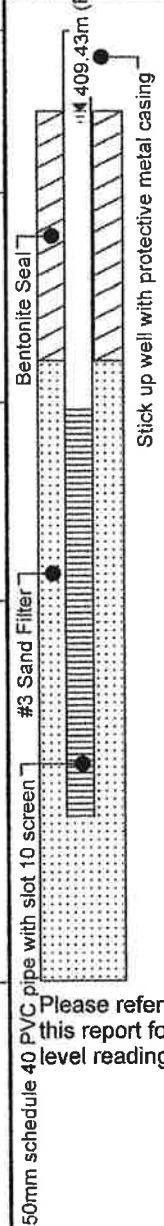
# BOREHOLE 107

**Date Drilled:** Jan. 24, 2006  
**Rig:** CME 55  
**Contractor:** Aardvark  
**Drilling Method:** HSA

**Elevation:** 409.58m  
**Logged by:** CD

**Project No.:** 06-004  
**Project:** Ainley Subdivision  
 Township of Centre Wellington  
**Location:** Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer
								kPa
0					Ground Surface (m) 409.58			
0					Topsoil Dark brown silt, frozen			
2					408.98			
2.60					Sand Loose red-brown sand, some silt, moist to wet			
3	SS		1				14.3	7
4					408.21			
4.137					Clayey Silt Compact brown clayey silt, trace sand, trace gravel, moist			
5	SS		2				13.6	11
6					407.48			
6.210					Sandy Silt Till Compact to loose brown sandy silt till, trace clay, saturated			
7	AS		3				19.1	25
8					406.08			
8.350					End of Borehole			
9	SS		4				17.3	7
10								
11								
12								
13								
14								



Please refer to section 4.7 of this report for additional water level readings

**CMT ENGINEERING INC.**  
 1011 Industrial Crescent, Unit 1  
 St. Clemente, Ontario N0B 2M0  
 phone 519-699-5775 fax 519-699-4664  
 www.cmlinc.net



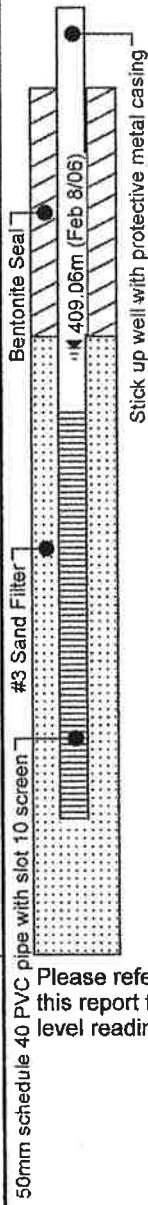
# BOREHOLE 108

Date Drilled: Jan. 25, 2006  
 Rig: CME 55  
 Contractor: Aardvark  
 Drilling Method: HSA

Elevation: 410.32m  
 Logged by: CD

Project No.: 06-004  
 Project: Ainley Subdivision  
 Township of Centre Wellington  
 Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content %		Pocket Penetrometer	
							Wp [---X---] Wl	SPT (N)	kPa	Blows/0.3 m
0					Ground Surface (m) 410.32					
0.00					Topsoil Dark brown silt, frozen					
2					409.74					
0.58					Silty Sand Compact mottled brown silty sand, trace gravel, moist					
1	SS		1				10.5		25	
4					408.95					
1.37					Sandy Silt Till Dense to loose brown sandy silt till, wet					
5	SS		2				16.8		38	
7										
8	SS		3				19.9		18	
9										
10	SS		4				26.7		7	
11					406.82					
3.50					End of Borehole					
12										
13										
14										

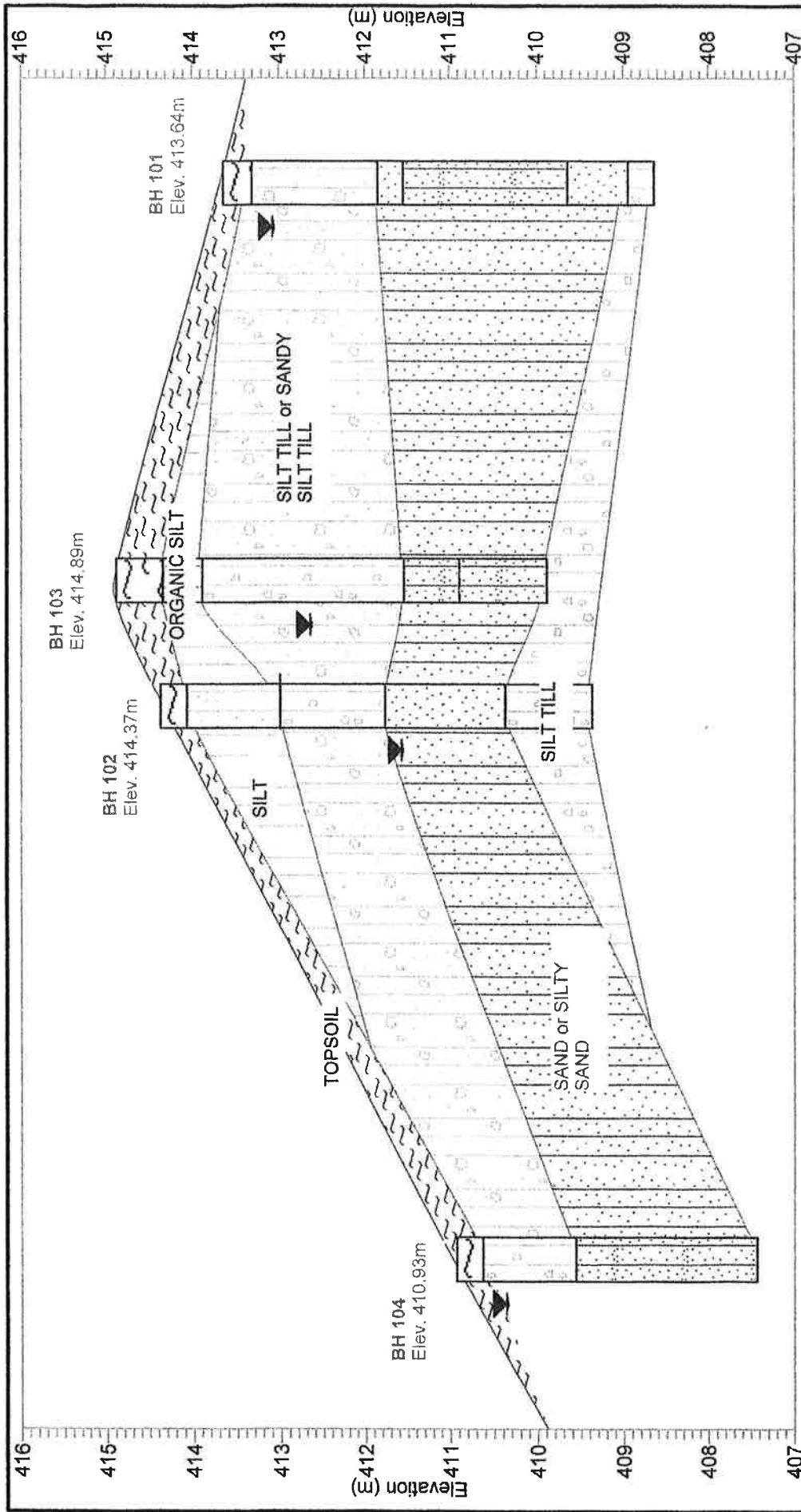


CMT ENGINEERING INC.  
 1011 Industrial Crescent, Unit 1  
 St. Clements, Ontario N0B 2M0  
 phone 519-699-5775 fax 519-699-4664  
 www.cmtinc.net



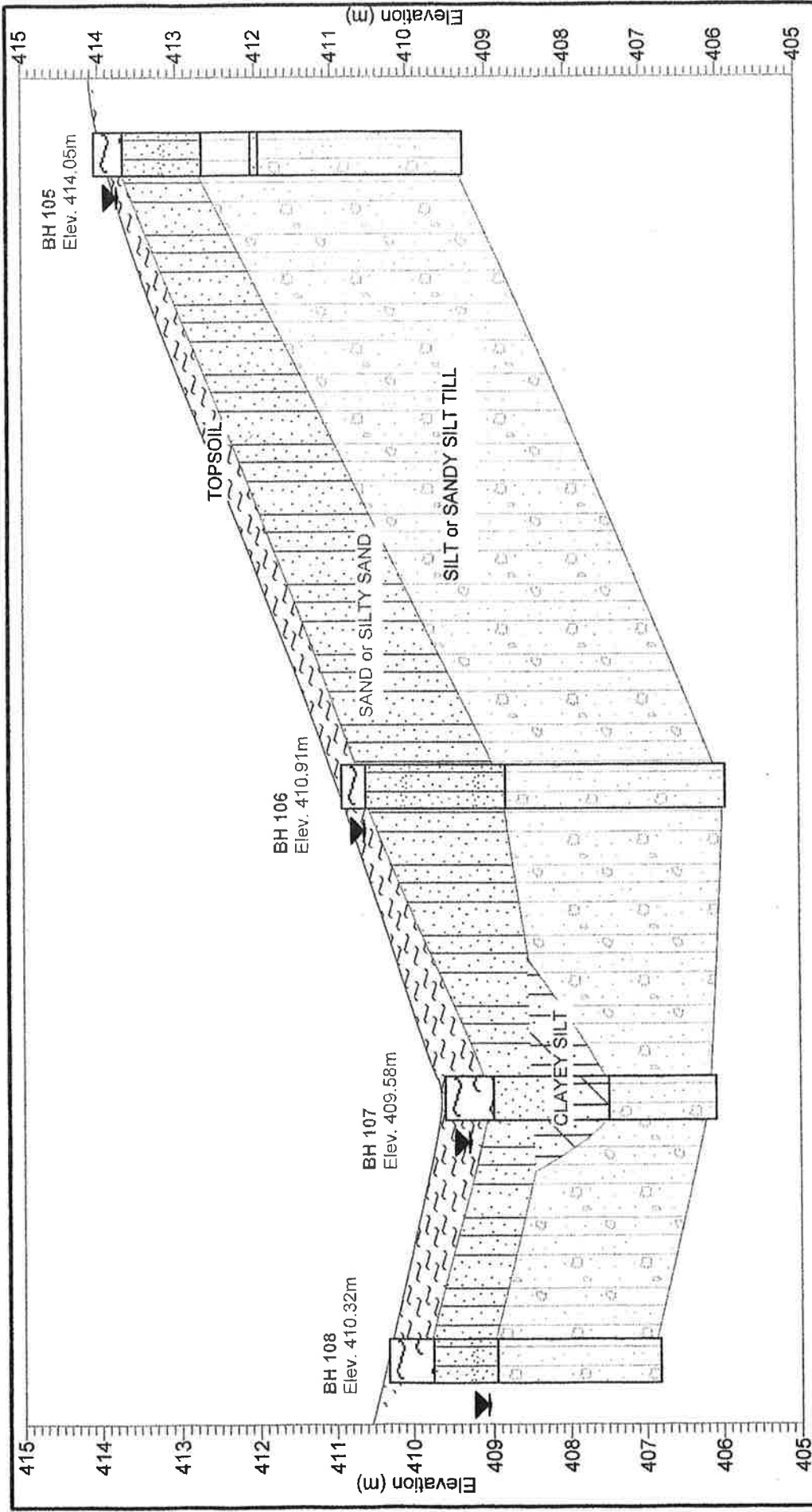


**APPENDIX B**  
**CROSS-SECTIONS**




**NOTE:** Water levels shown were measured on Feb. 8, 2006. Please refer to Section 4.7 of this report for additional water levels.

 <p>1011 Industrial Cres., Unit 1 St. Clements, Ontario</p>	<p><b>Project: AINLEY SUBDIVISION</b></p> <p>Project Number: 06-004</p> <p>Location: Elora, Ontario</p> <p>Drawn By: JS</p> <p>Date: February, 2006</p>
--	---



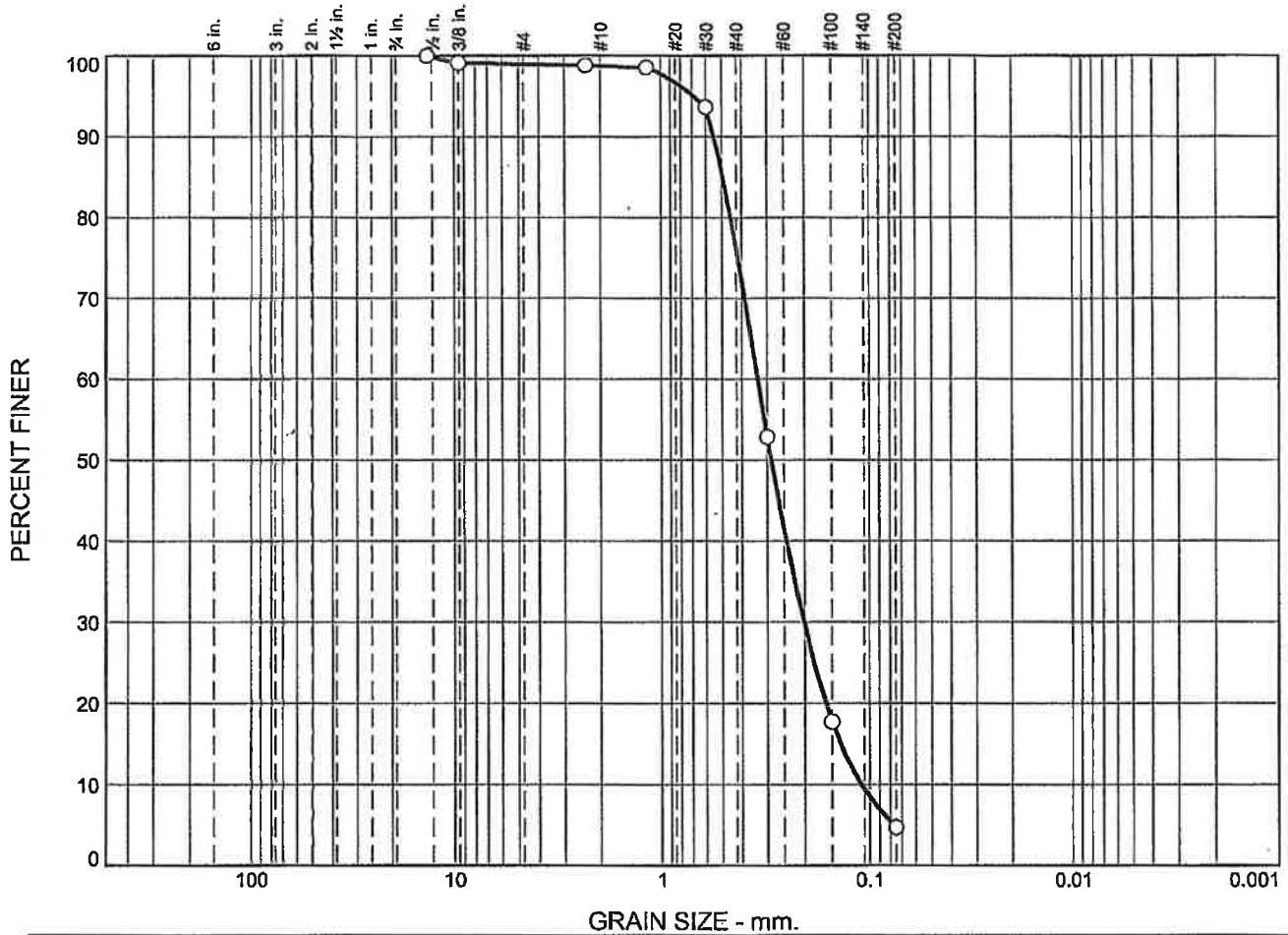
**NOTE:** Water levels shown were measured on Feb. 8, 2006. Please refer to Section 4.7 of this report for additional water levels.

 <p>1011 Industrial Cres., Unit 1 St. Clements, Ontario</p>	<p><b>Project: ANLEY SUBDIVISION</b></p>
	<p>Project Number: 06-004 Location: Elora, Ontario Drawn By: JS Date: February 2006</p>

**APPENDIX C**

**GRAIN SIZE ANALYSES**

# Particle Size Distribution Report



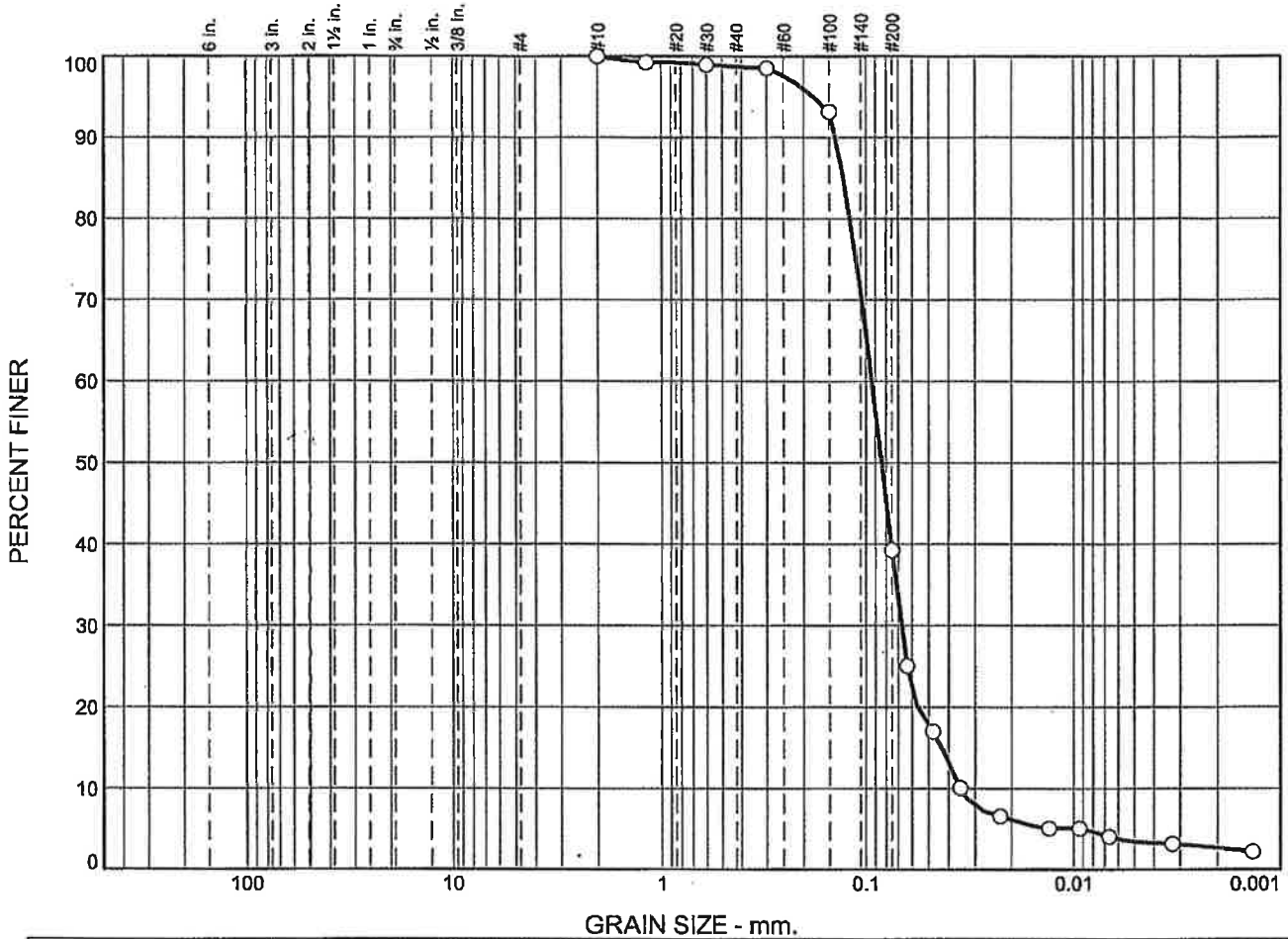
	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	1.0	0.2	22.7	71.4	4.7	

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH102 - SS4	1	3.05-3.51m	Sand, trace silt, trace gravel	SP
				Tested by CMT - January 27, 2006	

**CMT Engineering Inc.**  
**St. Clements, ON**

**Client:** Ainley Subdivision  
**Project:** Township of Centre Wellington  
Elora, Ontario  
**Project No.:** 06-004

# Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	0.0	0.0	1.2	59.5	36.6	2.7

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH103 - SS5	1	4.57-5.03m	Silty sand, trace clay	SM
Tested by CMT - January 27, 2006					

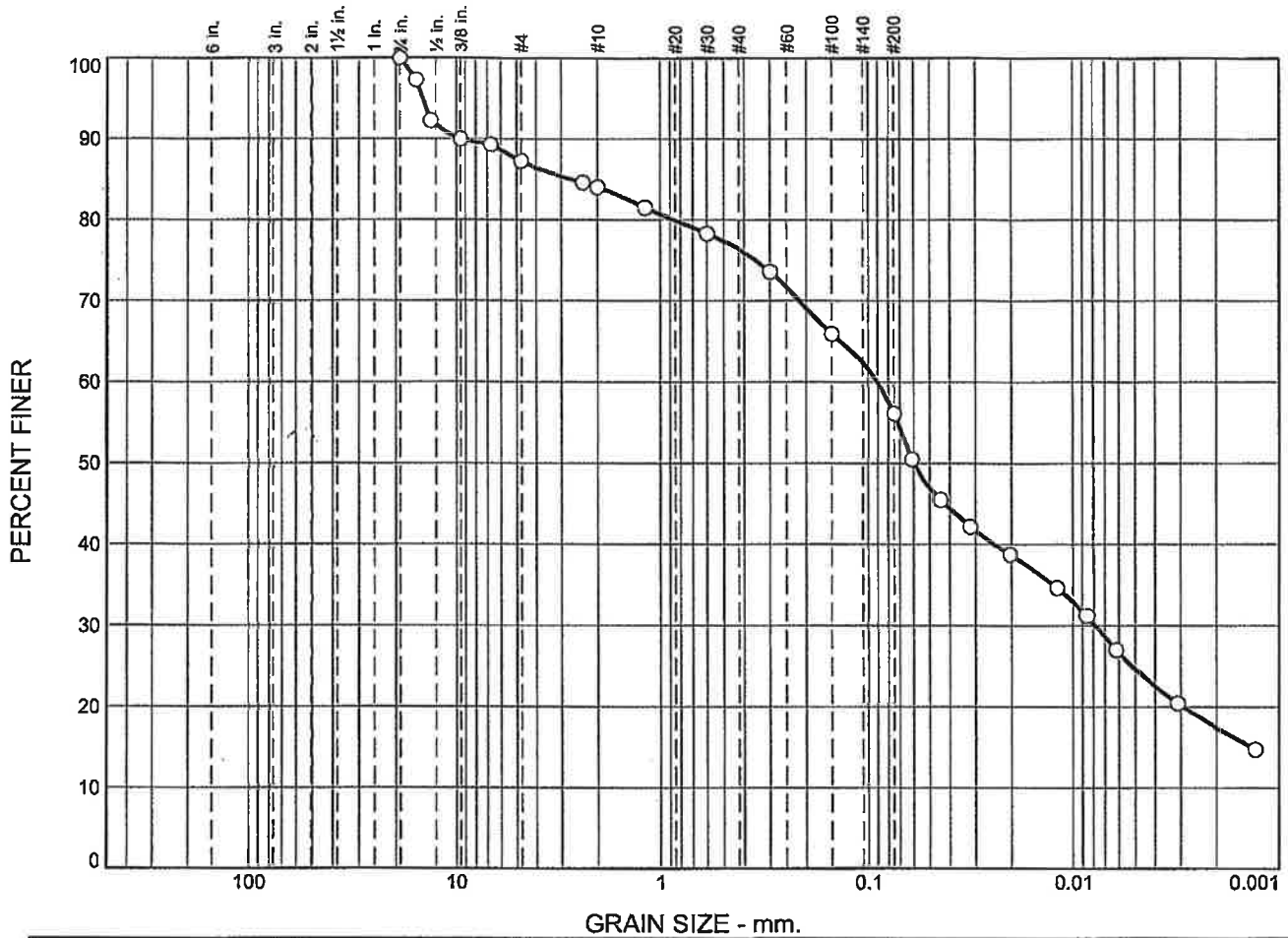
**CMT Engineering Inc.**

**St. Clements, ON**

Client: Ainley Subdivision  
 Project: Township of Centre Wellington  
 Elora, Ontario  
 Project No.: 06-004

Figure 2

# Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	12.8	3.2	7.6	20.3	38.7	17.4

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH105 - SS3	1	2.29-2.74m	Sandy silt, some clay, some gravel	ML
				Tested by CMT - January 27, 2006	

**CMT Engineering Inc.**

**St. Clements, ON**

**Client:** Ainley Subdivision

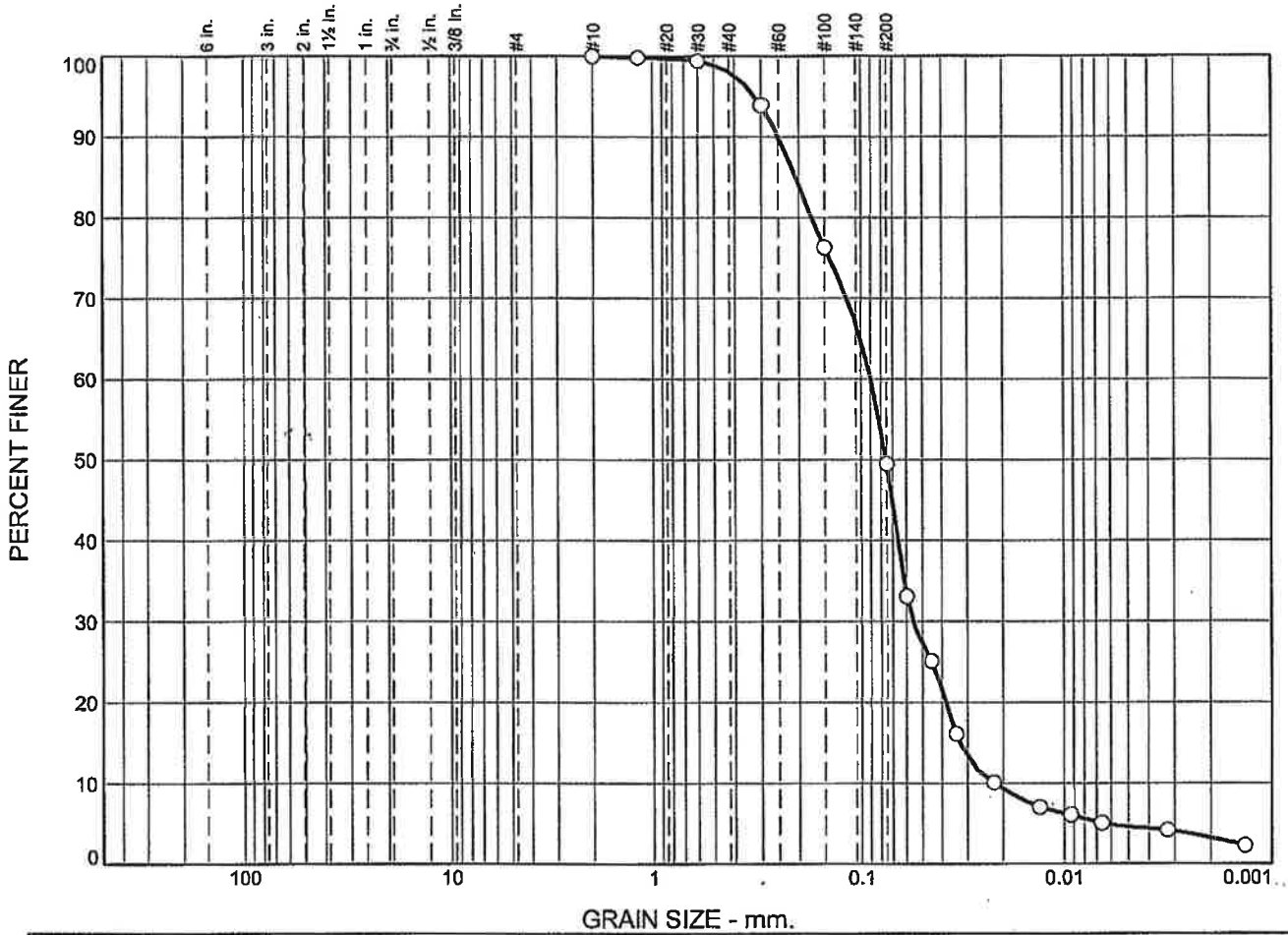
**Project:** Township of Centre Wellington  
Elora, Ontario

**Project No.:** 06-004

**Figure 3**



# Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	0.0	0.0	1.9	48.6	46.3	3.2

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH107 - SS3	1	2.29-2.74m	Sand and silt, trace clay	SM
Tested by CMT - January 27, 2006					

**CMT Engineering Inc.**

**St. Clements, ON**

**Client:** Ainley Subdivision

**Project:** Township of Centre Wellington  
Elora, Ontario

**Project No.:** 06-004

**Figure 4**

**APPENDIX D**  
**LABORATORY PROCTOR TEST**

# CMT ENGINEERING INC.

## LABORATORY PROCTOR TEST

PROJECT NO.: 06-004

PROJECT: Ainley Subdivision

PROJECT LOCATION: Township of Centre Wellington (Elora)

SAMPLED FROM: Borehole 103, BS, depth 1.5 to 2.0 m

DATE SAMPLED/BY: January 25, 2006 by C.D. of CMT Inc.

DATE TESTED/BY: January 26, 2006 by J.S. of CMT Inc.

SOIL TYPE: silt till

REMARKS:

TEST STANDARD:

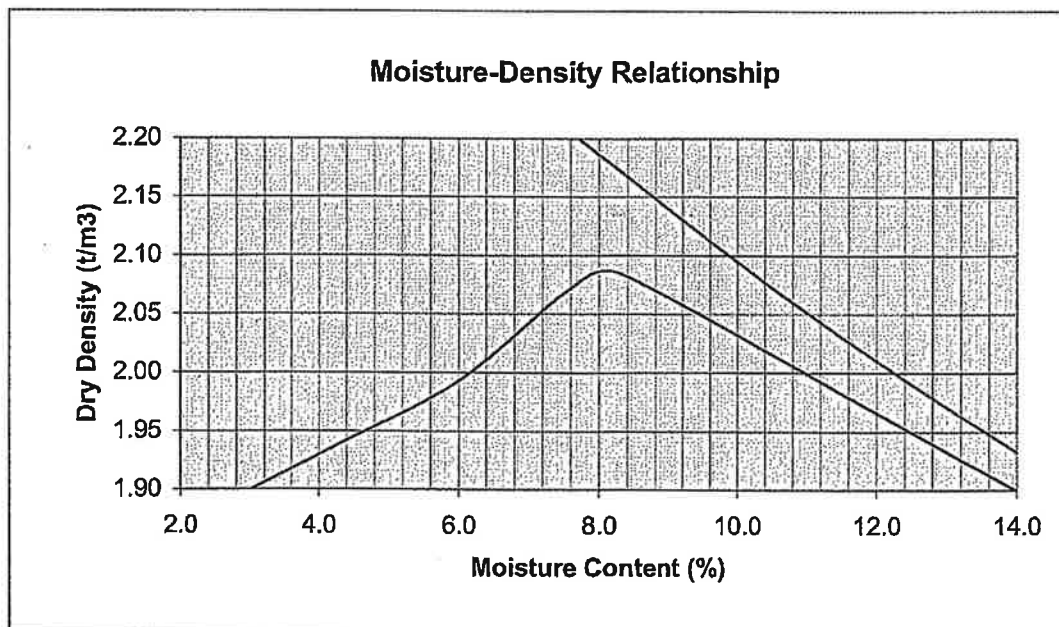
ASTM D698

PROCEDURE -  A  B  C

ASSUMED SPECIFIC GRAVITY: 2.65

MAXIMUM DRY DENSITY: 2.085 t/m<sup>3</sup>

OPTIMUM MOISTURE CONTENT: 8.3%



**APPENDIX E**

**WELL RECORD**

Instructions for Completing Form

- For use in the Province of Ontario only. This document is a permanent legal document. Please retain for future reference.
- All Sections must be completed in full to avoid delays in processing. Further instructions and explanations are available on the back of this form.
- Questions regarding completing this application can be directed to the Water Well Management Coordinator at 416-235-8203.
- All metre measurements shall be reported to 1/10<sup>th</sup> of a metre.
- Please print clearly in blue or black ink only.

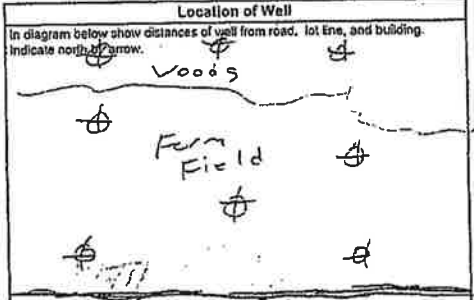
Ministry Use Only					
MUN	CON	LOT			
Well Owner's Information and Location of Well Information					
First Name James		Last Name Keating		Mailing Address (Street Number/Name, RR, Lot, Concession) Matheson St.	
County/District/Municipality		Township/City/Town/Village Elera		Province Ontario	Postal Code
Address of Well Location (County/District/Municipality) Gerric Rd.		Township Center Wellington	Lot 17-18	Concession 12	
RR#/Street Number/Name		City/Town/Village Elera		Site/Compartment/Block/Tract etc.	
GPS Reading	NAD 83	Zone	Easting N058214	Northing	Unit Make/Model
			Mode of Operation: <input type="checkbox"/> Undifferentiated <input type="checkbox"/> Averaged <input checked="" type="checkbox"/> Differentiated, specify		

Log of Overburden and Bedrock Materials (see instructions)

General Colour	Most common material	Other Materials	General Description	Depth From Metres	Metres To
Brown	SILT TILL			0	1.5
Brown	SILT SAND			1.5	4.5

Hole Diameter		Construction Record				Test of Well Yield					
Depth Metres	Diameter Centimetres	Inside diam centimetres	Material	Well thickness centimetres	Depth From Metres	Metres To	Pumping test method	Draw Down Time min	Water Level Metres	Recovery Time min	Water Level Metres
From	To										
0	4.5	72			0	1.5	Pump intake set at - (metres)	Static Level			
Water Record		Casing				Pumping rate - (litres/min)					
Water found at Metres	Kind of Water	<input type="checkbox"/> Steel <input type="checkbox"/> Fibreglass <input checked="" type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized				Duration of pumping _____ hrs + _____ min Final water level and of pumping _____ metres Recommended pump type <input type="checkbox"/> Shallow <input type="checkbox"/> Deep Recommended pump depth _____ metres Recommended pump rate _____ (litres/min) If flowing give rate - _____ (litres/min) If pumping discontinued, give reason.					
<input type="checkbox"/> m <input type="checkbox"/> Fresh <input type="checkbox"/> Sulphur <input type="checkbox"/> Gas <input type="checkbox"/> Salty <input type="checkbox"/> Minerals <input type="checkbox"/> Other:		<input type="checkbox"/> Steel <input type="checkbox"/> Fibreglass <input type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized				Recommended pump rate _____ (litres/min) If flowing give rate - _____ (litres/min) If pumping discontinued, give reason.					
<input type="checkbox"/> m <input type="checkbox"/> Fresh <input type="checkbox"/> Sulphur <input type="checkbox"/> Gas <input type="checkbox"/> Salty <input type="checkbox"/> Minerals <input type="checkbox"/> Other:		Screen				Recommended pump rate _____ (litres/min) If flowing give rate - _____ (litres/min) If pumping discontinued, give reason.					
After test of well yield, water was <input type="checkbox"/> Clear and sediment free <input type="checkbox"/> Other, specify		Outside diam	Material	Slot No.	1.5	4.5					
Chlorinated <input type="checkbox"/> Yes <input type="checkbox"/> No		4	<input checked="" type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized	10	No Casing or Screen						
					<input type="checkbox"/> Open hole						

Plugging and Sealing Record			<input type="checkbox"/> Annular space	<input type="checkbox"/> Abandonment
Depth set at - Metres	Material and type (benonite slurry, neat cement slurry) etc.	Volume Placed (cubic metres)		
From	To			
0	1	Benonite Pellets	100 Lts	
1	4.5	#3 Well Sand	300 Lts	
Method of Construction				
<input type="checkbox"/> Cable Tool	<input type="checkbox"/> Rotary (air)	<input type="checkbox"/> Diamond	<input type="checkbox"/> Digging	
<input checked="" type="checkbox"/> Rotary (conventional)	<input type="checkbox"/> Air percussion	<input type="checkbox"/> Jetting	<input type="checkbox"/> Other	
<input type="checkbox"/> Rotary (reverse)	<input type="checkbox"/> Boring	<input type="checkbox"/> Drilling		
Water Use				
<input type="checkbox"/> Domestic	<input type="checkbox"/> Industrial	<input type="checkbox"/> Public Supply	<input type="checkbox"/> Other	
<input type="checkbox"/> Stock	<input type="checkbox"/> Commercial	<input type="checkbox"/> Not used		
<input type="checkbox"/> Irrigation	<input type="checkbox"/> Municipal	<input type="checkbox"/> Cooling & air conditioning		
Final Status of Well				
<input type="checkbox"/> Water Supply	<input type="checkbox"/> Recharge well	<input type="checkbox"/> Unfinished	<input type="checkbox"/> Abandoned, (Other)	
<input checked="" type="checkbox"/> Observation well	<input type="checkbox"/> Abandoned, insufficient supply	<input type="checkbox"/> Dewatering		
<input checked="" type="checkbox"/> Test Hole	<input type="checkbox"/> Abandoned, poor quality	<input type="checkbox"/> Replacement well		



Gerric Rd	
Audit No. <b>Z 39731</b>	Date Well Completed <b>2006 10 25</b>
Was the well owner's information package delivered? <input checked="" type="checkbox"/> Yes <input type="checkbox"/> No	Date Delivered <b>2006 10 21</b>

Well Contractor/Technician Information	
Name of Well Contractor <b>Anchor Drilling Inc</b>	Well Contractor's Licence No. <b>7738</b>
Business Address (street name, number, city etc.) <b>C-25 Lewis Rd. Suelph ON N1H 1E9</b>	
Name of Well Technician (last name, first name) <b>Henry Matheson</b>	Well Technician's Licence No. <b>7-3305</b>
Signature of Technician/Contractor <i>[Signature]</i>	Date Submitted <b>2006 10 21</b>

Ministry Use Only	
Data Source	Contractor
Date Received <b>YYY MM DD</b>	Date of Inspection <b>YYY MM DD</b>
Remarks	Well Record Number



**APPENDIX B**  
**GROUNDWATER ELEVATION MONITORING**  
**CMT ENGINEERING INC.**  
**JANUARY 2016**



*CMT Engineering Inc.*  
**CONSULTING ENGINEERS**  
1011 Industrial Crescent, Unit 1  
St. Clements, Ontario N0B 2M0  
*Tel: 519-699-5775*  
*Fax: 519-699-4664*  
[www.cmtinc.net](http://www.cmtinc.net)

January 20, 2016

06-004.L85

GM BluePlan Engineering Limited  
330 Trillium Drive, Unit D  
Kitchener, Ontario  
N2E 3J2

Attention: Mr. Glenn Anderson, C.E.T.

Dear Sir:

**Re: Groundwater Monitoring  
Ainley Subdivision  
Elora, Ontario**

---

Attached is a current summary of the water level measurements for the above-referenced site. The graphs include total monthly precipitation as recorded at the Environment Canada Fergus MOE Weather Station.

I trust this information meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

A handwritten signature in blue ink, appearing to read 'Tim Salter', is written over the typed name.

Tim Salter, C.E.T.

ks

1cc: Tom Keating - James Keating Construction Ltd.

Encl - Water Level Measurements

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Feb 8/06	Water Elevation (m) Feb 20/06	Water Elevation (m) Mar 9/06	Water Elevation (m) Mar 25/06	Water Elevation (m) Mar 29/06	Water Elevation (m) Apr 28/06	Water Elevation (m) June 6/06	Water Elevation (m) July 8/06	Water Elevation (m) Aug 7/06	Water Elevation (m) Sept 7/06
101	413.64	413.07	413.11	412.83	412.96	--	412.94	412.59	411.70	411.34	411.00
102	414.37	411.57	411.96	411.91	412.48	--	412.43	412.12	411.78	411.43	411.14
103	414.89	412.65	412.98	412.88	412.77	--	412.76	411.55	410.95	410.43	410.31
104	410.93	410.36	410.60	410.17	410.66	--	410.69	410.15	409.15	408.71	408.52
105	414.05	414.05	414.07 (F)	414.15 (F)	414.15 (F)	414.68	413.44	412.86	412.27	412.06	411.59
106	410.91	410.67	410.86 (F)	410.93 (F)	410.75	--	410.54	410.36	409.93	409.89	409.39
107	409.58	409.43	409.06 (F)	409.12 (F)	409.41	--	409.42	409.03	408.11	408.00	407.63
108	410.32	409.06	409.21	408.82	409.01	--	408.99	408.43	407.94	407.76	407.11

\*(F) = Frozen



WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Oct 6/06	Water Elevation (m) Nov 11/06	Water Elevation (m) Dec 7/06	Water Elevation (m) Jan 9/07	Water Elevation (m) Feb 12/07	Water Elevation (m) Mar 8/07	Water Elevation (m) Apr 10/07	Water Elevation (m) May 12/07	Water Elevation (m) June 11/07	Water Elevation (m) July 11/07
101	413.64	410.83	412.67	412.97	413.03	412.11	411.61	413.02	412.75	411.87	411.42
102	414.37	411.00	411.25	411.71	411.99	411.69	411.45	412.14	411.96	411.67	411.38
103	414.89	410.36	411.10	411.91	412.27	411.05	410.66	412.50	411.59	410.97	410.54
104	410.93	408.71	409.13	409.45	409.65	409.12	408.96	409.78	409.39	409.24	408.92
105	414.05	411.95	413.94	413.71	413.90	412.95	412.65	413.72	413.29	412.51	412.11
106	410.91	410.13	410.74	410.57	410.59	410.28	410.22	410.55	410.50	410.07	409.59
107	409.58	408.28	408.94	409.11	409.16	408.60	408.43	409.25	409.01	408.24	407.71
108	410.32	407.62	408.21	408.48	408.57	408.07	407.93	408.69	408.36	407.96	407.57

\*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Aug 15/07	Water Elevation (m) Sept 13/07	Water Elevation (m) Oct 12/07	Water Elevation (m) Nov 8/07	Water Elevation (m) Dec 11/07	Water Elevation (m) Jan 15/08	Water Elevation (m) Feb 12/08	Water Elevation (m) Mar 8/08	Water Elevation (m) Apr 13/08	Water Elevation (m) May 8/08
101	413.64	411.01	410.72	410.50	410.35	410.33	412.73	412.92	413.00	413.19	413.05
102	414.37	411.13	410.98	410.87	410.81	410.77	411.26	411.53	411.80	412.92	412.46
103	414.89	410.34	410.36	410.36	410.36	410.36	411.29	411.74	412.03	413.40	412.50
104	410.93	408.73	408.61	408.57	408.58	408.76	409.70	409.66	409.81	410.70	410.35
105	414.05	411.67	411.34	411.22	411.22	411.22	414.00	414.11	414.10 frozen	413.99	413.71
106	410.91	409.22	408.99	408.89	408.89	409.44	410.63	410.61	410.64	410.77	410.69
107	409.58	407.49	407.40	407.43	407.52	407.91	409.20	409.05	409.10	409.52	409.35
108	410.32	407.36	407.37	407.37	407.37	407.52	408.52	408.50	408.57	409.19	408.89

\*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) June 10/08	Water Elevation (m) July 8/08	Water Elevation (m) Aug 1/08	Water Elevation (m) Sept 10/08	Water Elevation (m) Oct 8/08	Water Elevation (m) Nov 17/08	Water Elevation (m) Dec 17/08	Water Elevation (m) Jan 23/09	Water Elevation (m) Feb 20/09	Water Elevation (m) Mar 18/09
101	413.64	412.58	412.60	412.55	411.67	410.86	413.08	413.135	412.725	412.960	413.053
102	414.37	412.13	412.06	411.95	411.57	410.30	411.52	411.979	412.147	412.419	412.772
103	414.89	411.53	411.60	411.56	410.86	410.64	412.14	412.746	411.796	412.600	413.046
104	410.93	409.86	409.71	409.76	409.34	408.18	409.98	410.183	409.902	410.238	410.670
105	414.05	412.96	412.87	412.93	412.26	411.60	413.99	413.969	413.221	414.066 (F)	414.070 (F)
106	410.91	410.83	410.24	410.36	410.28	409.04	410.78	410.67	410.567	411.010 (F)	410.963 (F)
107	409.58	409.00	408.72	408.86	408.40	407.36	409.24	409.249	409.009	409.246	409.502
108	410.32	408.33	408.20	408.29	407.95	406.82	408.51	408.662	408.359	408.651	408.948

\*(F) = Frozen



WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Apr 21/09	Water Elevation (m) May 21/09	Water Elevation (m) June 26/09	Water Elevation (m) July 22/09	Water Elevation (m) Aug 27/09	Water Elevation (m) Sept 29/09	Water Elevation (m) Oct 30/09	Water Elevation (m) Dec 7/09	Water Elevation (m) Jan 5/10	Water Elevation (m) Feb 9/10
101	413.64	412.95	412.77	411.93	411.54	411.27	411.16	411.72	412.26	412.04	411.55
102	414.37	412.79	412.43	411.93	411.63	411.28	411.11	411.11	411.15	411.19	411.06
103	414.89	412.88	412.03	411.17	410.80	410.44	410.36	410.57	410.91	410.83	410.47
104	410.93	410.45	410.11	409.61	409.35	409.15	409.04	409.23	409.41	409.36	409.19
105	414.05	413.45	412.98	412.21	411.93	412.05	412.12	412.97	413.25	413.05	412.72
106	410.91	410.55	410.43	410.08	409.87	409.90	410.08	410.33	410.54	410.33	410.22
107	409.58	409.35	409.12	408.62	408.13	407.97	407.97	408.47	408.85	408.81	408.60
108	410.32	408.86	408.51	408.08	407.84	407.66	407.52	407.79	408.01	408.05	407.93

\*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Mar 2/10	Water Elevation (m) Apr 17/10	Water Elevation (m) May 11/10	Water Elevation (m) June 1/10	Water Elevation (m) June 29/10	Water Elevation (m) Aug 5/10	Water Elevation (m) Sept 22/10	Water Elevation (m) Oct 22/10	Water Elevation (m) Nov 9/10	Water Elevation (m) Dec 6/10
101	413.64	411.31	412.70	412.87	412.31	412.92	411.66	410.97	410.83	410.84	411.37
102	414.37	411.01	411.60	411.65	411.64	411.73	411.43	411.09	410.58	410.89	410.93
103	414.89	410.37	411.53	411.73	411.21	411.80	410.76	dry	dry	dry	dry
104	410.93	409.14	409.82	410.09	409.51	409.90	409.17	408.85	408.86	408.89	409.20
105	414.05	412.47	413.22	413.36	412.67	413.37	412.16	411.49	411.75	412.25	413.30
106	410.91	410.12	410.49	410.55	410.15	410.53	409.90	409.50	409.60	409.75	410.33
107	409.58	408.38	409.12	409.34	408.69	409.23	408.06	407.69	407.80	407.88	408.72
108	410.32	407.81	408.43	408.70	408.16	408.42	407.78	407.41	407.44	407.51	407.59

\*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Jan 11/11	Water Elevation (m) Feb 19/11	Water Elevation (m) Mar 31/11	Water Elevation (m) July 19/11	Water Elevation (m) Sept 30/11	Water Elevation (m) Dec 7/11	Water Elevation (m) Feb 10/12	Water Elevation (m) Apr 4/12	Water Elevation (m) June 27/12	Water Elevation (m) Aug 1/12
101	413.64	412.24	412.62	413.09	412.33	411.51	413.21	412.91	412.67	411.22	410.84
102	414.37	411.10	411.09	dry	412.09	411.22	412.23	412.16	412.07	411.28	411.05
103	414.89	410.76	411.02	dry	411.42	410.49	413.25	412.28	411.73	410.45	dry
104	410.93	409.27	409.20	410.05	409.54	409.24	410.45	409.95	409.86	409.06	408.73
105	414.05	413.52	413.93	413.86	412.53	412.83	413.93	413.53	412.99	411.75	411.30
106	410.91	410.37	410.61	410.66	410.09	410.23	410.73	410.52	410.46	409.64	409.17
107	409.58	408.85	408.70	409.29	408.21	408.18	409.44	409.14	409.10	407.93	407.52
108	410.32	408.02	407.89	408.66	407.92	407.73	408.92	408.53	408.38	407.69	dry

\*(F) = Frozen



WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Oct 11/12	Water Elevation (m) Dec 11/12	Water Elevation (m) Mar 9/13	Water Elevation (m) May 3/13	Water Elevation (m) July 3/13	Water Elevation (m) Sept 30/13	Water Elevation (m) Dec 19/13	Water Elevation (m) Feb 19/14	Water Elevation (m) Apr 8/14	Water Elevation (m) June 6/14
101	413.64	410.48	412.25	412.19	412.94	412.74	412.56	412.58	412.28	413.60	412.70
102	414.37	410.81	411.18	411.42	412.52	412.08	411.59	411.98	411.74	412.51	412.47
103	414.89	dry	410.87	411.00	412.71	411.74	411.30	411.60	411.28	413.77	411.95
104	410.93	408.69	409.36	408.32	410.21	409.94	409.63	409.77	409.56	410.71	409.94
105	414.05	dry	413.48	413.12	413.38	412.89	413.15	413.09	412.90	414.02	412.89
106	410.91	409.26	410.53	410.40	410.55	410.44	410.37	410.46	410.40	411.02	410.38
107	409.58	407.61	408.91	408.84	409.32	409.21	408.97	409.02	408.83	409.61	409.00
108	410.32	dry	418.10	408.09	408.74	408.50	408.22	408.28	408.13	409.33	408.35

\*(F) = Frozen

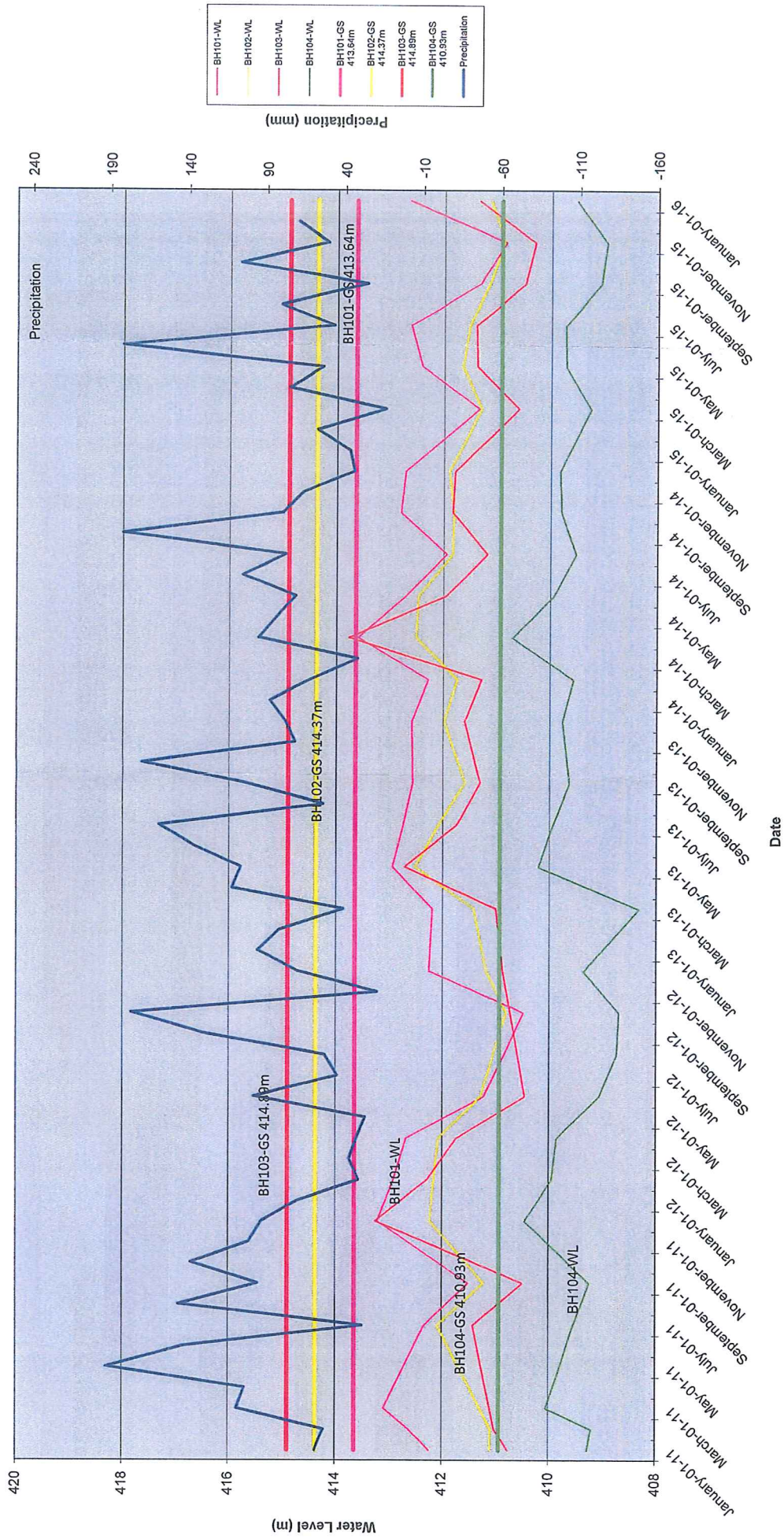
WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION  
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Aug 7/14	Water Elevation (m) Oct 27/14	Water Elevation (m) Dec 12/14	Water Elevation (m) Mar 11/15	Water Elevation (m) May 8/15	Water Elevation (m) July 8/15	Water Elevation (m) Sept 1/15	Water Elevation (m) Nov 5/15	Water Elevation (m) Jan 15/16	Water Elevation (m)
101	413.64	411.94	412.80	412.71	411.34	412.41	412.62	411.33	410.85	412.64	
102	414.37	411.83	411.81	411.90	411.30	411.66	411.53	411.16	410.88	411.14	
103	414.89	411.18	411.83	411.79	410.60	411.38	411.39	410.49	410.31	411.35	
104	410.93	409.52	409.80	409.83	409.25	409.72	409.73	409.07	408.96	409.52	
105	414.05	412.36	413.36	413.20	412.10	412.95	413.29	412.23	412.33	413.67	
106	410.91	410.11	410.55	410.48	410.11	410.33	410.55	409.82	409.91	410.57	
107	409.58	408.41	409.15	409.17	408.21	409.07	409.16	407.91	408.02	409.01	
108	410.32	407.92	408.34	408.40	407.90	408.43	408.48	407.71	407.60	408.36	

\*(F) = Frozen

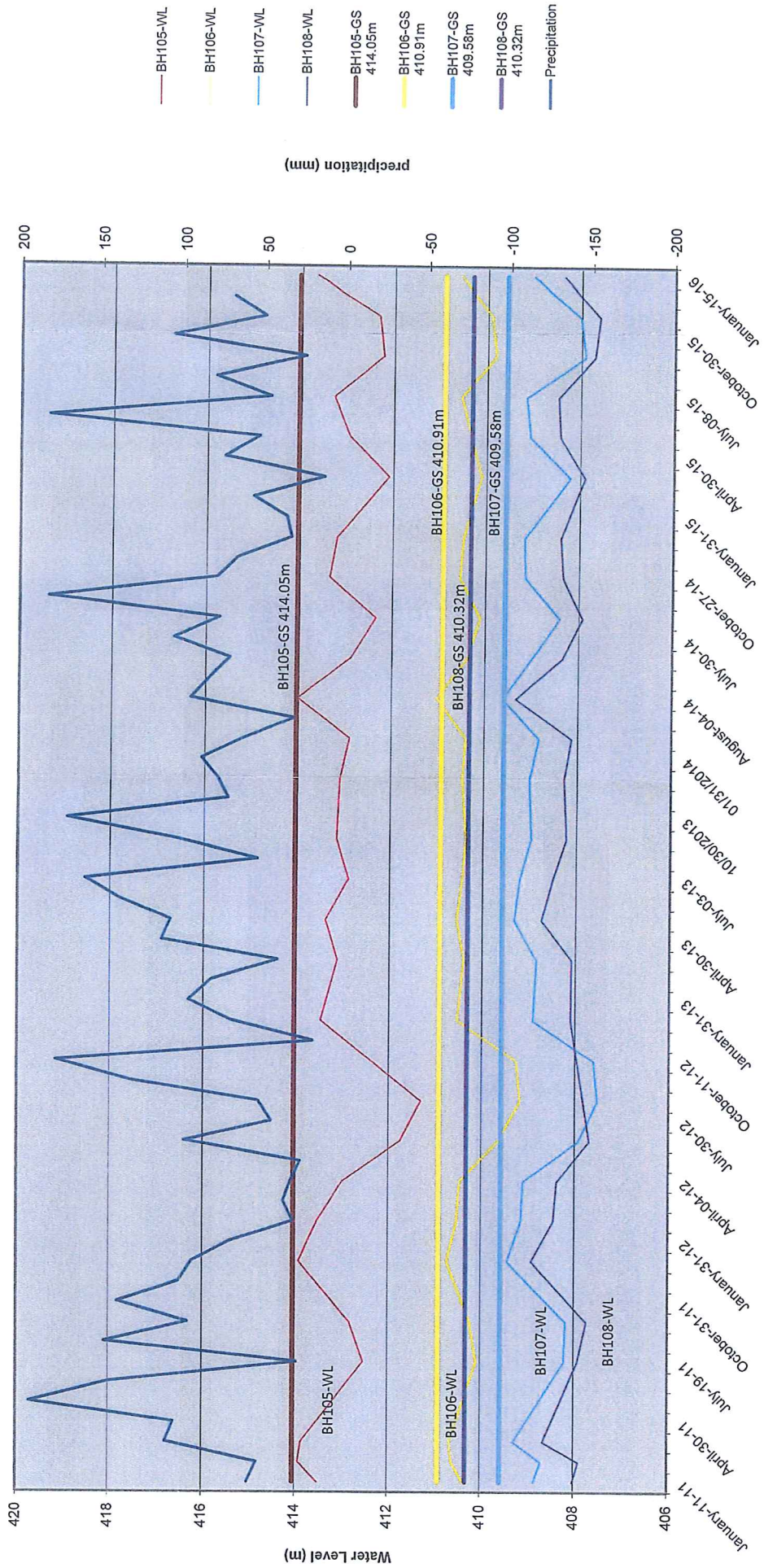


Water Level Measurements - Ainley Subdivision, Elora





### Water Level Measurements - Ainley Subdivision, Elora





**APPENDIX C**  
**SANITARY AND STORM SEWER DESIGN SHEETS**

q = average daily per capita flow (450 L/cap.d)  
 I = unit of peak extraneous flow (0.15 L/ha/s)  
 A = Tributary area in gross hectares  
 M = Peaking factor  
 Q(p) = peak population flow (L/s)  
 Q(i) = peak extraneous flow (L/s)  
 Q(d) = peak design flow

# SANITARY SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

$$M = 1 + \frac{14}{4 + (P)^{1/2}}$$

where P is population in 1000's

$$Q(p) = \frac{PqM}{86.4} \text{ (L/s)}$$

$$Q(i) = IA$$

$$Q(d) = Q(p) + Q(i) \text{ (L/s)}$$

### Ainley Farm Subdivision

Street	From	To	Individual Population	Cumulative Population	Individual Area (ha)	Cumulative Area (ha)	Peaking Factor (M)	Pop. Flow Q(p) (L/s)	Peak Extraneous Flow Q(i) (L/s)	Peak Design Flow Q(d) (m3/s)	Proposed Sewer						
											Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Actual velocity at Q(d)
Street 2	Stub	MH.I	149	149	1.86	1.86	4.000	3.10	0.279	0.0034	10.0	200	0.013	2.00	0.0464	1.476	0.856
Street 2	MH.I	MH.F	0	149	0.21	2.07	4.000	3.10	0.311	0.0034	97.0	200	0.013	1.00	0.0328	1.044	0.668
Street 2	Stub	MH.F	135	135	2.76	2.76	4.000	2.81	0.414	0.0032	43.3	200	0.013	1.00	0.0328	1.044	0.668
Walsler Street	MH.H	MH.G	18	18	0.71	0.71	4.000	0.38	0.107	0.0005	55.0	200	0.013	1.00	0.0328	1.044	0.240
Walsler Street	MH.G	MH.F	28	195	0.56	1.27	4.000	4.06	0.191	0.0043	70.0	200	0.013	0.50	0.0232	0.738	0.568
Walsler Street	MH.F	MH.E	18	362	0.42	3.76	4.000	7.54	0.564	0.0081	82.3	200	0.013	0.50	0.0232	0.738	0.664
Walsler Street	Stub	MH.E	87	87	0.79	0.79	4.000	1.81	0.119	0.0019	10.0	200	0.013	1.00	0.0328	1.044	0.606
Walsler Street	MH.E	MH.D	14	463	0.33	4.88	4.000	9.65	0.732	0.0104	69.8	200	0.013	0.50	0.0232	0.738	0.716
Street 1	Stub	MH.D	150	0	3.61	3.61	4.000	0.00	0.542	0.0005	43.6	200	0.013	1.01	0.0330	1.049	0.346
Walsler Street	MH.D	MH.C	7	470	0.20	8.69	4.000	9.79	1.304	0.0111	42.6	200	0.013	0.49	0.0230	0.731	0.716
Walsler Street	MH.C	MH.B	21	491	0.51	9.20	4.000	10.23	1.380	0.0116	100.0	200	0.013	0.50	0.0232	0.738	0.738
Walsler Street	MH.B	MH.A	14	505	0.32	9.52	4.000	10.52	1.428	0.0119	44.2	200	0.013	0.50	0.0232	0.738	0.746
Walsler Street	MH.A	EX.MH OUTLET A	14	519	0.34	9.86	4.000	10.81	1.479	0.0123	63.3	200	0.013	0.50	0.0232	0.738	0.753
Street 2	MH.J	MH.K	14	14	0.24	0.24	4.000	0.29	0.036	0.0003	42.2	200	0.013	0.80	0.0293	0.934	0.215
Street 2	MH.K	MH.L	28	42	0.54	0.78	4.000	0.88	0.117	0.0010	100.0	200	0.013	0.50	0.0232	0.738	0.362
Street 3	MH.M	MH.N	60	60	1.12	1.06	4.000	1.25	0.159	0.0014	100.0	200	0.013	1.00	0.0328	1.044	0.512
Street 3	MH.N	MH.O	7	67	0.11	1.17	4.000	1.40	0.176	0.0016	18.1	200	0.013	0.50	0.0232	0.738	0.428
Street 3	MH.O	MH.P	0	67	0.04	1.21	4.000	1.40	0.182	0.0016	19.2	200	0.013	1.00	0.0328	1.044	0.595
Street 3	MH.P	MH.Q	21	88	0.40	1.61	4.000	1.83	0.242	0.0021	68.0	200	0.013	0.50	0.0232	0.738	0.465

Project: Ainley Farm Subdivision

q = average daily per capita flow (450 L/cap.d)  
 l = unit of peak extraneous flow (0.15 L/ha/s)  
 A = Tributary area in gross hectares  
 M = Peaking factor  
 Q(p) = peak population flow (L/s)  
 Q(i) = peak extraneous flow (L/s)  
 Q(d) = peak design flow

# SANITARY SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

$$M = 1 + \frac{14}{4 + (P)^{1/2}}$$

where P is population in 1000's

$$Q(p) = \frac{PqM}{86.4} \text{ (L/s)}$$

$$Q(i) = IA$$

$$Q(d) = Q(p) + Q(i) \text{ (L/s)}$$

### Ainley Farm Subdivision

Street	From	To	Individual Population	Cumulative Population	Individual Area (ha)	Cumulative Area (ha)	Peaking Factor (M)	Pop. Flow Q(p) (L/s)	Peak Extraneous Flow Q(i) (L/s)	Peak Design Flow Q(d) (m3/s)	Proposed Sewer						
											Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Actual velocity at Q(d)
Street 3	MH.Q	MH.R	0	88	0.05	1.66	4.000	1.83	0.249	0.0021	20.8	200	0.013	0.50	0.0232	0.738	0.465
Street 3	MH.R	MH.S	4	92	0.13	1.79	4.000	1.92	0.269	0.0022	47.6	200	0.013	0.50	0.0232	0.738	0.465
Street 3	MH.T	MH.S	39	39	0.68	0.68	4.000	0.81	0.102	0.0009	67.6	200	0.013	1.00	0.0328	1.044	0.418
Street 4	MH.S	MH.U	28	159	0.50	2.97	4.000	3.31	0.446	0.0038	77.9	200	0.013	0.50	0.0232	0.738	0.539
Street 4	MH.U	MH.V	0	247	0.00	2.97	4.000	5.15	0.446	0.0056	17.6	200	0.013	0.50	0.0232	0.738	0.613
Street 4	MH.X	MH.W	32	32	0.87	0.87	4.000	0.67	0.131	0.0008	65.0	200	0.013	1.50	0.0402	1.279	0.422
Street 4	MH.W	MH.L	14	46	0.40	1.27	4.000	0.96	0.191	0.0011	65.0	200	0.013	1.50	0.0402	1.279	0.511
Street 2	MH.L	MH.V	39	127	0.87	2.92	4.000	2.65	0.438	0.0031	79.8	200	0.013	1.50	0.0402	1.279	0.780
Sanitary Easement	MH.V	MH.Y	0	374	0.06	5.95	4.000	7.79	0.893	0.0087	54.2	200	0.013	0.50	0.0232	0.738	0.679
Sanitary Easement	MH.Y	MH.Z	0	374	0.06	6.01	4.000	7.79	0.902	0.0087	82.8	200	0.013	0.50	0.0232	0.738	0.679
Sanitary Easement	MH.Z	MH.AA	0	374	0.04	9.02	4.000	7.79	1.353	0.0091	64.3	200	0.013	0.50	0.0232	0.738	0.694
Sanitary Easement	MH.AA	MH.BB	0	374	0.15	9.17	4.000	7.79	1.376	0.0092	62.0	200	0.013	0.50	0.0232	0.738	0.694
Sanitary Easement	MH.BB	MH.CC	0	374	0.04	9.21	4.000	7.79	1.382	0.0092	72.6	200	0.013	0.50	0.0232	0.738	0.694
Sanitary Easement	MH.CC	MH.DD OUTLET B	0	374	0.04	9.25	4.000	7.79	1.388	0.0092	69.8	200	0.013	0.50	0.0232	0.738	0.694

Project: Ainley Farm Subdivision

Chicago Storm Parameters  
 Township of Centre Wellington  
 A = 1459.072  
 B = 13.69  
 C = 0.85  
 Intensity =  $A / (t + B)^C$

# STORM SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

### 5 Year Design

### Ainley Farm Subdivision

$Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m <sup>3</sup> /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
<b>Outlet 1 - Walsler Street Outlet</b>																
WALSER STREET	MH.4	MH.3	0.21	0.45	0.09	0.09	10.00	99.01	<b>0.026</b>	27.4	1200	0.013	1.00	4.051	3.58	0.13
WALSER STREET	MH.3	CBMH.1	0.24	0.45	0.11	0.20	10.13	98.56	<b>0.055</b>	28.3	1200	0.013	1.00	4.051	3.58	0.13
WALSER STREET	CBMH.1	EX CBMH 200	0.35	0.45	0.16	0.36	10.26	98.10	<b>0.098</b>	48.7	300	0.013	1.70	0.131	1.85	0.44
<b>Outlet 2 - Stormwater Management Facility #1, Storm Outlet # 1</b>																
WALSER STREET	MH.5	MH.6	0.00	0.45	0.00	0.00	10.00	99.01	<b>0.000</b>	30.2	300	0.013	0.50	0.071	1.01	0.50
WALSER STREET	MH.6	CBMH.7	0.04	0.45	0.02	0.02	10.50	97.27	<b>0.005</b>	30.0	300	0.013	0.50	0.071	1.01	0.50
WALSER STREET	CBMH.7	MH.9	0.17	0.45	0.08	0.09	11.00	95.60	<b>0.025</b>	29.0	300	0.013	0.50	0.071	1.01	0.48
WALSER STREET	MH.9	CBMH.10	0.34	0.45	0.15	0.25	11.48	94.05	<b>0.065</b>	30.6	300	0.013	0.50	0.071	1.01	0.51
WALSER STREET	CBMH.10	DCBMH.12	0.18	0.45	0.08	0.33	11.99	92.46	<b>0.084</b>	10.0	375	0.013	0.50	0.129	1.17	0.14
STREET NO. 1	MH.39	CBMH.103	0.05	0.45	0.02	0.02	10.00	99.01	<b>0.006</b>	26.6	300	0.013	0.50	0.071	1.01	0.44
STREET NO. 1	CBMH.103	DCBMH.12	0.07	0.45	0.03	0.05	10.44	97.47	<b>0.015</b>	18.0	300	0.013	0.50	0.071	1.01	0.30
WALSER STREET	MH.15	CBMH.13	0.10	0.45	0.05	0.05	10.00	99.01	<b>0.012</b>	27.2	300	0.013	0.50	0.071	1.01	0.45
WALSER STREET	CBMH.13	DCBMH.12	0.16	0.45	0.07	0.12	10.45	97.44	<b>0.032</b>	18.5	300	0.013	0.50	0.071	1.01	0.31
STM EASEMENT	DCBMH.12	MH.64	0.06	0.45	0.03	0.50	12.13	92.03	<b>0.127</b>	74.9	450	0.013	0.50	0.209	1.32	0.95
STM EASEMENT	MH.64	MH.63	0.62	0.32	0.20	0.69	13.08	89.25	<b>0.172</b>	64.8	525	0.013	0.36	0.268	1.24	0.87

Minimum diameter = 300 mm  
 Minimum acceptable velocity = 0.75 m/s  
 Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision  
 File: 411009

Chicago Storm Parameters  
 Township of Centre Wellington  
 A = 1459.072  
 B = 13.69  
 C = 0.85  
 Intensity =  $A / (t + B)^C$

# STORM SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

### 5 Year Design

### Ainley Farm Subdivision

$Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m <sup>3</sup> /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
STREET NO. 3	STUB	MH.44	0.94	0.45	0.42	0.42	10.00	99.01	<b>0.116</b>	14.3	450	0.013	0.50	0.209	1.32	0.18
STREET NO. 3	MH.44	CBMH.59	0.22	0.45	0.10	0.52	10.18	98.38	<b>0.143</b>	60.9	450	0.013	0.50	0.209	1.32	0.77
STREET NO. 3	CBMH.59	DCBMH.61	0.36	0.45	0.16	0.68	10.95	95.75	<b>0.182</b>	66.8	525	0.013	0.50	0.316	1.46	0.76
STREET NO. 3	DCBMH.61	MH.63	0.44	0.45	0.20	0.88	11.71	93.31	<b>0.229</b>	22.3	525	0.013	0.50	0.316	1.46	0.25
STREET NO. 3	MH.63	OGS.3 STORMCEPTOR EF8	0.00	0.45	0.00	1.58	13.95	86.85	<b>0.380</b>	59.6	600	0.013	0.50	0.451	1.60	0.62
STREET NO. 3	OGS.3 STORMCEPTOR EF8	DCBMH.68	0.00	0.45	0.00	1.58	14.57	85.22	<b>0.373</b>	7.2	600	0.013	0.50	0.451	1.60	0.08
STREET NO. 3	DCBMH.68	MH.70	0.50	0.45	0.23	1.80	14.65	85.03	<b>0.425</b>	11.9	675	0.013	0.50	0.618	1.73	0.11
STREET NO. 2	CBMH.47	MH.49	0.17	0.45	0.08	0.08	10.00	99.01	<b>0.021</b>	16.0	300	0.013	0.50	0.071	1.01	0.27
STREET NO. 2	MH.49	MH.50	0.21	0.45	0.09	0.17	10.27	98.08	<b>0.047</b>	18.6	300	0.013	0.50	0.071	1.01	0.31
STREET NO. 2	MH.51	MH.50	0.49	0.45	0.22	0.22	10.00	99.01	<b>0.061</b>	25.5	375	0.013	0.50	0.129	1.17	0.36
STREET NO. 3	MH.50	MH.78	0.00	0.45	0.00	0.39	10.57	97.02	<b>0.106</b>	34.8	375	0.013	0.49	0.128	1.15	0.50
STREET NO. 3	MH.78	CBMH.76	0.17	0.45	0.08	0.47	10.36	97.74	<b>0.127</b>	26.0	450	0.013	0.50	0.209	1.32	0.33
STREET NO. 3	CBMH.76	MH.75	0.40	0.45	0.18	0.65	10.69	96.62	<b>0.174</b>	26.0	450	0.013	0.50	0.209	1.32	0.33
STREET NO. 3	MH.75	MH.74	0.15	0.45	0.07	0.72	11.02	95.52	<b>0.190</b>	14.0	450	0.013	0.50	0.209	1.32	0.18
STREET NO. 3	MH.74	DCBMH.72	0.21	0.45	0.09	0.81	11.20	94.94	<b>0.214</b>	26.9	525	0.013	0.50	0.316	1.46	0.31

Minimum diameter = 300 mm  
 Minimum acceptable velocity = 0.75 m/s  
 Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision  
 File: 411009

Chicago Storm Parameters  
 Township of Centre Wellington  
 A = 1459.072  
 B = 13.69  
 C = 0.85  
 Intensity =  $A / (t + B)^C$

# STORM SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

### 5 Year Design

### Ainley Farm Subdivision

$Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m <sup>3</sup> /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
STREET NO. 3	DCBMH.72	OGS.2 STORMCEPTOR EF6	0.19	0.45	0.09	0.90	11.51	93.96	<b>0.234</b>	11.1	525	0.013	0.50	0.316	1.46	0.13
STREET NO. 3	OGS.2 STORMCEPTOR EF6	MH.70	0.00	0.45	0.00	0.90	11.63	93.56	<b>0.233</b>	18.3	525	0.013	0.50	0.316	1.46	0.21
STREET NO. 3	MH.70	HEADWALL	0.00	0.45	0.00	2.70	14.76	84.74	<b>0.635</b>	18.7	750	0.013	0.50	0.818	1.85	0.17
Outlet 3 - Stormwater Management Facility #1, Storm Outlet # 2																
STREET NO. 4	DCBMH.104	MH.103	0.18	0.45	0.08	0.08	10.00	99.01	<b>0.022</b>	24.3	300	0.013	0.50	0.071	1.01	0.40
STREET NO. 4	MH.103	CBMH.102	0.15	0.45	0.07	0.15	10.40	97.60	<b>0.040</b>	2.8	300	0.013	0.50	0.071	1.01	0.05
STREET NO. 4	CBMH.102	CBMH.99	0.19	0.45	0.09	0.23	10.45	97.45	<b>0.063</b>	67.4	375	0.013	0.50	0.129	1.17	0.96
STREET NO. 4	CBMH.99	CBMH.96	0.20	0.45	0.09	0.32	11.41	94.26	<b>0.085</b>	31.5	450	0.013	0.50	0.209	1.32	0.40
STREET NO. 4	CBMH.96	MH.55	0.34	0.45	0.15	0.48	11.81	93.00	<b>0.123</b>	18.2	450	0.013	0.50	0.209	1.32	0.23
STREET NO. 2	CBMH.53	MH.55	0.16	0.45	0.07	0.07	10.00	99.01	<b>0.020</b>	8.9	300	0.013	0.50	0.071	1.01	0.15
STREET NO. 4	MH.55	MH.94	0.00	0.45	0.00	0.55	12.04	92.30	<b>0.141</b>	32.4	525	0.013	0.50	0.316	1.46	0.37
STREET NO. 4	MH.94	CBMH.91	0.15	0.45	0.07	0.62	12.41	91.18	<b>0.156</b>	24.5	525	0.013	0.50	0.316	1.46	0.28
STREET NO. 4	CBMH.91	MH.88	0.28	0.45	0.13	0.74	12.69	90.36	<b>0.186</b>	17.2	525	0.013	0.50	0.316	1.46	0.20
STREET NO. 4	CBMH.83	CBMH.86	0.19	0.45	0.09	0.09	10.00	99.01	<b>0.024</b>	62.1	300	0.013	0.50	0.071	1.01	1.03
STREET NO. 4	CBMH.86	MH.88	0.19	0.45	0.09	0.17	11.03	95.50	<b>0.045</b>	10.9	300	0.013	0.50	0.071	1.01	0.18

Minimum diameter = 300 mm  
 Minimum acceptable velocity = 0.75 m/s  
 Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision  
 File: 411009



Chicago Storm Parameters  
 Township of Centre Wellington  
 A = 1459.072  
 B = 13.69  
 C = 0.85  
 Intensity =  $A / (t + B)^C$

# STORM SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

### 5 Year Design

### Ainley Farm Subdivision

$Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m <sup>3</sup> /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
SWM EASEMENT	MH.88	OGS.4	0.00	0.45	0.00	0.91	12.89	89.79	<b>0.228</b>	63.9	525	0.013	1.05	0.458	2.12	0.50
SWM EASEMENT	OGS.4	HEADWALL	0.00	0.45	0.00	0.91	13.39	88.37	<b>0.224</b>	6.0	525	0.013	1.00	0.447	2.06	0.05
Outlet 4 - Stormwater Management Facility #2																
WALSER STREET	MH.16	MH.17	0.07	0.45	0.03	0.03	10.00	99.01	<b>0.009</b>	27.0	300	0.013	0.50	0.071	1.01	0.45
WALSER STREET	MH.17	MH.18	0.07	0.45	0.03	0.06	10.45	97.45	<b>0.017</b>	13.3	300	0.013	0.50	0.071	1.01	0.22
WALSER STREET	MH.18	MH.19	1.00	0.68	0.68	0.74	10.67	96.70	<b>0.200</b>	12.2	525	0.013	1.80	0.600	2.77	0.07
STREET NO. 2	MH.40	CBMH.105	0.05	0.45	0.02	0.02	10.00	99.01	<b>0.006</b>	25.7	300	0.013	0.50	0.071	1.01	0.43
STREET NO. 2	CBMH.105	MH.19	0.06	0.45	0.03	0.05	10.00	99.01	<b>0.014</b>	19.9	300	0.013	0.50	0.071	1.01	0.33
STREET NO. 2	DCBMH.41	MH.19	0.15	0.45	0.07	0.07	10.00	99.01	<b>0.019</b>	18.1	300	0.013	0.50	0.071	1.01	0.30
WALSER STREET	MH.19	MH.20	0.00	0.45	0.00	0.83	10.74	96.45	<b>0.223</b>	45.5	525	0.013	1.40	0.529	2.44	0.31
WALSER STREET	MH.20	CBMH.21	0.08	0.45	0.04	0.87	11.05	95.42	<b>0.230</b>	25.0	525	0.013	1.40	0.529	2.44	0.17
WALSER STREET	CBMH.21	DCBMH.23	0.39	0.45	0.18	1.04	11.22	94.87	<b>0.275</b>	63.2	525	0.013	0.80	0.799	1.81	0.58
WALSER STREET	DCBMH.23	DCBMH.24	0.41	0.45	0.18	1.23	11.80	93.02	<b>0.318</b>	8.5	2 - 450	0.013	0.54	0.435	0.99	0.14
WALSER STREET	DCBMH.24	OGS.1 Stormceptor EF8	0.00	0.45	0.00	1.23	11.95	92.58	<b>0.316</b>	5.9	2 - 450	0.013	0.49	0.415	0.94	0.10
WALSER STREET	OGS.1 Stormceptor EF8	POND 2	0.00	0.45	0.00	1.23	12.05	92.26	<b>0.315</b>	5.1	2 - 450	0.013	0.50	0.419	0.95	0.09
	RYCB 08	POND 2	0.06	0.45	0.03	0.03	10.00	99.01	<b>0.007</b>	15.8	300	0.013	0.50	0.071	1.01	0.26

Minimum diameter = 300 mm  
 Minimum acceptable velocity = 0.75 m/s  
 Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision  
 File: 411009

Chicago Storm Parameters  
 Township of Centre Wellington  
 A = 1459.072  
 B = 13.69  
 C = 0.85  
 Intensity =  $A / (t + B)^C$

# STORM SEWER DESIGN

## TOWNSHIP OF CENTRE WELLINGTON (ELORA)

### 5 Year Design

### Ainley Farm Subdivision

$Q = CiA \text{ (m}^3\text{/s)}$

Location			Area (ha)	Runoff Coefficient	A x C	Cumulative A x C	Time of Conc. (min.)	Intensity (mm/hr)	Flow (m <sup>3</sup> /s)	Proposed Sewer						
Street	From	To								Length (m)	Pipe Size (mm)	Type of Pipe	Grade %	Capacity (m <sup>3</sup> /s)	Full Flow Velocity (m/s)	Time of Flow (min.)
Outlet 5 - Infiltration Gallery #1																
STREET NO. 4	MH.101	MH.100	0.11	0.45	0.05	0.05	10.00	99.01	<b>0.014</b>	31.4	300	0.013	0.50	0.071	1.01	0.52
STREET NO. 4	MH.100	MH.96	0.11	0.45	0.05	0.10	10.52	97.20	<b>0.027</b>	31.4	300	0.013	0.50	0.071	1.01	0.52
STREET NO. 4	MH.96	MH.95	0.22	0.45	0.10	0.20	11.04	95.46	<b>0.053</b>	15.0	300	0.013	0.50	0.071	1.01	0.25
STREET NO. 4	MH.95	MH.93	0.05	0.45	0.02	0.22	11.29	94.65	<b>0.058</b>	47.0	300	0.013	0.50	0.071	1.01	0.78
STREET NO. 4	MH.93	MH.90	0.11	0.45	0.05	0.27	12.07	92.21	<b>0.069</b>	31.4	300	0.013	0.50	0.071	1.01	0.52
STREET NO. 4	MH.90	MH.89	0.24	0.45	0.11	0.38	12.59	90.66	<b>0.095</b>	12.3	375	0.013	0.50	0.129	1.17	0.18
STREET NO. 4	MH.85	MH.89	0.21	0.45	0.09	0.09	10.00	99.01	<b>0.026</b>	53.9	300	0.013	0.50	0.071	1.01	0.89
STM EASEMENT	MH.89	MH.119	0.00	0.45	0.00	0.47	12.77	90.14	<b>0.118</b>	37.5	375	0.013	0.50	0.129	1.17	0.54
STM EASEMENT	MH.119	MH.120	0.22	0.45	0.10	0.57	13.30	88.62	<b>0.141</b>	32.6	375	0.013	0.50	0.129	1.17	0.47
STM EASEMENT	MH.120	INFILTRATION GALLERY #1	0.00	0.45	0.00	0.57	13.77	87.34	<b>0.139</b>	8.0	375	0.013	0.50	0.129	1.17	0.11

Minimum diameter = 300 mm  
 Minimum acceptable velocity = 0.75 m/s  
 Maximum acceptable velocity = 4.5 m/s

Project: Ainley Farm Subdivision  
 File: 411009



**APPENDIX D**  
**STORMWATER MANAGEMENT ANALYSIS**

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Ex__2yr.out"
"          Licensee name:                      gmbp"
"          Company                             gmbp"
"          Date & Time last used:              7/25/2022 at 1:42:39 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          695.050 Coefficient A"
"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.293  mm/hr"
"          Total depth                          33.014  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.155  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.003	0.000	0.000	0.000	c.m/sec"
"	Catchment 30	Pervious	Impervious	Total Area	"
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	23.304	1.868	23.304	minutes"
"	Time to Centroid	130.781	88.659	130.781	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	79.23	0.00	79.23	c.m"
"	Rainfall losses	27.898	5.363	27.898	mm"
"	Runoff depth	5.116	27.651	5.116	mm"
"	Runoff volume	12.28	0.00	12.28	c.m"
"	Runoff coefficient	0.155	0.000	0.155	"
"	Maximum flow	0.003	0.000	0.003	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"	0.003	0.003	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"			
"	0.003	0.003	0.003	0.000"	
" 40	HYDROGRAPH Combine 2"				
"	6	Combine "			
"	2	Node #"			
"	To Walser Street"				
"	Maximum flow		0.003	c.m/sec"	
"	Hydrograph volume		12.279	c.m"	
"	0.003	0.003	0.003	0.003"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"			
"	0.003	0.000	0.003	0.003"	
" 33	CATCHMENT 10"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	10	Catchment 10"			
"	0.000	% Impervious"			
"	7.760	Total Area"			
"	150.000	Flow length"			
"	2.000	Overland Slope"			
"	7.760	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.155	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			

"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.044	0.000	0.003	0.003 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	78.068	6.258	78.068	minutes"
"		Time to Centroid	195.540	95.197	195.540	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	2561.88	0.00	2561.88	c.m"
"		Rainfall losses	27.894	5.228	27.894	mm"
"		Runoff depth	5.120	27.786	5.120	mm"
"		Runoff volume	397.31	0.00	397.31	c.m"
"		Runoff coefficient	0.155	0.000	0.155	"
"		Maximum flow	0.044	0.000	0.044	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.044	0.044	0.003	0.003"	
" 33		CATCHMENT 11"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	11	Catchment 11"				
"	0.000	% Impervious"				
"	0.130	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.130	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.155	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.001	0.044	0.003	0.003 c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"
"		Surface Area	0.130	0.000	0.130	hectare"
"		Time of concentration	35.323	2.832	35.323	minutes"
"		Time to Centroid	144.986	90.217	144.986	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	42.92	0.00	42.92	c.m"

"	Rainfall losses	27.897	5.467	27.897	mm"
"	Runoff depth	5.117	27.547	5.117	mm"
"	Runoff volume	6.65	0.00	6.65	c.m"
"	Runoff coefficient	0.155	0.000	0.155	"
"	Maximum flow	0.001	0.000	0.001	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.001 0.045 0.003 0.003"				
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.155 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.060 0.045 0.003 0.003 c.m/sec"				
"	Catchment 40 Pervious Impervious Total Area "				
"	Surface Area 7.120 0.000 7.120 hectare"				
"	Time of concentration 45.052 3.611 45.051 minutes"				
"	Time to Centroid 156.495 91.497 156.495 minutes"				
"	Rainfall depth 33.014 33.014 33.014 mm"				
"	Rainfall volume 2350.59 0.00 2350.59 c.m"				
"	Rainfall losses 27.895 5.642 27.895 mm"				
"	Runoff depth 5.119 27.372 5.119 mm"				
"	Runoff volume 364.45 0.00 364.45 c.m"				
"	Runoff coefficient 0.155 0.000 0.155 "				
"	Maximum flow 0.060 0.000 0.060 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.060 0.099 0.003 0.003"				
" 54	POND DESIGN"				
"	0.099 Current peak flow c.m/sec"				

```

"      0.050  Target outflow    c.m/sec"
"      768.4  Hydrograph volume   c.m"
"      6.     Number of stages"
"  409.630  Minimum water level   metre"
"  410.750  Maximum water level  metre"
"  409.630  Starting water level  metre"
"      0     Keep Design Data: 1 = True; 0 = False"
"          Level Discharge   Volume"
"      409.630    0.000    0.000"
"      409.750    0.6650   402.200"
"      410.000    3.601   2187.900"
"      410.250    7.811   5318.900"
"      410.500   12.984  9642.300"
"      410.750   18.965  15227.70"
"          Peak outflow                0.095    c.m/sec"
"          Maximum level                409.647   metre"
"          Maximum storage              57.616    c.m"
"          Centroidal lag              3.111    hours"
"          0.060    0.099    0.095    0.003 c.m/sec"
" 40     HYDROGRAPH Next link "
"          5     Next link "
"          0.060    0.095    0.095    0.003"
" 52     CHANNEL DESIGN"
"      0.095  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.     Cross-section type: 0=trapezoidal; 1=general"
"      0.000  Basewidth    metre"
"      7.410  Left bank slope"
"      6.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow                0.162    metre"
"          Velocity                    0.541    m/sec"
"          Channel capacity             10.655    c.m/sec"
"          Critical depth               0.133    metre"
" 53     ROUTE    Channel Route 72"
"      72.40   Channel Route 72 Reach length  ( metre)"
"      0.460   X-factor <= 0.5"
" 100.360   K-lag  ( seconds)"
"      0.000   Default(0) or user spec.(1) values used"
"      0.500   X-factor <= 0.5"
"      30.000  K-lag  ( seconds)"
"      0.500   Beta weighting factor"
" 100.000   Routing time step  ( seconds)"
"      1     No. of sub-reaches"
"          Peak outflow                0.095    c.m/sec"
"          0.060    0.095    0.095    0.003 c.m/sec"
" 40     HYDROGRAPH Next link "
"          5     Next link "
"          0.060    0.095    0.095    0.003"

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" 52      CHANNEL DESIGN"
"      0.095  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth    metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow          0.083    metre"
"          Velocity                0.512    m/sec"
"          Channel capacity        9.246    c.m/sec"
"          Critical depth          0.059    metre"
" 53      ROUTE    Channel Route 40"
"      39.80    Channel Route 40 Reach length    ( metre)"
"      0.442    X-factor <= 0.5"
"      58.297    K-lag    ( seconds)"
"      0.000    Default(0) or user spec.(1) values used"
"      0.500    X-factor <= 0.5"
"      30.000    K-lag    ( seconds)"
"      0.500    Beta weighting factor"
"      60.000    Routing time step    ( seconds)"
"          1    No. of sub-reaches"
"          Peak outflow          0.095    c.m/sec"
"              0.060    0.095    0.095    0.003 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5    Next link "
"              0.060    0.095    0.095    0.003"
" 40      HYDROGRAPH Copy to Outflow"
"          8    Copy to Outflow"
"              0.060    0.095    0.095    0.003"
" 40      HYDROGRAPH Combine    1"
"          6    Combine "
"          1    Node #"
"          Total"
"          Maximum flow          0.095    c.m/sec"
"          Hydrograph volume      768.416    c.m"
"              0.060    0.095    0.095    0.095"
" 40      HYDROGRAPH Start - New Tributary"
"          2    Start - New Tributary"
"              0.060    0.000    0.095    0.095"
" 33      CATCHMENT 20"
"          1    Triangular SCS"
"          1    Equal length"
"          1    SCS method"
"          20    Catchment 20"
"          0.000    % Impervious"
"          6.650    Total Area"
"          150.000    Flow length"
"          2.000    Overland Slope"

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"      6.650  Pervious Area"
"    150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"    150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.155  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"    98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.038      0.000      0.095      0.095 c.m/sec"
"      Catchment 20      Pervious      Impervious      Total Area  "
"      Surface Area      6.650      0.000      6.650      hectare"
"      Time of concentration  78.068      6.258      78.068      minutes"
"      Time to Centroid      195.540      95.197      195.539      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      2195.43      0.00      2195.43      c.m"
"      Rainfall losses      27.894      5.228      27.894      mm"
"      Runoff depth      5.120      27.786      5.120      mm"
"      Runoff volume      340.48      0.00      340.48      c.m"
"      Runoff coefficient      0.155      0.000      0.155      "
"      Maximum flow      0.038      0.000      0.038      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.038      0.038      0.095      0.095"
" 33      CATCHMENT 21"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      21      Catchment 20"
"    10.000  % Impervious"
"      0.830  Total Area"
"    40.000  Flow length"
"      2.000  Overland Slope"
"      0.747  Pervious Area"
"    40.000  Pervious length"
"      2.000  Pervious slope"
"      0.083  Impervious Area"
"    40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.155  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"

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"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"  98.000  Impervious SCS Curve No."
"      0.834  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.016      0.038      0.095      0.095 c.m/sec"
"      Catchment 21          Pervious  Impervious Total Area  "
"      Surface Area          0.747      0.083      0.830      hectare"
"      Time of concentration  35.323      2.832      23.162      minutes"
"      Time to Centroid      144.986     90.217     124.487     minutes"
"      Rainfall depth        33.014     33.014     33.014      mm"
"      Rainfall volume        246.61     27.40      274.02      c.m"
"      Rainfall losses        27.897     5.467     25.654      mm"
"      Runoff depth           5.117     27.547     7.360       mm"
"      Runoff volume          38.22     22.86     61.09       c.m"
"      Runoff coefficient      0.155     0.834     0.223       "
"      Maximum flow           0.007     0.016     0.016       c.m/sec"
"  40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.016      0.043      0.095      0.095"
"  40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.016      0.043      0.043      0.095"
"  64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow              0.043      c.m/sec"
"      Hydrograph volume          401.567     c.m"
"  40      HYDROGRAPH  Combine  1"
"      6      Combine  "
"      1      Node #"
"      Total"
"      Maximum flow              0.138     c.m/sec"
"      Hydrograph volume          1169.982     c.m"
"          0.016      0.043      0.043      0.138"
"  38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"      Total Catchment area              22.730     hectare"
"      Total Impervious area              0.083     hectare"
"      Total % impervious                0.365"
"  19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                      Ex__5yr.out"
"          Licensee name:                        gmbp"
"          Company                              gmbp"
"          Date & Time last used:                7/25/2022 at 1:44:05 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          113.586  mm/hr"
"          Total depth                49.792  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.257  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

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	0.010	0.000	0.000	0.000	c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	16.417	1.691	16.417	minutes"
"	Time to Centroid	118.292	87.210	118.292	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	119.50	0.00	119.50	c.m"
"	Rainfall losses	36.983	5.811	36.983	mm"
"	Runoff depth	12.809	43.981	12.809	mm"
"	Runoff volume	30.74	0.00	30.74	c.m"
"	Runoff coefficient	0.257	0.000	0.257	"
"	Maximum flow	0.010	0.000	0.010	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"		0.010	0.010	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"			
"		0.010	0.010	0.010	0.000"
" 40	HYDROGRAPH Combine 2"				
"	6	Combine "			
"	2	Node #"			
"		To Walser Street"			
"			0.010		c.m/sec"
"			30.741		c.m"
"		0.010	0.010	0.010	0.010"
" 40	HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"			
"		0.010	0.000	0.010	0.010"
" 33	CATCHMENT 10"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	10	Catchment 10"			
"	0.000	% Impervious"			
"	7.760	Total Area"			
"	150.000	Flow length"			
"	2.000	Overland Slope"			
"	7.760	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.258	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			

"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.157	0.000	0.010	0.010 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	54.995	5.665	54.994	minutes"
"		Time to Centroid	162.955	92.780	162.955	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	3863.83	0.00	3863.84	c.m"
"		Rainfall losses	36.958	5.466	36.958	mm"
"		Runoff depth	12.834	44.325	12.834	mm"
"		Runoff volume	995.89	0.00	995.90	c.m"
"		Runoff coefficient	0.258	0.000	0.258	"
"		Maximum flow	0.157	0.000	0.157	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.157	0.157	0.010	0.010"	
" 33		CATCHMENT 11"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	11	Catchment 11"				
"	0.000	% Impervious"				
"	0.130	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.130	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.258	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.004	0.157	0.010	0.010 c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"
"		Surface Area	0.130	0.000	0.130	hectare"
"		Time of concentration	24.883	2.563	24.883	minutes"
"		Time to Centroid	128.082	88.517	128.082	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	64.73	0.00	64.73	c.m"

"	Rainfall losses	36.970	6.066	36.969	mm"
"	Runoff depth	12.822	43.726	12.822	mm"
"	Runoff volume	16.67	0.00	16.67	c.m"
"	Runoff coefficient	0.258	0.000	0.258	"
"	Maximum flow	0.004	0.000	0.004	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.004	0.159	0.010	0.010"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.258 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.209	0.159	0.010	0.010 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	31.736	3.269	31.736	minutes"
"	Time to Centroid	136.024	89.581	136.024	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	3545.16	0.00	3545.17	c.m"
"	Rainfall losses	36.968	6.236	36.968	mm"
"	Runoff depth	12.824	43.556	12.824	mm"
"	Runoff volume	913.04	0.00	913.04	c.m"
"	Runoff coefficient	0.258	0.000	0.258	"
"	Maximum flow	0.209	0.000	0.209	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.209	0.353	0.010	0.010"
" 54	POND DESIGN"				
"	0.353 Current peak flow	c.m/sec"			

```

"      0.050  Target outflow    c.m/sec"
"      1925.6 Hydrograph volume  c.m"
"      6.     Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0     Keep Design Data: 1 = True; 0 = False"
"           Level Discharge    Volume"
"           409.630    0.000    0.000"
"           409.750    0.6650   402.200"
"           410.000    3.601   2187.900"
"           410.250    7.811   5318.900"
"           410.500   12.984  9642.300"
"           410.750   18.965  15227.70"
"           Peak outflow                0.324    c.m/sec"
"           Maximum level                409.688  metre"
"           Maximum storage              195.955   c.m"
"           Centroidal lag              2.666   hours"
"           0.209    0.353    0.324    0.010 c.m/sec"
" 40     HYDROGRAPH Next link "
"           5     Next link "
"           0.209    0.324    0.324    0.010"
" 52     CHANNEL DESIGN"
"           0.324  Current peak flow    c.m/sec"
"           0.035  Manning 'n'"
"           0.     Cross-section type: 0=trapezoidal; 1=general"
"           0.000  Basewidth    metre"
"           7.410  Left bank slope"
"           6.000  Right bank slope"
"           0.950  Channel depth    metre"
"           1.040  Gradient    %"
"           Depth of flow                0.256   metre"
"           Velocity                    0.735   m/sec"
"           Channel capacity             10.655   c.m/sec"
"           Critical depth                0.217   metre"
" 53     ROUTE    Channel Route 72"
"           72.40    Channel Route 72 Reach length  ( metre)"
"           0.436  X-factor <= 0.5"
"           73.851  K-lag  ( seconds)"
"           0.000  Default(0) or user spec.(1) values used"
"           0.500  X-factor <= 0.5"
"           30.000  K-lag  ( seconds)"
"           0.500  Beta weighting factor"
"           75.000  Routing time step  ( seconds)"
"           1     No. of sub-reaches"
"           Peak outflow                0.323    c.m/sec"
"           0.209    0.324    0.323    0.010 c.m/sec"
" 40     HYDROGRAPH Next link "
"           5     Next link "
"           0.209    0.323    0.323    0.010"

```



```

" 52      CHANNEL DESIGN"
"      0.323  Current peak flow      c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth      metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth      metre"
"      1.040  Gradient      %"
"          Depth of flow          0.167  metre"
"          Velocity                0.774  m/sec"
"          Channel capacity        9.246  c.m/sec"
"          Critical depth          0.129  metre"
" 53      ROUTE      Channel Route 40"
"      39.80   Channel Route 40 Reach length  ( metre)"
"      0.386   X-factor <= 0.5"
"      38.571  K-lag  ( seconds)"
"      0.000   Default(0) or user spec.(1) values used"
"      0.500   X-factor <= 0.5"
"      30.000  K-lag  ( seconds)"
"      0.500   Beta weighting factor"
"      42.857  Routing time step  ( seconds)"
"          1   No. of sub-reaches"
"          Peak outflow          0.322   c.m/sec"
"          0.209   0.323   0.322   0.010 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5   Next link "
"          0.209   0.322   0.322   0.010"
" 40      HYDROGRAPH Copy to Outflow"
"          8   Copy to Outflow"
"          0.209   0.322   0.322   0.010"
" 40      HYDROGRAPH Combine      1"
"          6   Combine "
"          1   Node #"
"          Total"
"          Maximum flow          0.322   c.m/sec"
"          Hydrograph volume      1925.607  c.m"
"          0.209   0.322   0.322   0.322"
" 40      HYDROGRAPH Start - New Tributary"
"          2   Start - New Tributary"
"          0.209   0.000   0.322   0.322"
" 33      CATCHMENT 20"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"          20  Catchment 20"
"          0.000  % Impervious"
"          6.650  Total Area"
"          150.000  Flow length"
"          2.000  Overland Slope"

```

"	6.650	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.258	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"			0.135	0.000	0.322	0.322 c.m/sec"
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	6.650	0.000	6.650	hectare"
"		Time of concentration	54.995	5.665	54.994	minutes"
"		Time to Centroid	162.956	92.780	162.955	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	3311.14	0.00	3311.15	c.m"
"		Rainfall losses	36.958	5.466	36.958	mm"
"		Runoff depth	12.834	44.325	12.834	mm"
"		Runoff volume	853.44	0.00	853.44	c.m"
"		Runoff coefficient	0.258	0.000	0.258	"
"		Maximum flow	0.135	0.000	0.135	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.135	0.135	0.322	0.322"
" 33		CATCHMENT 21"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	21	Catchment 20"				
"	10.000	% Impervious"				
"	0.830	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.747	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.083	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.258	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				

```

"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"  98.000  Impervious SCS Curve No."
"      0.878  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.030      0.135      0.322      0.322 c.m/sec"
"      Catchment 21          Pervious  Impervious Total Area "
"      Surface Area          0.747      0.083      0.830      hectare"
"      Time of concentration  24.883      2.563      18.750      minutes"
"      Time to Centroid      128.082     88.517     117.210     minutes"
"      Rainfall depth        49.792     49.792     49.792      mm"
"      Rainfall volume       371.94     41.33     413.27      c.m"
"      Rainfall losses       36.970     6.066     33.879      mm"
"      Runoff depth          12.822     43.726     15.913      mm"
"      Runoff volume         95.78     36.29     132.07      c.m"
"      Runoff coefficient     0.258     0.878     0.320      "
"      Maximum flow          0.025     0.021     0.030      c.m/sec"
"  40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.030      0.153      0.322      0.322"
"  40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.030      0.153      0.153      0.322"
"  64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"          Maximum flow          0.153      c.m/sec"
"          Hydrograph volume     985.517      c.m"
"  40      HYDROGRAPH  Combine  1"
"      6      Combine "
"      1      Node #"
"          Total"
"          Maximum flow          0.472      c.m/sec"
"          Hydrograph volume     2911.122      c.m"
"          0.030      0.153      0.153      0.472"
"  38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"          Total Catchment area          22.730      hectare"
"          Total Impervious area          0.083      hectare"
"          Total % impervious          0.365"
"  19      EXIT"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Ex__10yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:46:41 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          2327.596 Coefficient A"
"          19.500  Constant B"
"          0.894  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          126.171  mm/hr"
"          Total depth                61.359  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.316  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"		0.017	0.000	0.000	0.000	c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area	"
"	Surface Area	0.240	0.000	0.240		hectare"
"	Time of concentration	14.182	1.611	14.182		minutes"
"	Time to Centroid	113.893	86.563	113.893		minutes"
"	Rainfall depth	61.359	61.359	61.359		mm"
"	Rainfall volume	147.26	0.00	147.26		c.m"
"	Rainfall losses	41.992	6.044	41.992		mm"
"	Runoff depth	19.367	55.315	19.367		mm"
"	Runoff volume	46.48	0.00	46.48		c.m"
"	Runoff coefficient	0.316	0.000	0.316		"
"	Maximum flow	0.017	0.000	0.017		c.m/sec"
" 40	HYDROGRAPH Add Runoff "					
"	4	Add Runoff "				
"		0.017	0.017	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"					
"	8	Copy to Outflow"				
"		0.017	0.017	0.017	0.000"	
" 40	HYDROGRAPH Combine 2"					
"	6	Combine "				
"	2	Node #"				
"		To Walser Street"				
"			0.017			c.m/sec"
"			46.481			c.m"
"		0.017	0.017	0.017	0.017"	
" 40	HYDROGRAPH Start - New Tributary"					
"	2	Start - New Tributary"				
"		0.017	0.000	0.017	0.017"	
" 33	CATCHMENT 10"					
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	10	Catchment 10"				
"	0.000	% Impervious"				
"	7.760	Total Area"				
"	150.000	Flow length"				
"	2.000	Overland Slope"				
"	7.760	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.316	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				

"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.273	0.000	0.017	0.017 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	47.507	5.395	47.507	minutes"
"		Time to Centroid	151.963	91.698	151.963	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"
"		Rainfall volume	4761.47	0.00	4761.48	c.m"
"		Rainfall losses	41.963	5.633	41.963	mm"
"		Runoff depth	19.396	55.726	19.396	mm"
"		Runoff volume	1505.11	0.00	1505.12	c.m"
"		Runoff coefficient	0.316	0.000	0.316	"
"		Maximum flow	0.273	0.000	0.273	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.273	0.273	0.017	0.017"	
" 33		CATCHMENT 11"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	11	Catchment 11"				
"	0.000	% Impervious"				
"	0.130	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.130	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.316	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.008	0.273	0.017	0.017 c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"
"		Surface Area	0.130	0.000	0.130	hectare"
"		Time of concentration	21.495	2.441	21.495	minutes"
"		Time to Centroid	122.241	87.742	122.240	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"
"		Rainfall volume	79.77	0.00	79.77	c.m"

"	Rainfall losses	41.967	6.310	41.967	mm"
"	Runoff depth	19.392	55.050	19.393	mm"
"	Runoff volume	25.21	0.00	25.21	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.008	0.000	0.008	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.008	0.277	0.017	0.017"	
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.316 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.359	0.277	0.017	0.017 c.m/sec"	
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	27.416	3.114	27.416	minutes"
"	Time to Centroid	128.990	88.727	128.990	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	4368.77	0.00	4368.78	c.m"
"	Rainfall losses	41.968	6.469	41.968	mm"
"	Runoff depth	19.392	54.890	19.392	mm"
"	Runoff volume	1380.67	0.00	1380.68	c.m"
"	Runoff coefficient	0.316	0.000	0.316	"
"	Maximum flow	0.359	0.000	0.359	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.359	0.608	0.017	0.017"	
" 54	POND DESIGN"				
"	0.608 Current peak flow	c.m/sec"			

```

"      0.050  Target outflow    c.m/sec"
"      2911.0 Hydrograph volume  c.m"
"      6.     Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"              409.630    0.000    0.000"
"              409.750    0.6650   402.200"
"              410.000    3.601   2187.900"
"              410.250    7.811   5318.900"
"              410.500   12.984  9642.300"
"              410.750   18.965  15227.70"
"              Peak outflow          0.549    c.m/sec"
"              Maximum level          409.729  metre"
"              Maximum storage        332.166  c.m"
"              Centroidal lag          2.515   hours"
"              0.359    0.608    0.549    0.017 c.m/sec"
" 40      HYDROGRAPH Next link "
"              5      Next link "
"              0.359    0.549    0.549    0.017"
" 52      CHANNEL DESIGN"
"              0.549  Current peak flow  c.m/sec"
"              0.035  Manning 'n'"
"              0.     Cross-section type: 0=trapezoidal; 1=general"
"              0.000  Basewidth    metre"
"              7.410  Left bank slope"
"              6.000  Right bank slope"
"              0.950  Channel depth  metre"
"              1.040  Gradient    %"
"              Depth of flow          0.312   metre"
"              Velocity               0.839   m/sec"
"              Channel capacity        10.655  c.m/sec"
"              Critical depth          0.267   metre"
" 53      ROUTE    Channel Route 72"
"              72.40    Channel Route 72 Reach length  ( metre)"
"              0.422  X-factor <= 0.5"
"              64.729  K-lag  ( seconds)"
"              0.000  Default(0) or user spec.(1) values used"
"              0.500  X-factor <= 0.5"
"              30.000  K-lag  ( seconds)"
"              0.500  Beta weighting factor"
"              60.000  Routing time step  ( seconds)"
"              1      No. of sub-reaches"
"              Peak outflow          0.547    c.m/sec"
"              0.359    0.549    0.547    0.017 c.m/sec"
" 40      HYDROGRAPH Next link "
"              5      Next link "
"              0.359    0.547    0.547    0.017"

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" 52      CHANNEL DESIGN"
"      0.547  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth    metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow          0.224    metre"
"          Velocity                0.914    m/sec"
"          Channel capacity        9.246    c.m/sec"
"          Critical depth          0.179    metre"
" 53      ROUTE    Channel Route 40"
"      39.80   Channel Route 40 Reach length    ( metre)"
"      0.350   X-factor <= 0.5"
"      32.667  K-lag    ( seconds)"
"      0.000   Default(0) or user spec.(1) values used"
"      0.500   X-factor <= 0.5"
"      30.000  K-lag    ( seconds)"
"      0.500   Beta weighting factor"
"      37.500  Routing time step    ( seconds)"
"          1   No. of sub-reaches"
"          Peak outflow          0.546    c.m/sec"
"          0.359    0.547    0.546    0.017 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5   Next link "
"          0.359    0.546    0.546    0.017"
" 40      HYDROGRAPH Copy to Outflow"
"          8   Copy to Outflow"
"          0.359    0.546    0.546    0.017"
" 40      HYDROGRAPH Combine    1"
"          6   Combine "
"          1   Node #"
"          Total"
"          Maximum flow          0.546    c.m/sec"
"          Hydrograph volume      2911.006    c.m"
"          0.359    0.546    0.546    0.546"
" 40      HYDROGRAPH Start - New Tributary"
"          2   Start - New Tributary"
"          0.359    0.000    0.546    0.546"
" 33      CATCHMENT 20"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"          20  Catchment 20"
"          0.000 % Impervious"
"          6.650 Total Area"
"          150.000 Flow length"
"          2.000 Overland Slope"

```

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"      6.650  Pervious Area"
"    150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"    150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.316  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"    98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.234      0.000      0.546      0.546 c.m/sec"
"      Catchment 20      Pervious      Impervious      Total Area  "
"      Surface Area      6.650      0.000      6.650      hectare"
"      Time of concentration  47.507      5.395      47.507      minutes"
"      Time to Centroid      151.963      91.698      151.963      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      4080.39      0.00      4080.39      c.m"
"      Rainfall losses      41.963      5.633      41.963      mm"
"      Runoff depth      19.396      55.726      19.396      mm"
"      Runoff volume      1289.82      0.00      1289.82      c.m"
"      Runoff coefficient      0.316      0.000      0.316      "
"      Maximum flow      0.234      0.000      0.234      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.234      0.234      0.546      0.546"
" 33  CATCHMENT 21"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      21  Catchment 20"
"    10.000  % Impervious"
"      0.830  Total Area"
"    40.000  Flow length"
"      2.000  Overland Slope"
"      0.747  Pervious Area"
"    40.000  Pervious length"
"      2.000  Pervious slope"
"      0.083  Impervious Area"
"    40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.316  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"

```

```

"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"  98.000  Impervious SCS Curve No."
"      0.897  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.050      0.234      0.546      0.546 c.m/sec"
"      Catchment 21          Pervious  Impervious Total Area "
"      Surface Area          0.747      0.083      0.830      hectare"
"      Time of concentration  21.495      2.441      16.926      minutes"
"      Time to Centroid      122.241     87.742     113.968     minutes"
"      Rainfall depth        61.359     61.359     61.359     mm"
"      Rainfall volume        458.35     50.93      509.28     c.m"
"      Rainfall losses        41.967     6.310     38.401     mm"
"      Runoff depth           19.392     55.050     22.958     mm"
"      Runoff volume          144.86     45.69     190.55     c.m"
"      Runoff coefficient      0.316     0.897     0.374     "
"      Maximum flow           0.044     0.024     0.050     c.m/sec"
"  40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.050      0.264      0.546      0.546"
"  40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.050      0.264      0.264      0.546"
"  64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"          Maximum flow              0.264      c.m/sec"
"          Hydrograph volume          1480.378     c.m"
"  40      HYDROGRAPH Combine 1"
"      6      Combine "
"      1      Node #"
"          Total"
"          Maximum flow              0.806      c.m/sec"
"          Hydrograph volume          4391.384     c.m"
"          0.050      0.264      0.264      0.806"
"  38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"          Total Catchment area              22.730     hectare"
"          Total Impervious area              0.083     hectare"
"          Total % impervious                0.365"
"  19      EXIT"

```

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Ex__25yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:47:44 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          3701.648 Coefficient A"
"          25.500  Constant B"
"          0.937  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          143.371  mm/hr"
"          Total depth                75.581  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.376  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

"		0.026	0.000	0.000	0.000	c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area	"
"	Surface Area	0.240	0.000	0.240		hectare"
"	Time of concentration	12.370	1.523	12.370		minutes"
"	Time to Centroid	110.314	85.984	110.314		minutes"
"	Rainfall depth	75.581	75.581	75.581		mm"
"	Rainfall volume	181.39	0.00	181.39		c.m"
"	Rainfall losses	47.190	6.330	47.190		mm"
"	Runoff depth	28.391	69.250	28.391		mm"
"	Runoff volume	68.14	0.00	68.14		c.m"
"	Runoff coefficient	0.376	0.000	0.376		"
"	Maximum flow	0.026	0.000	0.026		c.m/sec"
" 40	HYDROGRAPH Add Runoff "					
"	4 Add Runoff "					
"		0.026	0.026	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"					
"	8 Copy to Outflow"					
"		0.026	0.026	0.026	0.000"	
" 40	HYDROGRAPH Combine 2"					
"	6 Combine "					
"	2 Node #"					
"	To Walser Street"					
"	Maximum flow		0.026			c.m/sec"
"	Hydrograph volume		68.139			c.m"
"		0.026	0.026	0.026	0.026"	
" 40	HYDROGRAPH Start - New Tributary"					
"	2 Start - New Tributary"					
"		0.026	0.000	0.026	0.026"	
" 33	CATCHMENT 10"					
"	1 Triangular SCS"					
"	1 Equal length"					
"	1 SCS method"					
"	10 Catchment 10"					
"	0.000 % Impervious"					
"	7.760 Total Area"					
"	150.000 Flow length"					
"	2.000 Overland Slope"					
"	7.760 Pervious Area"					
"	150.000 Pervious length"					
"	2.000 Pervious slope"					
"	0.000 Impervious Area"					
"	150.000 Impervious length"					
"	2.000 Impervious slope"					
"	0.250 Pervious Manning 'n'"					
"	74.000 Pervious SCS Curve No."					
"	0.377 Pervious Runoff coefficient"					
"	0.100 Pervious Ia/S coefficient"					
"	8.924 Pervious Initial abstraction"					
"	0.015 Impervious Manning 'n'"					
"	98.000 Impervious SCS Curve No."					

```

"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.454      0.000      0.026      0.026 c.m/sec"
"      Catchment 10      Pervious  Impervious  Total Area  "
"      Surface Area      7.760      0.000      7.760      hectare"
"      Time of concentration  41.437      5.102      41.437      minutes"
"      Time to Centroid      143.191      90.751      143.190      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      5865.07      0.01      5865.07      c.m"
"      Rainfall losses      47.093      5.908      47.093      mm"
"      Runoff depth      28.488      69.673      28.488      mm"
"      Runoff volume      2210.64      0.01      2210.65      c.m"
"      Runoff coefficient      0.377      0.000      0.377      "
"      Maximum flow      0.454      0.000      0.454      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.454      0.454      0.026      0.026"
" 33      CATCHMENT 11"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      11     Catchment 11"
"      0.000  % Impervious"
"      0.130  Total Area"
"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.130  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.376  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.012      0.454      0.026      0.026 c.m/sec"
"      Catchment 11      Pervious  Impervious  Total Area  "
"      Surface Area      0.130      0.000      0.130      hectare"
"      Time of concentration  18.749      2.308      18.749      minutes"
"      Time to Centroid      117.510      87.059      117.510      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      98.25      0.00      98.26      c.m"

```

"		Rainfall losses	47.127	6.593	47.127	mm"
"		Runoff depth	28.453	68.988	28.453	mm"
"		Runoff volume	36.99	0.00	36.99	c.m"
"		Runoff coefficient	0.376	0.000	0.376	"
"		Maximum flow	0.012	0.000	0.012	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.012	0.461	0.026	0.026"
" 33		CATCHMENT 40"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	40	Catchment 40"				
"	0.000	% Impervious"				
"	7.120	Total Area"				
"	60.000	Flow length"				
"	2.000	Overland Slope"				
"	7.120	Pervious Area"				
"	60.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	60.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.377	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"			0.584	0.461	0.026	0.026 c.m/sec"
"		Catchment 40	Pervious	Impervious	Total Area	"
"		Surface Area	7.120	0.000	7.120	hectare"
"		Time of concentration	23.913	2.944	23.913	minutes"
"		Time to Centroid	123.357	87.974	123.357	minutes"
"		Rainfall depth	75.581	75.581	75.581	mm"
"		Rainfall volume	5381.35	0.01	5381.36	c.m"
"		Rainfall losses	47.108	6.942	47.107	mm"
"		Runoff depth	28.473	68.639	28.473	mm"
"		Runoff volume	2027.30	0.00	2027.30	c.m"
"		Runoff coefficient	0.377	0.000	0.377	"
"		Maximum flow	0.584	0.000	0.584	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.584	1.001	0.026	0.026"
" 54		POND DESIGN"				
"	1.001	Current peak flow	c.m/sec"			

```

"      0.050  Target outflow    c.m/sec"
"      4274.9 Hydrograph volume  c.m"
"      6.    Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0     Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          409.630    0.000    0.000"
"          409.750    0.6650   402.200"
"          410.000    3.601   2187.900"
"          410.250    7.811   5318.900"
"          410.500   12.984  9642.300"
"          410.750   18.965  15227.70"
"          Peak outflow                0.886    c.m/sec"
"          Maximum level                409.769  metre"
"          Maximum storage              536.694   c.m"
"          Centroidal lag                2.394   hours"
"          0.584    1.001    0.886    0.026 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.584    0.886    0.886    0.026"
" 52    CHANNEL DESIGN"
"      0.886  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      0.000  Basewidth    metre"
"      7.410  Left bank slope"
"      6.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow                0.374   metre"
"          Velocity                    0.946   m/sec"
"          Channel capacity              10.655   c.m/sec"
"          Critical depth                0.324   metre"
" 53    ROUTE    Channel Route 72"
"      72.40   Channel Route 72 Reach length  ( metre)"
"      0.407   X-factor <= 0.5"
"      57.429  K-lag  ( seconds)"
"      0.000   Default(0) or user spec.(1) values used"
"      0.500   X-factor <= 0.5"
"      30.000  K-lag  ( seconds)"
"      0.500   Beta weighting factor"
"      60.000  Routing time step  ( seconds)"
"      1     No. of sub-reaches"
"          Peak outflow                0.880    c.m/sec"
"          0.584    0.886    0.880    0.026 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.584    0.880    0.880    0.026"

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" 52      CHANNEL DESIGN"
"      0.880  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth    metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow          0.291    metre"
"          Velocity                1.055    m/sec"
"          Channel capacity        9.246    c.m/sec"
"          Critical depth          0.239    metre"
" 53      ROUTE    Channel Route 40"
"      39.80   Channel Route 40 Reach length    ( metre)"
"      0.310   X-factor <= 0.5"
"      28.289  K-lag    ( seconds)"
"      0.000   Default(0) or user spec.(1) values used"
"      0.500   X-factor <= 0.5"
"      30.000  K-lag    ( seconds)"
"      0.500   Beta weighting factor"
"      37.500  Routing time step    ( seconds)"
"          1   No. of sub-reaches"
"          Peak outflow          0.877    c.m/sec"
"          0.584    0.880    0.877    0.026 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5   Next link "
"          0.584    0.877    0.877    0.026"
" 40      HYDROGRAPH Copy to Outflow"
"          8   Copy to Outflow"
"          0.584    0.877    0.877    0.026"
" 40      HYDROGRAPH Combine    1"
"          6   Combine "
"          1   Node #"
"          Total"
"          Maximum flow          0.877    c.m/sec"
"          Hydrograph volume      4274.952    c.m"
"          0.584    0.877    0.877    0.877"
" 40      HYDROGRAPH Start - New Tributary"
"          2   Start - New Tributary"
"          0.584    0.000    0.877    0.877"
" 33      CATCHMENT 20"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"          20  Catchment 20"
"          0.000 % Impervious"
"          6.650 Total Area"
"          150.000 Flow length"
"          2.000 Overland Slope"

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"      6.650  Pervious Area"
"    150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"    150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.377  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"    98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.389      0.000      0.877      0.877 c.m/sec"
"      Catchment 20      Pervious      Impervious Total Area  "
"      Surface Area      6.650      0.000      6.650      hectare"
"      Time of concentration  41.437      5.102      41.437      minutes"
"      Time to Centroid      143.191      90.751      143.190      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      5026.12      0.01      5026.13      c.m"
"      Rainfall losses      47.093      5.908      47.093      mm"
"      Runoff depth      28.488      69.673      28.488      mm"
"      Runoff volume      1894.43      0.00      1894.44      c.m"
"      Runoff coefficient      0.377      0.000      0.377      "
"      Maximum flow      0.389      0.000      0.389      c.m/sec"
" 40  HYDROGRAPH Add Runoff  "
"      4  Add Runoff  "
"          0.389      0.389      0.877      0.877"
" 33  CATCHMENT 21"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      21  Catchment 20"
"    10.000  % Impervious"
"      0.830  Total Area"
"    40.000  Flow length"
"      2.000  Overland Slope"
"      0.747  Pervious Area"
"    40.000  Pervious length"
"      2.000  Pervious slope"
"      0.083  Impervious Area"
"    40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.376  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"

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"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"  98.000  Impervious SCS Curve No."
"      0.913  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.079      0.389      0.877      0.877 c.m/sec"
"      Catchment 21          Pervious  Impervious Total Area  "
"      Surface Area          0.747      0.083      0.830      hectare"
"      Time of concentration  18.749      2.308      15.260      minutes"
"      Time to Centroid      117.510     87.059     111.048     minutes"
"      Rainfall depth        75.581     75.581     75.581     mm"
"      Rainfall volume        564.59     62.73     627.32     c.m"
"      Rainfall losses        47.127     6.593     43.074     mm"
"      Runoff depth           28.453     68.988     32.507     mm"
"      Runoff volume          212.55     57.26     269.81     c.m"
"      Runoff coefficient      0.376     0.913     0.430     "
"      Maximum flow           0.069     0.028     0.079     c.m/sec"
"  40      HYDROGRAPH Add Runoff  "
"      4  Add Runoff  "
"          0.079      0.436      0.877      0.877"
"  40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.079      0.436      0.436      0.877"
"  64      SHOW TABLE"
"      2  Flow hydrograph"
"      4  Inflow Hydrograph"
"          Maximum flow              0.436      c.m/sec"
"          Hydrograph volume          2164.242     c.m"
"  40      HYDROGRAPH  Combine  1"
"      6  Combine  "
"      1  Node #"
"          Total"
"          Maximum flow              1.308      c.m/sec"
"          Hydrograph volume          6439.194     c.m"
"          0.079      0.436      0.436      1.308"
"  38      START/RE-START TOTALS 21"
"      3  Runoff Totals on EXIT"
"          Total Catchment area              22.730     hectare"
"          Total Impervious area              0.083     hectare"
"          Total % impervious                0.365"
"  19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Ex__50yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 1:48:38 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          5089.418 Coefficient A"
"          30.000  Constant B"
"          0.967  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          156.350  mm/hr"
"          Total depth                86.737  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.417  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

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"		0.035	0.000	0.000	0.000	c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area	"
"	Surface Area	0.240	0.000	0.240		hectare"
"	Time of concentration	11.375	1.467	11.375		minutes"
"	Time to Centroid	108.305	85.675	108.305		minutes"
"	Rainfall depth	86.737	86.737	86.737		mm"
"	Rainfall volume	208.17	0.00	208.17		c.m"
"	Rainfall losses	50.570	6.561	50.570		mm"
"	Runoff depth	36.167	80.176	36.167		mm"
"	Runoff volume	86.80	0.00	86.80		c.m"
"	Runoff coefficient	0.417	0.000	0.417		"
"	Maximum flow	0.035	0.000	0.035		c.m/sec"
" 40	HYDROGRAPH Add Runoff "					
"	4 Add Runoff "					
"		0.035	0.035	0.000	0.000"	
" 40	HYDROGRAPH Copy to Outflow"					
"	8 Copy to Outflow"					
"		0.035	0.035	0.035	0.000"	
" 40	HYDROGRAPH Combine 2"					
"	6 Combine "					
"	2 Node #"					
"	To Walser Street"					
"	Maximum flow		0.035			c.m/sec"
"	Hydrograph volume		86.800			c.m"
"		0.035	0.035	0.035	0.035"	
" 40	HYDROGRAPH Start - New Tributary"					
"	2 Start - New Tributary"					
"		0.035	0.000	0.035	0.035"	
" 33	CATCHMENT 10"					
"	1 Triangular SCS"					
"	1 Equal length"					
"	1 SCS method"					
"	10 Catchment 10"					
"	0.000 % Impervious"					
"	7.760 Total Area"					
"	150.000 Flow length"					
"	2.000 Overland Slope"					
"	7.760 Pervious Area"					
"	150.000 Pervious length"					
"	2.000 Pervious slope"					
"	0.000 Impervious Area"					
"	150.000 Impervious length"					
"	2.000 Impervious slope"					
"	0.250 Pervious Manning 'n'"					
"	74.000 Pervious SCS Curve No."					
"	0.418 Pervious Runoff coefficient"					
"	0.100 Pervious Ia/S coefficient"					
"	8.924 Pervious Initial abstraction"					
"	0.015 Impervious Manning 'n'"					
"	98.000 Impervious SCS Curve No."					

"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.618	0.000	0.035	0.035 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	38.106	4.916	38.106	minutes"
"		Time to Centroid	138.366	90.175	138.366	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	6730.77	0.01	6730.77	c.m"
"		Rainfall losses	50.510	5.941	50.510	mm"
"		Runoff depth	36.227	80.796	36.227	mm"
"		Runoff volume	2811.21	0.01	2811.22	c.m"
"		Runoff coefficient	0.418	0.000	0.418	"
"		Maximum flow	0.618	0.000	0.618	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.618	0.618	0.035	0.035"	
" 33		CATCHMENT 11"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	11	Catchment 11"				
"	0.000	% Impervious"				
"	0.130	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.130	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.417	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.016	0.618	0.035	0.035 c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"
"		Surface Area	0.130	0.000	0.130	hectare"
"		Time of concentration	17.241	2.224	17.241	minutes"
"		Time to Centroid	114.897	86.667	114.896	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	112.76	0.00	112.76	c.m"

"	Rainfall losses	50.540	6.773	50.540	mm"
"	Runoff depth	36.197	79.963	36.197	mm"
"	Runoff volume	47.06	0.00	47.06	c.m"
"	Runoff coefficient	0.417	0.000	0.417	"
"	Maximum flow	0.016	0.000	0.016	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.016	0.628	0.035	0.035"	
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.417 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.789	0.628	0.035	0.035 c.m/sec"	
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	21.990	2.837	21.990	minutes"
"	Time to Centroid	120.254	87.552	120.254	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	6175.65	0.01	6175.66	c.m"
"	Rainfall losses	50.538	7.307	50.538	mm"
"	Runoff depth	36.199	79.429	36.199	mm"
"	Runoff volume	2577.38	0.01	2577.39	c.m"
"	Runoff coefficient	0.417	0.000	0.417	"
"	Maximum flow	0.789	0.000	0.789	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.789	1.341	0.035	0.035"	
" 54	POND DESIGN"				
"	1.341 Current peak flow	c.m/sec"			

```

"      0.050  Target outflow    c.m/sec"
"      5435.7 Hydrograph volume  c.m"
"      6.    Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0     Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          409.630    0.000    0.000"
"          409.750    0.6650   402.200"
"          410.000    3.601   2187.900"
"          410.250    7.811   5318.900"
"          410.500   12.984  9642.300"
"          410.750   18.965  15227.70"
"          Peak outflow                1.183    c.m/sec"
"          Maximum level                409.794  metre"
"          Maximum storage              718.586   c.m"
"          Centroidal lag                2.328   hours"
"          0.789    1.341    1.183    0.035 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.789    1.183    1.183    0.035"
" 52    CHANNEL DESIGN"
"          1.183  Current peak flow    c.m/sec"
"          0.035  Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          0.000  Basewidth    metre"
"          7.410  Left bank slope"
"          6.000  Right bank slope"
"          0.950  Channel depth    metre"
"          1.040  Gradient    %"
"          Depth of flow                0.417   metre"
"          Velocity                    1.016   m/sec"
"          Channel capacity             10.655   c.m/sec"
"          Critical depth                0.364   metre"
" 53    ROUTE Channel Route 72"
"          72.40  Channel Route 72 Reach length  ( metre)"
"          0.396  X-factor <= 0.5"
"          53.425 K-lag  ( seconds)"
"          0.000  Default(0) or user spec.(1) values used"
"          0.500  X-factor <= 0.5"
"          30.000 K-lag  ( seconds)"
"          0.500  Beta weighting factor"
"          60.000 Routing time step  ( seconds)"
"          1     No. of sub-reaches"
"          Peak outflow                1.180    c.m/sec"
"          0.789    1.183    1.180    0.035 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.789    1.180    1.180    0.035"

```



```

" 52      CHANNEL DESIGN"
"      1.180  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth    metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow          0.341    metre"
"          Velocity                1.150    m/sec"
"          Channel capacity        9.246    c.m/sec"
"          Critical depth          0.283    metre"
" 53      ROUTE    Channel Route 40"
"      39.80   Channel Route 40 Reach length    ( metre)"
"      0.282   X-factor <= 0.5"
"      25.955  K-lag    ( seconds)"
"      0.000   Default(0) or user spec.(1) values used"
"      0.500   X-factor <= 0.5"
"      30.000  K-lag    ( seconds)"
"      0.500   Beta weighting factor"
"      33.333  Routing time step    ( seconds)"
"          1   No. of sub-reaches"
"          Peak outflow          1.178    c.m/sec"
"          0.789    1.180    1.178    0.035 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5   Next link "
"          0.789    1.178    1.178    0.035"
" 40      HYDROGRAPH Copy to Outflow"
"          8   Copy to Outflow"
"          0.789    1.178    1.178    0.035"
" 40      HYDROGRAPH Combine    1"
"          6   Combine "
"          1   Node #"
"          Total"
"          Maximum flow          1.178    c.m/sec"
"          Hydrograph volume      5435.673    c.m"
"          0.789    1.178    1.178    1.178"
" 40      HYDROGRAPH Start - New Tributary"
"          2   Start - New Tributary"
"          0.789    0.000    1.178    1.178"
" 33      CATCHMENT 20"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"          20  Catchment 20"
"          0.000 % Impervious"
"          6.650 Total Area"
"          150.000 Flow length"
"          2.000 Overland Slope"

```

"	6.650	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.418	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.530	0.000	1.178	1.178 c.m/sec"	
"		Catchment 20	Pervious	Impervious	Total Area	"
"		Surface Area	6.650	0.000	6.650	hectare"
"		Time of concentration	38.106	4.916	38.106	minutes"
"		Time to Centroid	138.366	90.175	138.366	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	5767.99	0.01	5768.00	c.m"
"		Rainfall losses	50.510	5.941	50.510	mm"
"		Runoff depth	36.227	80.796	36.227	mm"
"		Runoff volume	2409.09	0.01	2409.10	c.m"
"		Runoff coefficient	0.418	0.000	0.418	"
"		Maximum flow	0.530	0.000	0.530	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.530	0.530	1.178	1.178"	
" 33		CATCHMENT 21"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	21	Catchment 20"				
"	10.000	% Impervious"				
"	0.830	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.747	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.083	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.417	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				

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"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"  98.000  Impervious SCS Curve No."
"      0.922  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.104      0.530      1.178      1.178 c.m/sec"
"      Catchment 21          Pervious  Impervious Total Area  "
"      Surface Area          0.747      0.083      0.830      hectare"
"      Time of concentration 17.241      2.224      14.282      minutes"
"      Time to Centroid      114.897      86.667      109.333      minutes"
"      Rainfall depth        86.737      86.737      86.737      mm"
"      Rainfall volume        647.92      71.99      719.92      c.m"
"      Rainfall losses        50.540      6.774      46.163      mm"
"      Runoff depth           36.197      79.963      40.574      mm"
"      Runoff volume          270.39      66.37      336.76      c.m"
"      Runoff coefficient      0.417      0.922      0.468      "
"      Maximum flow           0.092      0.031      0.104      c.m/sec"
"  40      HYDROGRAPH Add Runoff  "
"      4  Add Runoff  "
"          0.104      0.593      1.178      1.178"
"  40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.104      0.593      0.593      1.178"
"  64      SHOW TABLE"
"      2  Flow hydrograph"
"      4  Inflow Hydrograph"
"          Maximum flow              0.593      c.m/sec"
"          Hydrograph volume          2745.860      c.m"
"  40      HYDROGRAPH Combine  1"
"      6  Combine  "
"      1  Node #"
"          Total"
"          Maximum flow              1.755      c.m/sec"
"          Hydrograph volume          8181.533      c.m"
"          0.104      0.593      0.593      1.755"
"  38      START/RE-START TOTALS 21"
"      3  Runoff Totals on EXIT"
"          Total Catchment area              22.730      hectare"
"          Total Impervious area              0.083      hectare"
"          Total % impervious                0.365"
"  19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Ex_100yr.out"
"          Licensee name:                      gmbp"
"          Company                             gmbp"
"          Date & Time last used:              7/25/2022 at 1:49:40 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          6933.019 Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          168.777  mm/hr"
"          Total depth                97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.452  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

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"		0.043	0.000	0.000	0.000 c.m/sec"
"	Catchment 30		Pervious	Impervious	Total Area "
"	Surface Area	0.240	0.000	0.240	hectare"
"	Time of concentration	10.606	1.421	10.606	minutes"
"	Time to Centroid	106.728	85.423	106.728	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	235.01	0.00	235.01	c.m"
"	Rainfall losses	53.628	6.787	53.628	mm"
"	Runoff depth	44.294	91.134	44.294	mm"
"	Runoff volume	106.30	0.00	106.30	c.m"
"	Runoff coefficient	0.452	0.000	0.452	"
"	Maximum flow	0.043	0.000	0.043	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"		0.043	0.043	0.000	0.000"
" 40	HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"			
"		0.043	0.043	0.043	0.000"
" 40	HYDROGRAPH Combine 2"				
"	6	Combine "			
"	2	Node #"			
"		To Walser Street"			
"			0.043		c.m/sec"
"			106.305		c.m"
"		0.043	0.043	0.043	0.043"
" 40	HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"			
"		0.043	0.000	0.043	0.043"
" 33	CATCHMENT 10"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	10	Catchment 10"			
"	0.000	% Impervious"			
"	7.760	Total Area"			
"	150.000	Flow length"			
"	2.000	Overland Slope"			
"	7.760	Pervious Area"			
"	150.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	150.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.454	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			

"	0.000	Impervious	Runoff coefficient"				
"	0.100	Impervious	Ia/S coefficient"				
"	0.518	Impervious	Initial abstraction"				
"		0.801	0.000	0.043	0.043	c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"	
"		Surface Area	7.760	0.000	7.760	hectare"	
"		Time of concentration	35.531	4.759	35.530	minutes"	
"		Time to Centroid	134.554	89.737	134.554	minutes"	
"		Rainfall depth	97.921	97.921	97.921	mm"	
"		Rainfall volume	7598.69	0.01	7598.69	c.m"	
"		Rainfall losses	53.501	6.084	53.501	mm"	
"		Runoff depth	44.420	91.837	44.420	mm"	
"		Runoff volume	3447.00	0.01	3447.01	c.m"	
"		Runoff coefficient	0.454	0.000	0.454	"	
"		Maximum flow	0.801	0.000	0.801	c.m/sec"	
" 40		HYDROGRAPH Add Runoff "					
"	4	Add Runoff "					
"		0.801	0.801	0.043	0.043"		
" 33		CATCHMENT 11"					
"	1	Triangular SCS"					
"	1	Equal length"					
"	1	SCS method"					
"	11	Catchment 11"					
"	0.000	% Impervious"					
"	0.130	Total Area"					
"	40.000	Flow length"					
"	2.000	Overland Slope"					
"	0.130	Pervious Area"					
"	40.000	Pervious length"					
"	2.000	Pervious slope"					
"	0.000	Impervious Area"					
"	40.000	Impervious length"					
"	2.000	Impervious slope"					
"	0.250	Pervious Manning 'n'"					
"	74.000	Pervious SCS Curve No."					
"	0.453	Pervious Runoff coefficient"					
"	0.100	Pervious Ia/S coefficient"					
"	8.924	Pervious Initial abstraction"					
"	0.015	Impervious Manning 'n'"					
"	98.000	Impervious SCS Curve No."					
"	0.000	Impervious Runoff coefficient"					
"	0.100	Impervious Ia/S coefficient"					
"	0.518	Impervious Initial abstraction"					
"		0.020	0.801	0.043	0.043	c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"	
"		Surface Area	0.130	0.000	0.130	hectare"	
"		Time of concentration	16.076	2.153	16.076	minutes"	
"		Time to Centroid	112.853	86.345	112.853	minutes"	
"		Rainfall depth	97.921	97.921	97.921	mm"	
"		Rainfall volume	127.30	0.00	127.30	c.m"	

"	Rainfall losses	53.605	6.948	53.605	mm"
"	Runoff depth	44.316	90.973	44.316	mm"
"	Runoff volume	57.61	0.00	57.61	c.m"
"	Runoff coefficient	0.453	0.000	0.453	"
"	Maximum flow	0.020	0.000	0.020	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.020	0.813	0.043	0.043"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	7.120 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	7.120 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.453 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.985	0.813	0.043	0.043 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	20.504	2.747	20.504	minutes"
"	Time to Centroid	117.777	87.189	117.777	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	6971.99	0.01	6972.00	c.m"
"	Rainfall losses	53.532	7.496	53.532	mm"
"	Runoff depth	44.389	90.426	44.389	mm"
"	Runoff volume	3160.50	0.01	3160.50	c.m"
"	Runoff coefficient	0.453	0.000	0.453	"
"	Maximum flow	0.985	0.000	0.985	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.985	1.721	0.043	0.043"
" 54	POND DESIGN"				
"	1.721 Current peak flow	c.m/sec"			

```

"      0.050  Target outflow    c.m/sec"
"      6665.1 Hydrograph volume  c.m"
"      6.    Number of stages"
"      409.630 Minimum water level  metre"
"      410.750 Maximum water level  metre"
"      409.630 Starting water level  metre"
"      0     Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          409.630    0.000    0.000"
"          409.750    0.6650   402.200"
"          410.000    3.601   2187.900"
"          410.250    7.811   5318.900"
"          410.500   12.984  9642.300"
"          410.750   18.965  15227.70"
"          Peak outflow                1.507    c.m/sec"
"          Maximum level                409.822  metre"
"          Maximum storage              917.762   c.m"
"          Centroidal lag              2.275    hours"
"          0.985    1.721    1.507    0.043 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.985    1.507    1.507    0.043"
" 52    CHANNEL DESIGN"
"          1.507  Current peak flow    c.m/sec"
"          0.035  Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          0.000  Basewidth    metre"
"          7.410  Left bank slope"
"          6.000  Right bank slope"
"          0.950  Channel depth    metre"
"          1.040  Gradient    %"
"          Depth of flow                0.456    metre"
"          Velocity                    1.080    m/sec"
"          Channel capacity             10.655   c.m/sec"
"          Critical depth              0.400    metre"
" 53    ROUTE    Channel Route 72"
"          72.40    Channel Route 72 Reach length  ( metre)"
"          0.386  X-factor <= 0.5"
"          50.288  K-lag  ( seconds)"
"          0.000  Default(0) or user spec.(1) values used"
"          0.500  X-factor <= 0.5"
"          30.000  K-lag  ( seconds)"
"          0.500  Beta weighting factor"
"          60.000  Routing time step  ( seconds)"
"          1     No. of sub-reaches"
"          Peak outflow                1.499    c.m/sec"
"          0.985    1.507    1.499    0.043 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.985    1.499    1.499    0.043"

```



```

" 52      CHANNEL DESIGN"
"      1.499  Current peak flow    c.m/sec"
"      0.035  Manning 'n'"
"      0.    Cross-section type: 0=trapezoidal; 1=general"
"      2.000  Basewidth    metre"
"      2.950  Left bank slope"
"      3.000  Right bank slope"
"      0.950  Channel depth    metre"
"      1.040  Gradient    %"
"          Depth of flow          0.386    metre"
"          Velocity                1.232    m/sec"
"          Channel capacity        9.246    c.m/sec"
"          Critical depth          0.325    metre"
" 53      ROUTE    Channel Route 40"
"      39.80   Channel Route 40 Reach length ( metre)"
"      0.256  X-factor <= 0.5"
"      24.228 K-lag ( seconds)"
"      0.000  Default(0) or user spec.(1) values used"
"      0.500  X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500  Beta weighting factor"
"      33.333 Routing time step ( seconds)"
"          1  No. of sub-reaches"
"          Peak outflow          1.499    c.m/sec"
"          0.985    1.499    1.499    0.043 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"          0.985    1.499    1.499    0.043"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"          0.985    1.499    1.499    0.043"
" 40      HYDROGRAPH Combine    1"
"          6  Combine "
"          1  Node #"
"          Total"
"          Maximum flow          1.499    c.m/sec"
"          Hydrograph volume      6665.152  c.m"
"          0.985    1.499    1.499    1.499"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.985    0.000    1.499    1.499"
" 33      CATCHMENT 20"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          20  Catchment 20"
"          0.000 % Impervious"
"          6.650 Total Area"
"          150.000 Flow length"
"          2.000 Overland Slope"

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"      6.650  Pervious Area"
"    150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"    150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.454  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"    98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.687      0.000      1.499      1.499 c.m/sec"
"      Catchment 20      Pervious      Impervious Total Area  "
"      Surface Area      6.650      0.000      6.650      hectare"
"      Time of concentration  35.531      4.759      35.530      minutes"
"      Time to Centroid      134.554      89.737      134.554      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      6511.76      0.01      6511.77      c.m"
"      Rainfall losses      53.501      6.084      53.501      mm"
"      Runoff depth      44.420      91.837      44.420      mm"
"      Runoff volume      2953.94      0.01      2953.94      c.m"
"      Runoff coefficient      0.454      0.000      0.454      "
"      Maximum flow      0.687      0.000      0.687      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.687      0.687      1.499      1.499"
" 33      CATCHMENT 21"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      21      Catchment 20"
"    10.000  % Impervious"
"      0.830  Total Area"
"    40.000  Flow length"
"      2.000  Overland Slope"
"      0.747  Pervious Area"
"    40.000  Pervious length"
"      2.000  Pervious slope"
"      0.083  Impervious Area"
"    40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"      0.453  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"

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"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"  98.000  Impervious SCS Curve No."
"      0.929  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.135      0.687      1.499      1.499 c.m/sec"
"      Catchment 21          Pervious  Impervious Total Area "
"      Surface Area          0.747      0.083      0.830      hectare"
"      Time of concentration 16.076      2.153      13.490      minutes"
"      Time to Centroid      112.853      86.345      107.930      minutes"
"      Rainfall depth        97.921      97.921      97.921      mm"
"      Rainfall volume        731.47      81.27      812.75      c.m"
"      Rainfall losses        53.605      6.948      48.939      mm"
"      Runoff depth           44.316      90.973      48.982      mm"
"      Runoff volume          331.04      75.51      406.55      c.m"
"      Runoff coefficient      0.453      0.929      0.500      "
"      Maximum flow           0.117      0.034      0.135      c.m/sec"
"  40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.135      0.765      1.499      1.499"
"  40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.135      0.765      0.765      1.499"
"  64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"          Maximum flow              0.765      c.m/sec"
"          Hydrograph volume          3360.493      c.m"
"  40      HYDROGRAPH Combine 1"
"      6      Combine "
"      1      Node #"
"          Total"
"          Maximum flow              2.244      c.m/sec"
"          Hydrograph volume          10025.642      c.m"
"          0.135      0.765      0.765      2.244"
"  38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"          Total Catchment area              22.730      hectare"
"          Total Impervious area              0.083      hectare"
"          Total % impervious                0.365"
"  19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                Version 2.25  rev. 473"
"          MIDUSS created                Sunday, February 07, 2010"
"          10  Units used:                ie METRIC"
"          Job folder:                   W:\Kitchener\411-2011\411009\Design Data\
"                                         Modelling Files\2022-07-25"
"          Output filename:              Ex__REG.out"
"          Licensee name:                gmbp"
"          Company                       gmbp"
"          Date & Time last used:        7/25/2022 at 1:51:35 PM"
" 31          TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000  Max. Storm length"
"          12000.000  Max. Hydrograph"
" 32          STORM Historic"
"          5  Historic"
"          2880.000  Duration"
"          48.000  Rainfall intensity values"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.026    2.026    2.026    2.028"
"                   2.026    6.000    4.000    6.000    13.000"
"                   17.000    13.000    23.000    13.000    13.000"
"                   53.000    38.000    13.000"
"          Maximum intensity              53.000  mm/hr"
"          Total depth                    285.000  mm"
"          6  000hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.240  Total Area"
"          20.000  Flow length"
"          2.000  Overland Slope"
"          0.240  Pervious Area"
"          20.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          20.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.713  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"

```

"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.027	0.000	0.000	0.000	c.m/sec"
"		Catchment 30	Pervious	Impervious	Total Area	"
"		Surface Area	0.240	0.000	0.240	hectare"
"		Time of concentration	12.633	2.243	12.633	minutes"
"		Time to Centroid	2530.545	2290.972	2530.545	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	684.00	0.00	684.00	c.m"
"		Rainfall losses	81.839	43.972	81.839	mm"
"		Runoff depth	203.161	241.028	203.161	mm"
"		Runoff volume	487.58	0.00	487.59	c.m"
"		Runoff coefficient	0.713	0.000	0.713	"
"		Maximum flow	0.027	0.000	0.027	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.027	0.027	0.000	0.000"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.027	0.027	0.027	0.000"	
" 40		HYDROGRAPH Combine 2"				
"	6	Combine "				
"	2	Node #"				
"		To Walser Street"				
"		Maximum flow	0.027		c.m/sec"	
"		Hydrograph volume	487.585		c.m"	
"		0.027	0.027	0.027	0.027"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.027	0.000	0.027	0.027"	
" 33		CATCHMENT 10"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	10	Catchment 10"				
"	0.000	% Impervious"				
"	7.760	Total Area"				
"	150.000	Flow length"				
"	2.000	Overland Slope"				
"	7.760	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				

"	74.000	Pervious SCS Curve No."				
"	0.714	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.881	0.000	0.027	0.027 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	42.319	7.513	42.319	minutes"
"		Time to Centroid	2572.242	2276.224	2572.241	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	2.2116	0.0000	2.2116	ha-m"
"		Rainfall losses	81.644	25.621	81.644	mm"
"		Runoff depth	203.356	259.379	203.356	mm"
"		Runoff volume	1.5780	0.0000	1.5780	ha-m"
"		Runoff coefficient	0.714	0.000	0.714	"
"		Maximum flow	0.881	0.000	0.881	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.881	0.881	0.027	0.027"	
" 33		CATCHMENT 11"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	11	Catchment 11"				
"	0.000	% Impervious"				
"	0.130	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.130	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.723	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.014	0.881	0.027	0.027 c.m/sec"	

	Catchment 11	Pervious	Impervious	Total Area	
"	Surface Area	0.130	0.000	0.130	hectare"
"	Time of concentration	19.148	3.399	19.148	minutes"
"	Time to Centroid	2545.193	2266.333	2545.193	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	370.50	0.00	370.50	c.m"
"	Rainfall losses	78.940	42.646	78.940	mm"
"	Runoff depth	206.060	242.354	206.060	mm"
"	Runoff volume	267.88	0.00	267.88	c.m"
"	Runoff coefficient	0.723	0.000	0.723	"
"	Maximum flow	0.014	0.000	0.014	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

	4	Add Runoff "			
"	0.014	0.894	0.027	0.027"	

" 33 CATCHMENT 40"

"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	40	Catchment 40"			
"	0.000	% Impervious"			
"	7.120	Total Area"			
"	60.000	Flow length"			
"	2.000	Overland Slope"			
"	7.120	Pervious Area"			
"	60.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.000	Impervious Area"			
"	60.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	74.000	Pervious SCS Curve No."			
"	0.716	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	8.924	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.000	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"	0.772	0.894	0.027	0.027 c.m/sec"	

	Catchment 40	Pervious	Impervious	Total Area	
"	Surface Area	7.120	0.000	7.120	hectare"
"	Time of concentration	24.421	4.336	24.421	minutes"
"	Time to Centroid	2549.942	2258.969	2549.942	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	2.0292	0.0000	2.0292	ha-m"
"	Rainfall losses	80.848	39.404	80.848	mm"
"	Runoff depth	204.152	245.596	204.152	mm"
"	Runoff volume	1.4536	0.0000	1.4536	ha-m"
"	Runoff coefficient	0.716	0.000	0.716	"

```

"           Maximum flow           0.772      0.000      0.772      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"           4  Add Runoff "
"           0.772      1.667      0.027      0.027"
" 54      POND DESIGN"
"           1.667  Current peak flow    c.m/sec"
"           0.050  Target outflow    c.m/sec"
"           30583.9  Hydrograph volume    c.m"
"           6.    Number of stages"
"           409.630  Minimum water level    metre"
"           410.750  Maximum water level    metre"
"           409.630  Starting water level    metre"
"           0    Keep Design Data: 1 = True; 0 = False"
"           Level Discharge    Volume"
"           409.630    0.000    0.000"
"           409.750    0.6650    402.200"
"           410.000    3.601    2187.900"
"           410.250    7.811    5318.900"
"           410.500    12.984    9642.300"
"           410.750    18.965    15227.70"
"           Peak outflow           1.612    c.m/sec"
"           Maximum level           409.831    metre"
"           Maximum storage           977.909    c.m"
"           Centroidal lag           42.858    hours"
"           0.772    1.667    1.612    0.027 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5  Next link "
"           0.772    1.612    1.612    0.027"
" 52      CHANNEL DESIGN"
"           1.612  Current peak flow    c.m/sec"
"           0.035  Manning 'n'"
"           0.    Cross-section type: 0=trapezoidal; 1=general"
"           0.000  Basewidth    metre"
"           7.410  Left bank slope"
"           6.000  Right bank slope"
"           0.950  Channel depth    metre"
"           1.040  Gradient    %"
"           Depth of flow           0.468    metre"
"           Velocity                 1.098    m/sec"
"           Channel capacity           10.655    c.m/sec"
"           Critical depth           0.411    metre"
" 53      ROUTE    Channel Route 72"
"           72.40    Channel Route 72 Reach length    ( metre)"
"           0.383  X-factor <= 0.5"
"           49.448  K-lag    ( seconds)"
"           0.000  Default(0) or user spec.(1) values used"
"           0.500  X-factor <= 0.5"
"           30.000  K-lag    ( seconds)"
"           0.500  Beta weighting factor"
"           60.000  Routing time step    ( seconds)"

```



```

"          1  No. of sub-reaches"
"          Peak outflow          1.606   c.m/sec"
"              0.772   1.612   1.606   0.027 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.772   1.606   1.606   0.027"
" 52      CHANNEL DESIGN"
"          1.606  Current peak flow   c.m/sec"
"          0.035  Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          2.000  Basewidth   metre"
"          2.950  Left bank slope"
"          3.000  Right bank slope"
"          0.950  Channel depth   metre"
"          1.040  Gradient   %"
"          Depth of flow          0.401   metre"
"          Velocity                1.256   m/sec"
"          Channel capacity        9.246   c.m/sec"
"          Critical depth          0.339   metre"
" 53      ROUTE   Channel Route 40"
"          39.80  Channel Route 40 Reach length ( metre)"
"          0.248  X-factor <= 0.5"
"          23.758 K-lag ( seconds)"
"          0.000  Default(0) or user spec.(1) values used"
"          0.500  X-factor <= 0.5"
"          30.000 K-lag ( seconds)"
"          0.500  Beta weighting factor"
"          35.644 Routing time step ( seconds)"
"          1  No. of sub-reaches"
"          Peak outflow          1.603   c.m/sec"
"              0.772   1.606   1.603   0.027 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.772   1.603   1.603   0.027"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.772   1.603   1.603   0.027"
" 40      HYDROGRAPH Combine  1"
"          6  Combine "
"          1  Node #"
"          Total"
"          Maximum flow          1.603   c.m/sec"
"          Hydrograph volume      30582.604 c.m"
"              0.772   1.603   1.603   1.603"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.772   0.000   1.603   1.603"
" 33      CATCHMENT 20"
"          1  Triangular SCS"
"          1  Equal length"

```

```

"          1  SCS method"
"          20  Catchment 20"
"          0.000  % Impervious"
"          6.650  Total Area"
"    150.000  Flow length"
"          2.000  Overland Slope"
"          6.650  Pervious Area"
"    150.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"    150.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"    74.000  Pervious SCS Curve No."
"          0.714  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"    98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"                0.755      0.000      1.603      1.603 c.m/sec"
"          Catchment 20      Pervious      Impervious      Total Area  "
"          Surface Area      6.650      0.000      6.650      hectare"
"          Time of concentration  42.319      7.513      42.319      minutes"
"          Time to Centroid      2572.242      2276.224      2572.241      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      1.8952      0.0000      1.8952      ha-m"
"          Rainfall losses      81.644      25.621      81.644      mm"
"          Runoff depth      203.356      259.379      203.356      mm"
"          Runoff volume      1.3523      0.0000      1.3523      ha-m"
"          Runoff coefficient      0.714      0.000      0.714      "
"          Maximum flow      0.755      0.000      0.755      c.m/sec"
" 40  HYDROGRAPH Add Runoff  "
"          4  Add Runoff  "
"                0.755      0.755      1.603      1.603"
" 33  CATCHMENT 21"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          21  Catchment 20"
"    10.000  % Impervious"
"          0.830  Total Area"
"    40.000  Flow length"
"          2.000  Overland Slope"
"          0.747  Pervious Area"
"    40.000  Pervious length"
"          2.000  Pervious slope"
"          0.083  Impervious Area"

```

```

"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.723  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.850  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.088      0.755      1.603      1.603 c.m/sec"
"      Catchment 21      Pervious      Impervious      Total Area  "
"      Surface Area      0.747      0.083      0.830      hectare"
"      Time of concentration  19.148      3.399      17.327      minutes"
"      Time to Centroid      2545.193      2266.333      2512.963      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      2128.95      236.55      2365.50      c.m"
"      Rainfall losses      78.940      42.646      75.310      mm"
"      Runoff depth      206.060      242.354      209.690      mm"
"      Runoff volume      1539.27      201.15      1740.42      c.m"
"      Runoff coefficient      0.723      0.850      0.736      "
"      Maximum flow      0.079      0.010      0.088      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.088      0.841      1.603      1.603"
" 40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.088      0.841      0.841      1.603"
" 64      SHOW TABLE"
"      2  Flow hydrograph"
"      4  Inflow Hydrograph"
"      Maximum flow      0.841      c.m/sec"
"      Hydrograph volume      15263.571      c.m"
" 40      HYDROGRAPH Combine 1"
"      6  Combine "
"      1  Node #"
"      Total"
"      Maximum flow      2.444      c.m/sec"
"      Hydrograph volume      45846.176      c.m"
"          0.088      0.841      0.841      2.444"
" 38      START/RE-START TOTALS 21"
"      3  Runoff Totals on EXIT"
"      Total Catchment area      22.730      hectare"
"      Total Impervious area      0.083      hectare"
"      Total % impervious      0.365"
" 19      EXIT"

```

**Ainley Farm Subdivision  
Township of Centre Wellington (Elora)  
G&M File: 411009  
March 2023**

**Catchment 1000 : Stormwater Management Facility No. 1**

**Stage Storage Volume Calculations**

<b>Elevation</b>	<b>Stage</b>	<b>Surface Area</b>	<b>Increm. Storage</b>	<b>Accum. Storage</b>	
<b>(m)</b>	<b>(m)</b>	<b>(m<sup>2</sup>)</b>	<b>(m<sup>3</sup>)</b>	<b>(m<sup>3</sup>)</b>	
411.00	0.00	14.8	0	0.0	CB.1 Lip
411.10	0.10	353.3	18	18.4	
411.20	0.20	1,188.7	77	95.5	
411.30	0.30	1,699.2	144	239.9	
411.40	0.40	2,215.9	196	435.7	CB.2 Lip
411.50	0.50	2,897.0	256	691.3	
411.60	0.60	3,828.7	336	1,027.6	
411.70	0.70	4,620.0	422	1,450.0	
411.80	0.80	5,096.6	486	1,935.8	
411.85	0.85	5,297.4	260	2,195.7	Weir
411.90	0.90	5,498.2	270	2,465.6	
412.00	1.00	5,792.6	565	3,030.1	Top of bank

<b>Outlet #1</b>			<b>Outlet #2</b>			<b>Overflow Weir</b>		
450 mm diameter pipe			525 mm diameter pipe			Elevation = 411.85		
250 mm orifice			350 mm orifice					
Q =	0.133	m <sup>3</sup> /s	Q =	0.246	m <sup>3</sup> /s	d1 =	1.00	m
Cd =	0.600		Cd =	0.600		h =	0.85	m
H =	1.045	m	H =	0.925	m	H =	0.15	m
2g =	19.620		2g =	19.620		2g =	19.620	
						L =	20.00	m
A =	0.049	m <sup>2</sup>	A =	0.096	m <sup>2</sup>			
D =	0.250	m	D =	0.350	m	Q =	1.638	m <sup>3</sup> /s
D/2 =	0.125	m	D/2 =	0.175	m			
Invert =	410.83		Invert =	410.9				

**Stage/Storage/Discharge Table**

<b>Elevation</b>	<b>Stage</b>	<b>Storage</b>	<b>Outlet #1</b>	<b>Outlet #2</b>	<b>Overflow</b>	<b>Actual</b>	
<b>(m)</b>	<b>(m)</b>	<b>(m<sup>3</sup>)</b>	<b>250 mm</b>	<b>350mm</b>	<b>Weir</b>	<b>Discharge</b>	
			<b>(m<sup>3</sup>/s)</b>	<b>(m<sup>3</sup>/s)</b>	<b>(m<sup>3</sup>/s)</b>	<b>(m<sup>3</sup>/s)</b>	
411.00	0.00	0.0	0.000	0.000	0.000	0.000	CB.1 Lip
411.10	0.10	18.4	0.032	0.000	0.000	0.032	
411.20	0.20	95.5	0.065	0.000	0.000	0.065	
411.30	0.30	239.9	0.077	0.000	0.000	0.077	
411.40	0.40	435.7	0.087	0.000	0.000	0.087	
411.50	0.50	691.3	0.096	0.000	0.000	0.096	
411.60	0.60	1,027.6	0.105	0.000	0.000	0.105	CB.2 Lip
411.70	0.70	1,450.0	0.113	0.202	0.000	0.315	
411.80	0.80	1,935.8	0.120	0.218	0.000	0.338	
411.85	0.85	2,195.7	0.123	0.225	0.000	0.349	Weir
411.90	0.90	2,465.6	0.127	0.232	0.308	0.667	
412.00	1.00	3,030.1	0.133	0.246	1.638	2.018	Top of bank

**Ainley Farm Subdivision  
Township of Centre Wellington (Elora)  
G&M File: 411009  
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**Catchment 2100 : Stormwater Management Facility No. 2**

**Stage Storage Volume Calculations**

Elevation	Stage	Surface Area	Increm. Quality Storage	Accum. Quality Storage	Increm. Active Storage	Accum. Active Storage	
(m)	(m)	(m <sup>2</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	
410.51	0.00	795.09	0.00	0.00			Outlet Pipe Invert
410.65	0.14	835.70	114.16	114.16	0	0.0	Pond Bottom/Knockout
410.70	0.19	888.00			43	43.1	
410.80	0.29	941.90			91	134.6	
410.90	0.39	996.40			97	231.5	
411.00	0.49	1052.40			102	333.9	
411.10	0.59	1109.60			108	442.0	CB Lip
411.20	0.69	1167.90			114	555.9	
411.30	0.79	1227.50			120	675.7	
411.40	0.89	1288.15			126	801.5	
411.50	0.99	1350.10			132	933.4	
411.60	1.09	1413.30			138	1,071.6	
411.65	1.14	1444.90			71	1,143.0	Weir
411.70	1.19	1476.50			73	1,216.0	
411.80	1.29	1559.40			152	1,367.8	Top of bank

**Outlet #1**  
150 mm Diameter Knockout

$$Q = 0.032 \text{ m}^3/\text{s}$$

$$Cd = 0.600$$

$$H = 0.475 \text{ m}$$

$$2g = 19.620$$

$$A = 0.018 \text{ m}^2$$

$$D = 0.150 \text{ m}$$

$$D/2 = 0.075 \text{ m}$$

**Outlet #2**  
300 mm diameter orifice

$$Q = 0.201 \text{ m}^3/\text{s}$$

$$Cd = 0.600$$

$$H = 0.940 \text{ m}$$

$$2g = 19.620$$

$$A = 0.071 \text{ m}^2$$

$$D = 0.300 \text{ m}$$

$$D/2 = 0.150 \text{ m}$$

$$\text{Invert} = 410.51$$

**Overflow Weir**  
Elevation = 411.65

$$d1 = 1.15 \text{ m}$$

$$h = 1.00 \text{ m}$$

$$H = 0.15 \text{ m}$$

$$2g = 19.62$$

$$L = 10.00 \text{ m}$$

$$Q = 0.815 \text{ m}^3/\text{s}$$

**Stage/Storage/Discharge Table**

Elevation	Stage	Storage	Outlet #1 150 mm	Outlet #2 300 mm	Overflow Weir	Actual Discharge	
(m)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> /s)	(m <sup>3</sup> /s)	(m <sup>3</sup> /s)	(m <sup>3</sup> /s)	
410.65	0.00	0.0	0.000	0.000	0.000	0.000	Pond Bottom/Knockout
410.70	0.05	43.1	0.006	0.000	0.000	0.006	
410.80	0.15	134.6	0.013	0.000	0.000	0.013	
410.90	0.25	231.5	0.020	0.000	0.000	0.020	
411.00	0.35	333.9	0.025	0.000	0.000	0.025	
411.10	0.45	442.0	0.029	0.000	0.000	0.029	CB Lip
411.20	0.55	555.9	0.000	0.138	0.000	0.138	
411.30	0.65	675.7	0.000	0.150	0.000	0.150	
411.40	0.75	801.5	0.000	0.162	0.000	0.162	
411.50	0.85	933.4	0.000	0.172	0.000	0.172	
411.60	0.95	1,071.6	0.000	0.182	0.000	0.182	
411.65	1.00	1,143.0	0.000	0.187	0.000	0.187	Weir
411.70	1.05	1,216.0	0.000	0.192	0.153	0.345	
411.80	1.15	1,367.8	0.000	0.201	0.815	1.015	Top of bank

**AINLEY SUBDIVISION  
TOWNSHIP OF CENTRE WELLINGTON  
Our File: 411009  
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**Catchment 3200: Proposed Super Pipe**

Pipe Volumes

From	To	Length (m)	Diameter (m)	Volume (m3)	Initial Invert (Approx)	Final Invert (Approx)	Initial Obvert	Final Obvert
MH.4	MH.3	27.4	1.2	30.99	410.96	411.23	412.16	412.43
MH.3	CBMH.1	28.3	1.2	32.01	410.65	410.93	411.85	412.13
<b>Total</b>				<b>63.00</b>				

Structure Volumes

	Area	Inv.	T/G	Volume (m <sup>3</sup> ) - up to Weir Depth or Top of Grate
CBMH.1	1.13	410.62	413.98	3.6
MH.3	1.13	410.93	414.66	3.3
MH.4	1.13	411.23	415.44	2.9
<b>Total</b>				<b>9.8</b>

Total Volume from Pipes and Structures  
(Up to Weir Elevation of 413.98)

**72.84**

ELEV (m)	INC. DEPTH (m)	PIPE + MANHOLE STORAGE VOL (m <sup>3</sup> )	ACCUM STORAGE VOL (m <sup>3</sup> )	
410.62	0.00	0.00	0.00	Orifice Invert
410.87	0.25	4.85	4.85	
411.12	0.50	9.50	14.35	
411.37	0.75	9.72	24.07	
411.62	1.00	9.85	33.92	
411.87	1.25	9.85	43.77	
412.12	1.50	9.85	53.61	
412.37	1.75	9.85	63.46	
412.62	2.00	9.85	73.31	Pipe Obvert
412.87	2.25	0.85	74.16	
413.12	2.50	0.85	75.00	
413.37	2.75	0.85	75.85	
413.62	3.00	0.85	76.70	
413.98	3.36	1.22	77.92	CBMH.43 T/G Weir
414.23	3.61	0.57	78.48	Overflow

**ORIFICE CALCULATIONS**

Invert =	410.62	m
Q =	0.057	m <sup>3</sup> /s
Cd =	0.6	
H =	3.55	m
2g =	19.62	
A =	0.011	m <sup>2</sup>
D =	0.120	m

**OVERFLOW WEIR**

Q =	0.104	cu m/s
d1 =	3.610	m
h =	3.360	m
H =	0.250	m
2g =	19.620	
L =	0.600	m

**Super Pipe (continued)**

<b>ELEVATION</b>	<b>STAGE (m)</b>	<b>STORAGE (cu m)</b>	<b>ORIFICE DISCHARGE (cu m/s)</b>	<b>WEIR DISCHARGE (cu m/s)</b>	<b>TOTAL DISCHARGE (cu m/s)</b>	
410.62	0.000	0.00	0.000	0.000	0.000	Orifice Invert
410.87	0.250	4.85	0.013	0.000	0.013	
411.12	0.500	14.35	0.020	0.000	0.020	
411.37	0.750	24.07	0.025	0.000	0.025	
411.62	1.000	33.92	0.029	0.000	0.029	
411.87	1.250	43.77	0.033	0.000	0.033	
412.12	1.500	53.61	0.036	0.000	0.036	Pipe Obvert
412.37	1.750	63.46	0.039	0.000	0.039	
412.62	2.000	73.31	0.042	0.000	0.042	
412.87	2.250	74.16	0.044	0.000	0.044	
413.12	2.500	75.00	0.047	0.000	0.047	
413.37	2.750	75.85	0.049	0.000	0.049	
413.62	3.000	76.70	0.052	0.000	0.052	
413.98	3.360	77.92	0.055	0.000	0.055	CBMH.43 T/G Weir
414.23	3.610	78.48	0.057	0.104	0.160	Overflow

**Ainley Farm Subdivision  
Township of Centre Wellington (Elora)  
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**Catchment 4000: Wetland**

**Stage Storage Volume Calculations**

<b>Elevation</b>	<b>Stage</b>	<b>Surface Area</b>	<b>Increm. Storage</b>	<b>Accum. Storage</b>	
<b>(m)</b>	<b>(m)</b>	<b>(m<sup>2</sup>)</b>	<b>(m<sup>3</sup>)</b>	<b>(m<sup>3</sup>)</b>	
409.63	0.00	2,833	0	0.0	Wetland Bottom
409.75	0.12	3,871	402	402.2	
410.00	0.37	10,414	1786	2,187.9	
410.25	0.62	14,634	3131	5,318.9	
410.50	0.87	19,953	4323	9,642.3	
410.75	1.12	24,730	5585	15,227.7	Overflow

**WEIR CALCULATIONS**

H = 1.12 m

L = 10 m

Q = 18.965 m<sup>3</sup>/s

**Stage/Storage/Discharge Table**

<b>Elevation</b>	<b>Stage</b>	<b>Storage</b>	<b>Actual Discharge</b>	
<b>(m)</b>	<b>(m)</b>	<b>(m<sup>3</sup>)</b>	<b>(m<sup>3</sup>/s)</b>	
409.63	0.00	0.0	0.000	Wetland Bottom
409.75	0.12	402.2	0.665	
410.00	0.37	2,187.9	3.601	
410.25	0.62	5,318.9	7.811	
410.50	0.87	9,642.3	12.984	
410.75	1.12	15,227.7	18.965	Overflow



**Ainley Farm Subdivision  
Township of Centre Wellington (Elora)  
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**Catchment 2200 : Private Stormwater Management Facility Multi-Family Block**

**Stage Storage Volume Calculations**

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m <sup>2</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )	
413.70	0.00	847	0	0.0	Bottom of Pond/Knockout
413.80	0.10	924	89	88.6	
413.90	0.20	1,049	99	187.2	
414.00	0.30	1,175	111	298.4	
414.10	0.40	1,302	124	422.2	
414.20	0.50	1,431	137	558.9	
414.30	0.60	1,561	150	708.5	
414.40	0.70	1,692	163	871.1	CB Lip
414.50	0.80	1,825	176	1,046.9	
414.60	0.90	1,959	189	1,236.1	
414.70	1.00	2,094	203	1,438.7	Weir
415.00	1.30	2,231	649	2,087.4	Top of bank

**Outlet #1**  
110 mm Diameter Knockout

Q = 0.020 m<sup>3</sup>/s  
Cd = 0.600  
H = 0.645 m  
2g = 19.620

A = 0.010 m<sup>2</sup>  
D = 0.110 m  
D/2 = 0.055 m

**Outlet #2**  
450 mm diameter pipe

Q = 0.395 m<sup>3</sup>/s  
Cd = 0.600  
H = 0.875 m  
2g = 19.620

A = 0.159 m<sup>2</sup>  
D = 0.450 m  
D/2 = 0.225 m  
Invert = 413.6

**Overflow Weir**  
Elevation = 414.70

d1 = 1.30 m  
h = 1.00 m  
H = 0.30 m  
2g = 19.62  
L = 10.00 m

Q = 2.369 m<sup>3</sup>/s

**Stage/Storage/Discharge Table**

Elevation	Stage	Storage	Outlet #1 110 mm	Outlet #2 450 mm	Overflow Weir	Actual Discharge	
(m)	(m)	(m <sup>3</sup> )	(m <sup>3</sup> /s)	(m <sup>3</sup> /s)	(m <sup>3</sup> /s)	(m <sup>3</sup> /s)	
413.70	0.00	0.0	0.000	0.000	0.000	0.000	Bottom of Pond/Knockout
413.80	0.10	88.6	0.005	0.000	0.000	0.005	
413.90	0.20	187.2	0.010	0.000	0.000	0.010	
414.00	0.30	298.4	0.013	0.000	0.000	0.013	
414.10	0.40	422.2	0.015	0.000	0.000	0.015	CB Lip
414.20	0.50	558.9	0.000	0.259	0.000	0.259	
414.30	0.60	708.5	0.000	0.291	0.000	0.291	
414.40	0.70	871.1	0.000	0.321	0.000	0.321	
414.50	0.80	1,046.9	0.000	0.347	0.000	0.347	
414.60	0.90	1,236.1	0.000	0.372	0.000	0.372	
414.70	1.00	1,438.7	0.000	0.395	0.000	0.395	Weir
415.00	1.30	2,087.4	0.000	0.458	2.369	2.828	Top of bank

**Ainley Farm Subdivision**  
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**Catchment 1200: Proposed Infiltration Gallery**

**STORAGE VOLUME CALCULATIONS**

<b>ELEV</b>	<b>INC</b>	<b>SURFACE</b>	<b>INCR.</b>	<b>INCR.</b>	<b>ACCUM</b>	
<b>(m)</b>	<b>D</b>	<b>AREA</b>	<b>PIPE</b>	<b>GALLERY</b>	<b>STORAGE</b>	
	<b>(m)</b>	<b>(Infil. Gall)</b>	<b>STORAGE</b>	<b>STORAGE</b>	<b>VOL</b>	
		<b>(sq m)</b>	<b>VOL</b>	<b>VOL</b>	<b>(cu m)</b>	
			<b>(cu m)</b>	<b>(cu m)</b>		
412.000	0.000	539.90	0.00	0.00	0.00	Bottom of Stone
412.100	0.100	539.90	25.13	9.52	34.65	
412.200	0.200	539.90	25.13	9.52	69.31	
412.300	0.300	539.90	25.13	9.52	103.96	
412.400	0.400	539.90	25.13	9.52	138.61	
412.500	0.500	539.90	0.00	17.82	156.43	
412.600	0.600	539.90	0.00	17.82	174.25	
412.700	0.700	539.90	0.00	17.82	192.06	Top of Stone
412.900	0.900	1.85	0.00	0.37	192.43	
413.230	1.230	1.85	0.00	0.61	193.04	CB 46N Lip
413.430	1.430	5.00	0.00	0.68	193.73	
413.630	1.630	10.00	0.00	1.50	195.23	
413.830	1.830	15.00	0.00	2.50	197.73	
414.030	2.030	20.00	0.00	3.50	201.23	
414.090	2.090	25.00	0.00	1.35	202.58	RYCB 46O Lip
414.290	2.290	30.00	0.00	5.50	208.08	
414.490	2.490	35.00	0.00	6.50	214.58	Overflow

**BOTTOM INFILTRATION**

L1(dw) =	79.4	m
W1(dw) =	3.5	m
L2(dw) =	52.4	m
W2(dw) =	5.0	m
D(dw) =	0.70	m
A(c) =	539.9	sq m
VOL(dw)=	277.4	cu m
VOL(st)=	92.5	cu m
K =	4	mm/hr
=	1.11E-04	cm/s

**SIDE INFILTRATION**

**ALL SIDES**

L1(dw) =	79.4	m
W1(dw) =	3.5	m
L2(dw) =	52.4	m
W2(dw) =	5.0	m
D(dw) =	0.7	m
A(c) =	196.4	sq m
K =	4	mm/hr
=	1.11E-04	cm/s

**Weir (1)**

Elevation = 413.23

d1 =	2.49	m
h =	1.23	m
H =	1.26	m
2g =	19.620	
L =	0.36	m
Q =	0.799	m <sup>3</sup> /s

**Weir (2)**

Elevation = 414.09

d1 =	2.49	m
h =	2.09	m
H =	0.40	m
2g =	19.620	
L =	0.36	m
Q =	0.129	m <sup>3</sup> /s

ELEVATION	STAGE (m)	STORAGE (cu m)	INFILTRATION DISCHARGE (cu m/s)	OVERFLOW WEIR (cu m/s)	OVERFLOW WEIR (cu m/s)	TOTAL DISCHARGE (cu m/s)	
412.000	0.000	0.00	0.0003	0.000	0.000	0.0003	Bottom of Stone
412.100	0.100	34.65	0.0003	0.000	0.000	0.0003	
412.200	0.200	69.31	0.0003	0.000	0.000	0.0003	
412.300	0.300	103.96	0.0003	0.000	0.000	0.0003	
412.400	0.400	138.61	0.0003	0.000	0.000	0.0003	
412.500	0.500	156.43	0.0003	0.000	0.000	0.0003	
412.600	0.600	174.25	0.0003	0.000	0.000	0.0003	
412.700	0.700	192.06	0.0003	0.000	0.000	0.0003	Top of Stone
412.900	0.900	192.43	0.0003	0.000	0.000	0.0003	
413.230	1.230	193.04	0.0003	0.000	0.000	0.0003	CB 46N Lip
413.430	1.430	193.73	0.0000	0.045	0.000	0.0453	
413.630	1.630	195.23	0.0000	0.132	0.000	0.1319	
413.830	1.830	197.73	0.0000	0.248	0.000	0.2482	
414.030	2.030	201.23	0.0000	0.390	0.000	0.3899	
414.090	2.090	202.58	0.0000	0.437	0.000	0.4369	RYCB 46O Lip
414.290	2.290	208.08	0.0000	0.608	0.045	0.6524	
414.490	2.490	214.58	0.0000	0.799	0.129	0.9278	Overflow

**Ainley Farm Subdivision**  
**Our File: 411009**  
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**Catchment 1400: Proposed Infiltration Gallery**

**STORAGE VOLUME CALCULATIONS**

<b>ELEV</b>	<b>INC</b>	<b>SURFACE</b>	<b>INCR.</b>	<b>INCR.</b>	<b>ACCUM</b>	
<b>(m)</b>	<b>D</b>	<b>AREA</b>	<b>PIPE</b>	<b>GALLERY</b>	<b>STORAGE</b>	
	<b>(m)</b>	<b>(sq m)</b>	<b>STORAGE</b>	<b>STORAGE</b>	<b>VOL</b>	
			<b>VOL</b>	<b>VOL</b>	<b>(cu m)</b>	
			<b>(cu m)</b>	<b>(cu m)</b>		
413.920	0.000	790.50	0.00	0.00	0.00	Bottom of Stone
414.020	0.100	790.50	0.00	26.35	26.35	
414.120	0.200	790.50	3.68	25.12	55.15	
414.220	0.300	790.50	3.68	25.12	83.95	
414.320	0.400	790.50	3.68	25.12	112.75	
414.420	0.500	790.50	0.00	26.35	139.10	
414.520	0.600	790.50	0.00	26.35	165.45	
414.620	0.700	790.50	0.00	26.35	191.80	Top of Stone
414.720	0.800	0.72	0.07	0.00	191.87	Pipe Invert
414.820	0.900	0.72	0.07	0.00	191.95	
414.920	1.000	0.72	0.07	0.00	192.02	
415.020	1.100	0.72	0.07	0.00	192.09	
415.120	1.200	0.72	0.07	0.00	192.16	Top of Grate
415.220	1.300	185.00	9.29	0.00	201.45	
415.320	1.400	565.00	37.50	0.00	238.95	
415.420	1.500	750.00	65.75	0.00	304.70	Weir
415.520	1.600	800.00	77.50	0.00	382.20	Overflow

**BOTTOM INFILTRATION**

L1(dw) = 79.1 m  
W1(dw) = 10.0 m  
D(dw) = 0.70 m  
  
A(c) = 790.5 sq m  
VOL(dw)= 553.4 cu m  
VOL(st)= 184.5 cu m  
K = 4 mm/hr  
= 1.11E-04 cm/s

**SIDE INFILTRATION**

**ALL SIDES**

L1(dw) = 79.1 m  
W1(dw) = 10.0 m  
D(dw) = 0.7 m  
  
A(c) = 124.7 sq m  
  
K = 4 mm/hr  
= 1.11E-04 cm/s

**Pipe Outlet**

300 mm diameter pipe  
200 mm orifice  
Q = 0.075 m<sup>3</sup>/s  
Cd = 0.6  
H = 0.80 m  
2g = 19.62  
  
A = 0.031 m<sup>2</sup>  
D = 0.2 m  
D/2 = 0.1 m

**Overflow Weir**

Elevation = 411.90  
d1 = 1.600 m  
h = 1.500 m  
H = 0.100 m  
2g = 19.620  
L = 5.000 m  
  
Q = 0.218 m<sup>3</sup>/s

ELEVATION	STAGE (m)	STORAGE (cu m)	INFILTRATION DISCHARGE (cu m/s)	PIPE DISCHARGE (cu m/s)	OVERFLOW WEIR (cu m/s)	TOTAL DISCHARGE (cu m/s)	
413.920	0.000	0.00	0.0009	0.000	0.000	0.0009	Bottom of Stone
414.020	0.100	26.35	0.0009	0.000	0.000	0.0009	
414.120	0.200	55.15	0.0009	0.000	0.000	0.0009	
414.220	0.300	83.95	0.0009	0.000	0.000	0.0009	
414.320	0.400	112.75	0.0009	0.000	0.000	0.0009	
414.420	0.500	139.10	0.0009	0.000	0.000	0.0009	
414.520	0.600	165.45	0.0009	0.000	0.000	0.0009	
414.620	0.700	191.80	0.0009	0.000	0.000	0.0009	Top of Stone
414.720	0.800	191.87	0.0009	0.000	0.000	0.0009	Pipe Invert
414.820	0.900	191.95	0.0000	0.026	0.000	0.0264	
414.920	1.000	192.02	0.0000	0.037	0.000	0.0373	
415.020	1.100	192.09	0.0000	0.046	0.000	0.0457	
415.120	1.200	192.16	0.0000	0.053	0.000	0.0528	Top of Grate
415.220	1.300	201.45	0.0000	0.059	0.219	0.2777	
415.320	1.400	238.95	0.0000	0.065	0.629	0.6941	
415.420	1.500	304.70	0.0000	0.070	1.175	1.2444	Weir
415.520	1.600	382.20	0.0000	0.075	1.834	1.9086	Overflow

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                     Post__2yr.out"
"          Licensee name:                       gmbp"
"          Company                             gmbp"
"          Date & Time last used:               7/25/2022 at 11:17:12 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          695.050 Coefficient A"
"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.293  mm/hr"
"          Total depth                          33.014  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.207  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.829  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.021	0.000	0.000	0.000	c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	13.484	1.233	3.680	minutes"
"	Time to Centroid	116.839	87.775	93.582	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	36.32	36.32	72.63	c.m"
"	Rainfall losses	26.179	5.640	15.910	mm"
"	Runoff depth	6.835	27.374	17.104	mm"
"	Runoff volume	7.52	30.11	37.63	c.m"
"	Runoff coefficient	0.207	0.829	0.518	"
"	Maximum flow	0.003	0.021	0.021	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

	0.021	0.021	0.000	0.000"
--	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.207	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.838	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

	0.063	0.021	0.000	0.000	c.m/sec"
--	-------	-------	-------	-------	----------

"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	20.437	1.868	5.553	minutes"
"	Time to Centroid	125.085	88.659	95.888	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	115.55	115.55	231.10	c.m"
"	Rainfall losses	26.169	5.363	15.766	mm"
"	Runoff depth	6.845	27.651	17.248	mm"
"	Runoff volume	23.96	96.78	120.74	c.m"

"	Runoff coefficient	0.207	0.838	0.522	"
"	Maximum flow	0.007	0.063	0.063	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.063	0.084	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.207 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.836 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.020	0.084	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	17.197	1.572	4.676	minutes"
"	Time to Centroid	121.230	88.299	94.841	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	36.32	36.32	72.63	c.m"
"	Rainfall losses	26.170	5.407	15.789	mm"
"	Runoff depth	6.844	27.606	17.225	mm"
"	Runoff volume	7.53	30.37	37.90	c.m"
"	Runoff coefficient	0.207	0.836	0.522	"
"	Maximum flow	0.002	0.020	0.020	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.020	0.105	0.000	0.000"	
" 54	POND DESIGN"				
"	0.105 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	196.3 Hydrograph volume	c.m"			
"	17. Number of stages"				



```

" 412.000 Minimum water level metre"
" 414.490 Maximum water level metre"
" 412.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 412.000 0.00031 0.000"
" 412.100 0.00031 34.650"
" 412.200 0.00031 69.310"
" 412.300 0.00032 103.960"
" 412.400 0.00032 138.610"
" 412.500 0.00032 156.430"
" 412.600 0.00032 174.250"
" 412.700 0.00032 192.060"
" 412.900 0.00032 192.430"
" 413.230 0.00032 193.040"
" 413.430 0.04528 193.730"
" 413.630 0.1319 195.230"
" 413.830 0.2482 197.730"
" 414.030 0.3899 201.230"
" 414.090 0.4369 202.580"
" 414.290 0.6524 208.080"
" 414.490 0.9278 214.580"
" Peak outflow 0.000 c.m/sec"
" Maximum level 412.300 metre"
" Maximum storage 104.033 c.m"
" Centroidal lag 85.446 hours"
" 0.020 0.105 0.000 0.000 c.m/sec"
" 40 HYDROGRAPH Combine 1000"
" 6 Combine "
" 1000 Node #"
" Infiltrated On-Site"
" Maximum flow 0.000 c.m/sec"
" Hydrograph volume 190.804 c.m"
" 0.020 0.105 0.000 0.000"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.020 0.000 0.000 0.000"
" 33 CATCHMENT 1400"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 1400 Catchment 1400"
" 20.000 % Impervious"
" 0.620 Total Area"
" 30.000 Flow length"
" 2.000 Overland Slope"
" 0.496 Pervious Area"
" 30.000 Pervious length"
" 2.000 Pervious slope"
" 0.124 Impervious Area"

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.837  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.024      0.000      0.000      0.000 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration  26.066      2.383      14.173      minutes"
"      Time to Centroid      131.779      89.515      110.556      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      163.75      40.94      204.69      c.m"
"      Rainfall losses      26.167      5.389      22.011      mm"
"      Runoff depth      6.847      27.625      11.003      mm"
"      Runoff volume      33.96      34.26      68.22      c.m"
"      Runoff coefficient      0.207      0.837      0.333      "
"      Maximum flow      0.008      0.023      0.024      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.024      0.024      0.000      0.000"
" 54      POND DESIGN"
"      0.024      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      68.2      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"		
"		415.320	0.6941	238.950"		
"		415.420	1.244	304.700"		
"		415.520	1.909	382.200"		
"		Peak outflow		0.001	c.m/sec"	
"		Maximum level		414.120	metre"	
"		Maximum storage		55.240	c.m"	
"		Centroidal lag		13.537	hours"	
"		0.024	0.024	0.001	0.000	c.m/sec"
" 40		HYDROGRAPH	Combine	1000"		
"	6	Combine	"			
"	1000	Node #"				
"		Infiltrated On-Site"				
"		Maximum flow		0.001	c.m/sec"	
"		Hydrograph volume		259.018	c.m"	
"		0.024	0.024	0.001	0.001"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.024	0.000	0.001	0.001"	
" 33		CATCHMENT 1500"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1500	Catchment 1500"				
"	50.000	% Impervious"				
"	1.110	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.555	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.555	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.207	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.834	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.105	0.000	0.001	0.001	c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area	"
"		Surface Area	0.555	0.555	1.110	hectare"
"		Time of concentration	30.977	2.832	8.436	minutes"
"		Time to Centroid	137.612	90.217	99.655	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"

"	Rainfall volume	183.23	183.23	366.46	c.m"
"	Rainfall losses	26.164	5.467	15.815	mm"
"	Runoff depth	6.850	27.547	17.198	mm"
"	Runoff volume	38.02	152.89	190.90	c.m"
"	Runoff coefficient	0.207	0.834	0.521	"
"	Maximum flow	0.008	0.104	0.105	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.105 0.105 0.001 0.001"				
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.000		c.m/sec"	
"	Volume of diverted flow	0.000		c.m"	
"	DIV01500.002hyd"				
"	Major flow at 1500"				
"	0.105 0.105 0.105 0.001 c.m/sec"				
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.105 0.105 0.105 0.001"				
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.208 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.846 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.662 0.105 0.105 0.001 c.m/sec"				
"	Catchment 1000 Pervious Impervious Total Area "				
"	Surface Area 3.490 3.490 6.980 hectare"				

"	Time of concentration	53.679	4.907	14.516	minutes"
"	Time to Centroid	164.608	93.181	107.253	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	1152.19	1152.19	2304.37	c.m"
"	Rainfall losses	26.162	5.089	15.626	mm"
"	Runoff depth	6.851	27.925	17.388	mm"
"	Runoff volume	239.12	974.59	1213.71	c.m"
"	Runoff coefficient	0.208	0.846	0.527	"
"	Maximum flow	0.036	0.659	0.662	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.662	0.768	0.105	0.001"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.207 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.009	0.768	0.105	0.001 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	20.437	1.868	20.437	minutes"
"	Time to Centroid	125.085	88.659	125.085	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	158.47	0.00	158.47	c.m"
"	Rainfall losses	26.169	5.363	26.169	mm"
"	Runoff depth	6.845	27.651	6.845	mm"
"	Runoff volume	32.86	0.00	32.86	c.m"
"	Runoff coefficient	0.207	0.000	0.207	"
"	Maximum flow	0.009	0.000	0.009	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

```

"          4  Add Runoff "
"              0.009      0.770      0.105      0.001"
" 54      POND DESIGN"
"          0.770  Current peak flow    c.m/sec"
"          0.250  Target outflow    c.m/sec"
"          1437.5  Hydrograph volume    c.m"
"          12.    Number of stages"
"          411.000  Minimum water level    metre"
"          412.000  Maximum water level    metre"
"          411.000  Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow                0.097    c.m/sec"
"          Maximum level                411.511  metre"
"          Maximum storage              726.712  c.m"
"          Centroidal lag               3.225   hours"
"          0.009      0.770      0.097      0.001 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.009      0.097      0.097      0.001"
" 47      FILEI_0 Read/Open DIV01500.002hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.002hyd"
"          Major flow at 1500"
"          Total volume                0.000    c.m"
"          Maximum flow                0.000    c.m/sec"
"          0.000      0.097      0.097      0.001 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.000      0.097      0.097      0.001"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"      7.330  Total Area"
"     60.000  Flow length"
"      2.000  Overland Slope"
"      7.330  Pervious Area"
"     60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     50.000  Pervious SCS Curve No."
"      0.007  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"     25.400  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.001    0.097    0.097    0.001 c.m/sec"
"      Catchment 4000      Pervious      Impervious Total Area "
"      Surface Area      7.330      0.000      7.330      hectare"
"      Time of concentration 236.855    3.611    236.826    minutes"
"      Time to Centroid    321.151    91.497    321.122    minutes"
"      Rainfall depth      33.014    33.014    33.014    mm"
"      Rainfall volume     2419.92    0.00    2419.92    c.m"
"      Rainfall losses     32.792    5.642    32.792    mm"
"      Runoff depth        0.222    27.372    0.222    mm"
"      Runoff volume       16.24    0.00    16.24    c.m"
"      Runoff coefficient   0.007    0.000    0.007    "
"      Maximum flow       0.001    0.000    0.001    c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"          0.001    0.097    0.097    0.001"
" 54      POND DESIGN"
"      0.097  Current peak flow    c.m/sec"
"      0.250  Target outflow      c.m/sec"
"     1454.3  Hydrograph volume      c.m"
"          6.  Number of stages"
"     409.630  Minimum water level    metre"
"     410.750  Maximum water level    metre"
"     409.630  Starting water level    metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"     409.630    0.000    0.000"
"     409.750    0.6650    402.200"
"     410.000    3.601    2187.900"
"     410.250    7.811    5318.900"
"     410.500   12.984    9642.300"
"     410.750   18.965   15227.70"

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"          Peak outflow                0.097    c.m/sec"
"          Maximum level                409.647  metre"
"          Maximum storage              58.572   c.m"
"          Centroidal lag               3.417   hours"
"          0.001    0.097    0.097    0.001 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.001    0.097    0.097    0.001"
" 52      CHANNEL DESIGN"
"          0.097 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth    metre"
"          7.410 Left bank slope"
"          6.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.163    metre"
"          Velocity                    0.544    m/sec"
"          Channel capacity              10.655   c.m/sec"
"          Critical depth                0.134    metre"
" 53      ROUTE Channel Route 72"
"          72.40 Channel Route 72 Reach length ( metre)"
"          0.459 X-factor <= 0.5"
"          99.839 K-lag ( seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag ( seconds)"
"          0.500 Beta weighting factor"
"          100.000 Routing time step ( seconds)"
"          1 No. of sub-reaches"
"          Peak outflow                0.097    c.m/sec"
"          0.001    0.097    0.097    0.001 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.001    0.097    0.097    0.001"
" 52      CHANNEL DESIGN"
"          0.097 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          2.000 Basewidth    metre"
"          2.950 Left bank slope"
"          3.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.084    metre"
"          Velocity                    0.516    m/sec"
"          Channel capacity              9.246    c.m/sec"
"          Critical depth                0.060    metre"
" 53      ROUTE Channel Route 40"

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"      39.80      Channel Route 40 Reach length  ( metre)"
"      0.441      X-factor <= 0.5"
"      57.872     K-lag  ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000     K-lag  ( seconds)"
"      0.500      Beta weighting factor"
"      60.000     Routing time step  ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow          0.097      c.m/sec"
"          0.001      0.097      0.097      0.001 c.m/sec"
" 40      HYDROGRAPH  Combine      100"
"          6      Combine "
"          100     Node #"
"          Existing Wetland"
"          Maximum flow          0.097      c.m/sec"
"          Hydrograph volume      1454.315      c.m"
"          0.001      0.097      0.097      0.097"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.001      0.000      0.097      0.097"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100     Catchment 2100"
"          60.000     % Impervious"
"          1.960     Total Area"
"          40.000     Flow length"
"          2.000     Overland Slope"
"          0.784     Pervious Area"
"          40.000     Pervious length"
"          2.000     Pervious slope"
"          1.176     Impervious Area"
"          40.000     Impervious length"
"          2.000     Impervious slope"
"          0.250     Pervious Manning 'n'"
"          78.000     Pervious SCS Curve No."
"          0.207     Pervious Runoff coefficient"
"          0.100     Pervious Ia/S coefficient"
"          7.164     Pervious Initial abstraction"
"          0.015     Impervious Manning 'n'"
"          98.000     Impervious SCS Curve No."
"          0.834     Impervious Runoff coefficient"
"          0.100     Impervious Ia/S coefficient"
"          0.518     Impervious Initial abstraction"
"          0.223      0.000      0.097      0.097 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area "
"          Surface Area          0.784      1.176      1.960      hectare"
"          Time of concentration 30.977      2.832      6.834      minutes"

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"	Time to Centroid	137.612	90.217	96.956	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	258.83	388.24	647.07	c.m"
"	Rainfall losses	26.164	5.467	13.746	mm"
"	Runoff depth	6.850	27.547	19.268	mm"
"	Runoff volume	53.70	323.96	377.66	c.m"
"	Runoff coefficient	0.207	0.834	0.584	"
"	Maximum flow	0.012	0.221	0.223	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.223 0.223 0.097 0.097"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.207 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.838 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.129 0.223 0.097 0.097 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 20.437 1.868 2.365 minutes"				
"	Time to Centroid 125.085 88.659 89.634 minutes"				
"	Rainfall depth 33.014 33.014 33.014 mm"				
"	Rainfall volume 26.41 237.70 264.11 c.m"				
"	Rainfall losses 26.169 5.363 7.444 mm"				
"	Runoff depth 6.845 27.651 25.570 mm"				
"	Runoff volume 5.48 199.09 204.56 c.m"				
"	Runoff coefficient 0.207 0.838 0.775 "				
"	Maximum flow 0.002 0.129 0.129 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.129	0.349	0.097	0.097"
" 54	POND DESIGN"				
"	0.349	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	582.2	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.027	c.m/sec"	
"		Maximum level	411.061	metre"	
"		Maximum storage	400.164	c.m"	
"		Centroidal lag	4.889	hours"	
"		0.129	0.349	0.027	0.097 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.129	0.027	0.027	0.097"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.207	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.838	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.011	0.027	0.027	0.097 c.m/sec"	
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	20.437	1.868	14.685	minutes"
"		Time to Centroid	125.085	88.659	113.801	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	142.62	15.85	158.47	c.m"
"		Rainfall losses	26.169	5.363	24.088	mm"
"		Runoff depth	6.845	27.651	8.926	mm"
"		Runoff volume	29.57	13.27	42.84	c.m"
"		Runoff coefficient	0.207	0.838	0.270	"
"		Maximum flow	0.008	0.009	0.011	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.011	0.036	0.027	0.097"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.011	0.036	0.036	0.097"	
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.036		c.m/sec"	
"		Hydrograph volume	625.068		c.m"	
"		0.011	0.036	0.036	0.036"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.011	0.000	0.036	0.036"	
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				

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"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"    98.000  Impervious SCS Curve No."
"      0.834  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.130   0.000   0.036   0.036 c.m/sec"
"      Catchment 2200      Pervious      Impervious      Total Area  "
"      Surface Area      0.230      0.690      0.920      hectare"
"      Time of concentration 30.977      2.832      4.986      minutes"
"      Time to Centroid    137.612     90.217     93.844     minutes"
"      Rainfall depth      33.014      33.014     33.014     mm"
"      Rainfall volume      75.93       227.80     303.73     c.m"
"      Rainfall losses      26.164      5.467      10.641     mm"
"      Runoff depth         6.850       27.547     22.373     mm"
"      Runoff volume        15.75       190.08     205.83     c.m"
"      Runoff coefficient    0.207       0.834      0.678      "
"      Maximum flow         0.004       0.129      0.130      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"              0.130   0.130   0.036   0.036"
" 54      POND DESIGN"
"          0.130  Current peak flow  c.m/sec"
"          0.756  Target outflow  c.m/sec"
"         205.8  Hydrograph volume  c.m"
"          12.    Number of stages"
"        413.700  Minimum water level  metre"
"        415.000  Maximum water level  metre"
"        413.700  Starting water level  metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"        413.700   0.000   0.000"
"        413.800   0.00500   88.600"
"        413.900   0.01000  187.200"
"        414.000   0.01300  298.400"
"        414.100   0.01500  422.200"
"        414.200   0.2590   558.900"
"        414.300   0.2910   708.500"
"        414.400   0.3210   871.100"
"        414.500   0.3470  1046.900"
"        414.600   0.3720  1236.100"
"        414.700   0.3950  1438.700"
"        415.000   2.828  2087.400"
"          Peak outflow      0.008  c.m/sec"
"          Maximum level    413.864  metre"

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"	Maximum storage	151.520	c.m"		
"	Centroidal lag	6.574	hours"		
"	0.130 0.130 0.008 0.036		c.m/sec"		
" 40	HYDROGRAPH Combine 200"				
"	6 Combine "				
"	200 Node #"				
"	To Trib. of Grand River"				
"	Maximum flow	0.043	c.m/sec"		
"	Hydrograph volume	830.894	c.m"		
"	0.130 0.130 0.008 0.043"				
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.130 0.000 0.008 0.043"				
" 33	CATCHMENT 3100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	3100 Catchment 3100"				
"	60.000 % Impervious"				
"	0.420 Total Area"				
"	40.000 Flow length"				
"	1.000 Overland Slope"				
"	0.168 Pervious Area"				
"	40.000 Pervious length"				
"	1.000 Pervious slope"				
"	0.252 Impervious Area"				
"	40.000 Impervious length"				
"	1.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.208 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.830 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.049 0.000 0.008 0.043		c.m/sec"		
"	Catchment 3100 Pervious Impervious Total Area "				
"	Surface Area 0.168 0.252 0.420		hectare"		
"	Time of concentration 38.137 3.486 8.435		minutes"		
"	Time to Centroid 146.130 91.278 99.112		minutes"		
"	Rainfall depth 33.014 33.014 33.014		mm"		
"	Rainfall volume 55.46 83.20 138.66		c.m"		
"	Rainfall losses 26.163 5.605 13.828		mm"		
"	Runoff depth 6.851 27.409 19.186		mm"		
"	Runoff volume 11.51 69.07 80.58		c.m"		
"	Runoff coefficient 0.208 0.830 0.581		"		
"	Maximum flow 0.002 0.048 0.049		c.m/sec"		

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.049      0.049      0.008      0.043"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"          0      Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.000      c.m/sec"
"              Volume of diverted flow      0.000      c.m"
"              DIV32001.002hyd"
"              Major flow at 32001"
"                  0.049      0.049      0.049      0.043 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.049      0.049      0.049      0.043"
" 54      POND DESIGN"
"          0.049  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          80.6  Hydrograph volume      c.m"
"          15.   Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"                  410.620      0.000      0.000"
"                  410.870      0.01300      4.855"
"                  411.120      0.02000      14.351"
"                  411.370      0.02500      24.074"
"                  411.620      0.02900      33.921"
"                  411.870      0.03300      43.768"
"                  412.120      0.03600      53.614"
"                  412.370      0.03900      63.461"
"                  412.620      0.04200      73.308"
"                  412.870      0.04400      74.155"
"                  413.120      0.04700      75.003"
"                  413.370      0.04900      75.850"
"                  413.620      0.05200      76.698"
"                  413.980      0.05500      77.918"
"                  414.230      0.1600      78.483"
"              Peak outflow      0.022      c.m/sec"
"              Maximum level      411.239      metre"
"              Maximum storage      18.968      c.m"
"              Centroidal lag      1.810      hours"
"                  0.049      0.049      0.022      0.043 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.049      0.022      0.022      0.043"
" 33      CATCHMENT 3200"

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"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3200   Catchment 3200"
"    60.000   % Impervious"
"      0.130   Total Area"
"    20.000   Flow length"
"      1.000   Overland Slope"
"      0.052   Pervious Area"
"    20.000   Pervious length"
"      1.000   Pervious slope"
"      0.078   Impervious Area"
"    20.000   Impervious length"
"      1.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"    78.000   Pervious SCS Curve No."
"      0.207   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      7.164   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"    98.000   Impervious SCS Curve No."
"      0.837   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"
"          0.014   0.022   0.022   0.043 c.m/sec"
"      Catchment 3200      Pervious   Impervious Total Area "
"      Surface Area      0.052      0.078      0.130      hectare"
"      Time of concentration 25.161      2.300      5.542      minutes"
"      Time to Centroid    130.696      89.401      95.257      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      17.17      25.75      42.92      c.m"
"      Rainfall losses      26.167      5.393      13.702      mm"
"      Runoff depth        6.847      27.621      19.312      mm"
"      Runoff volume        3.56      21.54      25.11      c.m"
"      Runoff coefficient   0.207      0.837      0.585      "
"      Maximum flow        0.001      0.014      0.014      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4   Add Runoff "
"
"          0.014   0.034   0.022   0.043"
" 33      CATCHMENT 3300"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3300   Catchment 3300"
"    60.000   % Impervious"
"      0.240   Total Area"
"    20.000   Flow length"
"      2.000   Overland Slope"
"      0.096   Pervious Area"
"    20.000   Pervious length"

```



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"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.207 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.838 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.026      0.034      0.022      0.043 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 20.437      1.868      4.499      minutes"
"      Time to Centroid      125.085      88.659      93.819      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      31.69      47.54      79.23      c.m"
"      Rainfall losses      26.169      5.363      13.685      mm"
"      Runoff depth      6.845      27.651      19.329      mm"
"      Runoff volume      6.57      39.82      46.39      c.m"
"      Runoff coefficient      0.207      0.838      0.585      "
"      Maximum flow      0.002      0.026      0.026      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.026      0.060      0.022      0.043"
" 47      FILEI_0 Read/Open DIV32001.002hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.002hyd"
"      Major flow at 32001"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"          0.000      0.060      0.022      0.043 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.000      0.060      0.022      0.043"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.000      0.060      0.060      0.043"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"          To Walser Street"
"      Maximum flow      0.060      c.m/sec"
"      Hydrograph volume      152.160      c.m"

```

"		0.000	0.060	0.060	0.060"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.060		c.m/sec"	
"		Hydrograph volume	152.160		c.m"	
"		0.000	0.060	0.060	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.060	0.060	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.114		c.m/sec"	
"		Hydrograph volume	1606.475		c.m"	
"		0.000	0.060	0.060	0.114"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.114		c.m/sec"	
"		Hydrograph volume	1606.475		c.m"	
"		0.000	0.114	0.060	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.114	0.114	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.156		c.m/sec"	
"		Hydrograph volume	2437.367		c.m"	
"		0.000	0.114	0.114	0.156"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.156		c.m/sec"	
"		Hydrograph volume	2437.366		c.m"	
"		0.000	0.156	0.114	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Post__5yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:20:45 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          113.586  mm/hr"
"          Total depth                49.792  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000  % Impervious"
"          0.220  Total Area"
"          10.000  Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000  Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000  Pervious SCS Curve No."
"          0.317  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.871  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.029	0.000	0.000	0.000	c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	9.868	1.116	3.453	minutes"
"	Time to Centroid	109.069	86.405	92.457	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	54.77	54.77	109.54	c.m"
"	Rainfall losses	33.989	6.414	20.202	mm"
"	Runoff depth	15.803	43.377	29.590	mm"
"	Runoff volume	17.38	47.72	65.10	c.m"
"	Runoff coefficient	0.317	0.871	0.594	"
"	Maximum flow	0.007	0.028	0.029	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

	0.029	0.029	0.000	0.000"
"	4	Add Runoff	"	

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.319	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.883	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

	0.093	0.029	0.000	0.000	c.m/sec"
"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	14.957	1.691	5.209	minutes"
"	Time to Centroid	115.000	87.210	94.579	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	174.27	174.27	348.54	c.m"
"	Rainfall losses	33.921	5.811	19.866	mm"
"	Runoff depth	15.871	43.981	29.926	mm"
"	Runoff volume	55.55	153.93	209.48	c.m"

"	Runoff coefficient	0.319	0.883	0.601	"
"	Maximum flow	0.020	0.086	0.093	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.093	0.122	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.318 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.881 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.029	0.122	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	12.585	1.423	4.386	minutes"
"	Time to Centroid	112.243	86.878	93.610	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	54.77	54.77	109.54	c.m"
"	Rainfall losses	33.944	5.926	19.935	mm"
"	Runoff depth	15.848	43.866	29.857	mm"
"	Runoff volume	17.43	48.25	65.69	c.m"
"	Runoff coefficient	0.318	0.881	0.600	"
"	Maximum flow	0.007	0.028	0.029	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.029	0.151	0.000	0.000"	
" 54	POND DESIGN"				
"	0.151 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	340.3 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

" 412.000 Minimum water level metre"
" 414.490 Maximum water level metre"
" 412.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 412.000 0.00031 0.000"
" 412.100 0.00031 34.650"
" 412.200 0.00031 69.310"
" 412.300 0.00032 103.960"
" 412.400 0.00032 138.610"
" 412.500 0.00032 156.430"
" 412.600 0.00032 174.250"
" 412.700 0.00032 192.060"
" 412.900 0.00032 192.430"
" 413.230 0.00032 193.040"
" 413.430 0.04528 193.730"
" 413.630 0.1319 195.230"
" 413.830 0.2482 197.730"
" 414.030 0.3899 201.230"
" 414.090 0.4369 202.580"
" 414.290 0.6524 208.080"
" 414.490 0.9278 214.580"
" Peak outflow 0.074 c.m/sec"
" Maximum level 413.507 metre"
" Maximum storage 194.308 c.m"
" Centroidal lag 49.492 hours"
" 0.029 0.151 0.074 0.000 c.m/sec"
" 40 HYDROGRAPH Combine 1000"
" 6 Combine "
" 1000 Node #"
" Infiltrated On-Site"
" Maximum flow 0.074 c.m/sec"
" Hydrograph volume 335.882 c.m"
" 0.029 0.151 0.074 0.074"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.029 0.000 0.074 0.074"
" 33 CATCHMENT 1400"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 1400 Catchment 1400"
" 20.000 % Impervious"
" 0.620 Total Area"
" 30.000 Flow length"
" 2.000 Overland Slope"
" 0.496 Pervious Area"
" 30.000 Pervious length"
" 2.000 Pervious slope"
" 0.124 Impervious Area"

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.882  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.038      0.000      0.074      0.074 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration  19.076      2.157      12.156      minutes"
"      Time to Centroid      119.796      87.903      106.752      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      246.97      61.74      308.71      c.m"
"      Rainfall losses      33.923      5.866      28.312      mm"
"      Runoff depth      15.868      43.926      21.480      mm"
"      Runoff volume      78.71      54.47      133.17      c.m"
"      Runoff coefficient      0.319      0.882      0.431      "
"      Maximum flow      0.025      0.031      0.038      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.038      0.038      0.074      0.074"
" 54      POND DESIGN"
"      0.038      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"     133.2      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

```

"		415.220	0.2777	201.450"		
"		415.320	0.6941	238.950"		
"		415.420	1.244	304.700"		
"		415.520	1.909	382.200"		
"		Peak outflow		0.001	c.m/sec"	
"		Maximum level		414.361	metre"	
"		Maximum storage		123.613	c.m"	
"		Centroidal lag		22.741	hours"	
"		0.038	0.038	0.001	0.074	c.m/sec"
" 40		HYDROGRAPH	Combine	1000"		
"	6	Combine	"			
"	1000	Node #"				
"		Infiltrated On-Site"				
"		Maximum flow		0.075	c.m/sec"	
"		Hydrograph volume		469.052	c.m"	
"		0.038	0.038	0.001	0.075"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.038	0.000	0.001	0.075"	
" 33		CATCHMENT 1500"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1500	Catchment 1500"				
"	50.000	% Impervious"				
"	1.110	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.555	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.555	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.319	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.878	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.146	0.000	0.001	0.075	c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area	"
"		Surface Area	0.555	0.555	1.110	hectare"
"		Time of concentration	22.670	2.563	7.922	minutes"
"		Time to Centroid	124.006	88.517	97.975	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"



"	Rainfall volume	276.34	276.34	552.69	c.m"
"	Rainfall losses	33.904	6.066	19.985	mm"
"	Runoff depth	15.888	43.726	29.807	mm"
"	Runoff volume	88.18	242.68	330.86	c.m"
"	Runoff coefficient	0.319	0.878	0.599	"
"	Maximum flow	0.026	0.139	0.146	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.146	0.146	0.001	0.075"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.000			c.m/sec"
"	Volume of diverted flow	0.000			c.m"
"	DIV01500.005hyd"				
"	Major flow at 1500"				
"		0.146	0.146	0.146	0.075 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.146	0.146	0.146	0.075"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.319 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.888 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.935	0.146	0.146	0.075 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"

"	Time of concentration	39.284	4.442	13.658	minutes"
"	Time to Centroid	143.413	91.124	104.955	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	1737.73	1737.73	3475.46	c.m"
"	Rainfall losses	33.893	5.585	19.739	mm"
"	Runoff depth	15.898	44.206	30.052	mm"
"	Runoff volume	554.85	1542.80	2097.65	c.m"
"	Runoff coefficient	0.319	0.888	0.604	"
"	Maximum flow	0.113	0.920	0.935	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.935 1.081 0.146 0.075"				
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.319 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.028 1.081 0.146 0.075 c.m/sec"				
"	Catchment 1100 Pervious Impervious Total Area "				
"	Surface Area 0.480 0.000 0.480 hectare"				
"	Time of concentration 14.957 1.691 14.957 minutes"				
"	Time to Centroid 114.999 87.210 114.999 minutes"				
"	Rainfall depth 49.792 49.792 49.792 mm"				
"	Rainfall volume 239.00 0.00 239.00 c.m"				
"	Rainfall losses 33.921 5.811 33.921 mm"				
"	Runoff depth 15.871 43.981 15.871 mm"				
"	Runoff volume 76.18 0.00 76.18 c.m"				
"	Runoff coefficient 0.319 0.000 0.319 "				
"	Maximum flow 0.028 0.000 0.028 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				

```

"          4  Add Runoff "
"              0.028      1.091      0.146      0.075"
" 54      POND DESIGN"
"          1.091  Current peak flow    c.m/sec"
"          0.250  Target outflow    c.m/sec"
"          2504.7 Hydrograph volume    c.m"
"          12.    Number of stages"
"          411.000 Minimum water level    metre"
"          412.000 Maximum water level    metre"
"          411.000 Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"          411.000      0.000      0.000"
"          411.100      0.03200      18.400"
"          411.200      0.06500      95.500"
"          411.300      0.07700      239.900"
"          411.400      0.08700      435.700"
"          411.500      0.09600      691.300"
"          411.600      0.1050      1027.600"
"          411.700      0.3150      1450.000"
"          411.800      0.3380      1935.800"
"          411.850      0.3490      2195.700"
"          411.900      0.6670      2465.600"
"          412.000      2.018      3030.100"
"          Peak outflow                0.234    c.m/sec"
"          Maximum level                411.661  metre"
"          Maximum storage              1286.482  c.m"
"          Centroidal lag                3.422    hours"
"          0.028      1.091      0.234      0.075 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.028      0.234      0.234      0.075"
" 47      FILEI_0 Read/Open DIV01500.005hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.005hyd"
"          Major flow at 1500"
"          Total volume                0.000    c.m"
"          Maximum flow                0.000    c.m/sec"
"          0.000      0.234      0.234      0.075 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.000      0.234      0.234      0.075"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000 Catchment 4000"
"          0.000 % Impervious"

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"      7.330  Total Area"
"     60.000  Flow length"
"      2.000  Overland Slope"
"      7.330  Pervious Area"
"     60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     50.000  Pervious SCS Curve No."
"      0.043  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"     25.400  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.019    0.234    0.234    0.075 c.m/sec"
"      Catchment 4000      Pervious      Impervious Total Area "
"      Surface Area      7.330      0.000      7.330      hectare"
"      Time of concentration 82.074      3.269      82.072      minutes"
"      Time to Centroid    193.297      89.581      193.294      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume     3649.73      0.00      3649.73      c.m"
"      Rainfall losses     47.655      6.236      47.655      mm"
"      Runoff depth        2.137      43.556      2.137      mm"
"      Runoff volume       156.63      0.00      156.64      c.m"
"      Runoff coefficient   0.043      0.000      0.043      "
"      Maximum flow        0.019      0.000      0.019      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"          0.019    0.244    0.234    0.075"
" 54      POND DESIGN"
"      0.244  Current peak flow      c.m/sec"
"      0.250  Target outflow      c.m/sec"
"     2662.1  Hydrograph volume      c.m"
"          6.  Number of stages"
"     409.630  Minimum water level      metre"
"     410.750  Maximum water level      metre"
"     409.630  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     409.630    0.000    0.000"
"     409.750    0.6650    402.200"
"     410.000    3.601    2187.900"
"     410.250    7.811    5318.900"
"     410.500   12.984    9642.300"
"     410.750   18.965   15227.70"

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"          Peak outflow                0.237    c.m/sec"
"          Maximum level                409.673  metre"
"          Maximum storage              143.048  c.m"
"          Centroidal lag                3.578   hours"
"          0.019    0.244    0.237    0.075 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.019    0.237    0.237    0.075"
" 52      CHANNEL DESIGN"
"          0.237 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth    metre"
"          7.410 Left bank slope"
"          6.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.228    metre"
"          Velocity                    0.680    m/sec"
"          Channel capacity             10.655  c.m/sec"
"          Critical depth               0.191    metre"
" 53      ROUTE Channel Route 72"
"          72.40 Channel Route 72 Reach length ( metre)"
"          0.443 X-factor <= 0.5"
"          79.856 K-lag ( seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag ( seconds)"
"          0.500 Beta weighting factor"
"          75.000 Routing time step ( seconds)"
"          1 No. of sub-reaches"
"          Peak outflow                0.236    c.m/sec"
"          0.019    0.237    0.236    0.075 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.019    0.236    0.236    0.075"
" 52      CHANNEL DESIGN"
"          0.236 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          2.000 Basewidth    metre"
"          2.950 Left bank slope"
"          3.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.140    metre"
"          Velocity                    0.699    m/sec"
"          Channel capacity             9.246  c.m/sec"
"          Critical depth               0.106    metre"
" 53      ROUTE Channel Route 40"

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.403      X-factor <= 0.5"
"    42.730      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"    30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"    50.000      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                0.236      c.m/sec"
"          0.019      0.236      0.236      0.075 c.m/sec"
" 40      HYDROGRAPH   Combine      100"
"          6      Combine  "
"          100     Node #"
"          Existing Wetland"
"          Maximum flow                0.236      c.m/sec"
"          Hydrograph volume          2662.062      c.m"
"          0.019      0.236      0.236      0.236"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.019      0.000      0.236      0.236"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100   Catchment 2100"
"    60.000      % Impervious"
"          1.960   Total Area"
"    40.000      Flow length"
"          2.000   Overland Slope"
"          0.784   Pervious Area"
"    40.000      Pervious length"
"          2.000   Pervious slope"
"          1.176   Impervious Area"
"    40.000      Impervious length"
"          2.000   Impervious slope"
"          0.250   Pervious Manning 'n'"
"    78.000      Pervious SCS Curve No."
"          0.319   Pervious Runoff coefficient"
"          0.100   Pervious Ia/S coefficient"
"          7.164   Pervious Initial abstraction"
"          0.015   Impervious Manning 'n'"
"    98.000      Impervious SCS Curve No."
"          0.878   Impervious Runoff coefficient"
"          0.100   Impervious Ia/S coefficient"
"          0.518   Impervious Initial abstraction"
"          0.304      0.000      0.236      0.236 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area  "
"          Surface Area          0.784      1.176      1.960      hectare"
"          Time of concentration  22.670      2.563      6.484      minutes"

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"	Time to Centroid	124.006	88.517	95.437	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	390.37	585.55	975.92	c.m"
"	Rainfall losses	33.904	6.066	17.201	mm"
"	Runoff depth	15.888	43.726	32.591	mm"
"	Runoff volume	124.56	514.22	638.78	c.m"
"	Runoff coefficient	0.319	0.878	0.655	"
"	Maximum flow	0.036	0.295	0.304	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.304 0.304 0.236 0.236"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.319 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.883 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.177 0.304 0.236 0.236 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 14.957 1.691 2.203 minutes"				
"	Time to Centroid 114.999 87.210 88.281 minutes"				
"	Rainfall depth 49.792 49.792 49.792 mm"				
"	Rainfall volume 39.83 358.50 398.33 c.m"				
"	Rainfall losses 33.921 5.811 8.622 mm"				
"	Runoff depth 15.871 43.981 41.170 mm"				
"	Runoff volume 12.70 316.66 329.36 c.m"				
"	Runoff coefficient 0.319 0.883 0.827 "				
"	Maximum flow 0.005 0.177 0.177 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.177	0.481	0.236	0.236"
" 54	POND DESIGN"				
"	0.481	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	968.1	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.121	c.m/sec"	
"		Maximum level	411.195	metre"	
"		Maximum storage	550.354	c.m"	
"		Centroidal lag	4.085	hours"	
"		0.177	0.481	0.121	0.236 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.177	0.121	0.121	0.236"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			



"	0.319	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.883	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.030	0.121	0.121	0.236 c.m/sec"	
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	14.957	1.691	11.834	minutes"
"		Time to Centroid	114.999	87.210	108.457	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	215.10	23.90	239.00	c.m"
"		Rainfall losses	33.921	5.811	31.110	mm"
"		Runoff depth	15.871	43.981	18.682	mm"
"		Runoff volume	68.56	21.11	89.67	c.m"
"		Runoff coefficient	0.319	0.883	0.375	"
"		Maximum flow	0.025	0.012	0.030	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.030	0.141	0.121	0.236"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.030	0.141	0.141	0.236"	
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.141		c.m/sec"	
"		Hydrograph volume	1056.969		c.m"	
"		0.030	0.141	0.141	0.141"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.030	0.000	0.141	0.141"	
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				

```

"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.878  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.176   0.000   0.141   0.141 c.m/sec"
"      Catchment 2200      Pervious  Impervious Total Area "
"      Surface Area      0.230   0.690   0.920   hectare"
"      Time of concentration  22.670   2.563   4.735   minutes"
"      Time to Centroid      124.006   88.517   92.351   minutes"
"      Rainfall depth      49.792   49.792   49.792   mm"
"      Rainfall volume     114.52   343.56   458.08   c.m"
"      Rainfall losses     33.904   6.066   13.025   mm"
"      Runoff depth       15.888   43.726   36.767   mm"
"      Runoff volume       36.54   301.71   338.25   c.m"
"      Runoff coefficient   0.319   0.878   0.738   "
"      Maximum flow       0.011   0.173   0.176   c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.176   0.176   0.141   0.141"
" 54      POND DESIGN"
"          0.176  Current peak flow  c.m/sec"
"          0.756  Target outflow  c.m/sec"
"         338.3  Hydrograph volume  c.m"
"          12.    Number of stages"
"        413.700  Minimum water level  metre"
"        415.000  Maximum water level  metre"
"        413.700  Starting water level  metre"
"          0     Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"        413.700   0.000   0.000"
"        413.800   0.00500   88.600"
"        413.900   0.01000  187.200"
"        414.000   0.01300  298.400"
"        414.100   0.01500  422.200"
"        414.200   0.2590   558.900"
"        414.300   0.2910   708.500"
"        414.400   0.3210   871.100"
"        414.500   0.3470  1046.900"
"        414.600   0.3720  1236.100"
"        414.700   0.3950  1438.700"
"        415.000   2.828  2087.400"
"          Peak outflow      0.012   c.m/sec"
"          Maximum level    413.963   metre"

```

"		Maximum storage	257.625	c.m"	
"		Centroidal lag	6.867	hours"	
"		0.176 0.176 0.012 0.141		c.m/sec"	
" 40		HYDROGRAPH Combine	200"		
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.151	c.m/sec"	
"		Hydrograph volume	1395.219	c.m"	
"		0.176 0.176 0.012		0.151"	
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.176 0.000 0.012		0.151"	
" 33		CATCHMENT 3100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3100	Catchment 3100"			
"	60.000	% Impervious"			
"	0.420	Total Area"			
"	40.000	Flow length"			
"	1.000	Overland Slope"			
"	0.168	Pervious Area"			
"	40.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.252	Impervious Area"			
"	40.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.319	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.877	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.067 0.000 0.012		0.151 c.m/sec"	
"		Catchment 3100	Pervious	Impervious	Total Area "
"		Surface Area	0.168	0.252	0.420 hectare"
"		Time of concentration	27.910	3.156	7.991 minutes"
"		Time to Centroid	130.112	89.411	97.361 minutes"
"		Rainfall depth	49.792	49.792	49.792 mm"
"		Rainfall volume	83.65	125.48	209.13 c.m"
"		Rainfall losses	33.900	6.148	17.249 mm"
"		Runoff depth	15.891	43.643	32.543 mm"
"		Runoff volume	26.70	109.98	136.68 c.m"
"		Runoff coefficient	0.319	0.877	0.654 "
"		Maximum flow	0.007	0.066	0.067 c.m/sec"

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" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.067      0.067      0.012      0.151"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"          0      Conduit type; 1=Pipe;2=Channel"
"              Peak of diverted flow      0.000      c.m/sec"
"              Volume of diverted flow      0.000      c.m"
"              DIV32001.005hyd"
"              Major flow at 32001"
"                  0.067      0.067      0.067      0.151 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.067      0.067      0.067      0.151"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          136.7  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"
"          410.620      0.000      0.000"
"          410.870      0.01300      4.855"
"          411.120      0.02000      14.351"
"          411.370      0.02500      24.074"
"          411.620      0.02900      33.921"
"          411.870      0.03300      43.768"
"          412.120      0.03600      53.614"
"          412.370      0.03900      63.461"
"          412.620      0.04200      73.308"
"          412.870      0.04400      74.155"
"          413.120      0.04700      75.003"
"          413.370      0.04900      75.850"
"          413.620      0.05200      76.698"
"          413.980      0.05500      77.918"
"          414.230      0.1600      78.483"
"              Peak outflow      0.029      c.m/sec"
"              Maximum level      411.630      metre"
"              Maximum storage      34.317      c.m"
"              Centroidal lag      1.847      hours"
"                  0.067      0.067      0.029      0.151 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.067      0.029      0.029      0.151"
" 33      CATCHMENT 3200"

```

```

"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3200   Catchment 3200"
"    60.000   % Impervious"
"      0.130   Total Area"
"    20.000   Flow length"
"      1.000   Overland Slope"
"      0.052   Pervious Area"
"    20.000   Pervious length"
"      1.000   Pervious slope"
"      0.078   Impervious Area"
"    20.000   Impervious length"
"      1.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"    78.000   Pervious SCS Curve No."
"      0.319   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      7.164   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"    98.000   Impervious SCS Curve No."
"      0.883   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"
"          0.020   0.029   0.029   0.151 c.m/sec"
"      Catchment 3200      Pervious   Impervious Total Area "
"      Surface Area      0.052      0.078      0.130      hectare"
"      Time of concentration 18.414      2.082      5.253      minutes"
"      Time to Centroid    119.025      87.780      93.846      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume     25.89      38.84      64.73      c.m"
"      Rainfall losses     33.912      5.846      17.073      mm"
"      Runoff depth        15.879      43.946      32.719      mm"
"      Runoff volume       8.26      34.28      42.53      c.m"
"      Runoff coefficient   0.319      0.883      0.657      "
"      Maximum flow        0.003      0.019      0.020      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4   Add Runoff "
"
"          0.020   0.045   0.029   0.151"
" 33      CATCHMENT 3300"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3300   Catchment 3300"
"    60.000   % Impervious"
"      0.240   Total Area"
"    20.000   Flow length"
"      2.000   Overland Slope"
"      0.096   Pervious Area"
"    20.000   Pervious length"

```

```

"      2.000  Pervious slope"
"      0.144  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.883  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.037      0.045      0.029      0.151 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area  "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration  14.957      1.691      4.264      minutes"
"      Time to Centroid      115.000      87.210      92.599      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      47.80      71.70      119.50      c.m"
"      Rainfall losses      33.921      5.811      17.055      mm"
"      Runoff depth      15.871      43.981      32.737      mm"
"      Runoff volume      15.24      63.33      78.57      c.m"
"      Runoff coefficient      0.319      0.883      0.657      "
"      Maximum flow      0.006      0.035      0.037      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.037      0.082      0.029      0.151"
" 47      FILEI_0 Read/Open DIV32001.005hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.005hyd"
"      Major flow at 32001"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"          0.000      0.082      0.029      0.151 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.000      0.082      0.029      0.151"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.000      0.082      0.082      0.151"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"          To Walser Street"
"      Maximum flow      0.082      c.m/sec"
"      Hydrograph volume      257.887      c.m"

```

"		0.000	0.082	0.082	0.082"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.082		c.m/sec"	
"		Hydrograph volume	257.887		c.m"	
"		0.000	0.082	0.082	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.082	0.082	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.258		c.m/sec"	
"		Hydrograph volume	2919.949		c.m"	
"		0.000	0.082	0.082	0.258"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.258		c.m/sec"	
"		Hydrograph volume	2919.948		c.m"	
"		0.000	0.258	0.082	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.000	0.258	0.258	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.366		c.m/sec"	
"		Hydrograph volume	4315.163		c.m"	
"		0.000	0.258	0.258	0.366"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.366		c.m/sec"	
"		Hydrograph volume	4315.163		c.m"	
"		0.000	0.366	0.258	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Post__10yr.out"
"          Licensee name:                      gmbp"
"          Company                             gmbp"
"          Date & Time last used:              7/25/2022 at 11:41:13 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          2327.596 Coefficient A"
"          19.500  Constant B"
"          0.894  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          126.171  mm/hr"
"          Total depth                 61.359  mm"
"          6  010hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.379  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.887  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```



	0.037	0.000	0.000	0.000	c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	8.639	1.063	3.329	minutes"
"	Time to Centroid	106.058	85.846	91.893	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	67.50	67.50	134.99	c.m"
"	Rainfall losses	38.124	6.938	22.531	mm"
"	Runoff depth	23.235	54.421	38.828	mm"
"	Runoff volume	25.56	59.86	85.42	c.m"
"	Runoff coefficient	0.379	0.887	0.633	"
"	Maximum flow	0.011	0.032	0.037	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

	0.037	0.037	0.000	0.000"
--	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.379	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.901	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

	0.114	0.037	0.000	0.000	c.m/sec"
--	-------	-------	-------	-------	----------

"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	13.094	1.611	5.010	minutes"
"	Time to Centroid	111.234	86.563	93.866	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	214.76	214.76	429.51	c.m"
"	Rainfall losses	38.098	6.044	22.071	mm"
"	Runoff depth	23.262	55.315	39.288	mm"
"	Runoff volume	81.42	193.60	275.02	c.m"

"	Runoff coefficient	0.379	0.901	0.640	"
"	Maximum flow	0.032	0.100	0.114	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.114	0.151	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.380 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.898 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.036	0.151	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	11.018	1.355	4.226	minutes"
"	Time to Centroid	108.801	86.227	92.933	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	67.50	67.50	134.99	c.m"
"	Rainfall losses	38.068	6.251	22.159	mm"
"	Runoff depth	23.291	55.108	39.200	mm"
"	Runoff volume	25.62	60.62	86.24	c.m"
"	Runoff coefficient	0.380	0.898	0.639	"
"	Maximum flow	0.011	0.032	0.036	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.036	0.188	0.000	0.000"	
" 54	POND DESIGN"				
"	0.188 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	446.7 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

" 412.000 Minimum water level metre"
" 414.490 Maximum water level metre"
" 412.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 412.000 0.00031 0.000"
" 412.100 0.00031 34.650"
" 412.200 0.00031 69.310"
" 412.300 0.00032 103.960"
" 412.400 0.00032 138.610"
" 412.500 0.00032 156.430"
" 412.600 0.00032 174.250"
" 412.700 0.00032 192.060"
" 412.900 0.00032 192.430"
" 413.230 0.00032 193.040"
" 413.430 0.04528 193.730"
" 413.630 0.1319 195.230"
" 413.830 0.2482 197.730"
" 414.030 0.3899 201.230"
" 414.090 0.4369 202.580"
" 414.290 0.6524 208.080"
" 414.490 0.9278 214.580"
" Peak outflow 0.130 c.m/sec"
" Maximum level 413.661 metre"
" Maximum storage 195.615 c.m"
" Centroidal lag 39.670 hours"
" 0.036 0.188 0.130 0.000 c.m/sec"
" 40 HYDROGRAPH Combine 1000"
" 6 Combine "
" 1000 Node #"
" Infiltrated On-Site"
" Maximum flow 0.130 c.m/sec"
" Hydrograph volume 422.535 c.m"
" 0.036 0.188 0.130 0.130"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.036 0.000 0.130 0.130"
" 33 CATCHMENT 1400"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 1400 Catchment 1400"
" 20.000 % Impervious"
" 0.620 Total Area"
" 30.000 Flow length"
" 2.000 Overland Slope"
" 0.496 Pervious Area"
" 30.000 Pervious length"
" 2.000 Pervious slope"
" 0.124 Impervious Area"

```

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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.380  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.900  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.056      0.000      0.130      0.130 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration 16.700      2.054      11.252      minutes"
"      Time to Centroid      115.361      87.160      104.869      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      304.34      76.09      380.43      c.m"
"      Rainfall losses      38.059      6.144      31.676      mm"
"      Runoff depth      23.300      55.215      29.683      mm"
"      Runoff volume      115.57      68.47      184.04      c.m"
"      Runoff coefficient      0.380      0.900      0.484      "
"      Maximum flow      0.040      0.036      0.056      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.056      0.056      0.130      0.130"
" 54      POND DESIGN"
"      0.056      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"     184.0      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

```

"		415.220	0.2777	201.450"		
"		415.320	0.6941	238.950"		
"		415.420	1.244	304.700"		
"		415.520	1.909	382.200"		
"		Peak outflow		0.001	c.m/sec"	
"		Maximum level		414.422	metre"	
"		Maximum storage		139.619	c.m"	
"		Centroidal lag		30.244	hours"	
"		0.056	0.056	0.001	0.130	c.m/sec"
" 40		HYDROGRAPH Combine		1000"		
"	6	Combine "				
"	1000	Node #"				
"		Infiltrated On-Site"				
"		Maximum flow		0.130	c.m/sec"	
"		Hydrograph volume		606.570	c.m"	
"		0.056	0.056	0.001	0.130"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.056	0.000	0.001	0.130"	
" 33		CATCHMENT 1500"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1500	Catchment 1500"				
"	50.000	% Impervious"				
"	1.110	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.555	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.555	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.380	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.897	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.174	0.000	0.001	0.130	c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area	"
"		Surface Area	0.555	0.555	1.110	hectare"
"		Time of concentration	19.847	2.441	7.618	minutes"
"		Time to Centroid	118.992	87.742	97.037	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"

"	Rainfall volume	340.54	340.54	681.09	c.m"
"	Rainfall losses	38.054	6.310	22.182	mm"
"	Runoff depth	23.305	55.050	39.177	mm"
"	Runoff volume	129.34	305.52	434.87	c.m"
"	Runoff coefficient	0.380	0.897	0.638	"
"	Maximum flow	0.041	0.162	0.174	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.174 0.174 0.001 0.130"				
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.028		c.m/sec"	
"	Volume of diverted flow	10.733		c.m"	
"	DIV01500.010hyd"				
"	Major flow at 1500"				
"	0.174 0.174 0.146 0.130 c.m/sec"				
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.174 0.146 0.146 0.130"				
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.380 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.905 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	1.094 0.146 0.146 0.130 c.m/sec"				
"	Catchment 1000 Pervious Impervious Total Area "				
"	Surface Area 3.490 3.490 6.980 hectare"				

"	Time of concentration	34.392	4.230	13.155	minutes"
"	Time to Centroid	135.773	90.174	103.666	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	2141.44	2141.44	4282.88	c.m"
"	Rainfall losses	38.029	5.843	21.936	mm"
"	Runoff depth	23.330	55.517	39.424	mm"
"	Runoff volume	814.23	1937.53	2751.76	c.m"
"	Runoff coefficient	0.380	0.905	0.643	"
"	Maximum flow	0.188	1.064	1.094	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		1.094	1.240	0.146	0.130"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.379 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.043	1.240	0.146	0.130 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	13.094	1.611	13.094	minutes"
"	Time to Centroid	111.234	86.563	111.234	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	294.52	0.00	294.52	c.m"
"	Rainfall losses	38.098	6.044	38.097	mm"
"	Runoff depth	23.262	55.315	23.262	mm"
"	Runoff volume	111.66	0.00	111.66	c.m"
"	Runoff coefficient	0.379	0.000	0.379	"
"	Maximum flow	0.043	0.000	0.043	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"              0.043      1.260      0.146      0.130"
" 54      POND DESIGN"
"          1.260  Current peak flow    c.m/sec"
"          0.250  Target outflow    c.m/sec"
"          3287.6  Hydrograph volume    c.m"
"          12.    Number of stages"
"          411.000  Minimum water level    metre"
"          412.000  Maximum water level    metre"
"          411.000  Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"          411.000    0.000    0.000"
"          411.100    0.03200    18.400"
"          411.200    0.06500    95.500"
"          411.300    0.07700    239.900"
"          411.400    0.08700    435.700"
"          411.500    0.09600    691.300"
"          411.600    0.1050    1027.600"
"          411.700    0.3150    1450.000"
"          411.800    0.3380    1935.800"
"          411.850    0.3490    2195.700"
"          411.900    0.6670    2465.600"
"          412.000    2.018    3030.100"
"          Peak outflow                0.323    c.m/sec"
"          Maximum level                411.736    metre"
"          Maximum storage              1623.071    c.m"
"          Centroidal lag                3.233    hours"
"          0.043      1.260      0.323      0.130 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.043      0.323      0.323      0.130"
" 47      FILEI_0 Read/Open DIV01500.010hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.010hyd"
"          Major flow at 1500"
"          Total volume                10.733    c.m"
"          Maximum flow                0.028    c.m/sec"
"          0.028      0.323      0.323      0.130 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.028      0.323      0.323      0.130"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"      7.330  Total Area"
"     60.000  Flow length"
"      2.000  Overland Slope"
"      7.330  Pervious Area"
"     60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     50.000  Pervious SCS Curve No."
"      0.073  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"     25.400  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.050   0.323   0.323   0.130 c.m/sec"
"      Catchment 4000      Pervious      Impervious Total Area "
"      Surface Area      7.330      0.000      7.330      hectare"
"      Time of concentration  57.122      3.114      57.121      minutes"
"      Time to Centroid      167.459      88.727      167.458      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      4497.63      0.00      4497.63      c.m"
"      Rainfall losses      56.901      6.470      56.901      mm"
"      Runoff depth      4.458      54.890      4.458      mm"
"      Runoff volume      326.76      0.00      326.77      c.m"
"      Runoff coefficient      0.073      0.000      0.073      "
"      Maximum flow      0.050      0.000      0.050      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"          0.050   0.372   0.323   0.130"
" 54      POND DESIGN"
"      0.372  Current peak flow      c.m/sec"
"      0.250  Target outflow      c.m/sec"
"     3625.2  Hydrograph volume      c.m"
"          6.  Number of stages"
"     409.630  Minimum water level      metre"
"     410.750  Maximum water level      metre"
"     409.630  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     409.630   0.000   0.000"
"     409.750   0.6650  402.200"
"     410.000   3.601  2187.900"
"     410.250   7.811  5318.900"
"     410.500  12.984  9642.300"
"     410.750  18.965  15227.70"

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"          Peak outflow                0.367    c.m/sec"
"          Maximum level                409.696  metre"
"          Maximum storage              222.278  c.m"
"          Centroidal lag                3.355   hours"
"          0.050    0.372    0.367    0.130 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.050    0.367    0.367    0.130"
" 52      CHANNEL DESIGN"
"          0.367 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth    metre"
"          7.410 Left bank slope"
"          6.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.269    metre"
"          Velocity                    0.759    m/sec"
"          Channel capacity             10.655  c.m/sec"
"          Critical depth                0.228    metre"
" 53      ROUTE Channel Route 72"
"          72.40 Channel Route 72 Reach length ( metre)"
"          0.433 X-factor <= 0.5"
"          71.586 K-lag ( seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag ( seconds)"
"          0.500 Beta weighting factor"
"          75.000 Routing time step ( seconds)"
"          1 No. of sub-reaches"
"          Peak outflow                0.367    c.m/sec"
"          0.050    0.367    0.367    0.130 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.050    0.367    0.367    0.130"
" 52      CHANNEL DESIGN"
"          0.367 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          2.000 Basewidth    metre"
"          2.950 Left bank slope"
"          3.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.180    metre"
"          Velocity                    0.806    m/sec"
"          Channel capacity             9.246  c.m/sec"
"          Critical depth                0.140    metre"
" 53      ROUTE Channel Route 40"

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.378      X-factor <= 0.5"
"      37.024      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"      30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"      42.857      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                0.367      c.m/sec"
"          0.050      0.367      0.367      0.130 c.m/sec"
" 40      HYDROGRAPH   Combine      100"
"          6      Combine "
"          100     Node #"
"          Existing Wetland"
"          Maximum flow                0.367      c.m/sec"
"          Hydrograph volume           3625.235      c.m"
"          0.050      0.367      0.367      0.367"
" 40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.050      0.000      0.367      0.367"
" 33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100     Catchment 2100"
"          60.000     % Impervious"
"          1.960     Total Area"
"          40.000     Flow length"
"          2.000     Overland Slope"
"          0.784     Pervious Area"
"          40.000     Pervious length"
"          2.000     Pervious slope"
"          1.176     Impervious Area"
"          40.000     Impervious length"
"          2.000     Impervious slope"
"          0.250     Pervious Manning 'n'"
"          78.000     Pervious SCS Curve No."
"          0.380     Pervious Runoff coefficient"
"          0.100     Pervious Ia/S coefficient"
"          7.164     Pervious Initial abstraction"
"          0.015     Impervious Manning 'n'"
"          98.000     Impervious SCS Curve No."
"          0.897     Impervious Runoff coefficient"
"          0.100     Impervious Ia/S coefficient"
"          0.518     Impervious Initial abstraction"
"          0.359      0.000      0.367      0.367 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area "
"          Surface Area          0.784      1.176      1.960      hectare"
"          Time of concentration 19.847      2.441      6.272      minutes"

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"	Time to Centroid	118.992	87.742	94.620	minutes"
"	Rainfall depth	61.359	61.359	61.359	mm"
"	Rainfall volume	481.06	721.58	1202.64	c.m"
"	Rainfall losses	38.054	6.310	19.008	mm"
"	Runoff depth	23.305	55.050	42.352	mm"
"	Runoff volume	182.71	647.38	830.09	c.m"
"	Runoff coefficient	0.380	0.897	0.690	"
"	Maximum flow	0.058	0.342	0.359	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.359 0.359 0.367 0.367"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.379 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.901 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.208 0.359 0.367 0.367 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 13.094 1.611 2.123 minutes"				
"	Time to Centroid 111.234 86.563 87.664 minutes"				
"	Rainfall depth 61.359 61.359 61.359 mm"				
"	Rainfall volume 49.09 441.79 490.87 c.m"				
"	Rainfall losses 38.098 6.044 9.250 mm"				
"	Runoff depth 23.262 55.315 52.110 mm"				
"	Runoff volume 18.61 398.27 416.88 c.m"				
"	Runoff coefficient 0.379 0.901 0.849 "				
"	Maximum flow 0.007 0.207 0.208 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.208	0.567	0.367	0.367"
" 54	POND DESIGN"				
"	0.567	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	1247.0	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.141	c.m/sec"	
"		Maximum level	411.314	metre"	
"		Maximum storage	693.286	c.m"	
"		Centroidal lag	3.701	hours"	
"		0.208	0.567	0.141	0.367 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.208	0.141	0.141	0.367"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.379	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.901	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.045	0.141	0.141	0.367	c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	13.094	1.611	10.694	minutes"
"		Time to Centroid	111.234	86.563	106.078	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"
"		Rainfall volume	265.07	29.45	294.52	c.m"
"		Rainfall losses	38.098	6.044	34.892	mm"
"		Runoff depth	23.262	55.315	26.467	mm"
"		Runoff volume	100.49	26.55	127.04	c.m"
"		Runoff coefficient	0.379	0.901	0.431	"
"		Maximum flow	0.039	0.014	0.045	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.045	0.178	0.141	0.367"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.045	0.178	0.178	0.367"	
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.178			c.m/sec"
"		Hydrograph volume	1373.708			c.m"
"		0.045	0.178	0.178	0.178"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.045	0.000	0.178	0.178"	
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				

```

"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.380  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.897  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.206      0.000      0.178      0.178 c.m/sec"
"      Catchment 2200      Pervious      Impervious      Total Area  "
"      Surface Area      0.230      0.690      0.920      hectare"
"      Time of concentration  19.847      2.441      4.594      minutes"
"      Time to Centroid      118.992      87.742      91.606      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      141.13      423.38      564.51      c.m"
"      Rainfall losses      38.054      6.310      14.246      mm"
"      Runoff depth      23.305      55.050      47.113      mm"
"      Runoff volume      53.60      379.84      433.44      c.m"
"      Runoff coefficient      0.380      0.897      0.768      "
"      Maximum flow      0.017      0.201      0.206      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.206      0.206      0.178      0.178"
" 54      POND DESIGN"
"      0.206      Current peak flow      c.m/sec"
"      0.756      Target outflow      c.m/sec"
"      433.4      Hydrograph volume      c.m"
"      12.      Number of stages"
"      413.700      Minimum water level      metre"
"      415.000      Maximum water level      metre"
"      413.700      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      413.700      0.000      0.000"
"      413.800      0.00500      88.600"
"      413.900      0.01000      187.200"
"      414.000      0.01300      298.400"
"      414.100      0.01500      422.200"
"      414.200      0.2590      558.900"
"      414.300      0.2910      708.500"
"      414.400      0.3210      871.100"
"      414.500      0.3470      1046.900"
"      414.600      0.3720      1236.100"
"      414.700      0.3950      1438.700"
"      415.000      2.828      2087.400"
"      Peak outflow      0.014      c.m/sec"
"      Maximum level      414.033      metre"

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"		Maximum storage	338.951	c.m"	
"		Centroidal lag	7.231	hours"	
"		0.206 0.206 0.014 0.178		c.m/sec"	
" 40		HYDROGRAPH Combine	200"		
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.190	c.m/sec"	
"		Hydrograph volume	1807.152	c.m"	
"		0.206 0.206 0.014		0.190"	
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.206 0.000 0.014		0.190"	
" 33		CATCHMENT 3100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3100	Catchment 3100"			
"	60.000	% Impervious"			
"	0.420	Total Area"			
"	40.000	Flow length"			
"	1.000	Overland Slope"			
"	0.168	Pervious Area"			
"	40.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.252	Impervious Area"			
"	40.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.380	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.895	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.078 0.000 0.014		0.190 c.m/sec"	
"		Catchment 3100	Pervious	Impervious	Total Area "
"		Surface Area	0.168	0.252	0.420 hectare"
"		Time of concentration	24.434	3.005	7.732 minutes"
"		Time to Centroid	124.292	88.564	96.444 minutes"
"		Rainfall depth	61.359	61.359	61.359 mm"
"		Rainfall volume	103.08	154.63	257.71 c.m"
"		Rainfall losses	38.047	6.437	19.081 mm"
"		Runoff depth	23.312	54.922	42.278 mm"
"		Runoff volume	39.16	138.40	177.57 c.m"
"		Runoff coefficient	0.380	0.895	0.689 "
"		Maximum flow	0.011	0.075	0.078 c.m/sec"



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" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.078      0.078      0.014      0.190"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"          0      Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow      0.011      c.m/sec"
"          Volume of diverted flow      3.174      c.m"
"          DIV32001.010hyd"
"          Major flow at 32001"
"              0.078      0.078      0.067      0.190 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.078      0.067      0.067      0.190"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          174.4  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          410.620      0.000      0.000"
"          410.870      0.01300      4.855"
"          411.120      0.02000      14.351"
"          411.370      0.02500      24.074"
"          411.620      0.02900      33.921"
"          411.870      0.03300      43.768"
"          412.120      0.03600      53.614"
"          412.370      0.03900      63.461"
"          412.620      0.04200      73.308"
"          412.870      0.04400      74.155"
"          413.120      0.04700      75.003"
"          413.370      0.04900      75.850"
"          413.620      0.05200      76.698"
"          413.980      0.05500      77.918"
"          414.230      0.1600      78.483"
"          Peak outflow      0.033      c.m/sec"
"          Maximum level      411.906      metre"
"          Maximum storage      45.197      c.m"
"          Centroidal lag      1.881      hours"
"              0.078      0.067      0.033      0.190 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.078      0.033      0.033      0.190"
" 33      CATCHMENT 3200"

```

```

"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3200   Catchment 3200"
"    60.000   % Impervious"
"      0.130   Total Area"
"    20.000   Flow length"
"      1.000   Overland Slope"
"      0.052   Pervious Area"
"    20.000   Pervious length"
"      1.000   Pervious slope"
"      0.078   Impervious Area"
"    20.000   Impervious length"
"      1.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"    78.000   Pervious SCS Curve No."
"      0.379   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      7.164   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"    98.000   Impervious SCS Curve No."
"      0.901   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"
"          0.024   0.033   0.033   0.190 c.m/sec"
"      Catchment 3200      Pervious   Impervious Total Area "
"      Surface Area      0.052      0.078      0.130      hectare"
"      Time of concentration 16.121      1.983      5.083      minutes"
"      Time to Centroid    114.721      87.040      93.109      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume     31.91      47.86      79.77      c.m"
"      Rainfall losses     38.081      6.103      18.894      mm"
"      Runoff depth        23.278      55.256      42.465      mm"
"      Runoff volume       12.10      43.10      55.20      c.m"
"      Runoff coefficient   0.379      0.901      0.692      "
"      Maximum flow        0.004      0.023      0.024      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4   Add Runoff "
"
"          0.024   0.051   0.033   0.190"
" 33      CATCHMENT 3300"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3300   Catchment 3300"
"    60.000   % Impervious"
"      0.240   Total Area"
"    20.000   Flow length"
"      2.000   Overland Slope"
"      0.096   Pervious Area"
"    20.000   Pervious length"

```

```

"      2.000  Pervious slope"
"      0.144  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.379  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.901  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.045      0.051      0.033      0.190 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 13.094      1.611      4.125      minutes"
"      Time to Centroid      111.234      86.563      91.965      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      58.90      88.36      147.26      c.m"
"      Rainfall losses      38.097      6.044      18.866      mm"
"      Runoff depth      23.262      55.315      42.494      mm"
"      Runoff volume      22.33      79.65      101.98      c.m"
"      Runoff coefficient      0.379      0.901      0.693      "
"      Maximum flow      0.009      0.041      0.045      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.045      0.096      0.033      0.190"
" 47      FILEI_0 Read/Open DIV32001.010hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.010hyd"
"      Major flow at 32001"
"      Total volume      3.174      c.m"
"      Maximum flow      0.011      c.m/sec"
"          0.011      0.096      0.033      0.190 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.011      0.107      0.033      0.190"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.011      0.107      0.107      0.190"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"          To Walser Street"
"      Maximum flow      0.107      c.m/sec"
"      Hydrograph volume      334.694      c.m"

```

"		0.011	0.107	0.107	0.107"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.107		c.m/sec"	
"		Hydrograph volume	334.694		c.m"	
"		0.011	0.107	0.107	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.011	0.107	0.107	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.395		c.m/sec"	
"		Hydrograph volume	3959.929		c.m"	
"		0.011	0.107	0.107	0.395"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.395		c.m/sec"	
"		Hydrograph volume	3959.929		c.m"	
"		0.011	0.395	0.107	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.011	0.395	0.395	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.551		c.m/sec"	
"		Hydrograph volume	5767.070		c.m"	
"		0.011	0.395	0.395	0.551"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.551		c.m/sec"	
"		Hydrograph volume	5767.070		c.m"	
"		0.011	0.551	0.395	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Post__25yr.out"
"          Licensee name:                      gmbp"
"          Company                             gmbp"
"          Date & Time last used:              7/25/2022 at 11:43:00 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          3701.648 Coefficient A"
"          25.500  Constant B"
"          0.937  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          143.371  mm/hr"
"          Total depth                 75.581  mm"
"          6  025hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000  % Impervious"
"          0.220  Total Area"
"          10.000  Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000  Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000  Pervious SCS Curve No."
"          0.440  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.899  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.047	0.000	0.000	0.000	c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	7.622	1.005	3.181	minutes"
"	Time to Centroid	103.628	85.398	91.394	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	83.14	83.14	166.28	c.m"
"	Rainfall losses	42.296	7.669	24.982	mm"
"	Runoff depth	33.285	67.912	50.598	mm"
"	Runoff volume	36.61	74.70	111.32	c.m"
"	Runoff coefficient	0.440	0.899	0.669	"
"	Maximum flow	0.018	0.038	0.047	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

	0.047	0.047	0.000	0.000"
--	-------	-------	-------	--------

" 33 CATCHMENT 1300"

"	1	Triangular SCS"
"	1	Equal length"
"	1	SCS method"
"	1300	Catchment 1300"
"	50.000	% Impervious"
"	0.700	Total Area"
"	20.000	Flow length"
"	2.000	Overland Slope"
"	0.350	Pervious Area"
"	20.000	Pervious length"
"	2.000	Pervious slope"
"	0.350	Impervious Area"
"	20.000	Impervious length"
"	2.000	Impervious slope"
"	0.250	Pervious Manning 'n'"
"	78.000	Pervious SCS Curve No."
"	0.441	Pervious Runoff coefficient"
"	0.100	Pervious Ia/S coefficient"
"	7.164	Pervious Initial abstraction"
"	0.015	Impervious Manning 'n'"
"	98.000	Impervious SCS Curve No."
"	0.916	Impervious Runoff coefficient"
"	0.100	Impervious Ia/S coefficient"
"	0.518	Impervious Initial abstraction"

	0.144	0.047	0.000	0.000	c.m/sec"
--	-------	-------	-------	-------	----------

"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	11.553	1.523	4.782	minutes"
"	Time to Centroid	108.042	85.984	93.151	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	264.53	264.53	529.07	c.m"
"	Rainfall losses	42.253	6.330	24.292	mm"
"	Runoff depth	33.328	69.250	51.289	mm"
"	Runoff volume	116.65	242.38	359.02	c.m"

"	Runoff coefficient	0.441	0.916	0.679	"
"	Maximum flow	0.047	0.118	0.144	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.144	0.191	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.439 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.912 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.046	0.191	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	9.721	1.281	4.026	minutes"
"	Time to Centroid	106.001	85.673	92.284	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	83.14	83.14	166.28	c.m"
"	Rainfall losses	42.377	6.681	24.529	mm"
"	Runoff depth	33.204	68.900	51.052	mm"
"	Runoff volume	36.52	75.79	112.31	c.m"
"	Runoff coefficient	0.439	0.912	0.675	"
"	Maximum flow	0.016	0.038	0.046	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.046	0.238	0.000	0.000"	
" 54	POND DESIGN"				
"	0.238 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	582.7 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

" 412.000 Minimum water level metre"
" 414.490 Maximum water level metre"
" 412.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 412.000 0.00031 0.000"
" 412.100 0.00031 34.650"
" 412.200 0.00031 69.310"
" 412.300 0.00032 103.960"
" 412.400 0.00032 138.610"
" 412.500 0.00032 156.430"
" 412.600 0.00032 174.250"
" 412.700 0.00032 192.060"
" 412.900 0.00032 192.430"
" 413.230 0.00032 193.040"
" 413.430 0.04528 193.730"
" 413.630 0.1319 195.230"
" 413.830 0.2482 197.730"
" 414.030 0.3899 201.230"
" 414.090 0.4369 202.580"
" 414.290 0.6524 208.080"
" 414.490 0.9278 214.580"
" Peak outflow 0.213 c.m/sec"
" Maximum level 413.791 metre"
" Maximum storage 197.238 c.m"
" Centroidal lag 30.349 hours"
" 0.046 0.238 0.213 0.000 c.m/sec"
" 40 HYDROGRAPH Combine 1000"
" 6 Combine "
" 1000 Node #"
" Infiltrated On-Site"
" Maximum flow 0.213 c.m/sec"
" Hydrograph volume 559.248 c.m"
" 0.046 0.238 0.213 0.213"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.046 0.000 0.213 0.213"
" 33 CATCHMENT 1400"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 1400 Catchment 1400"
" 20.000 % Impervious"
" 0.620 Total Area"
" 30.000 Flow length"
" 2.000 Overland Slope"
" 0.496 Pervious Area"
" 30.000 Pervious length"
" 2.000 Pervious slope"
" 0.124 Impervious Area"

```



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"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.441  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.916  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.082      0.000      0.213      0.213 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration  14.734      1.942      10.366      minutes"
"      Time to Centroid      111.700      86.497      103.093      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      374.88      93.72      468.60      c.m"
"      Rainfall losses      42.213      6.364      35.043      mm"
"      Runoff depth      33.368      69.217      40.538      mm"
"      Runoff volume      165.50      85.83      251.33      c.m"
"      Runoff coefficient      0.441      0.916      0.536      "
"      Maximum flow      0.061      0.042      0.082      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.082      0.082      0.213      0.213"
" 54      POND DESIGN"
"      0.082      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      251.3      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

```

"		415.220	0.2777	201.450"		
"		415.320	0.6941	238.950"		
"		415.420	1.244	304.700"		
"		415.520	1.909	382.200"		
"		Peak outflow		0.017	c.m/sec"	
"		Maximum level		414.821	metre"	
"		Maximum storage		191.951	c.m"	
"		Centroidal lag		27.025	hours"	
"		0.082	0.082	0.017	0.213	c.m/sec"
" 40		HYDROGRAPH	Combine	1000"		
"	6	Combine	"			
"	1000	Node #"				
"		Infiltrated On-Site"				
"		Maximum flow		0.213	c.m/sec"	
"		Hydrograph volume		808.641	c.m"	
"		0.082	0.082	0.017	0.213"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.082	0.000	0.017	0.213"	
" 33		CATCHMENT 1500"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1500	Catchment 1500"				
"	50.000	% Impervious"				
"	1.110	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.555	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.555	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.442	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.913	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.213	0.000	0.017	0.213	c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area	"
"		Surface Area	0.555	0.555	1.110	hectare"
"		Time of concentration	17.510	2.308	7.265	minutes"
"		Time to Centroid	114.842	87.059	96.118	minutes"
"		Rainfall depth	75.581	75.581	75.581	mm"

"	Rainfall volume	419.47	419.47	838.95	c.m"
"	Rainfall losses	42.200	6.593	24.396	mm"
"	Runoff depth	33.381	68.988	51.185	mm"
"	Runoff volume	185.27	382.88	568.15	c.m"
"	Runoff coefficient	0.442	0.913	0.677	"
"	Maximum flow	0.064	0.190	0.213	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.213	0.213	0.017	0.213"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.067			c.m/sec"
"	Volume of diverted flow	46.077			c.m"
"	DIV01500.025hyd"				
"	Major flow at 1500"				
"		0.213	0.213	0.146	0.213 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.213	0.146	0.146	0.213"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.442 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.917 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		1.292	0.146	0.146	0.213 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"

"	Time of concentration	30.343	4.000	12.568	minutes"
"	Time to Centroid	129.499	89.355	102.412	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	2637.77	2637.77	5275.54	c.m"
"	Rainfall losses	42.180	6.290	24.235	mm"
"	Runoff depth	33.401	69.291	51.346	mm"
"	Runoff volume	1165.68	2418.25	3583.93	c.m"
"	Runoff coefficient	0.442	0.917	0.679	"
"	Maximum flow	0.298	1.235	1.292	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		1.292	1.438	0.146	0.213"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.441 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.065	1.438	0.146	0.213 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	11.553	1.523	11.553	minutes"
"	Time to Centroid	108.042	85.984	108.042	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	362.79	0.00	362.79	c.m"
"	Rainfall losses	42.253	6.330	42.253	mm"
"	Runoff depth	33.328	69.250	33.328	mm"
"	Runoff volume	159.97	0.00	159.98	c.m"
"	Runoff coefficient	0.441	0.000	0.441	"
"	Maximum flow	0.065	0.000	0.065	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"              0.065      1.476      0.146      0.213"
" 54      POND DESIGN"
"          1.476  Current peak flow    c.m/sec"
"          0.250  Target outflow    c.m/sec"
"          4266.0  Hydrograph volume    c.m"
"          12.    Number of stages"
"          411.000  Minimum water level    metre"
"          412.000  Maximum water level    metre"
"          411.000  Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"          411.000    0.000    0.000"
"          411.100    0.03200    18.400"
"          411.200    0.06500    95.500"
"          411.300    0.07700    239.900"
"          411.400    0.08700    435.700"
"          411.500    0.09600    691.300"
"          411.600    0.1050    1027.600"
"          411.700    0.3150    1450.000"
"          411.800    0.3380    1935.800"
"          411.850    0.3490    2195.700"
"          411.900    0.6670    2465.600"
"          412.000    2.018    3030.100"
"          Peak outflow                0.398    c.m/sec"
"          Maximum level                411.858    metre"
"          Maximum storage                2237.928    c.m"
"          Centroidal lag                3.270    hours"
"          0.065      1.476      0.398      0.213 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.065      0.398      0.398      0.213"
" 47      FILEI_0 Read/Open DIV01500.025hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.025hyd"
"          Major flow at 1500"
"          Total volume                46.077    c.m"
"          Maximum flow                0.067    c.m/sec"
"          0.067      0.398      0.398      0.213 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.067      0.398      0.398      0.213"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"      7.330  Total Area"
"     60.000  Flow length"
"      2.000  Overland Slope"
"      7.330  Pervious Area"
"     60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     50.000  Pervious SCS Curve No."
"      0.109  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"     25.400  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.114    0.398    0.398    0.213 c.m/sec"
"      Catchment 4000      Pervious      Impervious      Total Area  "
"      Surface Area      7.330      0.000      7.330      hectare"
"      Time of concentration  43.720      2.944      43.719      minutes"
"      Time to Centroid      151.037      87.974      151.037      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      5540.07      0.01      5540.08      c.m"
"      Rainfall losses      67.305      6.942      67.305      mm"
"      Runoff depth      8.276      68.639      8.276      mm"
"      Runoff volume      606.62      0.01      606.62      c.m"
"      Runoff coefficient      0.109      0.000      0.109      "
"      Maximum flow      0.114      0.000      0.114      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.114    0.512    0.398    0.213"
" 54      POND DESIGN"
"      0.512  Current peak flow      c.m/sec"
"      0.250  Target outflow      c.m/sec"
"     4920.7  Hydrograph volume      c.m"
"      6.     Number of stages"
"     409.630  Minimum water level      metre"
"     410.750  Maximum water level      metre"
"     409.630  Starting water level      metre"
"      0     Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     409.630    0.000    0.000"
"     409.750    0.6650   402.200"
"     410.000    3.601   2187.900"
"     410.250    7.811   5318.900"
"     410.500   12.984   9642.300"
"     410.750   18.965  15227.70"

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"          Peak outflow          0.485    c.m/sec"
"          Maximum level        409.717    metre"
"          Maximum storage      293.288    c.m"
"          Centroidal lag       3.326    hours"
"          0.114    0.512    0.485    0.213 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.114    0.485    0.485    0.213"
" 52      CHANNEL DESIGN"
"          0.485 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth    metre"
"          7.410 Left bank slope"
"          6.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow          0.298    metre"
"          Velocity                0.813    m/sec"
"          Channel capacity       10.655    c.m/sec"
"          Critical depth         0.254    metre"
" 53      ROUTE Channel Route 72"
"          72.40 Channel Route 72 Reach length ( metre)"
"          0.426 X-factor <= 0.5"
"          66.766 K-lag ( seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag ( seconds)"
"          0.500 Beta weighting factor"
"          75.000 Routing time step ( seconds)"
"          1 No. of sub-reaches"
"          Peak outflow          0.484    c.m/sec"
"          0.114    0.485    0.484    0.213 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.114    0.484    0.484    0.213"
" 52      CHANNEL DESIGN"
"          0.484 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          2.000 Basewidth    metre"
"          2.950 Left bank slope"
"          3.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow          0.210    metre"
"          Velocity                0.880    m/sec"
"          Channel capacity       9.246    c.m/sec"
"          Critical depth         0.166    metre"
" 53      ROUTE Channel Route 40"

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"      39.80      Channel Route 40 Reach length  ( metre)"
"      0.359      X-factor <= 0.5"
"  33.931      K-lag  ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"  30.000      K-lag  ( seconds)"
"      0.500      Beta weighting factor"
"  42.857      Routing time step  ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow              0.482      c.m/sec"
"          0.114      0.484      0.482      0.213 c.m/sec"
"  40      HYDROGRAPH  Combine      100"
"          6      Combine  "
"          100     Node #"
"          Existing Wetland"
"          Maximum flow              0.482      c.m/sec"
"          Hydrograph volume          4920.682      c.m"
"          0.114      0.484      0.482      0.482"
"  40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.114      0.000      0.482      0.482"
"  33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100     Catchment 2100"
"  60.000     % Impervious"
"          1.960     Total Area"
"  40.000     Flow length"
"          2.000     Overland Slope"
"          0.784     Pervious Area"
"  40.000     Pervious length"
"          2.000     Pervious slope"
"          1.176     Impervious Area"
"  40.000     Impervious length"
"          2.000     Impervious slope"
"          0.250     Pervious Manning 'n'"
"  78.000     Pervious SCS Curve No."
"          0.442     Pervious Runoff coefficient"
"          0.100     Pervious Ia/S coefficient"
"          7.164     Pervious Initial abstraction"
"          0.015     Impervious Manning 'n'"
"  98.000     Impervious SCS Curve No."
"          0.913     Impervious Runoff coefficient"
"          0.100     Impervious Ia/S coefficient"
"          0.518     Impervious Initial abstraction"
"          0.435      0.000      0.482      0.482 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area  "
"          Surface Area          0.784      1.176      1.960      hectare"
"          Time of concentration  17.510      2.308      6.016      minutes"

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"	Time to Centroid	114.842	87.059	93.835	minutes"
"	Rainfall depth	75.581	75.581	75.581	mm"
"	Rainfall volume	592.55	888.83	1481.38	c.m"
"	Rainfall losses	42.200	6.593	20.835	mm"
"	Runoff depth	33.381	68.988	54.745	mm"
"	Runoff volume	261.71	811.30	1073.01	c.m"
"	Runoff coefficient	0.442	0.913	0.724	"
"	Maximum flow	0.090	0.402	0.435	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.435 0.435 0.482 0.482"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.441 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.916 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.246 0.435 0.482 0.482 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 11.553 1.523 2.032 minutes"				
"	Time to Centroid 108.042 85.984 87.103 minutes"				
"	Rainfall depth 75.581 75.581 75.581 mm"				
"	Rainfall volume 60.46 544.18 604.65 c.m"				
"	Rainfall losses 42.253 6.330 9.923 mm"				
"	Runoff depth 33.328 69.250 65.658 mm"				
"	Runoff volume 26.66 498.60 525.27 c.m"				
"	Runoff coefficient 0.441 0.916 0.869 "				
"	Maximum flow 0.011 0.243 0.246 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.246	0.680	0.482	0.482"
" 54	POND DESIGN"				
"	0.680	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	1598.3	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.161	c.m/sec"	
"		Maximum level	411.484	metre"	
"		Maximum storage	911.685	c.m"	
"		Centroidal lag	3.538	hours"	
"		0.246	0.680	0.161	0.482 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.246	0.161	0.161	0.482"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.441	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.916	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.067	0.161	0.161	0.482 c.m/sec"	
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	11.553	1.523	9.671	minutes"
"		Time to Centroid	108.042	85.984	103.905	minutes"
"		Rainfall depth	75.581	75.581	75.581	mm"
"		Rainfall volume	326.51	36.28	362.79	c.m"
"		Rainfall losses	42.253	6.330	38.660	mm"
"		Runoff depth	33.328	69.250	36.920	mm"
"		Runoff volume	143.98	33.24	177.22	c.m"
"		Runoff coefficient	0.441	0.916	0.488	"
"		Maximum flow	0.059	0.016	0.067	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.067	0.214	0.161	0.482"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.067	0.214	0.214	0.482"	
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.214		c.m/sec"	
"		Hydrograph volume	1777.141		c.m"	
"		0.067	0.214	0.214	0.214"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.067	0.000	0.214	0.214"	
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				

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"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.442  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.913  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.245   0.000   0.214   0.214 c.m/sec"
"      Catchment 2200      Pervious  Impervious Total Area "
"      Surface Area      0.230   0.690   0.920   hectare"
"      Time of concentration 17.510   2.308   4.420   minutes"
"      Time to Centroid    114.842  87.059  90.917  minutes"
"      Rainfall depth     75.581  75.581  75.581  mm"
"      Rainfall volume    173.84  521.51  695.34  c.m"
"      Rainfall losses    42.200  6.593  15.494  mm"
"      Runoff depth       33.381  68.988  60.086  mm"
"      Runoff volume      76.78  476.02  552.80  c.m"
"      Runoff coefficient  0.442  0.913  0.795   "
"      Maximum flow      0.026  0.236  0.245   c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.245   0.245   0.214   0.214"
" 54      POND DESIGN"
"          0.245  Current peak flow  c.m/sec"
"          0.756  Target outflow  c.m/sec"
"          552.8  Hydrograph volume  c.m"
"          12.    Number of stages"
"         413.700  Minimum water level  metre"
"         415.000  Maximum water level  metre"
"         413.700  Starting water level  metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge  Volume"
"         413.700   0.000   0.000"
"         413.800   0.00500   88.600"
"         413.900   0.01000  187.200"
"         414.000   0.01300  298.400"
"         414.100   0.01500  422.200"
"         414.200   0.2590   558.900"
"         414.300   0.2910   708.500"
"         414.400   0.3210   871.100"
"         414.500   0.3470  1046.900"
"         414.600   0.3720  1236.100"
"         414.700   0.3950  1438.700"
"         415.000   2.828  2087.400"
"          Peak outflow      0.027  c.m/sec"
"          Maximum level    414.105  metre"

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"		Maximum storage	428.861	c.m"	
"		Centroidal lag	7.439	hours"	
"		0.245 0.245 0.027 0.214		c.m/sec"	
" 40		HYDROGRAPH Combine	200"		
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.227	c.m/sec"	
"		Hydrograph volume	2329.845	c.m"	
"		0.245 0.245 0.027		0.227"	
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.245 0.000 0.027		0.227"	
" 33		CATCHMENT 3100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3100	Catchment 3100"			
"	60.000	% Impervious"			
"	0.420	Total Area"			
"	40.000	Flow length"			
"	1.000	Overland Slope"			
"	0.168	Pervious Area"			
"	40.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.252	Impervious Area"			
"	40.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.442	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.908	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.091 0.000 0.027		0.227 c.m/sec"	
"		Catchment 3100	Pervious	Impervious	Total Area "
"		Surface Area	0.168	0.252	0.420 hectare"
"		Time of concentration	21.558	2.842	7.426 minutes"
"		Time to Centroid	119.480	87.822	95.576 minutes"
"		Rainfall depth	75.581	75.581	75.581 mm"
"		Rainfall volume	126.98	190.46	317.44 c.m"
"		Rainfall losses	42.201	6.986	21.072 mm"
"		Runoff depth	33.380	68.594	54.509 mm"
"		Runoff volume	56.08	172.86	228.94 c.m"
"		Runoff coefficient	0.442	0.908	0.721 "
"		Maximum flow	0.017	0.086	0.091 c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.091      0.091      0.027      0.227"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"          0      Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow      0.024      c.m/sec"
"          Volume of diverted flow      12.772      c.m"
"          DIV32001.025hyd"
"          Major flow at 32001"
"              0.091      0.091      0.067      0.227 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.091      0.067      0.067      0.227"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          216.2  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          410.620      0.000      0.000"
"          410.870      0.01300      4.855"
"          411.120      0.02000      14.351"
"          411.370      0.02500      24.074"
"          411.620      0.02900      33.921"
"          411.870      0.03300      43.768"
"          412.120      0.03600      53.614"
"          412.370      0.03900      63.461"
"          412.620      0.04200      73.308"
"          412.870      0.04400      74.155"
"          413.120      0.04700      75.003"
"          413.370      0.04900      75.850"
"          413.620      0.05200      76.698"
"          413.980      0.05500      77.918"
"          414.230      0.1600      78.483"
"          Peak outflow      0.038      c.m/sec"
"          Maximum level      412.267      metre"
"          Maximum storage      59.408      c.m"
"          Centroidal lag      1.926      hours"
"              0.091      0.067      0.038      0.227 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.091      0.038      0.038      0.227"
" 33      CATCHMENT 3200"

```

```

"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3200   Catchment 3200"
"    60.000   % Impervious"
"      0.130   Total Area"
"    20.000   Flow length"
"      1.000   Overland Slope"
"      0.052   Pervious Area"
"    20.000   Pervious length"
"      1.000   Pervious slope"
"      0.078   Impervious Area"
"    20.000   Impervious length"
"      1.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"    78.000   Pervious SCS Curve No."
"      0.441   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      7.164   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"    98.000   Impervious SCS Curve No."
"      0.916   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"                0.029      0.038      0.038      0.227 c.m/sec"
"      Catchment 3200      Pervious      Impervious Total Area "
"      Surface Area      0.052      0.078      0.130      hectare"
"      Time of concentration 14.223      1.875      4.878      minutes"
"      Time to Centroid 111.113      86.404      92.413      minutes"
"      Rainfall depth 75.581      75.581      75.581      mm"
"      Rainfall volume 39.30      58.95      98.26      c.m"
"      Rainfall losses 42.219      6.365      20.706      mm"
"      Runoff depth 33.362      69.216      54.874      mm"
"      Runoff volume 17.35      53.99      71.34      c.m"
"      Runoff coefficient 0.441      0.916      0.726      "
"      Maximum flow 0.007      0.026      0.029      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"                0.029      0.058      0.038      0.227"
" 33      CATCHMENT 3300"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3300   Catchment 3300"
"    60.000   % Impervious"
"      0.240   Total Area"
"    20.000   Flow length"
"      2.000   Overland Slope"
"      0.096   Pervious Area"
"    20.000   Pervious length"

```

```

"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.441 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.916 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.055      0.058      0.038      0.227 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 11.553      1.523      3.959      minutes"
"      Time to Centroid      108.042      85.984      91.342      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      72.56      108.84      181.39      c.m"
"      Rainfall losses      42.253      6.330      20.699      mm"
"      Runoff depth      33.328      69.250      54.881      mm"
"      Runoff volume      31.99      99.72      131.72      c.m"
"      Runoff coefficient      0.441      0.916      0.726      "
"      Maximum flow      0.013      0.049      0.055      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.055      0.113      0.038      0.227"
" 47      FILEI_0 Read/Open DIV32001.025hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.025hyd"
"      Major flow at 32001"
"      Total volume      12.772      c.m"
"      Maximum flow      0.024      c.m/sec"
"          0.024      0.113      0.038      0.227 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.024      0.137      0.038      0.227"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.024      0.137      0.137      0.227"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"          To Walser Street"
"      Maximum flow      0.137      c.m/sec"
"      Hydrograph volume      431.922      c.m"

```



"		0.024	0.137	0.137	0.137"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.137		c.m/sec"	
"		Hydrograph volume	431.922		c.m"	
"		0.024	0.137	0.137	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.024	0.137	0.137	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.524		c.m/sec"	
"		Hydrograph volume	5352.604		c.m"	
"		0.024	0.137	0.137	0.524"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.524		c.m/sec"	
"		Hydrograph volume	5352.604		c.m"	
"		0.024	0.524	0.137	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.024	0.524	0.524	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.719		c.m/sec"	
"		Hydrograph volume	7682.439		c.m"	
"		0.024	0.524	0.524	0.719"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.719		c.m/sec"	
"		Hydrograph volume	7682.439		c.m"	
"		0.024	0.719	0.524	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Post__50yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:46:50 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          5089.418 Coefficient A"
"          30.000  Constant B"
"          0.967  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          156.350  mm/hr"
"          Total depth                 86.737  mm"
"          6  050hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000  % Impervious"
"          0.220  Total Area"
"          10.000  Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000  Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000  Pervious SCS Curve No."
"          0.479  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.904  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.055	0.000	0.000	0.000	c.m/sec"
"	Catchment 1200	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	7.056	0.968	3.076	minutes"
"	Time to Centroid	102.283	85.178	91.100	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	95.41	95.41	190.82	c.m"
"	Rainfall losses	45.201	8.296	26.748	mm"
"	Runoff depth	41.536	78.441	59.988	mm"
"	Runoff volume	45.69	86.28	131.97	c.m"
"	Runoff coefficient	0.479	0.904	0.692	"
"	Maximum flow	0.021	0.041	0.055	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

	0.055	0.055	0.000	0.000"
"	4	Add Runoff	"	

	0.055	0.055	0.000	0.000"
" 33	CATCHMENT 1300"			
"	1	Triangular SCS"		
"	1	Equal length"		
"	1	SCS method"		
"	1300	Catchment 1300"		
"	50.000	% Impervious"		
"	0.700	Total Area"		
"	20.000	Flow length"		
"	2.000	Overland Slope"		
"	0.350	Pervious Area"		
"	20.000	Pervious length"		
"	2.000	Pervious slope"		
"	0.350	Impervious Area"		
"	20.000	Impervious length"		
"	2.000	Impervious slope"		
"	0.250	Pervious Manning 'n'"		
"	78.000	Pervious SCS Curve No."		
"	0.481	Pervious Runoff coefficient"		
"	0.100	Pervious Ia/S coefficient"		
"	7.164	Pervious Initial abstraction"		
"	0.015	Impervious Manning 'n'"		
"	98.000	Impervious SCS Curve No."		
"	0.924	Impervious Runoff coefficient"		
"	0.100	Impervious Ia/S coefficient"		
"	0.518	Impervious Initial abstraction"		

	0.169	0.055	0.000	0.000	c.m/sec"
"	Catchment 1300	Pervious	Impervious	Total Area	"
"	Surface Area	0.350	0.350	0.700	hectare"
"	Time of concentration	10.695	1.467	4.627	minutes"
"	Time to Centroid	106.283	85.675	92.731	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	303.58	303.58	607.16	c.m"
"	Rainfall losses	44.994	6.561	25.777	mm"
"	Runoff depth	41.743	80.176	60.959	mm"
"	Runoff volume	146.10	280.61	426.72	c.m"

"	Runoff coefficient	0.481	0.924	0.703	"
"	Maximum flow	0.060	0.131	0.169	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.169	0.224	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.480 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.919 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.054	0.224	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	9.000	1.235	3.900	minutes"
"	Time to Centroid	104.357	85.382	91.895	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	95.41	95.41	190.82	c.m"
"	Rainfall losses	45.077	7.038	26.057	mm"
"	Runoff depth	41.660	79.699	60.679	mm"
"	Runoff volume	45.83	87.67	133.49	c.m"
"	Runoff coefficient	0.480	0.919	0.700	"
"	Maximum flow	0.020	0.042	0.054	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.054	0.279	0.000	0.000"	
" 54	POND DESIGN"				
"	0.279 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	692.2 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

" 412.000 Minimum water level metre"
" 414.490 Maximum water level metre"
" 412.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 412.000 0.00031 0.000"
" 412.100 0.00031 34.650"
" 412.200 0.00031 69.310"
" 412.300 0.00032 103.960"
" 412.400 0.00032 138.610"
" 412.500 0.00032 156.430"
" 412.600 0.00032 174.250"
" 412.700 0.00032 192.060"
" 412.900 0.00032 192.430"
" 413.230 0.00032 193.040"
" 413.430 0.04528 193.730"
" 413.630 0.1319 195.230"
" 413.830 0.2482 197.730"
" 414.030 0.3899 201.230"
" 414.090 0.4369 202.580"
" 414.290 0.6524 208.080"
" 414.490 0.9278 214.580"
" Peak outflow 0.270 c.m/sec"
" Maximum level 413.868 metre"
" Maximum storage 198.404 c.m"
" Centroidal lag 24.205 hours"
" 0.054 0.279 0.270 0.000 c.m/sec"
" 40 HYDROGRAPH Combine 1000"
" 6 Combine "
" 1000 Node #"
" Infiltrated On-Site"
" Maximum flow 0.270 c.m/sec"
" Hydrograph volume 710.278 c.m"
" 0.054 0.279 0.270 0.270"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.054 0.000 0.270 0.270"
" 33 CATCHMENT 1400"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 1400 Catchment 1400"
" 20.000 % Impervious"
" 0.620 Total Area"
" 30.000 Flow length"
" 2.000 Overland Slope"
" 0.496 Pervious Area"
" 30.000 Pervious length"
" 2.000 Pervious slope"
" 0.124 Impervious Area"

```

```

"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.482  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.925  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.105      0.000      0.270      0.270 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration  13.641      1.872      9.825      minutes"
"      Time to Centroid      109.632      86.148      102.018      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      430.21      107.55      537.77      c.m"
"      Rainfall losses      44.941      6.526      37.258      mm"
"      Runoff depth      41.796      80.211      49.479      mm"
"      Runoff volume      207.31      99.46      306.77      c.m"
"      Runoff coefficient      0.482      0.925      0.570      "
"      Maximum flow      0.079      0.047      0.105      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.105      0.105      0.270      0.270"
" 54      POND DESIGN"
"      0.105      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      306.8      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

```

"		415.220	0.2777	201.450"		
"		415.320	0.6941	238.950"		
"		415.420	1.244	304.700"		
"		415.520	1.909	382.200"		
"		Peak outflow		0.046	c.m/sec"	
"		Maximum level		415.121	metre"	
"		Maximum storage		192.296	c.m"	
"		Centroidal lag		22.464	hours"	
"		0.105	0.105	0.046	0.270	c.m/sec"
" 40		HYDROGRAPH	Combine	1000"		
"	6	Combine	"			
"	1000	Node #"				
"		Infiltrated On-Site"				
"		Maximum flow		0.271	c.m/sec"	
"		Hydrograph volume		1014.930	c.m"	
"		0.105	0.105	0.046	0.271"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.105	0.000	0.046	0.271"	
" 33		CATCHMENT 1500"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1500	Catchment 1500"				
"	50.000	% Impervious"				
"	1.110	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.555	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.555	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.481	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.922	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.242	0.000	0.046	0.271	c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area	"
"		Surface Area	0.555	0.555	1.110	hectare"
"		Time of concentration	16.211	2.224	7.023	minutes"
"		Time to Centroid	112.570	86.667	95.554	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"

"	Rainfall volume	481.39	481.39	962.78	c.m"
"	Rainfall losses	44.974	6.774	25.874	mm"
"	Runoff depth	41.763	79.963	60.863	mm"
"	Runoff volume	231.78	443.80	675.58	c.m"
"	Runoff coefficient	0.481	0.922	0.702	"
"	Maximum flow	0.084	0.210	0.242	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.242	0.242	0.046	0.271"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.096			c.m/sec"
"	Volume of diverted flow	90.793			c.m"
"	DIV01500.050hyd"				
"	Major flow at 1500"				
"		0.242	0.242	0.146	0.271 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.242	0.146	0.146	0.271"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.482 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.923 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		1.452	0.146	0.146	0.271 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"



"	Time of concentration	28.092	3.854	12.172	minutes"
"	Time to Centroid	125.992	88.892	101.624	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	3027.11	3027.11	6054.23	c.m"
"	Rainfall losses	44.916	6.695	25.806	mm"
"	Runoff depth	41.821	80.042	60.931	mm"
"	Runoff volume	1459.54	2793.46	4253.00	c.m"
"	Runoff coefficient	0.482	0.923	0.702	"
"	Maximum flow	0.393	1.367	1.452	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		1.452	1.598	0.146	0.271"
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.481 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.082	1.598	0.146	0.271 c.m/sec"
"	Catchment 1100	Pervious	Impervious	Total Area	"
"	Surface Area	0.480	0.000	0.480	hectare"
"	Time of concentration	10.695	1.467	10.695	minutes"
"	Time to Centroid	106.283	85.675	106.283	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	416.34	0.00	416.34	c.m"
"	Rainfall losses	44.994	6.561	44.994	mm"
"	Runoff depth	41.743	80.176	41.743	mm"
"	Runoff volume	200.37	0.00	200.37	c.m"
"	Runoff coefficient	0.481	0.000	0.481	"
"	Maximum flow	0.082	0.000	0.082	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"              0.082      1.654      0.146      0.271"
" 54      POND DESIGN"
"          1.654  Current peak flow    c.m/sec"
"          0.250  Target outflow    c.m/sec"
"          5038.2  Hydrograph volume    c.m"
"          12.    Number of stages"
"          411.000  Minimum water level    metre"
"          412.000  Maximum water level    metre"
"          411.000  Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"          411.000    0.000    0.000"
"          411.100    0.03200    18.400"
"          411.200    0.06500    95.500"
"          411.300    0.07700    239.900"
"          411.400    0.08700    435.700"
"          411.500    0.09600    691.300"
"          411.600    0.1050    1027.600"
"          411.700    0.3150    1450.000"
"          411.800    0.3380    1935.800"
"          411.850    0.3490    2195.700"
"          411.900    0.6670    2465.600"
"          412.000    2.018    3030.100"
"          Peak outflow                0.658    c.m/sec"
"          Maximum level                411.899    metre"
"          Maximum storage                2458.114    c.m"
"          Centroidal lag                3.134    hours"
"          0.082      1.654      0.658      0.271 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.082      0.658      0.658      0.271"
" 47      FILEI_0 Read/Open DIV01500.050hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.050hyd"
"          Major flow at 1500"
"          Total volume                90.793    c.m"
"          Maximum flow                0.096    c.m/sec"
"          0.096    0.658    0.658    0.271 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.096    0.658    0.658    0.271"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"      7.330  Total Area"
"      60.000  Flow length"
"      2.000  Overland Slope"
"      7.330  Pervious Area"
"      60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"      60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      50.000  Pervious SCS Curve No."
"      0.137  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      25.400  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.183      0.658      0.658      0.271 c.m/sec"
"      Catchment 4000      Pervious      Impervious      Total Area  "
"      Surface Area      7.330      0.000      7.330      hectare"
"      Time of concentration  37.472      2.837      37.472      minutes"
"      Time to Centroid      142.983      87.552      142.982      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      6357.80      0.01      6357.80      c.m"
"      Rainfall losses      74.812      7.307      74.812      mm"
"      Runoff depth      11.925      79.429      11.925      mm"
"      Runoff volume      874.09      0.01      874.10      c.m"
"      Runoff coefficient      0.137      0.000      0.137      "
"      Maximum flow      0.183      0.000      0.183      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.183      0.841      0.658      0.271"
" 54      POND DESIGN"
"      0.841  Current peak flow      c.m/sec"
"      0.250  Target outflow      c.m/sec"
"      6000.2  Hydrograph volume      c.m"
"      6.      Number of stages"
"      409.630  Minimum water level      metre"
"      410.750  Maximum water level      metre"
"      409.630  Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      409.630      0.000      0.000"
"      409.750      0.6650      402.200"
"      410.000      3.601      2187.900"
"      410.250      7.811      5318.900"
"      410.500      12.984      9642.300"
"      410.750      18.965      15227.70"

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"          Peak outflow          0.768    c.m/sec"
"          Maximum level         409.759    metre"
"          Maximum storage       466.326    c.m"
"          Centroidal lag        3.164    hours"
"          0.183    0.841    0.768    0.271 c.m/sec"
40  HYDROGRAPH Next link "
"          5 Next link "
"          0.183    0.768    0.768    0.271"
52  CHANNEL DESIGN"
"          0.768 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth    metre"
"          7.410 Left bank slope"
"          6.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow          0.354    metre"
"          Velocity                0.912    m/sec"
"          Channel capacity       10.655    c.m/sec"
"          Critical depth         0.306    metre"
53  ROUTE Channel Route 72"
"          72.40 Channel Route 72 Reach length ( metre)"
"          0.412 X-factor <= 0.5"
"          59.519 K-lag ( seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag ( seconds)"
"          0.500 Beta weighting factor"
"          60.000 Routing time step ( seconds)"
"          1 No. of sub-reaches"
"          Peak outflow          0.767    c.m/sec"
"          0.183    0.768    0.767    0.271 c.m/sec"
40  HYDROGRAPH Next link "
"          5 Next link "
"          0.183    0.767    0.767    0.271"
52  CHANNEL DESIGN"
"          0.767 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0. Cross-section type: 0=trapezoidal; 1=general"
"          2.000 Basewidth    metre"
"          2.950 Left bank slope"
"          3.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow          0.270    metre"
"          Velocity                1.013    m/sec"
"          Channel capacity       9.246    c.m/sec"
"          Critical depth         0.220    metre"
53  ROUTE Channel Route 40"

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"      39.80      Channel Route 40 Reach length   ( metre)"
"      0.323      X-factor <= 0.5"
"    29.473      K-lag   ( seconds)"
"      0.000      Default(0) or user spec.(1) values used"
"      0.500      X-factor <= 0.5"
"    30.000      K-lag   ( seconds)"
"      0.500      Beta weighting factor"
"    37.500      Routing time step   ( seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                0.766      c.m/sec"
"          0.183      0.767      0.766      0.271 c.m/sec"
"  40      HYDROGRAPH   Combine   100"
"          6      Combine  "
"          100     Node #"
"          Existing Wetland"
"          Maximum flow                0.766      c.m/sec"
"          Hydrograph volume            6000.176      c.m"
"          0.183      0.767      0.766      0.766"
"  40      HYDROGRAPH Start - New Tributary"
"          2      Start - New Tributary"
"          0.183      0.000      0.766      0.766"
"  33      CATCHMENT 2100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          2100   Catchment 2100"
"    60.000      % Impervious"
"          1.960   Total Area"
"    40.000      Flow length"
"          2.000   Overland Slope"
"          0.784   Pervious Area"
"    40.000      Pervious length"
"          2.000   Pervious slope"
"          1.176   Impervious Area"
"    40.000      Impervious length"
"          2.000   Impervious slope"
"          0.250   Pervious Manning 'n'"
"    78.000      Pervious SCS Curve No."
"          0.481   Pervious Runoff coefficient"
"          0.100   Pervious Ia/S coefficient"
"          7.164   Pervious Initial abstraction"
"          0.015   Impervious Manning 'n'"
"    98.000      Impervious SCS Curve No."
"          0.922   Impervious Runoff coefficient"
"          0.100   Impervious Ia/S coefficient"
"          0.518   Impervious Initial abstraction"
"          0.491      0.000      0.766      0.766 c.m/sec"
"          Catchment 2100      Pervious      Impervious Total Area  "
"          Surface Area                0.784      1.176      1.960      hectare"
"          Time of concentration  16.211      2.224      5.836      minutes"

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"	Time to Centroid	112.570	86.667	93.357	minutes"
"	Rainfall depth	86.737	86.737	86.737	mm"
"	Rainfall volume	680.02	1020.02	1700.04	c.m"
"	Rainfall losses	44.974	6.773	22.054	mm"
"	Runoff depth	41.763	79.963	64.683	mm"
"	Runoff volume	327.42	940.37	1267.79	c.m"
"	Runoff coefficient	0.481	0.922	0.746	"
"	Maximum flow	0.119	0.446	0.491	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.491 0.491 0.766 0.766"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.481 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.924 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.274 0.491 0.766 0.766 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 10.695 1.467 1.972 minutes"				
"	Time to Centroid 106.283 85.675 86.802 minutes"				
"	Rainfall depth 86.737 86.737 86.737 mm"				
"	Rainfall volume 69.39 624.50 693.89 c.m"				
"	Rainfall losses 44.994 6.561 10.404 mm"				
"	Runoff depth 41.743 80.176 76.332 mm"				
"	Runoff volume 33.39 577.26 610.66 c.m"				
"	Runoff coefficient 0.481 0.924 0.880 "				
"	Maximum flow 0.014 0.270 0.274 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.274	0.764	0.766	0.766"
" 54	POND DESIGN"				
"	0.764	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	1878.4	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.175	c.m/sec"	
"		Maximum level	411.616	metre"	
"		Maximum storage	1094.876	c.m"	
"		Centroidal lag	3.520	hours"	
"		0.274	0.764	0.175	0.766 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.274	0.175	0.175	0.766"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.481	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.924	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.086	0.175	0.175	0.766 c.m/sec"	
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	10.695	1.467	9.072	minutes"
"		Time to Centroid	106.283	85.675	102.658	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	374.70	41.63	416.34	c.m"
"		Rainfall losses	44.994	6.561	41.150	mm"
"		Runoff depth	41.743	80.176	45.586	mm"
"		Runoff volume	180.33	38.48	218.81	c.m"
"		Runoff coefficient	0.481	0.924	0.526	"
"		Maximum flow	0.074	0.018	0.086	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.086	0.242	0.175	0.766"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.086	0.242	0.242	0.766"	
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.242		c.m/sec"	
"		Hydrograph volume	2097.427		c.m"	
"		0.086	0.242	0.242	0.242"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.086	0.000	0.242	0.242"	
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				



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"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.481  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.922  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.275   0.000   0.242   0.242 c.m/sec"
"      Catchment 2200      Pervious  Impervious Total Area "
"      Surface Area      0.230    0.690    0.920    hectare"
"      Time of concentration 16.211    2.224    4.298    minutes"
"      Time to Centroid    112.570   86.667   90.508   minutes"
"      Rainfall depth      86.737   86.737   86.737   mm"
"      Rainfall volume     199.49   598.48   797.98   c.m"
"      Rainfall losses     44.974    6.774   16.324   mm"
"      Runoff depth        41.763   79.963   70.413   mm"
"      Runoff volume       96.05   551.75   647.80   c.m"
"      Runoff coefficient   0.481    0.922    0.812    "
"      Maximum flow       0.035    0.262    0.275    c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.275   0.275   0.242   0.242"
" 54      POND DESIGN"
"          0.275  Current peak flow    c.m/sec"
"          0.756  Target outflow    c.m/sec"
"          647.8  Hydrograph volume    c.m"
"          12.    Number of stages"
"         413.700  Minimum water level    metre"
"         415.000  Maximum water level    metre"
"         413.700  Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"         413.700    0.000    0.000"
"         413.800    0.00500    88.600"
"         413.900    0.01000    187.200"
"         414.000    0.01300    298.400"
"         414.100    0.01500    422.200"
"         414.200    0.2590    558.900"
"         414.300    0.2910    708.500"
"         414.400    0.3210    871.100"
"         414.500    0.3470   1046.900"
"         414.600    0.3720   1236.100"
"         414.700    0.3950   1438.700"
"         415.000    2.828   2087.400"
"          Peak outflow      0.061    c.m/sec"
"          Maximum level    414.119    metre"

```

"		Maximum storage	447.957	c.m"	
"		Centroidal lag	6.667	hours"	
"		0.275 0.275 0.061 0.242		c.m/sec"	
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.267	c.m/sec"	
"		Hydrograph volume	2744.608	c.m"	
"		0.275 0.275 0.061		0.267"	
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.275 0.000 0.061		0.267"	
" 33		CATCHMENT 3100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3100	Catchment 3100"			
"	60.000	% Impervious"			
"	0.420	Total Area"			
"	40.000	Flow length"			
"	1.000	Overland Slope"			
"	0.168	Pervious Area"			
"	40.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.252	Impervious Area"			
"	40.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.482	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.917	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.102 0.000 0.061		0.267 c.m/sec"	
"		Catchment 3100	Pervious	Impervious	Total Area "
"		Surface Area	0.168	0.252	0.420 hectare"
"		Time of concentration	19.958	2.738	7.208 minutes"
"		Time to Centroid	116.779	87.402	95.027 minutes"
"		Rainfall depth	86.737	86.737	86.737 mm"
"		Rainfall volume	145.72	218.58	364.29 c.m"
"		Rainfall losses	44.916	7.204	22.289 mm"
"		Runoff depth	41.821	79.533	64.448 mm"
"		Runoff volume	70.26	200.42	270.68 c.m"
"		Runoff coefficient	0.482	0.917	0.743 "
"		Maximum flow	0.023	0.095	0.102 c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.102      0.102      0.061      0.267"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"          0  Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow      0.035      c.m/sec"
"          Volume of diverted flow      24.243      c.m"
"          DIV32001.050hyd"
"          Major flow at 32001"
"              0.102      0.102      0.067      0.267 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.102      0.067      0.067      0.267"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          246.4  Hydrograph volume      c.m"
"          15.  Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          410.620      0.000      0.000"
"          410.870      0.01300      4.855"
"          411.120      0.02000      14.351"
"          411.370      0.02500      24.074"
"          411.620      0.02900      33.921"
"          411.870      0.03300      43.768"
"          412.120      0.03600      53.614"
"          412.370      0.03900      63.461"
"          412.620      0.04200      73.308"
"          412.870      0.04400      74.155"
"          413.120      0.04700      75.003"
"          413.370      0.04900      75.850"
"          413.620      0.05200      76.698"
"          413.980      0.05500      77.918"
"          414.230      0.1600      78.483"
"          Peak outflow      0.041      c.m/sec"
"          Maximum level      412.539      metre"
"          Maximum storage      70.123      c.m"
"          Centroidal lag      1.956      hours"
"              0.102      0.067      0.041      0.267 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.102      0.041      0.041      0.267"
" 33      CATCHMENT 3200"

```

```

"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3200   Catchment 3200"
"    60.000   % Impervious"
"      0.130   Total Area"
"    20.000   Flow length"
"      1.000   Overland Slope"
"      0.052   Pervious Area"
"    20.000   Pervious length"
"      1.000   Pervious slope"
"      0.078   Impervious Area"
"    20.000   Impervious length"
"      1.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"    78.000   Pervious SCS Curve No."
"      0.481   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      7.164   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"    98.000   Impervious SCS Curve No."
"      0.925   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"
"          0.034   0.041   0.041   0.267 c.m/sec"
"      Catchment 3200      Pervious   Impervious Total Area "
"      Surface Area          0.052   0.078   0.130   hectare"
"      Time of concentration 13.168   1.807   4.730   minutes"
"      Time to Centroid     109.105  86.056  91.988  minutes"
"      Rainfall depth       86.737   86.737  86.737  mm"
"      Rainfall volume      45.10    67.65   112.76  c.m"
"      Rainfall losses      45.027   6.490   21.905  mm"
"      Runoff depth         41.710   80.247  64.832  mm"
"      Runoff volume        21.69    62.59   84.28   c.m"
"      Runoff coefficient    0.481    0.925   0.747   "
"      Maximum flow         0.008    0.029   0.034   c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4   Add Runoff "
"
"          0.034   0.064   0.041   0.267"
" 33      CATCHMENT 3300"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3300   Catchment 3300"
"    60.000   % Impervious"
"      0.240   Total Area"
"    20.000   Flow length"
"      2.000   Overland Slope"
"      0.096   Pervious Area"
"    20.000   Pervious length"

```

```

"      2.000  Pervious slope"
"      0.144  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.481  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.924  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.064      0.064      0.041      0.267 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration  10.695      1.467      3.845      minutes"
"      Time to Centroid      106.283      85.675      90.985      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      83.27      124.90      208.17      c.m"
"      Rainfall losses      44.994      6.561      21.934      mm"
"      Runoff depth      41.743      80.176      64.803      mm"
"      Runoff volume      40.07      115.45      155.53      c.m"
"      Runoff coefficient      0.481      0.924      0.747      "
"      Maximum flow      0.016      0.054      0.064      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.064      0.128      0.041      0.267"
" 47      FILEI_0 Read/Open DIV32001.050hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.050hyd"
"      Major flow at 32001"
"      Total volume      24.243      c.m"
"      Maximum flow      0.035      c.m/sec"
"          0.035      0.128      0.041      0.267 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.035      0.163      0.041      0.267"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.035      0.163      0.163      0.267"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"          To Walser Street"
"      Maximum flow      0.163      c.m/sec"
"      Hydrograph volume      510.569      c.m"

```

"		0.035	0.163	0.163	0.163"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.163		c.m/sec"	
"		Hydrograph volume	510.569		c.m"	
"		0.035	0.163	0.163	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.035	0.163	0.163	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.817		c.m/sec"	
"		Hydrograph volume	6510.743		c.m"	
"		0.035	0.163	0.163	0.817"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	0.817		c.m/sec"	
"		Hydrograph volume	6510.742		c.m"	
"		0.035	0.817	0.163	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.035	0.817	0.817	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.063		c.m/sec"	
"		Hydrograph volume	9255.348		c.m"	
"		0.035	0.817	0.817	1.063"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.063		c.m/sec"	
"		Hydrograph volume	9255.348		c.m"	
"		0.035	1.063	0.817	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Post_100yr.out"
"          Licensee name:                      gmbp"
"          Company                            gmbp"
"          Date & Time last used:              7/25/2022 at 11:49:23 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          6933.019 Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity          168.777  mm/hr"
"          Total depth                97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000 % Impervious"
"          0.220  Total Area"
"          10.000 Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000 Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.511  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.908  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"

```

	0.063	0.000	0.000	0.000	c.m/sec"
Catchment 1200		Pervious	Impervious	Total Area	"
Surface Area	0.110	0.110	0.220	hectare"	
Time of concentration	6.616	0.937	2.983	minutes"	
Time to Centroid	101.199	85.009	90.841	minutes"	
Rainfall depth	97.921	97.921	97.921	mm"	
Rainfall volume	107.71	107.71	215.43	c.m"	
Rainfall losses	47.838	8.977	28.408	mm"	
Runoff depth	50.084	88.944	69.514	mm"	
Runoff volume	55.09	97.84	152.93	c.m"	
Runoff coefficient	0.511	0.908	0.710	"	
Maximum flow	0.026	0.045	0.063	c.m/sec"	

" 40 HYDROGRAPH Add Runoff "

	0.063	0.063	0.000	0.000"
4 Add Runoff "				

" 33 CATCHMENT 1300"

1	Triangular SCS"
1	Equal length"
1	SCS method"
1300	Catchment 1300"
50.000	% Impervious"
0.700	Total Area"
20.000	Flow length"
2.000	Overland Slope"
0.350	Pervious Area"
20.000	Pervious length"
2.000	Pervious slope"
0.350	Impervious Area"
20.000	Impervious length"
2.000	Impervious slope"
0.250	Pervious Manning 'n'"
78.000	Pervious SCS Curve No."
0.515	Pervious Runoff coefficient"
0.100	Pervious Ia/S coefficient"
7.164	Pervious Initial abstraction"
0.015	Impervious Manning 'n'"
98.000	Impervious SCS Curve No."
0.931	Impervious Runoff coefficient"
0.100	Impervious Ia/S coefficient"
0.518	Impervious Initial abstraction"

	0.194	0.063	0.000	0.000	c.m/sec"
Catchment 1300		Pervious	Impervious	Total Area	"
Surface Area	0.350	0.350	0.700	hectare"	
Time of concentration	10.027	1.421	4.487	minutes"	
Time to Centroid	104.871	85.423	92.352	minutes"	
Rainfall depth	97.921	97.921	97.921	mm"	
Rainfall volume	342.72	342.72	685.45	c.m"	
Rainfall losses	47.483	6.787	27.135	mm"	
Runoff depth	50.438	91.134	70.786	mm"	
Runoff volume	176.53	318.97	495.50	c.m"	



"	Runoff coefficient	0.515	0.931	0.723	"
"	Maximum flow	0.074	0.144	0.194	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.194	0.257	0.000	0.000"	
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.516 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.924 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.062	0.257	0.000	0.000 c.m/sec"	
"	Catchment 1600	Pervious	Impervious	Total Area	"
"	Surface Area	0.110	0.110	0.220	hectare"
"	Time of concentration	8.438	1.196	3.791	minutes"
"	Time to Centroid	103.033	85.162	91.567	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	107.71	107.71	215.43	c.m"
"	Rainfall losses	47.375	7.430	27.402	mm"
"	Runoff depth	50.546	90.491	70.519	mm"
"	Runoff volume	55.60	99.54	155.14	c.m"
"	Runoff coefficient	0.516	0.924	0.720	"
"	Maximum flow	0.025	0.045	0.062	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.062	0.319	0.000	0.000"	
" 54	POND DESIGN"				
"	0.319 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	803.6 Hydrograph volume	c.m"			
"	17. Number of stages"				

```

" 412.000 Minimum water level metre"
" 414.490 Maximum water level metre"
" 412.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 412.000 0.00031 0.000"
" 412.100 0.00031 34.650"
" 412.200 0.00031 69.310"
" 412.300 0.00032 103.960"
" 412.400 0.00032 138.610"
" 412.500 0.00032 156.430"
" 412.600 0.00032 174.250"
" 412.700 0.00032 192.060"
" 412.900 0.00032 192.430"
" 413.230 0.00032 193.040"
" 413.430 0.04528 193.730"
" 413.630 0.1319 195.230"
" 413.830 0.2482 197.730"
" 414.030 0.3899 201.230"
" 414.090 0.4369 202.580"
" 414.290 0.6524 208.080"
" 414.490 0.9278 214.580"
" Peak outflow 0.316 c.m/sec"
" Maximum level 413.928 metre"
" Maximum storage 199.438 c.m"
" Centroidal lag 21.816 hours"
" 0.062 0.319 0.316 0.000 c.m/sec"
" 40 HYDROGRAPH Combine 1000"
" 6 Combine "
" 1000 Node #"
" Infiltrated On-Site"
" Maximum flow 0.316 c.m/sec"
" Hydrograph volume 794.278 c.m"
" 0.062 0.319 0.316 0.316"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.062 0.000 0.316 0.316"
" 33 CATCHMENT 1400"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 1400 Catchment 1400"
" 20.000 % Impervious"
" 0.620 Total Area"
" 30.000 Flow length"
" 2.000 Overland Slope"
" 0.496 Pervious Area"
" 30.000 Pervious length"
" 2.000 Pervious slope"
" 0.124 Impervious Area"

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```

"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.516  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.932  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.129      0.000      0.316      0.316 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration  12.789      1.812      9.373      minutes"
"      Time to Centroid      107.990      85.865      101.105      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      485.69      121.42      607.11      c.m"
"      Rainfall losses      47.421      6.662      39.270      mm"
"      Runoff depth      50.500      91.259      58.652      mm"
"      Runoff volume      250.48      113.16      363.64      c.m"
"      Runoff coefficient      0.516      0.932      0.599      "
"      Maximum flow      0.098      0.051      0.129      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.129      0.129      0.316      0.316"
" 54      POND DESIGN"
"      0.129      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      363.6      Hydrograph volume      c.m"
"      17.      Number of stages"
"     413.920      Minimum water level      metre"
"     415.520      Maximum water level      metre"
"     413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.920      0.00088      0.000"
"     414.020      0.00088      26.350"
"     414.120      0.00089      55.150"
"     414.220      0.00089      83.950"
"     414.320      0.00089      112.750"
"     414.420      0.00090      139.100"
"     414.520      0.00090      165.450"
"     414.620      0.00090      191.800"
"     414.720      0.00090      191.870"
"     414.820      0.02640      191.950"
"     414.920      0.03734      192.020"
"     415.020      0.04573      192.090"
"     415.120      0.05281      192.160"

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"		415.220	0.2777	201.450"		
"		415.320	0.6941	238.950"		
"		415.420	1.244	304.700"		
"		415.520	1.909	382.200"		
"		Peak outflow		0.090	c.m/sec"	
"		Maximum level		415.137	metre"	
"		Maximum storage		193.705	c.m"	
"		Centroidal lag		18.767	hours"	
"		0.129	0.129	0.090	0.316	c.m/sec"
" 40		HYDROGRAPH	Combine	1000"		
"	6	Combine	"			
"	1000	Node #"				
"		Infiltrated On-Site"				
"		Maximum flow		0.317	c.m/sec"	
"		Hydrograph volume		1164.892	c.m"	
"		0.129	0.129	0.090	0.317"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.129	0.000	0.090	0.317"	
" 33		CATCHMENT 1500"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1500	Catchment 1500"				
"	50.000	% Impervious"				
"	1.110	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.555	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.555	Impervious Area"				
"	40.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.517	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.929	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.277	0.000	0.090	0.317	c.m/sec"
"		Catchment 1500	Pervious	Impervious	Total Area	"
"		Surface Area	0.555	0.555	1.110	hectare"
"		Time of concentration	15.199	2.153	6.817	minutes"
"		Time to Centroid	110.688	86.345	95.048	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"

"	Rainfall volume	543.46	543.46	1086.93	c.m"
"	Rainfall losses	47.301	6.948	27.124	mm"
"	Runoff depth	50.621	90.973	70.797	mm"
"	Runoff volume	280.94	504.90	785.85	c.m"
"	Runoff coefficient	0.517	0.929	0.723	"
"	Maximum flow	0.105	0.230	0.277	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.277	0.277	0.090	0.317"
" 56	DIVERSION"				
"	1500 Node number"				
"	0.146 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.131			c.m/sec"
"	Volume of diverted flow	145.528			c.m"
"	DIV01500.100hyd"				
"	Major flow at 1500"				
"		0.277	0.277	0.146	0.317 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.277	0.146	0.146	0.317"
" 33	CATCHMENT 1000"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1000 Catchment 1000"				
"	50.000 % Impervious"				
"	6.980 Total Area"				
"	100.000 Flow length"				
"	2.000 Overland Slope"				
"	3.490 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	3.490 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.517 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.926 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		1.610	0.146	0.146	0.317 c.m/sec"
"	Catchment 1000	Pervious	Impervious	Total Area	"
"	Surface Area	3.490	3.490	6.980	hectare"

"	Time of concentration	26.337	3.732	11.833	minutes"
"	Time to Centroid	123.197	88.516	100.945	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	3417.45	3417.45	6834.91	c.m"
"	Rainfall losses	47.249	7.203	27.226	mm"
"	Runoff depth	50.672	90.719	70.695	mm"
"	Runoff volume	1768.46	3166.08	4934.54	c.m"
"	Runoff coefficient	0.517	0.926	0.722	"
"	Maximum flow	0.497	1.489	1.610	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	1.610 1.756 0.146 0.317"				
" 33	CATCHMENT 1100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1100 Catchment 1100"				
"	0.000 % Impervious"				
"	0.480 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.480 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.515 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.101 1.756 0.146 0.317 c.m/sec"				
"	Catchment 1100 Pervious Impervious Total Area "				
"	Surface Area 0.480 0.000 0.480 hectare"				
"	Time of concentration 10.027 1.421 10.027 minutes"				
"	Time to Centroid 104.871 85.423 104.871 minutes"				
"	Rainfall depth 97.921 97.921 97.921 mm"				
"	Rainfall volume 470.02 0.00 470.02 c.m"				
"	Rainfall losses 47.483 6.787 47.483 mm"				
"	Runoff depth 50.438 91.134 50.438 mm"				
"	Runoff volume 242.10 0.00 242.10 c.m"				
"	Runoff coefficient 0.515 0.000 0.515 "				
"	Maximum flow 0.101 0.000 0.101 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				

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"          4  Add Runoff "
"              0.101      1.830      0.146      0.317"
" 54      POND DESIGN"
"          1.830  Current peak flow    c.m/sec"
"          0.250  Target outflow    c.m/sec"
"          5817.0  Hydrograph volume    c.m"
"          12.    Number of stages"
"          411.000  Minimum water level    metre"
"          412.000  Maximum water level    metre"
"          411.000  Starting water level    metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"              Level Discharge    Volume"
"          411.000    0.000    0.000"
"          411.100    0.03200    18.400"
"          411.200    0.06500    95.500"
"          411.300    0.07700    239.900"
"          411.400    0.08700    435.700"
"          411.500    0.09600    691.300"
"          411.600    0.1050    1027.600"
"          411.700    0.3150    1450.000"
"          411.800    0.3380    1935.800"
"          411.850    0.3490    2195.700"
"          411.900    0.6670    2465.600"
"          412.000    2.018    3030.100"
"          Peak outflow                0.970    c.m/sec"
"          Maximum level                411.923    metre"
"          Maximum storage                2593.056    c.m"
"          Centroidal lag                2.996    hours"
"          0.101      1.830      0.970      0.317 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.101      0.970      0.970      0.317"
" 47      FILEI_0 Read/Open DIV01500.100hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV01500.100hyd"
"          Major flow at 1500"
"          Total volume                145.528    c.m"
"          Maximum flow                0.131    c.m/sec"
"          0.131      0.970      0.970      0.317 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.131      0.970      0.970      0.317"
" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000  Catchment 4000"
"          0.000  % Impervious"

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"      7.330  Total Area"
"     60.000  Flow length"
"      2.000  Overland Slope"
"      7.330  Pervious Area"
"     60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     50.000  Pervious SCS Curve No."
"      0.164  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"     25.400  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.268    0.970    0.970    0.317 c.m/sec"
"      Catchment 4000      Pervious  Impervious Total Area "
"      Surface Area      7.330    0.000    7.330    hectare"
"      Time of concentration  32.980    2.747    32.980    minutes"
"      Time to Centroid      137.344    87.189    137.344    minutes"
"      Rainfall depth      97.921    97.921    97.921    mm"
"      Rainfall volume      7177.62    0.01    7177.63    c.m"
"      Rainfall losses      81.821    7.496    81.821    mm"
"      Runoff depth      16.100    90.426    16.100    mm"
"      Runoff volume      1180.13    0.01    1180.14    c.m"
"      Runoff coefficient    0.164    0.000    0.164    "
"      Maximum flow      0.268    0.000    0.268    c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.268    1.235    0.970    0.317"
" 54      POND DESIGN"
"      1.235  Current peak flow    c.m/sec"
"      0.250  Target outflow    c.m/sec"
"     7146.6  Hydrograph volume    c.m"
"      6.     Number of stages"
"     409.630  Minimum water level    metre"
"     410.750  Maximum water level    metre"
"     409.630  Starting water level    metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"     409.630    0.000    0.000"
"     409.750    0.6650    402.200"
"     410.000    3.601    2187.900"
"     410.250    7.811    5318.900"
"     410.500   12.984    9642.300"
"     410.750   18.965   15227.70"

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"           Peak outflow                1.083    c.m/sec"
"           Maximum level                409.786  metre"
"           Maximum storage              656.386  c.m"
"           Centroidal lag               3.013   hours"
"           0.268    1.235    1.083    0.317 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5 Next link "
"           0.268    1.083    1.083    0.317"
" 52      CHANNEL DESIGN"
"           1.083 Current peak flow    c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           0.000 Basewidth    metre"
"           7.410 Left bank slope"
"           6.000 Right bank slope"
"           0.950 Channel depth    metre"
"           1.040 Gradient    %"
"           Depth of flow                0.403    metre"
"           Velocity                    0.994    m/sec"
"           Channel capacity             10.655  c.m/sec"
"           Critical depth               0.351    metre"
" 53      ROUTE Channel Route 72"
"           72.40 Channel Route 72 Reach length ( metre)"
"           0.400 X-factor <= 0.5"
"           54.618 K-lag ( seconds)"
"           0.000 Default(0) or user spec.(1) values used"
"           0.500 X-factor <= 0.5"
"           30.000 K-lag ( seconds)"
"           0.500 Beta weighting factor"
"           60.000 Routing time step ( seconds)"
"           1 No. of sub-reaches"
"           Peak outflow                1.078    c.m/sec"
"           0.268    1.083    1.078    0.317 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5 Next link "
"           0.268    1.078    1.078    0.317"
" 52      CHANNEL DESIGN"
"           1.078 Current peak flow    c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           2.000 Basewidth    metre"
"           2.950 Left bank slope"
"           3.000 Right bank slope"
"           0.950 Channel depth    metre"
"           1.040 Gradient    %"
"           Depth of flow                0.325    metre"
"           Velocity                    1.120    m/sec"
"           Channel capacity             9.246    c.m/sec"
"           Critical depth               0.269    metre"
" 53      ROUTE Channel Route 40"

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"      39.80      Channel Route 40 Reach length  ( metre)"
"      0.291      X-factor <= 0.5"
"      26.647     K-lag  ( seconds)"
"      0.000     Default(0) or user spec.(1) values used"
"      0.500     X-factor <= 0.5"
"      30.000     K-lag  ( seconds)"
"      0.500     Beta weighting factor"
"      37.500     Routing time step  ( seconds)"
"          1     No. of sub-reaches"
"          Peak outflow                1.074    c.m/sec"
"          0.268    1.078    1.074    0.317 c.m/sec"
" 40      HYDROGRAPH  Combine    100"
"          6     Combine  "
"          100    Node #"
"          Existing Wetland"
"          Maximum flow                1.074    c.m/sec"
"          Hydrograph volume            7146.616    c.m"
"          0.268    1.078    1.074    1.074"
" 40      HYDROGRAPH Start - New Tributary"
"          2     Start - New Tributary"
"          0.268    0.000    1.074    1.074"
" 33      CATCHMENT 2100"
"          1     Triangular SCS"
"          1     Equal length"
"          1     SCS method"
"          2100  Catchment 2100"
"          60.000 % Impervious"
"          1.960  Total Area"
"          40.000 Flow length"
"          2.000  Overland Slope"
"          0.784  Pervious Area"
"          40.000 Pervious length"
"          2.000  Pervious slope"
"          1.176  Impervious Area"
"          40.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.517  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.929  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.554    0.000    1.074    1.074 c.m/sec"
"          Catchment 2100      Pervious  Impervious Total Area  "
"          Surface Area        0.784    1.176    1.960    hectare"
"          Time of concentration 15.199    2.153    5.683    minutes"

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"	Time to Centroid	110.688	86.345	92.932	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	767.70	1151.55	1919.26	c.m"
"	Rainfall losses	47.301	6.948	23.089	mm"
"	Runoff depth	50.621	90.973	74.832	mm"
"	Runoff volume	396.87	1069.84	1466.71	c.m"
"	Runoff coefficient	0.517	0.929	0.764	"
"	Maximum flow	0.148	0.488	0.554	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.554 0.554 1.074 1.074"				
" 33	CATCHMENT 2400"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	2400 Catchment 2400"				
"	90.000 % Impervious"				
"	0.800 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.080 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.720 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.515 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.931 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.301 0.554 1.074 1.074 c.m/sec"				
"	Catchment 2400 Pervious Impervious Total Area "				
"	Surface Area 0.080 0.720 0.800 hectare"				
"	Time of concentration 10.027 1.421 1.919 minutes"				
"	Time to Centroid 104.871 85.423 86.549 minutes"				
"	Rainfall depth 97.921 97.921 97.921 mm"				
"	Rainfall volume 78.34 705.03 783.37 c.m"				
"	Rainfall losses 47.483 6.787 10.857 mm"				
"	Runoff depth 50.438 91.134 87.065 mm"				
"	Runoff volume 40.35 656.17 696.52 c.m"				
"	Runoff coefficient 0.515 0.931 0.889 "				
"	Maximum flow 0.017 0.296 0.301 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

"		0.301	0.854	1.074	1.074"
" 54	POND DESIGN"				
"	0.854	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	2163.2	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"		Peak outflow	0.306	c.m/sec"	
"		Maximum level	411.690	metre"	
"		Maximum storage	1201.847	c.m"	
"		Centroidal lag	3.398	hours"	
"		0.301	0.854	0.306	1.074 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5	Next link "			
"		0.301	0.306	0.306	1.074"
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.048	Impervious Area"			
"	20.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			

"	0.515	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.931	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.105	0.306	0.306	1.074 c.m/sec"	
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	10.027	1.421	8.588	minutes"
"		Time to Centroid	104.871	85.423	101.619	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	423.02	47.00	470.02	c.m"
"		Rainfall losses	47.483	6.787	43.413	mm"
"		Runoff depth	50.438	91.134	54.508	mm"
"		Runoff volume	217.89	43.74	261.64	c.m"
"		Runoff coefficient	0.515	0.931	0.557	"
"		Maximum flow	0.091	0.020	0.105	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.105	0.355	0.306	1.074"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.105	0.355	0.355	1.074"	
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	0.355		c.m/sec"	
"		Hydrograph volume	2419.666		c.m"	
"		0.105	0.355	0.355	0.355"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.105	0.000	0.355	0.355"	
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				
"	2.000	Overland Slope"				
"	0.230	Pervious Area"				
"	40.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.690	Impervious Area"				
"	40.000	Impervious length"				

```

"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.517  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.929  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.306      0.000      0.355      0.355 c.m/sec"
"      Catchment 2200      Pervious      Impervious      Total Area  "
"      Surface Area      0.230      0.690      0.920      hectare"
"      Time of concentration  15.199      2.153      4.194      minutes"
"      Time to Centroid      110.688      86.345      90.153      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      225.22      675.66      900.88      c.m"
"      Rainfall losses      47.301      6.948      17.036      mm"
"      Runoff depth      50.621      90.973      80.885      mm"
"      Runoff volume      116.43      627.71      744.14      c.m"
"      Runoff coefficient      0.517      0.929      0.826      "
"      Maximum flow      0.043      0.286      0.306      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.306      0.306      0.355      0.355"
" 54      POND DESIGN"
"      0.306      Current peak flow      c.m/sec"
"      0.756      Target outflow      c.m/sec"
"      744.1      Hydrograph volume      c.m"
"      12.      Number of stages"
"     413.700      Minimum water level      metre"
"     415.000      Maximum water level      metre"
"     413.700      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.700      0.000      0.000"
"     413.800      0.00500      88.600"
"     413.900      0.01000      187.200"
"     414.000      0.01300      298.400"
"     414.100      0.01500      422.200"
"     414.200      0.2590      558.900"
"     414.300      0.2910      708.500"
"     414.400      0.3210      871.100"
"     414.500      0.3470      1046.900"
"     414.600      0.3720      1236.100"
"     414.700      0.3950      1438.700"
"     415.000      2.828      2087.400"
"      Peak outflow      0.099      c.m/sec"
"      Maximum level      414.134      metre"

```

"		Maximum storage	469.233	c.m"	
"		Centroidal lag	6.040	hours"	
"		0.306 0.306 0.099 0.355		c.m/sec"	
" 40		HYDROGRAPH Combine 200"			
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	0.454	c.m/sec"	
"		Hydrograph volume	3165.424	c.m"	
"		0.306 0.306 0.099 0.454"			
" 40		HYDROGRAPH Start - New Tributary"			
"	2	Start - New Tributary"			
"		0.306 0.000 0.099 0.454"			
" 33		CATCHMENT 3100"			
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	3100	Catchment 3100"			
"	60.000	% Impervious"			
"	0.420	Total Area"			
"	40.000	Flow length"			
"	1.000	Overland Slope"			
"	0.168	Pervious Area"			
"	40.000	Pervious length"			
"	1.000	Pervious slope"			
"	0.252	Impervious Area"			
"	40.000	Impervious length"			
"	1.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	78.000	Pervious SCS Curve No."			
"	0.517	Pervious Runoff coefficient"			
"	0.100	Pervious Ia/S coefficient"			
"	7.164	Pervious Initial abstraction"			
"	0.015	Impervious Manning 'n'"			
"	98.000	Impervious SCS Curve No."			
"	0.924	Impervious Runoff coefficient"			
"	0.100	Impervious Ia/S coefficient"			
"	0.518	Impervious Initial abstraction"			
"		0.114 0.000 0.099 0.454		c.m/sec"	
"		Catchment 3100 Pervious Impervious Total Area "			
"		Surface Area 0.168 0.252 0.420		hectare"	
"		Time of concentration 18.712 2.651 7.014		minutes"	
"		Time to Centroid 114.625 87.045 94.538		minutes"	
"		Rainfall depth 97.921 97.921 97.921		mm"	
"		Rainfall volume 164.51 246.76 411.27		c.m"	
"		Rainfall losses 47.295 7.442 23.384		mm"	
"		Runoff depth 50.626 90.479 74.538		mm"	
"		Runoff volume 85.05 228.01 313.06		c.m"	
"		Runoff coefficient 0.517 0.924 0.761		"	
"		Maximum flow 0.029 0.104 0.114		c.m/sec"	

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.114      0.114      0.099      0.454"
" 56      DIVERSION"
"          32001  Node number"
"          0.067  Overflow threshold"
"          1.000  Required diverted fraction"
"          0      Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow      0.047      c.m/sec"
"          Volume of diverted flow      39.992      c.m"
"          DIV32001.100hyd"
"          Major flow at 32001"
"              0.114      0.114      0.067      0.454 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.114      0.067      0.067      0.454"
" 54      POND DESIGN"
"          0.067  Current peak flow      c.m/sec"
"          0.756  Target outflow      c.m/sec"
"          273.1  Hydrograph volume      c.m"
"          15.    Number of stages"
"          410.620  Minimum water level      metre"
"          414.230  Maximum water level      metre"
"          410.620  Starting water level      metre"
"          0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          410.620      0.000      0.000"
"          410.870      0.01300      4.855"
"          411.120      0.02000      14.351"
"          411.370      0.02500      24.074"
"          411.620      0.02900      33.921"
"          411.870      0.03300      43.768"
"          412.120      0.03600      53.614"
"          412.370      0.03900      63.461"
"          412.620      0.04200      73.308"
"          412.870      0.04400      74.155"
"          413.120      0.04700      75.003"
"          413.370      0.04900      75.850"
"          413.620      0.05200      76.698"
"          413.980      0.05500      77.918"
"          414.230      0.1600      78.483"
"          Peak outflow      0.050      c.m/sec"
"          Maximum level      413.465      metre"
"          Maximum storage      76.172      c.m"
"          Centroidal lag      1.972      hours"
"              0.114      0.067      0.050      0.454 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.114      0.050      0.050      0.454"
" 33      CATCHMENT 3200"

```



```

"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3200   Catchment 3200"
"    60.000   % Impervious"
"      0.130   Total Area"
"    20.000   Flow length"
"      1.000   Overland Slope"
"      0.052   Pervious Area"
"    20.000   Pervious length"
"      1.000   Pervious slope"
"      0.078   Impervious Area"
"    20.000   Impervious length"
"      1.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"    78.000   Pervious SCS Curve No."
"      0.516   Pervious Runoff coefficient"
"      0.100   Pervious Ia/S coefficient"
"      7.164   Pervious Initial abstraction"
"      0.015   Impervious Manning 'n'"
"    98.000   Impervious SCS Curve No."
"      0.932   Impervious Runoff coefficient"
"      0.100   Impervious Ia/S coefficient"
"      0.518   Impervious Initial abstraction"
"
"          0.038   0.050   0.050   0.454 c.m/sec"
"      Catchment 3200      Pervious   Impervious Total Area "
"      Surface Area      0.052      0.078      0.130      hectare"
"      Time of concentration 12.345      1.749      4.607      minutes"
"      Time to Centroid    107.474     85.796     91.642     minutes"
"      Rainfall depth      97.921     97.921     97.921     mm"
"      Rainfall volume      50.92      76.38      127.30     c.m"
"      Rainfall losses      47.364     6.648      22.934     mm"
"      Runoff depth        50.557     91.273     74.987     mm"
"      Runoff volume        26.29      71.19      97.48      c.m"
"      Runoff coefficient    0.516      0.932      0.766      "
"      Maximum flow        0.010      0.032      0.038      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4   Add Runoff "
"
"          0.038   0.070   0.050   0.454"
" 33      CATCHMENT 3300"
"          1   Triangular SCS"
"          1   Equal length"
"          1   SCS method"
"      3300   Catchment 3300"
"    60.000   % Impervious"
"      0.240   Total Area"
"    20.000   Flow length"
"      2.000   Overland Slope"
"      0.096   Pervious Area"
"    20.000   Pervious length"

```

```

"      2.000 Pervious slope"
"      0.144 Impervious Area"
"     20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     78.000 Pervious SCS Curve No."
"      0.515 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"     98.000 Impervious SCS Curve No."
"      0.931 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"              0.072      0.070      0.050      0.454 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration 10.027      1.421      3.740      minutes"
"      Time to Centroid      104.871      85.423      90.664      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      94.00      141.01      235.01      c.m"
"      Rainfall losses      47.483      6.787      23.065      mm"
"      Runoff depth      50.438      91.134      74.856      mm"
"      Runoff volume      48.42      131.23      179.65      c.m"
"      Runoff coefficient      0.515      0.931      0.764      "
"      Maximum flow      0.020      0.059      0.072      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.072      0.142      0.050      0.454"
" 47      FILEI_0 Read/Open DIV32001.100hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.100hyd"
"      Major flow at 32001"
"      Total volume      39.992      c.m"
"      Maximum flow      0.047      c.m/sec"
"              0.047      0.142      0.050      0.454 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.047      0.190      0.050      0.454"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"              0.047      0.190      0.190      0.454"
" 40      HYDROGRAPH Combine 300"
"      6      Combine "
"      300      Node #"
"      To Walser Street"
"      Maximum flow      0.190      c.m/sec"
"      Hydrograph volume      590.014      c.m"

```

"		0.047	0.190	0.190	0.190"	
" 40	HYDROGRAPH	Confluence	300"			
"	7	Confluence "				
"	300	Node #"				
"		To Walser Street"				
"		Maximum flow	0.190		c.m/sec"	
"		Hydrograph volume	590.014		c.m"	
"		0.047	0.190	0.190	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.047	0.190	0.190	0.000"	
" 40	HYDROGRAPH	Combine	100"			
"	6	Combine "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	1.138		c.m/sec"	
"		Hydrograph volume	7736.632		c.m"	
"		0.047	0.190	0.190	1.138"	
" 40	HYDROGRAPH	Confluence	100"			
"	7	Confluence "				
"	100	Node #"				
"		Existing Wetland"				
"		Maximum flow	1.138		c.m/sec"	
"		Hydrograph volume	7736.631		c.m"	
"		0.047	1.138	0.190	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.047	1.138	1.138	0.000"	
" 40	HYDROGRAPH	Combine	200"			
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.475		c.m/sec"	
"		Hydrograph volume	10902.038		c.m"	
"		0.047	1.138	1.138	1.475"	
" 40	HYDROGRAPH	Confluence	200"			
"	7	Confluence "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow	1.475		c.m/sec"	
"		Hydrograph volume	10902.038		c.m"	
"		0.047	1.475	1.138	0.000"	
" 38	START/RE-START TOTALS	200"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area		22.610	hectare"	
"		Total Impervious area		7.847	hectare"	
"		Total % impervious		34.706"		
" 19	EXIT"					

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         W:\Kitchener\411-2011\411009\Design Data\
"                                               Modelling Files\2022-07-25"
"          Output filename:                    Post__REG.out"
"          Licensee name:                      gmbp"
"          Company                             gmbp"
"          Date & Time last used:              7/25/2022 at 12:28:09 PM"
" 31      TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000  Max. Storm length"
"          12000.000  Max. Hydrograph"
" 32      STORM Historic"
"          5  Historic"
"          2880.000  Duration"
"          48.000  Rainfall intensity values"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.026    2.026    2.026    2.028"
"                   2.026    6.000    4.000    6.000    13.000"
"                   17.000    13.000    23.000    13.000    13.000"
"                   53.000    38.000    13.000"
"          Maximum intensity                    53.000  mm/hr"
"          Total depth                          285.000  mm"
"          6  000hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 1200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          1200  Catchment 1200"
"          50.000  % Impervious"
"          0.220  Total Area"
"          10.000  Flow length"
"          2.000  Overland Slope"
"          0.110  Pervious Area"
"          10.000  Pervious length"
"          2.000  Pervious slope"
"          0.110  Impervious Area"
"          10.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000  Pervious SCS Curve No."
"          0.719  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"

```

"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.027	0.000	0.000	0.000	c.m/sec"
"		Catchment 1200	Pervious	Impervious	Total Area	"
"		Surface Area	0.110	0.110	0.220	hectare"
"		Time of concentration	8.237	1.480	4.583	minutes"
"		Time to Centroid	2489.666	2307.003	2390.883	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	313.50	313.50	627.00	c.m"
"		Rainfall losses	80.153	43.756	61.955	mm"
"		Runoff depth	204.847	241.244	223.045	mm"
"		Runoff volume	225.33	265.37	490.70	c.m"
"		Runoff coefficient	0.719	0.846	0.783	"
"		Maximum flow	0.013	0.014	0.027	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.027	0.027	0.000	0.000"	
" 33		CATCHMENT 1300"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1300	Catchment 1300"				
"	50.000	% Impervious"				
"	0.700	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.350	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.350	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.085	0.027	0.000	0.000	c.m/sec"
"		Catchment 1300	Pervious	Impervious	Total Area	"
"		Surface Area	0.350	0.350	0.700	hectare"
"		Time of concentration	12.485	2.243	7.071	minutes"

"	Time to Centroid	2505.277	2290.972	2391.991	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	997.50	997.50	1995.00	c.m"
"	Rainfall losses	70.073	43.972	57.023	mm"
"	Runoff depth	214.927	241.028	227.977	mm"
"	Runoff volume	752.24	843.60	1595.84	c.m"
"	Runoff coefficient	0.754	0.846	0.800	"
"	Maximum flow	0.040	0.045	0.085	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.085 0.112 0.000 0.000"				
" 33	CATCHMENT 1600"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	1600 Catchment 1600"				
"	50.000 % Impervious"				
"	0.220 Total Area"				
"	15.000 Flow length"				
"	2.000 Overland Slope"				
"	0.110 Pervious Area"				
"	15.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.110 Impervious Area"				
"	15.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.739 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.844 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.027 0.112 0.000 0.000 c.m/sec"				
"	Catchment 1600 Pervious Impervious Total Area "				
"	Surface Area 0.110 0.110 0.220 hectare"				
"	Time of concentration 10.506 1.887 5.911 minutes"				
"	Time to Centroid 2500.832 2301.045 2394.331 minutes"				
"	Rainfall depth 285.000 285.000 285.000 mm"				
"	Rainfall volume 313.50 313.50 627.00 c.m"				
"	Rainfall losses 74.380 44.547 59.463 mm"				
"	Runoff depth 210.620 240.453 225.537 mm"				
"	Runoff volume 231.68 264.50 496.18 c.m"				
"	Runoff coefficient 0.739 0.844 0.791 "				
"	Maximum flow 0.013 0.014 0.027 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				

```

"          0.027    0.139    0.000    0.000"
" 54      POND DESIGN"
"      0.139    Current peak flow    c.m/sec"
"      0.250    Target outflow    c.m/sec"
"      2582.7    Hydrograph volume    c.m"
"      17.      Number of stages"
"      412.000    Minimum water level    metre"
"      414.490    Maximum water level    metre"
"      412.000    Starting water level    metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"      412.000    0.00031    0.000"
"      412.100    0.00031    34.650"
"      412.200    0.00031    69.310"
"      412.300    0.00032    103.960"
"      412.400    0.00032    138.610"
"      412.500    0.00032    156.430"
"      412.600    0.00032    174.250"
"      412.700    0.00032    192.060"
"      412.900    0.00032    192.430"
"      413.230    0.00032    193.040"
"      413.430    0.04528    193.730"
"      413.630    0.1319    195.230"
"      413.830    0.2482    197.730"
"      414.030    0.3899    201.230"
"      414.090    0.4369    202.580"
"      414.290    0.6524    208.080"
"      414.490    0.9278    214.580"
"      Peak outflow          0.138    c.m/sec"
"      Maximum level          413.642    metre"
"      Maximum storage          195.375    c.m"
"      Centroidal lag          49.356    hours"
"          0.027    0.139    0.138    0.000 c.m/sec"
" 40      HYDROGRAPH Combine    1000"
"      6      Combine "
"      1000    Node #"
"          Infiltrated On-Site"
"      Maximum flow          0.138    c.m/sec"
"      Hydrograph volume          2556.484    c.m"
"          0.027    0.139    0.138    0.138"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"          0.027    0.000    0.138    0.138"
" 33      CATCHMENT 1400"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      1400    Catchment 1400"
"      20.000    % Impervious"
"      0.620    Total Area"

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"      30.000  Flow length"
"      2.000  Overland Slope"
"      0.496  Pervious Area"
"      30.000  Pervious length"
"      2.000  Pervious slope"
"      0.124  Impervious Area"
"      30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.764  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.849  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.071      0.000      0.138      0.138 c.m/sec"
"      Catchment 1400      Pervious      Impervious      Total Area  "
"      Surface Area      0.496      0.124      0.620      hectare"
"      Time of concentration  15.924      2.860      13.084      minutes"
"      Time to Centroid      2515.051      2276.456      2463.191      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1413.60      353.40      1767.00      c.m"
"      Rainfall losses      67.258      43.116      62.430      mm"
"      Runoff depth      217.742      241.884      222.570      mm"
"      Runoff volume      1080.00      299.94      1379.93      c.m"
"      Runoff coefficient      0.764      0.849      0.781      "
"      Maximum flow      0.055      0.016      0.071      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"          0.071      0.071      0.138      0.138"
" 54      POND DESIGN"
"      0.071      Current peak flow      c.m/sec"
"      0.250      Target outflow      c.m/sec"
"      1379.9      Hydrograph volume      c.m"
"      17.      Number of stages"
"      413.920      Minimum water level      metre"
"      415.520      Maximum water level      metre"
"      413.920      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      413.920      0.00088      0.000"
"      414.020      0.00088      26.350"
"      414.120      0.00089      55.150"
"      414.220      0.00089      83.950"
"      414.320      0.00089      112.750"
"      414.420      0.00090      139.100"
"      414.520      0.00090      165.450"

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"		414.620	0.00090	191.800"	
"		414.720	0.00090	191.870"	
"		414.820	0.02640	191.950"	
"		414.920	0.03734	192.020"	
"		415.020	0.04573	192.090"	
"		415.120	0.05281	192.160"	
"		415.220	0.2777	201.450"	
"		415.320	0.6941	238.950"	
"		415.420	1.244	304.700"	
"		415.520	1.909	382.200"	
"		Peak outflow	0.070	c.m/sec"	
"		Maximum level	415.128	metre"	
"		Maximum storage	192.887	c.m"	
"		Centroidal lag	48.418	hours"	
"		0.071	0.071	0.070	0.138 c.m/sec"
"	40	HYDROGRAPH	Combine	1000"	
"		6	Combine	"	
"		1000	Node #"		
"			Infiltrated On-Site"		
"		Maximum flow	0.208	c.m/sec"	
"		Hydrograph volume	3933.528	c.m"	
"		0.071	0.071	0.070	0.208"
"	40	HYDROGRAPH	Start - New Tributary"		
"		2	Start - New Tributary"		
"		0.071	0.000	0.070	0.208"
"	33	CATCHMENT	1500"		
"		1	Triangular SCS"		
"		1	Equal length"		
"		1	SCS method"		
"		1500	Catchment 1500"		
"		50.000	% Impervious"		
"		1.110	Total Area"		
"		40.000	Flow length"		
"		2.000	Overland Slope"		
"		0.555	Pervious Area"		
"		40.000	Pervious length"		
"		2.000	Pervious slope"		
"		0.555	Impervious Area"		
"		40.000	Impervious length"		
"		2.000	Impervious slope"		
"		0.250	Pervious Manning 'n'"		
"		78.000	Pervious SCS Curve No."		
"		0.765	Pervious Runoff coefficient"		
"		0.100	Pervious Ia/S coefficient"		
"		7.164	Pervious Initial abstraction"		
"		0.015	Impervious Manning 'n'"		
"		98.000	Impervious SCS Curve No."		
"		0.850	Impervious Runoff coefficient"		
"		0.100	Impervious Ia/S coefficient"		
"		0.518	Impervious Initial abstraction"		

"		0.129	0.000	0.070	0.208	c.m/sec"
"	Catchment 1500		Pervious	Impervious	Total Area	"
"	Surface Area	0.555	0.555	1.110	hectare"	
"	Time of concentration	18.924	3.399	10.752	minutes"	
"	Time to Centroid	2520.774	2266.333	2386.847	minutes"	
"	Rainfall depth	285.000	285.000	285.000	mm"	
"	Rainfall volume	1581.75	1581.75	3163.50	c.m"	
"	Rainfall losses	66.918	42.646	54.782	mm"	
"	Runoff depth	218.082	242.354	230.218	mm"	
"	Runoff volume	1210.36	1345.07	2555.42	c.m"	
"	Runoff coefficient	0.765	0.850	0.808	"	
"	Maximum flow	0.060	0.070	0.129	c.m/sec"	
" 40	HYDROGRAPH Add Runoff "					
"	4	Add Runoff "				
"		0.129	0.129	0.070	0.208"	
" 56	DIVERSION"					
"	1500	Node number"				
"	0.146	Overflow threshold"				
"	1.000	Required diverted fraction"				
"	0	Conduit type; 1=Pipe;2=Channel"				
"			Peak of diverted flow	0.000	c.m/sec"	
"			Volume of diverted flow	0.000	c.m"	
"		DIV01500.000hyd"				
"		Major flow at 1500"				
"		0.129	0.129	0.129	0.208	c.m/sec"
" 40	HYDROGRAPH Next link "					
"	5	Next link "				
"		0.129	0.129	0.129	0.208"	
" 33	CATCHMENT 1000"					
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1000	Catchment 1000"				
"	50.000	% Impervious"				
"	6.980	Total Area"				
"	100.000	Flow length"				
"	2.000	Overland Slope"				
"	3.490	Pervious Area"				
"	100.000	Pervious length"				
"	2.000	Pervious slope"				
"	3.490	Impervious Area"				
"	100.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.764	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				

"	0.884	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.768	0.129	0.129	0.208	c.m/sec"
"		Catchment 1000	Pervious	Impervious	Total Area	"
"		Surface Area	3.490	3.490	6.980	hectare"
"		Time of concentration	32.793	5.891	18.359	minutes"
"		Time to Centroid	2537.302	2260.661	2388.880	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	0.9947	0.9947	1.9893	ha-m"
"		Rainfall losses	67.335	33.040	50.188	mm"
"		Runoff depth	217.665	251.960	234.812	mm"
"		Runoff volume	0.7597	0.8793	1.6390	ha-m"
"		Runoff coefficient	0.764	0.884	0.824	"
"		Maximum flow	0.407	0.441	0.768	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.768	0.898	0.129	0.208"	
" 33		CATCHMENT 1100"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	1100	Catchment 1100"				
"	0.000	% Impervious"				
"	0.480	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.480	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.055	0.898	0.129	0.208	c.m/sec"
"		Catchment 1100	Pervious	Impervious	Total Area	"
"		Surface Area	0.480	0.000	0.480	hectare"
"		Time of concentration	12.485	2.243	12.485	minutes"
"		Time to Centroid	2505.277	2290.972	2505.276	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	1368.00	0.00	1368.00	c.m"

"	Rainfall losses	70.073	43.972	70.073	mm"
"	Runoff depth	214.927	241.028	214.927	mm"
"	Runoff volume	1031.65	0.00	1031.65	c.m"
"	Runoff coefficient	0.754	0.000	0.754	"
"	Maximum flow	0.055	0.000	0.055	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.055 0.952 0.129 0.208"				
" 54	POND DESIGN"				
"	0.952 Current peak flow	c.m/sec"			
"	0.250 Target outflow	c.m/sec"			
"	19977.0 Hydrograph volume	c.m"			
"	12. Number of stages"				
"	411.000 Minimum water level	metre"			
"	412.000 Maximum water level	metre"			
"	411.000 Starting water level	metre"			
"	0 Keep Design Data: 1 = True; 0 = False"				
"	Level Discharge Volume"				
"	411.000 0.000 0.000"				
"	411.100 0.03200 18.400"				
"	411.200 0.06500 95.500"				
"	411.300 0.07700 239.900"				
"	411.400 0.08700 435.700"				
"	411.500 0.09600 691.300"				
"	411.600 0.1050 1027.600"				
"	411.700 0.3150 1450.000"				
"	411.800 0.3380 1935.800"				
"	411.850 0.3490 2195.700"				
"	411.900 0.6670 2465.600"				
"	412.000 2.018 3030.100"				
"	Peak outflow	0.875	c.m/sec"		
"	Maximum level	411.919	metre"		
"	Maximum storage	2573.825	c.m"		
"	Centroidal lag	40.971	hours"		
"	0.055 0.952 0.875 0.208 c.m/sec"				
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.055 0.875 0.875 0.208"				
" 47	FILEI_0 Read/Open DIV01500.000hyd"				
"	1 1=read/open; 2=write/save"				
"	2 1=rainfall; 2=hydrograph"				
"	1 1=runoff; 2=inflow; 3=outflow; 4=junction"				
"	DIV01500.000hyd"				
"	Major flow at 1500"				
"	Total volume	0.000	c.m"		
"	Maximum flow	0.000	c.m/sec"		
"	0.000 0.875 0.875 0.208 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.000 0.875 0.875 0.208"				

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" 33      CATCHMENT 4000"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          4000 Catchment 4000"
"          0.000 % Impervious"
"          7.330 Total Area"
"          60.000 Flow length"
"          2.000 Overland Slope"
"          7.330 Pervious Area"
"          60.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          60.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          50.000 Pervious SCS Curve No."
"          0.453 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          25.400 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"              0.619      0.875      0.875      0.208 c.m/sec"
"          Catchment 4000      Pervious      Impervious Total Area "
"          Surface Area      7.330      0.000      7.330      hectare"
"          Time of concentration 27.692      4.336      27.692      minutes"
"          Time to Centroid      2672.198      2258.968      2672.197      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      2.0890      0.0000      2.0891      ha-m"
"          Rainfall losses      155.800      39.404      155.800      mm"
"          Runoff depth      129.200      245.596      129.200      mm"
"          Runoff volume      9470.35      0.02      9470.37      c.m"
"          Runoff coefficient      0.453      0.000      0.453      "
"          Maximum flow      0.619      0.000      0.619      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.619      1.494      0.875      0.208"
" 54      POND DESIGN"
"          1.494 Current peak flow      c.m/sec"
"          0.250 Target outflow      c.m/sec"
"          29575.5 Hydrograph volume      c.m"
"          6. Number of stages"
"          409.630 Minimum water level      metre"
"          410.750 Maximum water level      metre"
"          409.630 Starting water level      metre"
"          0 Keep Design Data: 1 = True; 0 = False"
"              Level Discharge      Volume"

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"           409.630    0.000    0.000"
"           409.750    0.6650   402.200"
"           410.000    3.601   2187.900"
"           410.250    7.811   5318.900"
"           410.500   12.984   9642.300"
"           410.750   18.965  15227.70"
"           Peak outflow                1.444    c.m/sec"
"           Maximum level                409.816   metre"
"           Maximum storage              875.731    c.m"
"           Centroidal lag               42.281   hours"
"           0.619    1.494    1.444    0.208 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5 Next link "
"           0.619    1.444    1.444    0.208"
" 52      CHANNEL DESIGN"
"           1.444 Current peak flow    c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           0.000 Basewidth    metre"
"           7.410 Left bank slope"
"           6.000 Right bank slope"
"           0.950 Channel depth    metre"
"           1.040 Gradient    %"
"           Depth of flow                0.449    metre"
"           Velocity                    1.068    m/sec"
"           Channel capacity             10.655   c.m/sec"
"           Critical depth               0.394    metre"
" 53      ROUTE Channel Route 72"
"           72.40 Channel Route 72 Reach length ( metre)"
"           0.388 X-factor <= 0.5"
"           50.828 K-lag ( seconds)"
"           0.000 Default(0) or user spec.(1) values used"
"           0.500 X-factor <= 0.5"
"           30.000 K-lag ( seconds)"
"           0.500 Beta weighting factor"
"           62.069 Routing time step ( seconds)"
"           1 No. of sub-reaches"
"           Peak outflow                1.438    c.m/sec"
"           0.619    1.444    1.438    0.208 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5 Next link "
"           0.619    1.438    1.438    0.208"
" 52      CHANNEL DESIGN"
"           1.438 Current peak flow    c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           2.000 Basewidth    metre"
"           2.950 Left bank slope"
"           3.000 Right bank slope"
"           0.950 Channel depth    metre"

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"      1.040 Gradient %"
"      Depth of flow          0.378 metre"
"      Velocity              1.217 m/sec"
"      Channel capacity      9.246 c.m/sec"
"      Critical depth        0.318 metre"
" 53    ROUTE Channel Route 40"
"      39.80 Channel Route 40 Reach length ( metre)"
"      0.260 X-factor <= 0.5"
"      24.517 K-lag ( seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag ( seconds)"
"      0.500 Beta weighting factor"
"      36.000 Routing time step ( seconds)"
"      1 No. of sub-reaches"
"      Peak outflow          1.436 c.m/sec"
"      0.619 1.438 1.436 0.208 c.m/sec"
" 40    HYDROGRAPH Combine 100"
"      6 Combine "
"      100 Node #"
"      Existing Wetland"
"      Maximum flow          1.436 c.m/sec"
"      Hydrograph volume     29574.402 c.m"
"      0.619 1.438 1.436 1.436"
" 40    HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.619 0.000 1.436 1.436"
" 33    CATCHMENT 2100"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2100 Catchment 2100"
"      60.000 % Impervious"
"      1.960 Total Area"
"      40.000 Flow length"
"      2.000 Overland Slope"
"      0.784 Pervious Area"
"      40.000 Pervious length"
"      2.000 Pervious slope"
"      1.176 Impervious Area"
"      40.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.765 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.850 Impervious Runoff coefficient"

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"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.232	0.000	1.436	1.436 c.m/sec"	
"		Catchment 2100	Pervious	Impervious	Total Area	"
"		Surface Area	0.784	1.176	1.960	hectare"
"		Time of concentration	18.924	3.399	9.220	minutes"
"		Time to Centroid	2520.773	2266.333	2361.739	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	2234.40	3351.60	5586.00	c.m"
"		Rainfall losses	66.918	42.646	52.354	mm"
"		Runoff depth	218.082	242.354	232.646	mm"
"		Runoff volume	1709.77	2850.09	4559.85	c.m"
"		Runoff coefficient	0.765	0.850	0.816	"
"		Maximum flow	0.085	0.148	0.232	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.232	0.232	1.436	1.436"	
" 33		CATCHMENT 2400"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2400	Catchment 2400"				
"	90.000	% Impervious"				
"	0.800	Total Area"				
"	20.000	Flow length"				
"	2.000	Overland Slope"				
"	0.080	Pervious Area"				
"	20.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.720	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.102	0.232	1.436	1.436 c.m/sec"	
"		Catchment 2400	Pervious	Impervious	Total Area	"
"		Surface Area	0.080	0.720	0.800	hectare"
"		Time of concentration	12.485	2.243	3.166	minutes"
"		Time to Centroid	2505.277	2290.972	2310.291	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	228.00	2052.00	2280.00	c.m"
"		Rainfall losses	70.073	43.972	46.582	mm"



"	Runoff depth	214.927	241.028	238.418	mm"
"	Runoff volume	171.94	1735.40	1907.34	c.m"
"	Runoff coefficient	0.754	0.846	0.837	"
"	Maximum flow	0.009	0.092	0.102	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.102	0.334	1.436	1.436"	
" 54	POND DESIGN"				
"	0.334	Current peak flow	c.m/sec"		
"	0.020	Target outflow	c.m/sec"		
"	6467.2	Hydrograph volume	c.m"		
"	14.	Number of stages"			
"	410.650	Minimum water level	metre"		
"	411.950	Maximum water level	metre"		
"	410.650	Starting water level	metre"		
"	0	Keep Design Data: 1 = True; 0 = False"			
"		Level Discharge	Volume"		
"	410.650	0.000	0.000"		
"	410.700	0.00600	43.000"		
"	410.800	0.01300	135.000"		
"	410.900	0.02000	232.000"		
"	411.000	0.02500	334.000"		
"	411.100	0.02900	442.000"		
"	411.200	0.1260	556.000"		
"	411.300	0.1390	676.000"		
"	411.400	0.1510	801.000"		
"	411.500	0.1630	933.000"		
"	411.600	0.1730	1072.000"		
"	411.650	0.1780	1143.000"		
"	411.700	0.3370	1216.000"		
"	411.800	1.007	1368.000"		
"	Peak outflow	0.268	c.m/sec"		
"	Maximum level	411.678	metre"		
"	Maximum storage	1184.491	c.m"		
"	Centroidal lag	41.101	hours"		
"	0.102	0.334	0.268	1.436 c.m/sec"	
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.102	0.268	0.268	1.436"	
" 33	CATCHMENT 2300"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	1	SCS method"			
"	2300	Catchment 2300"			
"	10.000	% Impervious"			
"	0.480	Total Area"			
"	20.000	Flow length"			
"	2.000	Overland Slope"			
"	0.432	Pervious Area"			
"	20.000	Pervious length"			

"	2.000	Pervious slope"				
"	0.048	Impervious Area"				
"	20.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.754	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.846	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"			0.055	0.268	0.268	1.436 c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.432	0.048	0.480	hectare"
"		Time of concentration	12.485	2.243	11.350	minutes"
"		Time to Centroid	2505.277	2290.972	2481.532	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	1231.20	136.80	1368.00	c.m"
"		Rainfall losses	70.073	43.972	67.463	mm"
"		Runoff depth	214.927	241.028	217.537	mm"
"		Runoff volume	928.48	115.69	1044.18	c.m"
"		Runoff coefficient	0.754	0.846	0.763	"
"		Maximum flow	0.049	0.006	0.055	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.055	0.315	0.268	1.436"
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"			0.055	0.315	0.315	1.436"
" 40		HYDROGRAPH Combine 200"				
"	6	Combine "				
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow		0.315		c.m/sec"
"		Hydrograph volume		7582.879		c.m"
"			0.055	0.315	0.315	0.315"
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"			0.055	0.000	0.315	0.315"
" 33		CATCHMENT 2200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	2200	Catchment 2200"				
"	75.000	% Impervious"				
"	0.920	Total Area"				
"	40.000	Flow length"				

```

"      2.000  Overland Slope"
"      0.230  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.690  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.765  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.850  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.112      0.000      0.315      0.315 c.m/sec"
"      Catchment 2200      Pervious      Impervious      Total Area  "
"      Surface Area      0.230      0.690      0.920      hectare"
"      Time of concentration  18.924      3.399      6.981      minutes"
"      Time to Centroid      2520.774      2266.333      2325.042      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      655.50      1966.50      2622.00      c.m"
"      Rainfall losses      66.918      42.646      48.714      mm"
"      Runoff depth      218.082      242.354      236.286      mm"
"      Runoff volume      501.59      1672.25      2173.83      c.m"
"      Runoff coefficient      0.765      0.850      0.829      "
"      Maximum flow      0.025      0.087      0.112      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"          0.112      0.112      0.315      0.315"
" 54      POND DESIGN"
"      0.112  Current peak flow      c.m/sec"
"      0.756  Target outflow      c.m/sec"
"     2173.8  Hydrograph volume      c.m"
"          12.  Number of stages"
"     413.700  Minimum water level      metre"
"     415.000  Maximum water level      metre"
"     413.700  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     413.700      0.000      0.000"
"     413.800      0.00500      88.600"
"     413.900      0.01000      187.200"
"     414.000      0.01300      298.400"
"     414.100      0.01500      422.200"
"     414.200      0.2590      558.900"
"     414.300      0.2910      708.500"
"     414.400      0.3210      871.100"

```

"		414.500	0.3470	1046.900"		
"		414.600	0.3720	1236.100"		
"		414.700	0.3950	1438.700"		
"		415.000	2.828	2087.400"		
"		Peak outflow		0.099	c.m/sec"	
"		Maximum level		414.137	metre"	
"		Maximum storage		473.254	c.m"	
"		Centroidal lag		42.637	hours"	
"		0.112	0.112	0.099	0.315	c.m/sec"
" 40		HYDROGRAPH	Combine	200"		
"	6	Combine	"			
"	200	Node #"				
"		To Trib. of Grand River"				
"		Maximum flow		0.406	c.m/sec"	
"		Hydrograph volume		9743.426	c.m"	
"		0.112	0.112	0.099	0.406"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.112	0.000	0.099	0.406"	
" 33		CATCHMENT 3100"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	1	SCS method"				
"	3100	Catchment 3100"				
"	60.000	% Impervious"				
"	0.420	Total Area"				
"	40.000	Flow length"				
"	1.000	Overland Slope"				
"	0.168	Pervious Area"				
"	40.000	Pervious length"				
"	1.000	Pervious slope"				
"	0.252	Impervious Area"				
"	40.000	Impervious length"				
"	1.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	78.000	Pervious SCS Curve No."				
"	0.761	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	7.164	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.860	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.049	0.000	0.099	0.406	c.m/sec"
"		Catchment 3100	Pervious	Impervious	Total Area	"
"		Surface Area	0.168	0.252	0.420	hectare"
"		Time of concentration	23.298	4.185	11.277	minutes"
"		Time to Centroid	2524.169	2259.596	2357.767	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"

"	Rainfall volume	478.80	718.20	1197.00	c.m"
"	Rainfall losses	68.199	40.012	51.287	mm"
"	Runoff depth	216.801	244.988	233.713	mm"
"	Runoff volume	364.23	617.37	981.59	c.m"
"	Runoff coefficient	0.761	0.860	0.820	"
"	Maximum flow	0.019	0.032	0.049	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.049	0.049	0.099	0.406"
" 56	DIVERSION"				
"	32001 Node number"				
"	0.067 Overflow threshold"				
"	1.000 Required diverted fraction"				
"	0 Conduit type; 1=Pipe;2=Channel"				
"	Peak of diverted flow	0.000			c.m/sec"
"	Volume of diverted flow	0.000			c.m"
"	DIV32001.000hyd"				
"	Major flow at 32001"				
"		0.049	0.049	0.049	0.406 c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"		0.049	0.049	0.049	0.406"
" 54	POND DESIGN"				
"	0.049 Current peak flow				c.m/sec"
"	0.756 Target outflow				c.m/sec"
"	981.6 Hydrograph volume				c.m"
"	15. Number of stages"				
"	410.620 Minimum water level				metre"
"	414.230 Maximum water level				metre"
"	410.620 Starting water level				metre"
"	0 Keep Design Data: 1 = True; 0 = False"				
"	Level Discharge				Volume"
"	410.620	0.000			0.000"
"	410.870	0.01300			4.855"
"	411.120	0.02000			14.351"
"	411.370	0.02500			24.074"
"	411.620	0.02900			33.921"
"	411.870	0.03300			43.768"
"	412.120	0.03600			53.614"
"	412.370	0.03900			63.461"
"	412.620	0.04200			73.308"
"	412.870	0.04400			74.155"
"	413.120	0.04700			75.003"
"	413.370	0.04900			75.850"
"	413.620	0.05200			76.698"
"	413.980	0.05500			77.918"
"	414.230	0.1600			78.483"
"	Peak outflow		0.040		c.m/sec"
"	Maximum level		412.466		metre"
"	Maximum storage		67.240		c.m"

"	Centroidal lag	39.444	hours"		
"	0.049	0.049	0.040	0.406	c.m/sec"
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.049	0.040	0.040	0.406"	
" 33	CATCHMENT 3200"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	3200 Catchment 3200"				
"	60.000 % Impervious"				
"	0.130 Total Area"				
"	20.000 Flow length"				
"	1.000 Overland Slope"				
"	0.052 Pervious Area"				
"	20.000 Pervious length"				
"	1.000 Pervious slope"				
"	0.078 Impervious Area"				
"	20.000 Impervious length"				
"	1.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.763 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.848 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.016	0.040	0.040	0.406	c.m/sec"
"	Catchment 3200	Pervious	Impervious	Total Area	"
"	Surface Area	0.052	0.078	0.130	hectare"
"	Time of concentration	15.371	2.761	7.490	minutes"
"	Time to Centroid	2513.554	2278.554	2366.679	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	148.20	222.30	370.50	c.m"
"	Rainfall losses	67.436	43.264	52.933	mm"
"	Runoff depth	217.564	241.736	232.067	mm"
"	Runoff volume	113.13	188.55	301.69	c.m"
"	Runoff coefficient	0.763	0.848	0.814	"
"	Maximum flow	0.006	0.010	0.016	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.016	0.053	0.040	0.406"	
" 33	CATCHMENT 3300"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	3300 Catchment 3300"				

```

"      60.000  % Impervious"
"      0.240  Total Area"
"      20.000  Flow length"
"      2.000  Overland Slope"
"      0.096  Pervious Area"
"      20.000  Pervious length"
"      2.000  Pervious slope"
"      0.144  Impervious Area"
"      20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.754  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.846  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.029      0.053      0.040      0.406 c.m/sec"
"      Catchment 3300      Pervious      Impervious      Total Area  "
"      Surface Area      0.096      0.144      0.240      hectare"
"      Time of concentration  12.485      2.243      6.061      minutes"
"      Time to Centroid      2505.277      2290.972      2370.872      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      273.60      410.40      684.00      c.m"
"      Rainfall losses      70.073      43.972      54.413      mm"
"      Runoff depth      214.927      241.028      230.587      mm"
"      Runoff volume      206.33      347.08      553.41      c.m"
"      Runoff coefficient      0.754      0.846      0.809      "
"      Maximum flow      0.011      0.018      0.029      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.029      0.077      0.040      0.406"
" 47      FILEI_0 Read/Open DIV32001.000hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV32001.000hyd"
"      Major flow at 32001"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"          0.000      0.077      0.040      0.406 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.000      0.077      0.040      0.406"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.000      0.077      0.077      0.406"

```

" 40	HYDROGRAPH	Combine	300"		
"	6	Combine "			
"	300	Node #"			
"		To Walser Street"			
"		Maximum flow	0.077	c.m/sec"	
"		Hydrograph volume	1827.049	c.m"	
"		0.000 0.077	0.077	0.077"	
" 40	HYDROGRAPH	Confluence	300"		
"	7	Confluence "			
"	300	Node #"			
"		To Walser Street"			
"		Maximum flow	0.077	c.m/sec"	
"		Hydrograph volume	1827.049	c.m"	
"		0.000 0.077	0.077	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.000 0.077	0.077	0.000"	
" 40	HYDROGRAPH	Combine	100"		
"	6	Combine "			
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow	1.511	c.m/sec"	
"		Hydrograph volume	31401.453	c.m"	
"		0.000 0.077	0.077	1.511"	
" 40	HYDROGRAPH	Confluence	100"		
"	7	Confluence "			
"	100	Node #"			
"		Existing Wetland"			
"		Maximum flow	1.511	c.m/sec"	
"		Hydrograph volume	31401.457	c.m"	
"		0.000 1.511	0.077	0.000"	
" 40	HYDROGRAPH	Copy to Outflow"			
"	8	Copy to Outflow"			
"		0.000 1.511	1.511	0.000"	
" 40	HYDROGRAPH	Combine	200"		
"	6	Combine "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	1.917	c.m/sec"	
"		Hydrograph volume	41144.883	c.m"	
"		0.000 1.511	1.511	1.917"	
" 40	HYDROGRAPH	Confluence	200"		
"	7	Confluence "			
"	200	Node #"			
"		To Trib. of Grand River"			
"		Maximum flow	1.917	c.m/sec"	
"		Hydrograph volume	41144.879	c.m"	
"		0.000 1.917	1.511	0.000"	
" 38	START/RE-START	TOTALS	200"		
"	3	Runoff Totals on EXIT"			

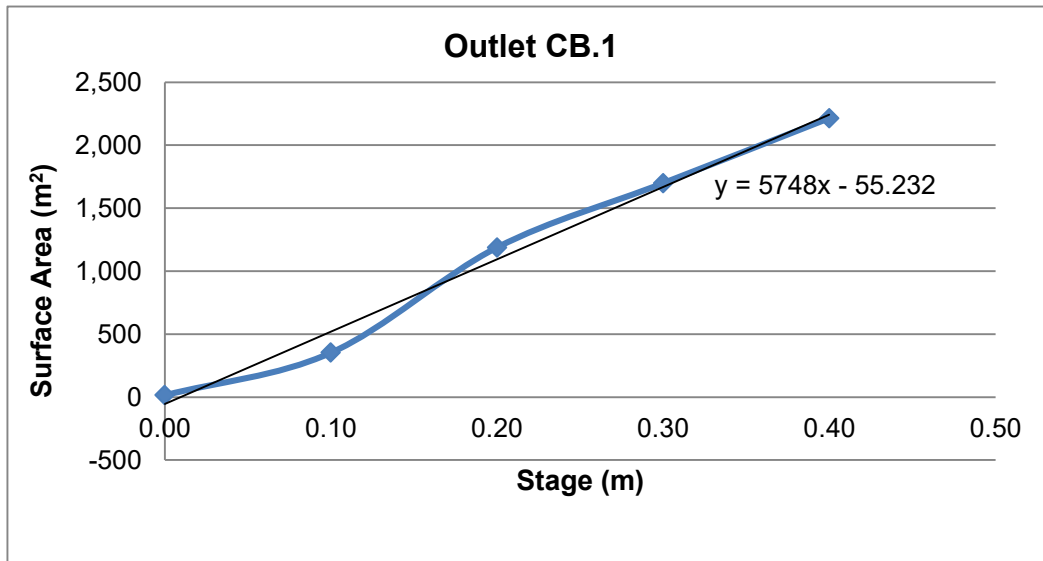


"	Total Catchment area	22.610	hectare"
"	Total Impervious area	7.847	hectare"
"	Total % impervious	34.706"	
" 19	EXIT"		

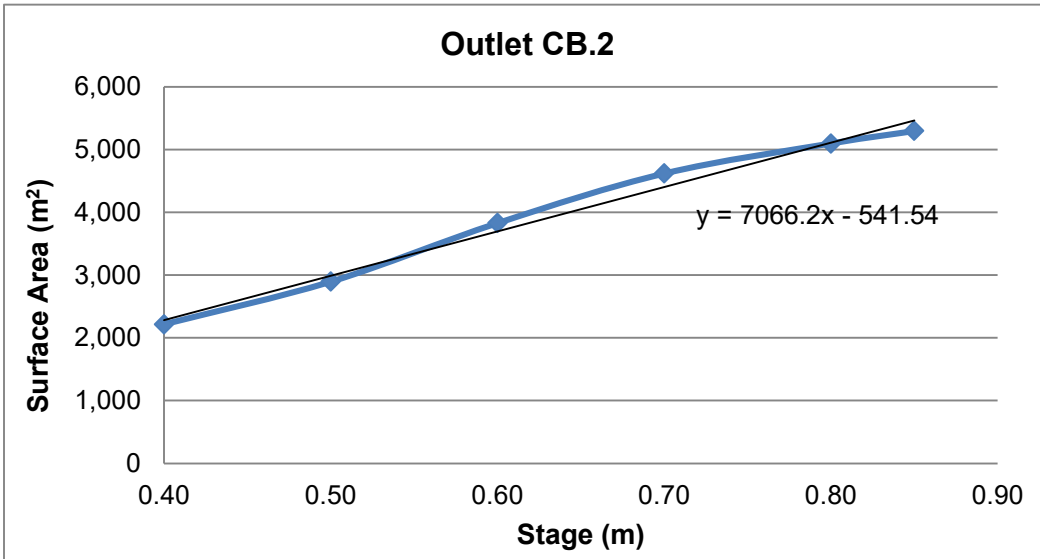
**Ainley Farm Subdivision**  
**Our File: 411009**  
**March 14, 2023**

**Stormwater Management Facility No. 1 - Drawdown Calculations**

<b>Elevation</b>	<b>Stage (m)</b>	<b>Surface area (m<sup>2</sup>)</b>	
411.00	0.00	15	CB.1 Lip
411.10	0.10	353	
411.20	0.20	1,189	
411.30	0.30	1,699	
411.40	0.40	2,216	CB.2 Lip
411.50	0.50	2,897	
411.60	0.60	3,829	
411.70	0.70	4,620	
411.80	0.80	5,097	
411.85	0.85	5,297	Weir
411.90	0.90	5,498	Top of Bank
412.00	1.00	5792.6	



**Ainley Farm Subdivision**  
**Our File: 411009**  
**March 14, 2023**



$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o}$$

Eq. 4.11 (MOE, 2003)

**Outlet - CB.1**  
**250mm Orifice**

Given: d = 0.250  
A<sub>o</sub> = 0.049  
C<sub>2</sub> = 5748  
C<sub>3</sub> = 55.232

**Outlet - CB.2**  
**350mm Orifice**

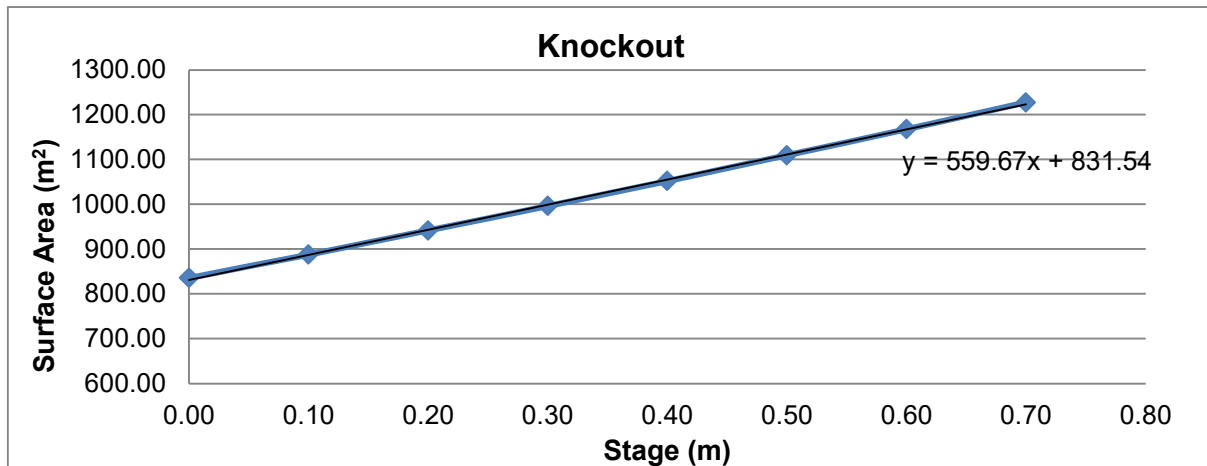
Given: d = 0.350  
A<sub>o</sub> = 0.096  
C<sub>2</sub> = 7066.2  
C<sub>3</sub> = 541.54

Storm	Ponding Depth	CB 1 Outlet h (m)	CB.1 Outlet Drawdown (hr)	CB.2 Outlet h (m)	CB.2 Outlet Drawdown (hr)	Total Drawdown (hr)
2 Year	411.50	0.50	2.9	--	--	<b>2.9</b>
5 Year	411.66	0.60	3.8	0.1	0.4	<b>4.2</b>
10 Year	411.73	0.60	3.8	0.1	0.6	<b>4.4</b>
25 Year	411.85	0.60	3.8	0.3	1.2	<b>5.0</b>
50 Year	411.89	0.60	3.8	0.3	1.2	<b>5.0</b>
100 Year	411.92	0.60	3.8	0.3	1.2	<b>5.0</b>
Regional Storm	411.92	0.60	3.8	0.3	1.2	<b>5.0</b>

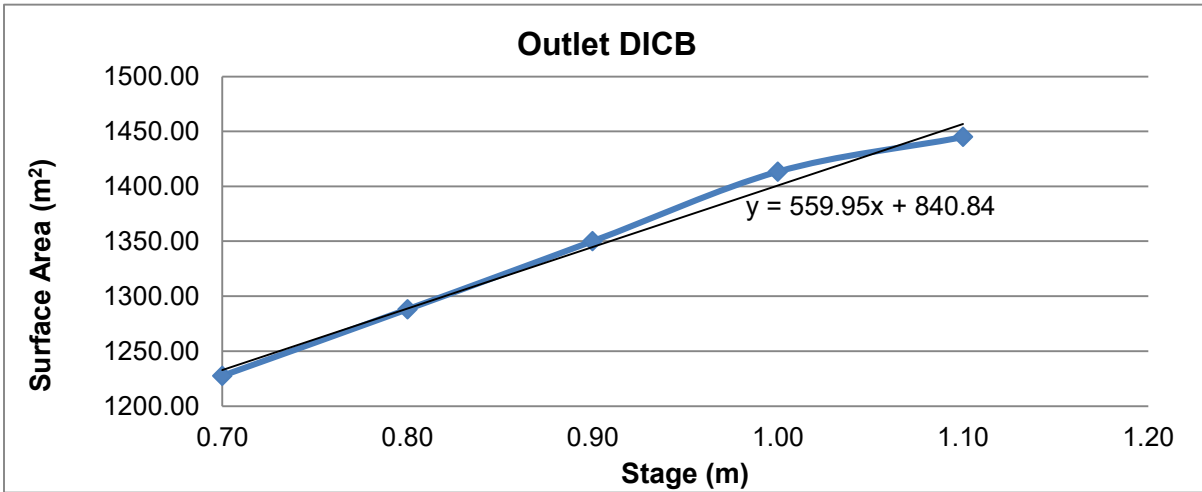
**Ainley Farm Subdivision**  
**Our File: 411009**  
**March 14, 2023**

**Stormwater Management Facility No. 2 Drawdown Calculations**

<b>Elevation</b>	<b>Stage (m)</b>	<b>Surface area (m<sup>2</sup>)</b>	
410.65	0.00	835.70	Knockout
410.70	0.10	888.00	
410.80	0.20	941.90	
410.90	0.30	996.40	
411.00	0.40	1052.40	
411.10	0.50	1109.60	
411.20	0.60	1167.90	
411.30	0.70	1227.50	CB Lip
411.40	0.80	1288.15	
411.50	0.90	1350.10	
411.60	1.00	1413.30	
411.65	1.10	1444.90	Weir
411.70	1.20	1476.50	
411.80	1.30	1559.40	Top of bank



**Ainley Farm Subdivision**  
**Our File: 411009**  
**March 14, 2023**



$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o} \quad \text{Eq. 4.11 (MOE, 2003)}$$

<b>Knockout 150mm Orifice</b>	<b>Outlet - DICB 300mm Orifice</b>
Given: d = 0.150	Given: d = 0.300
A <sub>o</sub> = 0.018	A <sub>o</sub> = 0.071
C <sub>2</sub> = 559.67	C <sub>2</sub> = 559.95
C <sub>3</sub> = 831.54	C <sub>3</sub> = 840.84

Storm	Ponding Depth	Knockout h (m)	Knockout Drawdown (hr)	Outlet - DICB h (m)	Outlet - DICB Drawdown (hr)	Total Drawdown (hr)
2 Year	411.06	0.41	6.6	--	--	<b>6.6</b>
5 Year	411.19	0.54	7.8	--	--	<b>7.8</b>
10 Year	411.31	0.66	8.9	0.01	0.2	<b>9.1</b>
25 Year	411.48	0.95	11.2	0.18	1.1	<b>12.3</b>
50 Year	411.61	0.95	11.2	0.31	1.4	<b>12.6</b>
100 Year	411.69	0.95	11.2	0.35	1.5	<b>12.8</b>
Regional Storm	411.68	0.95	11.2	0.35	1.5	<b>12.8</b>

Stormceptor® EF Sizing Report

**STORMCEPTOR®**

**ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION**

07/15/2022

Province:	Ontario
City:	Elora
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	Ainley Farm Subdivision
Project Number:	411009
Designer Name:	Patricia Wiebe
Designer Company:	GM BluePlan Engineering Ltd.
Designer Email:	patricia.wiebe@gmblueplan.ca
Designer Phone:	519-748-1440
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	SWM Pond 1, Outlet 1 - South
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Drainage Area (ha):	2.32
% Imperviousness:	50.00

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	43.29
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EF4	72
<b>EF6</b>	<b>83</b>
EF8	88
EF10	92
EF12	95

**Recommended Stormceptor EF Model: EF6**  
**Estimated Net Annual Sediment (TSS) Load Reduction (%): 83**  
**Water Quality Runoff Volume Capture (%): > 90**

## Stormceptor® EF Sizing Report

### THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

### PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

### PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m <sup>2</sup> )	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	1.93	116.0	44.0	100	8.5	8.5
1	20.6	29.1	3.87	232.0	88.0	98	20.3	28.8
2	16.8	45.9	7.74	464.0	177.0	87	14.6	43.4
3	10.8	56.7	11.61	697.0	265.0	80	8.6	52.1
4	8.5	65.2	15.48	929.0	353.0	76	6.4	58.5
5	6.4	71.6	19.35	1161.0	441.0	73	4.7	63.2
6	5.5	77.0	23.22	1393.0	530.0	72	3.9	67.1
7	3.9	81.0	27.09	1625.0	618.0	71	2.8	69.9
8	2.9	83.9	30.96	1857.0	706.0	70	2.0	71.9
9	2.7	86.5	34.83	2090.0	795.0	69	1.9	73.8
10	2.2	88.7	38.70	2322.0	883.0	69	1.5	75.3
11	1.0	89.7	42.57	2554.0	971.0	68	0.7	75.9
12	1.7	91.3	46.44	2786.0	1059.0	69	1.1	77.1
13	1.4	92.8	50.31	3018.0	1148.0	70	1.0	78.1
14	1.0	93.7	54.18	3251.0	1236.0	72	0.7	78.8
15	0.3	94.0	58.05	3483.0	1324.0	74	0.2	79.0
16	0.8	94.8	61.92	3715.0	1413.0	75	0.6	79.6
17	0.8	95.7	65.79	3947.0	1501.0	70	0.6	80.2
18	0.2	95.8	69.66	4179.0	1589.0	66	0.1	80.3
19	1.5	97.3	73.53	4412.0	1677.0	63	0.9	81.2
20	0.2	97.5	77.40	4644.0	1766.0	60	0.1	81.4
21	0.6	98.2	81.26	4876.0	1854.0	57	0.4	81.7
22	0.0	98.2	85.13	5108.0	1942.0	54	0.0	81.7
23	0.2	98.4	89.00	5340.0	2031.0	52	0.1	81.8
24	0.2	98.6	92.87	5572.0	2119.0	50	0.1	82.0
25	0.2	98.9	96.74	5805.0	2207.0	48	0.1	82.1
30	1.1	100.0	116.09	6966.0	2649.0	41	0.5	82.5
35	0.0	100.0	135.44	8126.0	3090.0	35	0.0	82.5
40	0.0	100.0	154.79	9287.0	3531.0	30	0.0	82.5
45	0.0	100.0	174.14	10448.0	3973.0	27	0.0	82.5
<b>Estimated Net Annual Sediment (TSS) Load Reduction =</b>								<b>83 %</b>

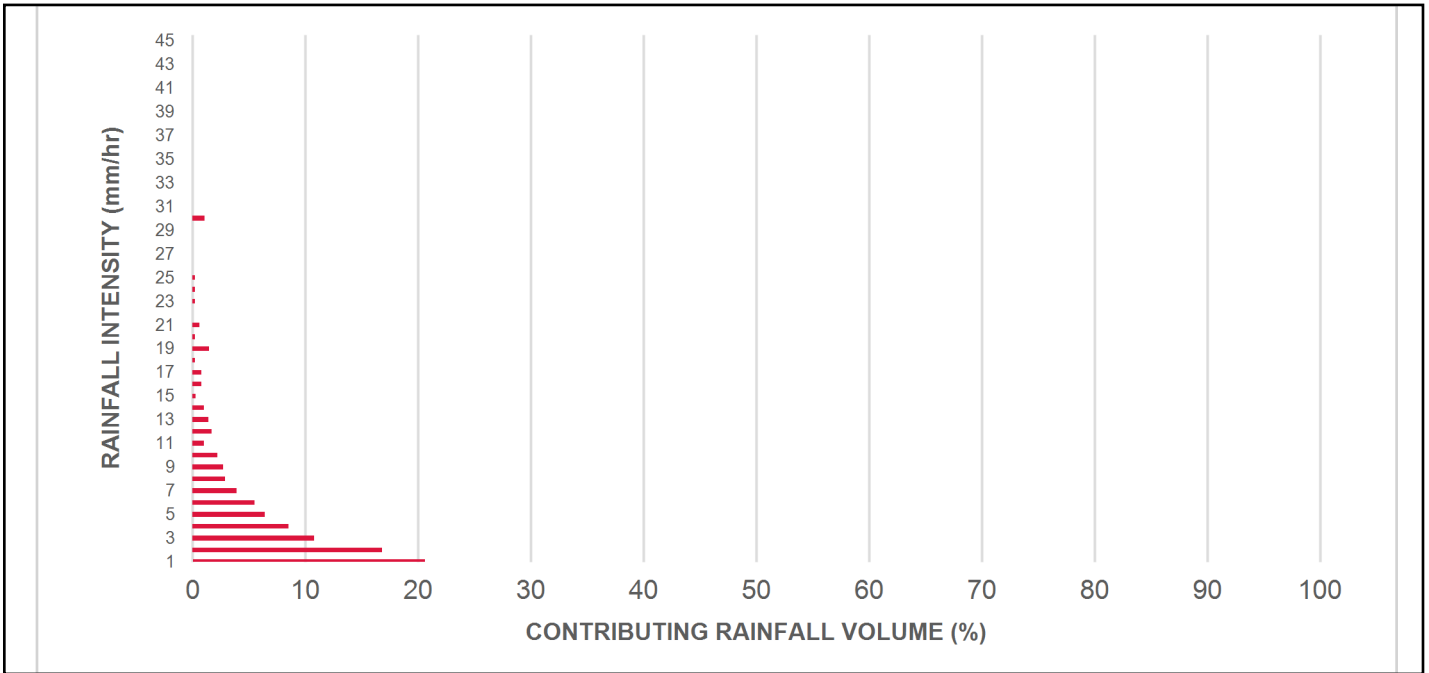
Climate Station ID: 6158731 Years of Rainfall Data: 20



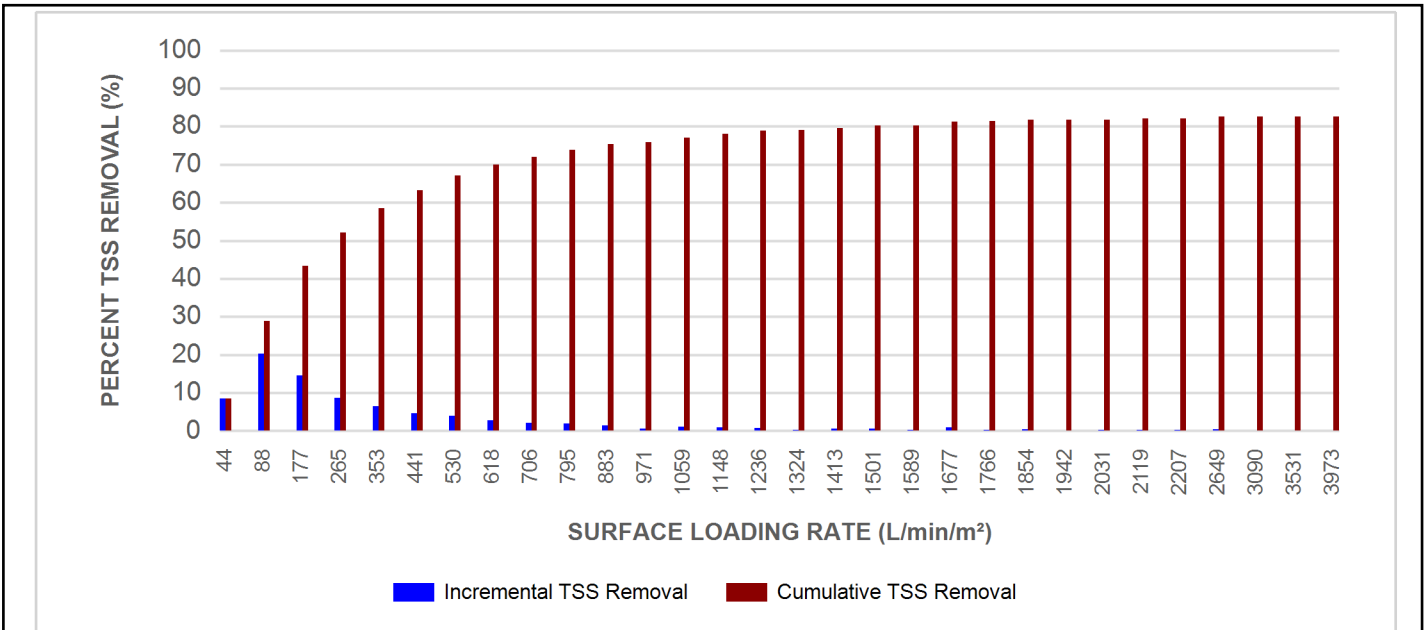


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

**SCOUR PREVENTION AND ONLINE CONFIGURATION**

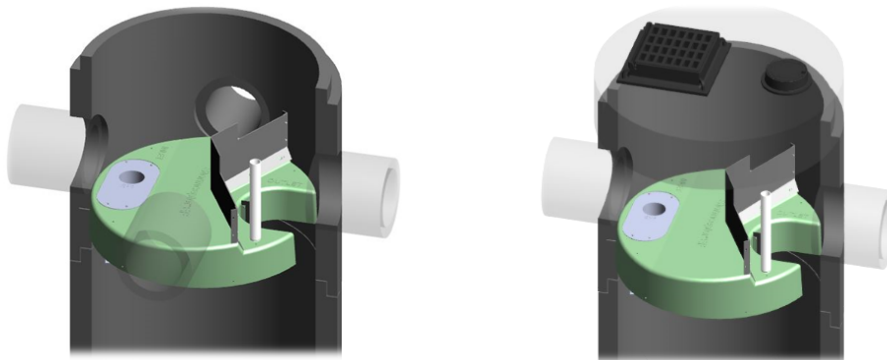
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

**DESIGN FLEXIBILITY**

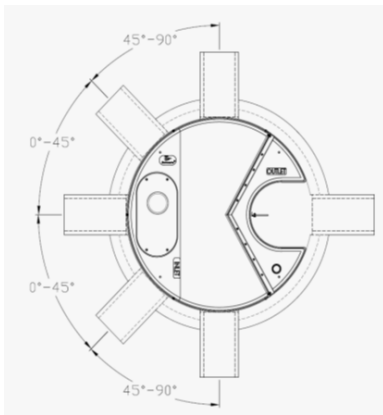
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

**OIL CAPTURE AND RETENTION**

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



## Stormceptor® EF Sizing Report



### INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

### HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

### Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

\*Increased sump depth may be added to increase sediment storage capacity

\*\* Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³ )

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

### STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

### STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR  
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

**PART 1 – GENERAL**

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators.**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

**PART 2 – PRODUCTS**

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m <sup>3</sup> sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m <sup>3</sup> sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m <sup>3</sup> sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m <sup>3</sup> sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m <sup>3</sup> sediment / 2,476 L oil

**PART 3 – PERFORMANCE & DESIGN**

3.1 GENERAL



## Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

### 3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m<sup>2</sup> to 1400 L/min/m<sup>2</sup>, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m<sup>2</sup> and 1400 L/min/m<sup>2</sup> shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m<sup>2</sup> shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m<sup>2</sup>. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m<sup>2</sup>.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m<sup>2</sup> shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m<sup>2</sup>, and shall be calculated using a simple proportioning formula, with 1400 L/min/m<sup>2</sup> in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m<sup>2</sup>.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

### 3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m<sup>2</sup>.

Stormceptor® EF Sizing Report

**STORMCEPTOR®**

**ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION**

07/15/2022

Province:	Ontario
City:	Elora
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	Ainley Farm Subdivision
Project Number:	411009
Designer Name:	Patricia Wiebe
Designer Company:	GM BluePlan Engineering Ltd.
Designer Email:	patricia.wiebe@gmblueplan.ca
Designer Phone:	519-748-1440
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	SWM Pond 1, Outlet 1 - North
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Drainage Area (ha):	3.15
% Imperviousness:	50.00

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	58.77
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EF4	67
EF6	78
<b>EF8</b>	<b>85</b>
EF10	90
EF12	92

**Recommended Stormceptor EF Model: EF8**  
**Estimated Net Annual Sediment (TSS) Load Reduction (%): 85**  
**Water Quality Runoff Volume Capture (%): > 90**

## Stormceptor® EF Sizing Report

### THIRD-PARTY TESTING AND VERIFICATION

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### PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

### PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m <sup>2</sup> )	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	2.63	158.0	34.0	100	8.5	8.5
1	20.6	29.1	5.25	315.0	67.0	100	20.6	29.1
2	16.8	45.9	10.51	631.0	134.0	92	15.5	44.6
3	10.8	56.7	15.76	946.0	201.0	83	8.9	53.5
4	8.5	65.2	21.02	1261.0	268.0	80	6.8	60.3
5	6.4	71.6	26.27	1576.0	335.0	77	4.9	65.3
6	5.5	77.0	31.53	1892.0	402.0	74	4.0	69.3
7	3.9	81.0	36.78	2207.0	470.0	73	2.9	72.2
8	2.9	83.9	42.03	2522.0	537.0	72	2.1	74.3
9	2.7	86.5	47.29	2837.0	604.0	71	1.9	76.1
10	2.2	88.7	52.54	3153.0	671.0	70	1.5	77.7
11	1.0	89.7	57.80	3468.0	738.0	70	0.7	78.4
12	1.7	91.3	63.05	3783.0	805.0	69	1.1	79.5
13	1.4	92.8	68.30	4098.0	872.0	69	1.0	80.5
14	1.0	93.7	73.56	4414.0	939.0	68	0.7	81.1
15	0.3	94.0	78.81	4729.0	1006.0	68	0.2	81.3
16	0.8	94.8	84.07	5044.0	1073.0	69	0.5	81.9
17	0.8	95.7	89.32	5359.0	1140.0	70	0.6	82.5
18	0.2	95.8	94.58	5675.0	1207.0	72	0.1	82.6
19	1.5	97.3	99.83	5990.0	1274.0	73	1.1	83.7
20	0.2	97.5	105.08	6305.0	1341.0	74	0.1	83.8
21	0.6	98.2	110.34	6620.0	1409.0	75	0.5	84.3
22	0.0	98.2	115.59	6936.0	1476.0	72	0.0	84.3
23	0.2	98.4	120.85	7251.0	1543.0	69	0.2	84.5
24	0.2	98.6	126.10	7566.0	1610.0	66	0.2	84.6
25	0.2	98.9	131.36	7881.0	1677.0	63	0.2	84.8
30	1.1	100.0	157.63	9458.0	2012.0	53	0.6	85.4
35	0.0	100.0	183.90	11034.0	2348.0	45	0.0	85.4
40	0.0	100.0	210.17	12610.0	2683.0	41	0.0	85.4
45	0.0	100.0	236.44	14186.0	3018.0	35	0.0	85.4
<b>Estimated Net Annual Sediment (TSS) Load Reduction =</b>								<b>85 %</b>

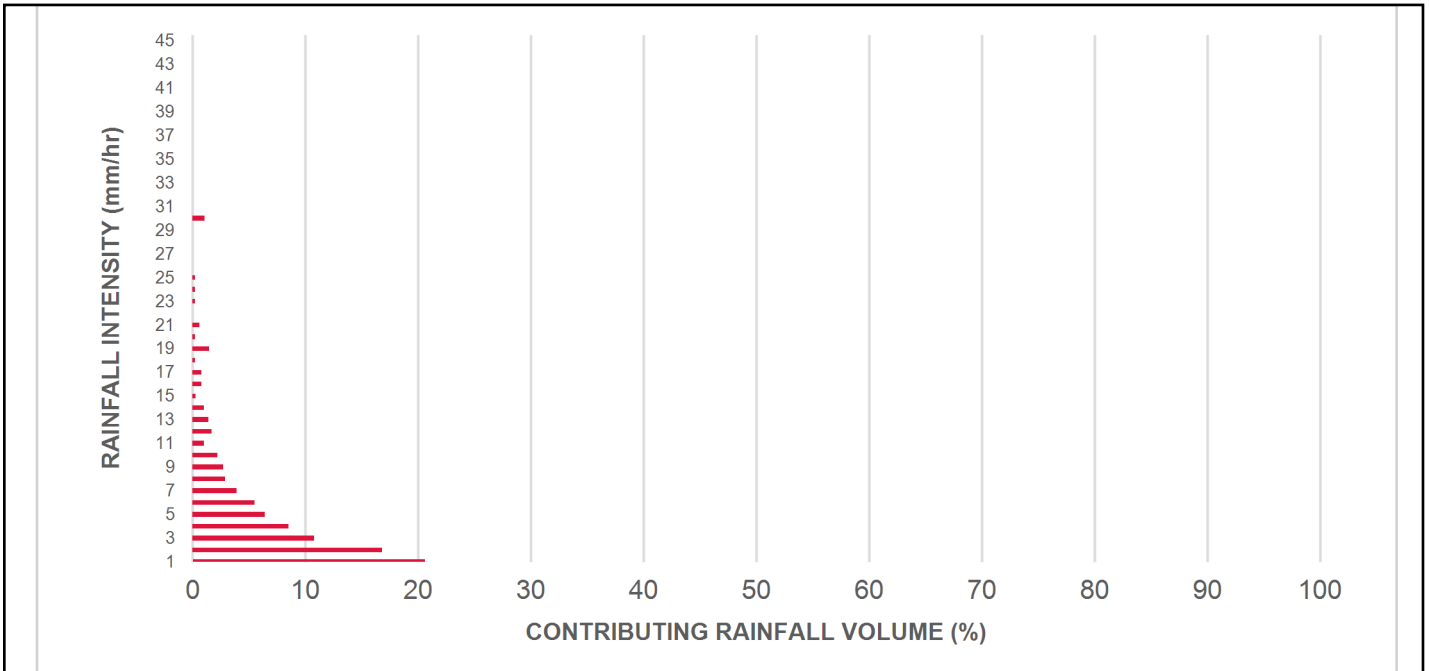
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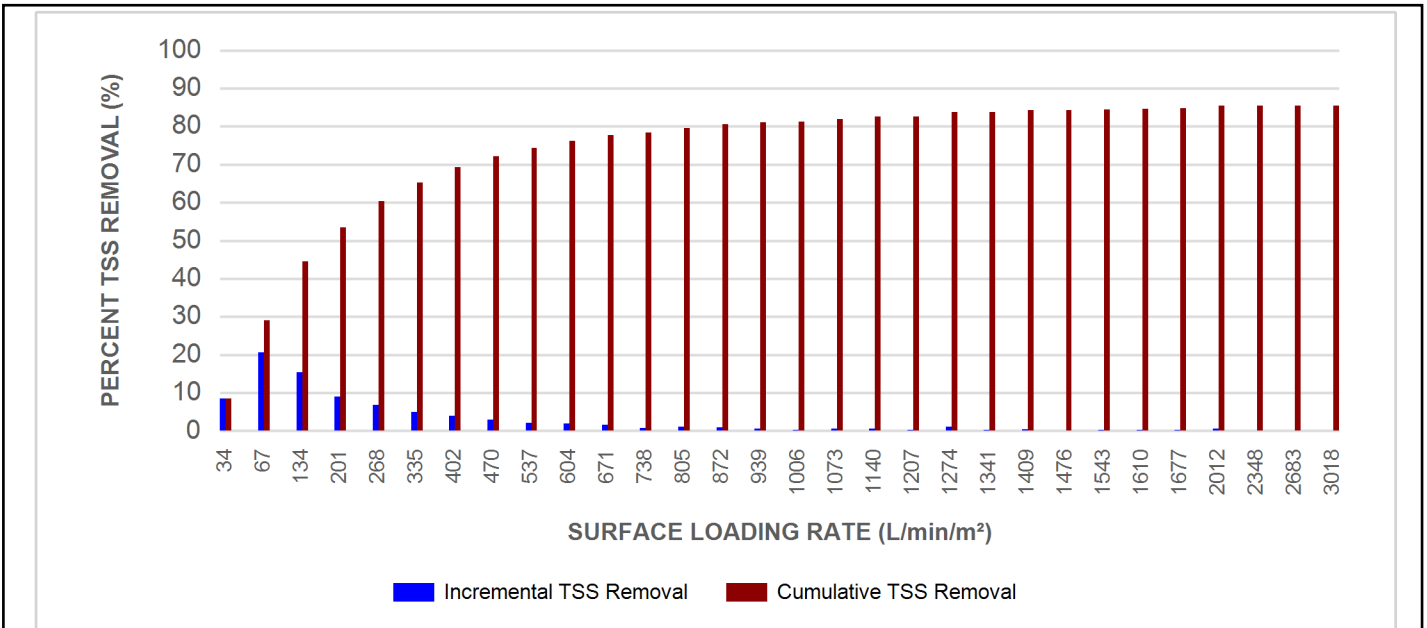


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

**SCOUR PREVENTION AND ONLINE CONFIGURATION**

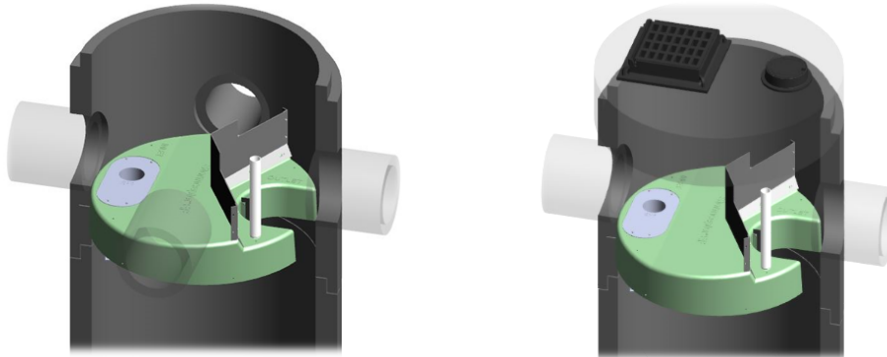
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

**DESIGN FLEXIBILITY**

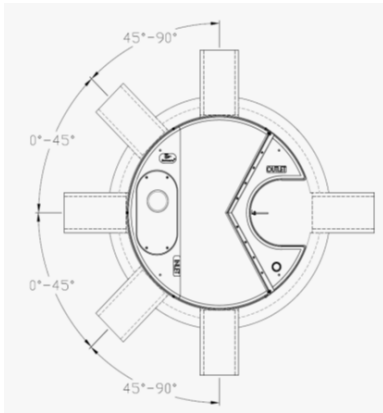
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

**OIL CAPTURE AND RETENTION**

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



## Stormceptor® EF Sizing Report



### INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

### HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

### Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

\*Increased sump depth may be added to increase sediment storage capacity

\*\* Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

### STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

### STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR  
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

**PART 1 – GENERAL**

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators.**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

**PART 2 – PRODUCTS**

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m <sup>3</sup> sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m <sup>3</sup> sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m <sup>3</sup> sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m <sup>3</sup> sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m <sup>3</sup> sediment / 2,476 L oil

**PART 3 – PERFORMANCE & DESIGN**

3.1 GENERAL



## Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

### 3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m<sup>2</sup> to 1400 L/min/m<sup>2</sup>, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m<sup>2</sup> and 1400 L/min/m<sup>2</sup> shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m<sup>2</sup> shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m<sup>2</sup>. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m<sup>2</sup>.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m<sup>2</sup> shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m<sup>2</sup>, and shall be calculated using a simple proportioning formula, with 1400 L/min/m<sup>2</sup> in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m<sup>2</sup>.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

### 3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m<sup>2</sup>.

Stormceptor® EF Sizing Report

**STORMCEPTOR®**

**ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION**

07/15/2022

Province:	Ontario
City:	Elora
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	Ainley Farm Subdivision
Project Number:	411009
Designer Name:	Patricia Wiebe
Designer Company:	GM BluePlan Engineering Ltd.
Designer Email:	patricia.wiebe@gmblueplan.ca
Designer Phone:	519-748-1440
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	SWM Pond 1, Outlet 2
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Drainage Area (ha):	2.32
% Imperviousness:	50.00

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	43.29
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EF4	72
<b>EF6</b>	<b>83</b>
EF8	88
EF10	92
EF12	95

**Recommended Stormceptor EF Model: EF6**  
**Estimated Net Annual Sediment (TSS) Load Reduction (%): 83**  
**Water Quality Runoff Volume Capture (%): > 90**

## Stormceptor® EF Sizing Report

### THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

### PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

### PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m <sup>2</sup> )	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	1.93	116.0	44.0	100	8.5	8.5
1	20.6	29.1	3.87	232.0	88.0	98	20.3	28.8
2	16.8	45.9	7.74	464.0	177.0	87	14.6	43.4
3	10.8	56.7	11.61	697.0	265.0	80	8.6	52.1
4	8.5	65.2	15.48	929.0	353.0	76	6.4	58.5
5	6.4	71.6	19.35	1161.0	441.0	73	4.7	63.2
6	5.5	77.0	23.22	1393.0	530.0	72	3.9	67.1
7	3.9	81.0	27.09	1625.0	618.0	71	2.8	69.9
8	2.9	83.9	30.96	1857.0	706.0	70	2.0	71.9
9	2.7	86.5	34.83	2090.0	795.0	69	1.9	73.8
10	2.2	88.7	38.70	2322.0	883.0	69	1.5	75.3
11	1.0	89.7	42.57	2554.0	971.0	68	0.7	75.9
12	1.7	91.3	46.44	2786.0	1059.0	69	1.1	77.1
13	1.4	92.8	50.31	3018.0	1148.0	70	1.0	78.1
14	1.0	93.7	54.18	3251.0	1236.0	72	0.7	78.8
15	0.3	94.0	58.05	3483.0	1324.0	74	0.2	79.0
16	0.8	94.8	61.92	3715.0	1413.0	75	0.6	79.6
17	0.8	95.7	65.79	3947.0	1501.0	70	0.6	80.2
18	0.2	95.8	69.66	4179.0	1589.0	66	0.1	80.3
19	1.5	97.3	73.53	4412.0	1677.0	63	0.9	81.2
20	0.2	97.5	77.40	4644.0	1766.0	60	0.1	81.4
21	0.6	98.2	81.26	4876.0	1854.0	57	0.4	81.7
22	0.0	98.2	85.13	5108.0	1942.0	54	0.0	81.7
23	0.2	98.4	89.00	5340.0	2031.0	52	0.1	81.8
24	0.2	98.6	92.87	5572.0	2119.0	50	0.1	82.0
25	0.2	98.9	96.74	5805.0	2207.0	48	0.1	82.1
30	1.1	100.0	116.09	6966.0	2649.0	41	0.5	82.5
35	0.0	100.0	135.44	8126.0	3090.0	35	0.0	82.5
40	0.0	100.0	154.79	9287.0	3531.0	30	0.0	82.5
45	0.0	100.0	174.14	10448.0	3973.0	27	0.0	82.5
<b>Estimated Net Annual Sediment (TSS) Load Reduction =</b>								<b>83 %</b>

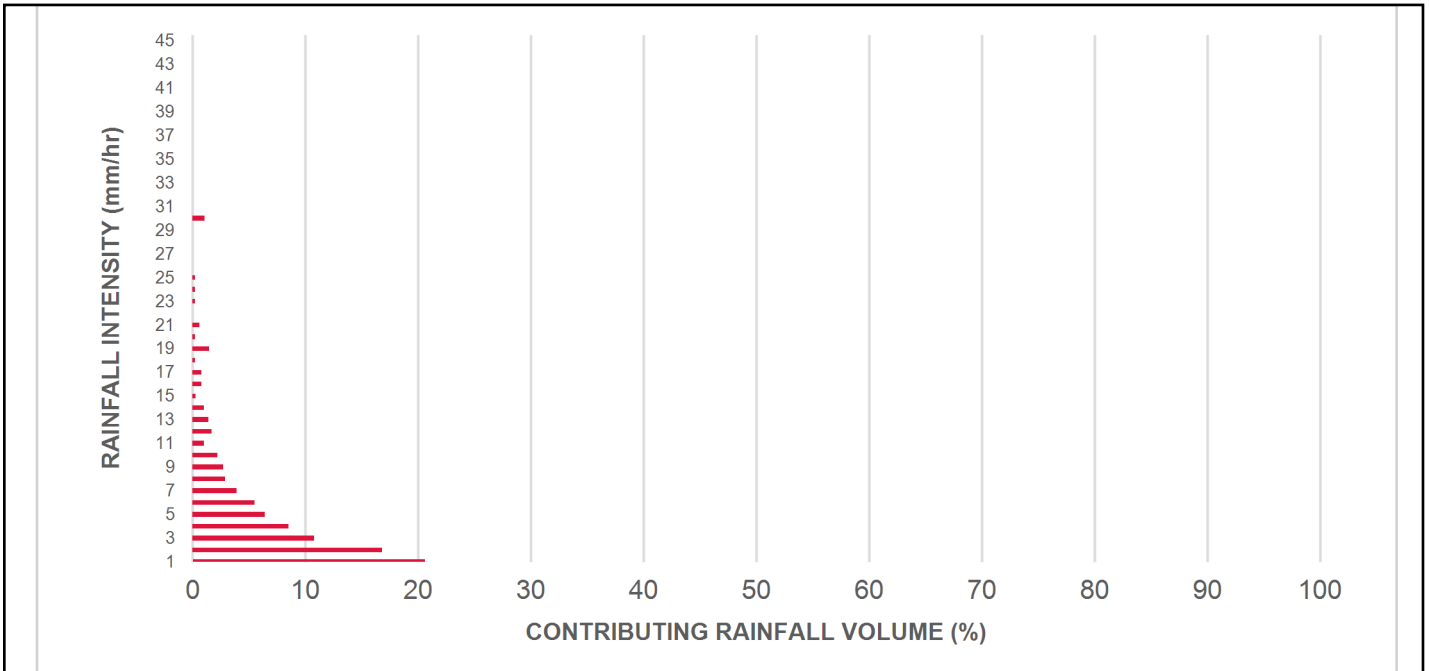
Climate Station ID: 6158731 Years of Rainfall Data: 20



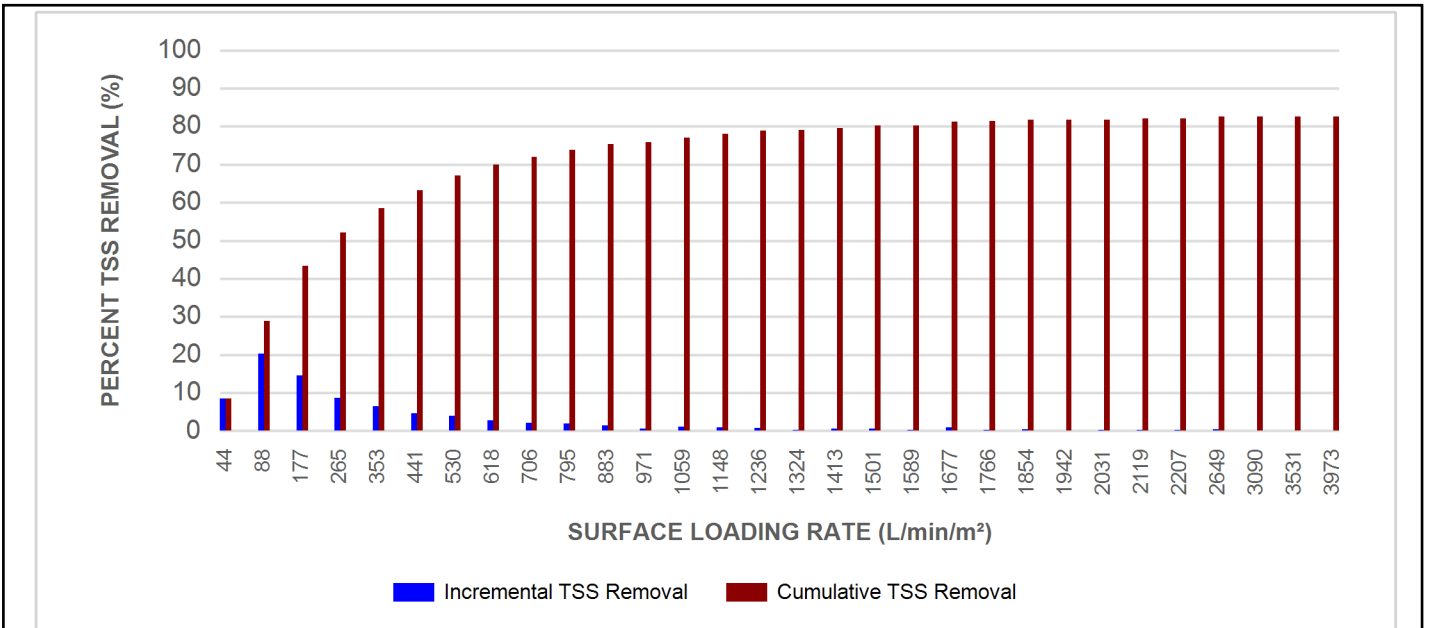


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

**SCOUR PREVENTION AND ONLINE CONFIGURATION**

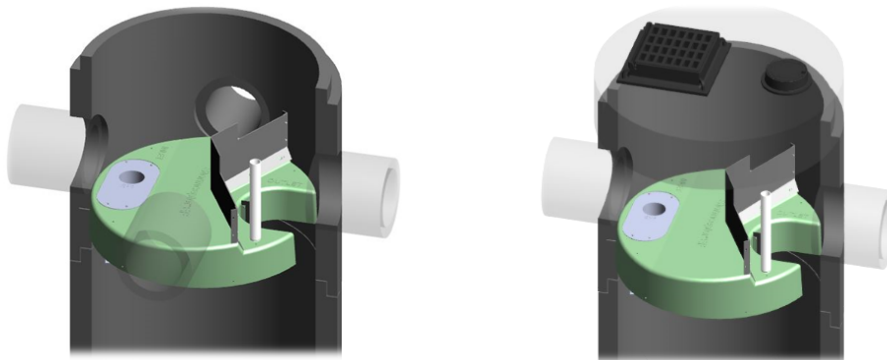
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

**DESIGN FLEXIBILITY**

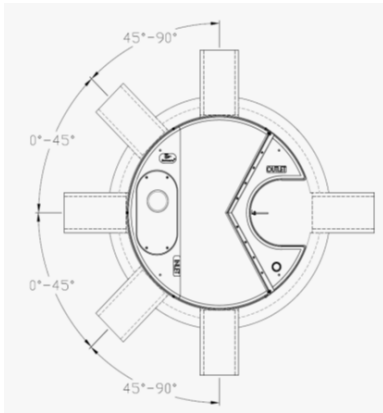
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

**OIL CAPTURE AND RETENTION**

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



## Stormceptor® EF Sizing Report



### INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

### HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

### Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
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EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

\*Increased sump depth may be added to increase sediment storage capacity

\*\* Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³ )

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

### STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

### STANDARD STORMCEPTOR EF/EFO SPECIFICATION

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Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR  
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

**PART 1 – GENERAL**

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators.**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

**PART 2 – PRODUCTS**

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m <sup>3</sup> sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m <sup>3</sup> sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m <sup>3</sup> sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m <sup>3</sup> sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m <sup>3</sup> sediment / 2,476 L oil

**PART 3 – PERFORMANCE & DESIGN**

3.1 GENERAL



## Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

### 3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m<sup>2</sup> to 1400 L/min/m<sup>2</sup>, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m<sup>2</sup> and 1400 L/min/m<sup>2</sup> shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m<sup>2</sup> shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m<sup>2</sup>. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m<sup>2</sup>.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m<sup>2</sup> shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m<sup>2</sup>, and shall be calculated using a simple proportioning formula, with 1400 L/min/m<sup>2</sup> in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m<sup>2</sup>.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

### 3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m<sup>2</sup>.

Stormceptor® EF Sizing Report

**STORMCEPTOR®**

**ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION**

07/15/2022

Province:	Ontario
City:	Elora
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	Ainley Farm Subdivision
Project Number:	411009
Designer Name:	Patricia Wiebe
Designer Company:	GM BluePlan Engineering Ltd.
Designer Email:	patricia.wiebe@gmblueplan.ca
Designer Phone:	519-748-1440
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	SWM Pond 2
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Drainage Area (ha):	2.75
% Imperviousness:	69.00

Runoff Coefficient 'c': 0.71

Particle Size Distribution:	Fine
Target TSS Removal (%):	80.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	61.06
Oil / Fuel Spill Risk Site?	No
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EF4	66
EF6	78
<b>EF8</b>	<b>85</b>
EF10	89
EF12	92

**Recommended Stormceptor EF Model: EF8**  
**Estimated Net Annual Sediment (TSS) Load Reduction (%): 85**  
**Water Quality Runoff Volume Capture (%): > 90**

## Stormceptor® EF Sizing Report

### THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

### PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

### PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor®EF Sizing Report

Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m <sup>2</sup> )	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	2.73	164.0	35.0	100	8.5	8.5
1	20.6	29.1	5.46	328.0	70.0	100	20.6	29.1
2	16.8	45.9	10.92	655.0	139.0	91	15.3	44.4
3	10.8	56.7	16.38	983.0	209.0	83	8.9	53.3
4	8.5	65.2	21.83	1310.0	279.0	80	6.7	60.0
5	6.4	71.6	27.29	1638.0	348.0	77	4.9	64.9
6	5.5	77.0	32.75	1965.0	418.0	74	4.0	69.0
7	3.9	81.0	38.21	2293.0	488.0	73	2.9	71.8
8	2.9	83.9	43.67	2620.0	557.0	72	2.1	73.9
9	2.7	86.5	49.13	2948.0	627.0	71	1.9	75.8
10	2.2	88.7	54.59	3275.0	697.0	70	1.5	77.3
11	1.0	89.7	60.04	3603.0	767.0	70	0.7	78.0
12	1.7	91.3	65.50	3930.0	836.0	69	1.1	79.1
13	1.4	92.8	70.96	4258.0	906.0	68	1.0	80.1
14	1.0	93.7	76.42	4585.0	976.0	68	0.7	80.8
15	0.3	94.0	81.88	4913.0	1045.0	68	0.2	81.0
16	0.8	94.8	87.34	5240.0	1115.0	70	0.6	81.5
17	0.8	95.7	92.80	5568.0	1185.0	71	0.6	82.1
18	0.2	95.8	98.25	5895.0	1254.0	73	0.1	82.2
19	1.5	97.3	103.71	6223.0	1324.0	74	1.1	83.3
20	0.2	97.5	109.17	6550.0	1394.0	75	0.2	83.5
21	0.6	98.2	114.63	6878.0	1463.0	72	0.4	83.9
22	0.0	98.2	120.09	7205.0	1533.0	69	0.0	83.9
23	0.2	98.4	125.55	7533.0	1603.0	66	0.1	84.1
24	0.2	98.6	131.00	7860.0	1672.0	63	0.2	84.2
25	0.2	98.9	136.46	8188.0	1742.0	61	0.1	84.4
30	1.1	100.0	163.76	9825.0	2091.0	50	0.6	85.0
35	0.0	100.0	191.05	11463.0	2439.0	43	0.0	85.0
40	0.0	100.0	218.34	13100.0	2787.0	39	0.0	85.0
45	0.0	100.0	245.63	14738.0	3136.0	34	0.0	85.0
<b>Estimated Net Annual Sediment (TSS) Load Reduction =</b>								<b>85 %</b>

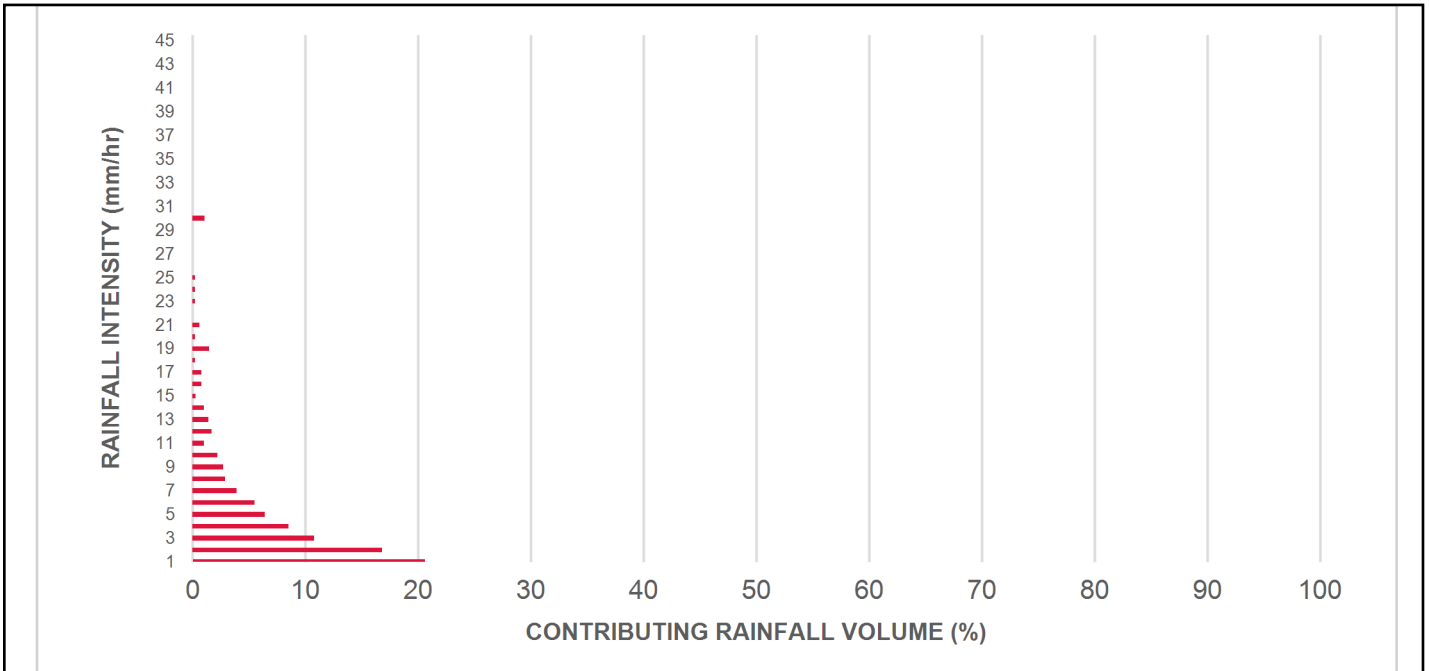
Climate Station ID: 6158731 Years of Rainfall Data: 20



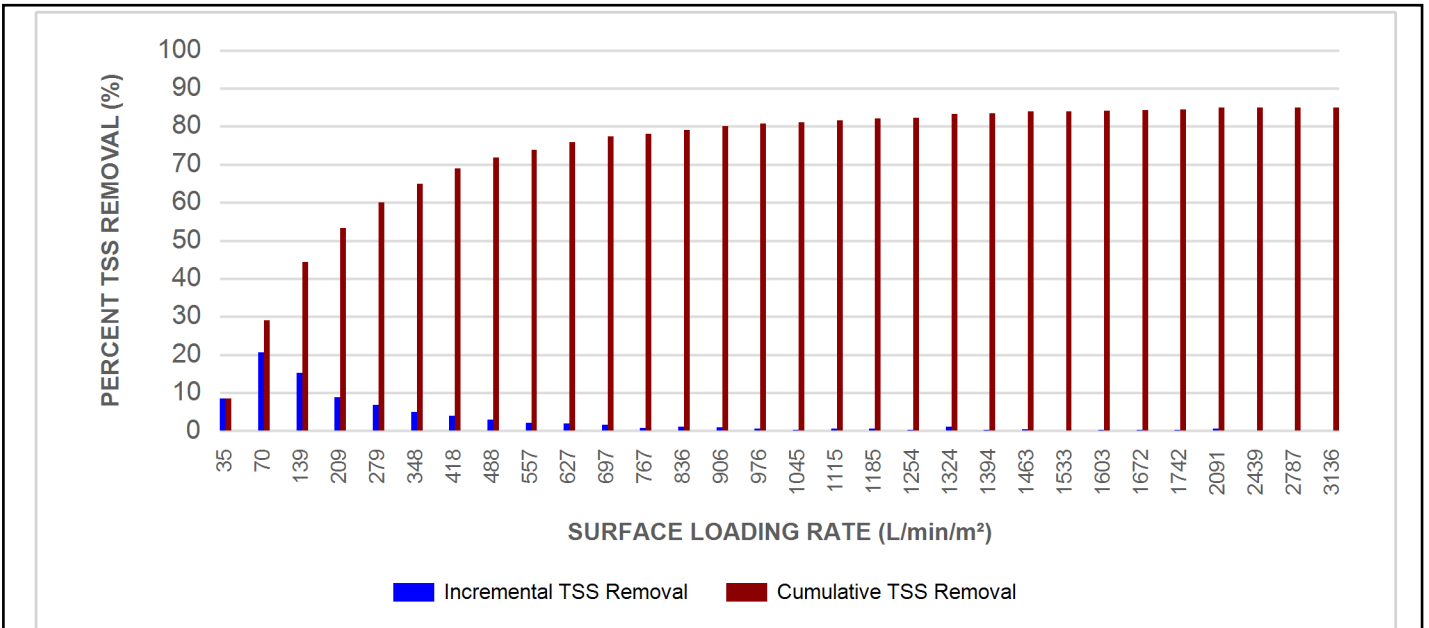


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

**SCOUR PREVENTION AND ONLINE CONFIGURATION**

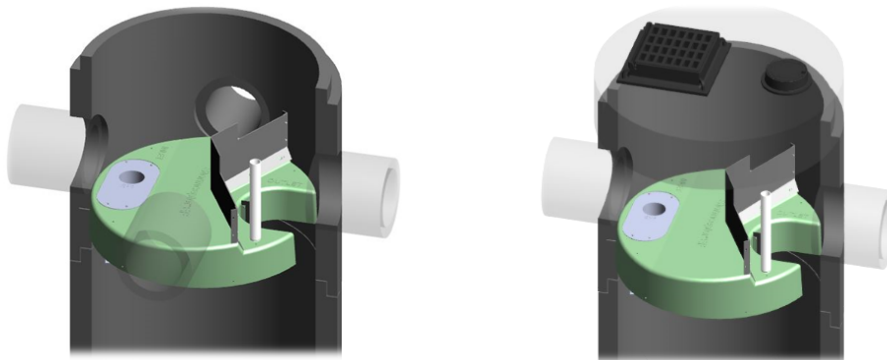
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

**DESIGN FLEXIBILITY**

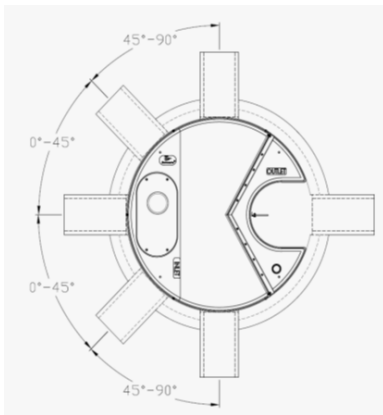
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

**OIL CAPTURE AND RETENTION**

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



## Stormceptor® EF Sizing Report



### INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

### HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

### Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

\*Increased sump depth may be added to increase sediment storage capacity

\*\* Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³ )

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

### STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

### STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR  
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

**PART 1 – GENERAL**

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators.**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

**PART 2 – PRODUCTS**

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The **minimum** sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m <sup>3</sup> sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m <sup>3</sup> sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m <sup>3</sup> sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m <sup>3</sup> sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m <sup>3</sup> sediment / 2,476 L oil

**PART 3 – PERFORMANCE & DESIGN**

3.1 GENERAL



## Stormceptor®EF Sizing Report

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

### 3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m<sup>2</sup> to 1400 L/min/m<sup>2</sup>, and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m<sup>2</sup> and 1400 L/min/m<sup>2</sup> shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m<sup>2</sup> shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m<sup>2</sup>. No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m<sup>2</sup>.

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m<sup>2</sup> shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m<sup>2</sup>, and shall be calculated using a simple proportioning formula, with 1400 L/min/m<sup>2</sup> in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m<sup>2</sup>.

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

### 3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m<sup>2</sup>.



**APPENDIX E**  
**WATER BUDGET ANALYSES**

**Ainley Farm Subdivision**  
**Township of Centre Wellington (Elora)**  
**Monthly Water Balance (Thornthwaite and Mather Method)**

Date: July 2022

**EXISTING CONDITION**

Contributing Catchments:

Contributing Area = 22.62 ha

Percent Impervious = 0% %

All Soil Type: Clay Loam

Vegetation: Shallow-rooted crops

Root Zone Depth = 0.40m

Soil Moisture Retention Capacity = 100mm

Runoff Factor = 0.84

Evapotranspiration Factor for

Impervious Surfaces = 0.36

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily Potential Evapotranspiration	Correction Factors	Adjusted Potential Evapotranspiration	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual Evapotranspiration	Pervious ET - Actual ET	Moisture Deficit	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Actual Runoff	Runoff Volume	Recharge Volume
	(°C)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m <sup>3</sup> )	(m <sup>3</sup> )
Jan	-7.40	0.00	0.00	24.3	0.00	67.90	67.9		236.5	0.0		0.0	0.0	0.0	0.0	9.9	0.0	9.9	8.3	1,887	351
Feb	-6.30	0.00	0.00	24.6	0.00	55.90	55.9		292.4	0.0		0.0	0.0	0.0	0.0	4.9	0.0	4.9	4.2	944	176
Mar	-1.90	0.00	0.00	30.6	0.00	59.60	59.6		352.0	0.0		0.0	0.0	0.0	0.0	2.5	0.0	2.5	2.1	472	88
Apr	5.70	1.22	0.90	33.6	30.24	74.10	43.9		100.0	0.0	30.2	30.2	0.0	0.0	43.9	21.9	25.2	47.1	39.7	8,988	1,673
May	12.20	3.86	2.00	37.8	75.60	86.90	11.3		100.0	0.0	75.6	75.6	0.0	0.0	11.3	16.6	113.4	130.0	109.6	24,795	4,614
Jun	17.50	6.66	2.90	38.4	111.36	83.80	-27.6	-27.6	75.0	-25.0	108.8	108.8	0.0	2.6	-2.6	7.0	56.7	63.7	53.7	12,154	2,262
Jul	20.00	8.16	3.40	38.7	131.58	89.20	-42.4	-69.9	49.0	-26.0	115.2	115.2	0.0	16.4	-16.4	-4.7	28.4	23.7	20.0	4,515	840
Aug	19.00	7.55	3.20	36.0	115.20	96.60	-18.6	-88.5	40.0	-9.0	105.6	105.6	0.0	9.6	-9.6	-7.1	14.2	7.0	5.9	1,342	250
Sep	14.90	5.22	2.50	31.2	78.00	93.10	15.1		55.1	15.1	78.0	78.0	0.0	0.0	0.0	-3.6	7.7	4.1	3.5	788	147
Oct	8.30	2.15	1.30	28.5	37.05	77.20	40.2		95.3	40.2	37.1	37.1	0.0	0.0	0.0	-1.8	4.0	2.2	1.9	423	79
Nov	2.10	0.27	0.30	24.3	7.29	93.00	85.7		100.0	4.8	7.3	7.3	0.0	0.0	81.0	39.6	2.5	42.1	35.5	8,027	1,494
Dec	-3.90	0.00	0.00	23.1	0.00	68.60	68.6		168.6	0.0		0.0	0.0	0.0	0.0	19.8	0.0	19.8	16.7	3,775	702
<b>Total</b>		<b>35.1</b>				<b>945.9</b>	<b>359.6</b>				<b>557.8</b>	<b>557.8</b>		<b>28.5</b>	<b>107.6</b>	<b>105.1</b>	<b>252.0</b>	<b>357.1</b>	<b>301.1</b>	<b>68,109</b>	<b>12,674</b>

Notes: Precipitation and Temperature data from Environment Canada Climate Normals 1981-2010 for Fergus Shand Dam  
Monthly water balance strategy as outlined in the document *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance (Thornthwaite and Mather, 1957)*  
Monthly Heat Index (I) from Table 2 of *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*  
Correction Factors from Table 6 of *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*  
Evaporation Factor for Impervious Surfaces = Average Annual Evapotranspiration for Impervious Surfaces (200mm/year) / Average Annual Evapotranspiration for Pervious Surfaces (558mm/year) = 0.36  
Runoff Factor = [(Impervious Percentage of Site x Average Annual Runoff for Impervious Surfaces (745.9mm/year)) + (Pervious Silt Till Percentage of Site x Average Annual Runoff for Pervious Silt Till Surfaces (301.1 mm/year))] / Total Annual Recharge & Runoff

**Ainley Farm Subdivision**  
**Township of Centre Wellington (Elora)**  
**Monthly Water Balance (Thorntwaite and Mather Method)**  
**Date: July 2022**

**POST-DEVELOPMENT CONDITIONS - TO WETLAND**

Contributing Catchments: All  
 Contributing Area = 22.62 ha  
 Percent Impervious = 36%

Soil Type: Clay Loam  
 Vegetation: Shallow-rooted crops  
 Root Zone Depth = 0.40m  
 Soil Moisture Retention Capacity = 100mm

Runoff Factor = 0.95  
 Evapotranspiration Factor for Impervious = 0.36

Month	Daily Average Temperature	Monthly Heat Index	Unadjusted Daily Potential Evapotranspiration	Correction Factors	Adjusted Potential Evapotranspiration	Average Precipitation	P-PE	Accum. Pot. Water Loss	Storage	ΔS	Pervious ET	Actual Evapotranspiration	Pervious ET - Actual ET	Moisture Deficit	Moisture Surplus	Water Runoff	Snow Melt Runoff	Total Recharge & Runoff	Actual Runoff	Runoff Volume	Recharge Volume	Enhanced Recharge	
	(°C)		(mm)		(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(mm)	(m <sup>3</sup> )	(m <sup>3</sup> )	(m <sup>3</sup> )
Jan	-7.40	0.00	0.00	24.3	0.00	67.90	67.9		236.5	0.0		0.0	0.0	0.0	0.0	11.9	0.0	11.9	11.3	2,381	135	184	
Feb	-6.30	0.00	0.00	24.6	0.00	55.90	55.9		292.4	0.0		0.0	0.0	0.0	0.0	6.0	0.0	6.0	5.7	1,190	67	92	
Mar	-1.90	0.00	0.00	30.6	0.00	59.60	59.6		352.0	0.0		0.0	0.0	0.0	0.0	3.0	0.0	3.0	2.8	595	34	46	
Apr	5.70	1.22	0.90	33.6	30.24	74.10	43.9		100.0	0.0	30.2	23.3	7.0	7.0	50.8	25.4	25.2	50.6	48.1	10,095	571	785	
May	12.20	3.86	2.00	37.8	75.60	86.90	11.3		100.0	0.0	75.6	58.1	17.5	17.5	28.8	27.1	113.4	140.5	133.5	28,773	1,584	1,421	
Jun	17.50	6.66	2.90	38.4	111.36	83.80	-27.6	-27.6	75.0	-25.0	108.8	83.7	25.1	27.7	22.6	24.8	56.7	81.5	77.5	16,455	919	1,067	
Jul	20.00	8.16	3.40	38.7	131.58	89.20	-42.4	-69.9	49.0	-26.0	115.2	88.6	26.6	43.0	10.2	17.5	28.4	45.9	43.6	9,165	517	695	
Aug	19.00	7.55	3.20	36.0	115.20	96.60	-18.6	-88.5	40.0	-9.0	105.6	81.2	24.4	34.0	14.8	16.2	14.2	30.3	28.8	6,066	342	452	
Sep	14.90	5.22	2.50	31.2	78.00	93.10	15.1		55.1	15.1	78.0	60.0	18.0	18.0	18.0	17.1	7.7	24.8	23.5	4,958	279	368	
Oct	8.30	2.15	1.30	28.5	37.05	77.20	40.2		95.3	40.2	37.1	28.5	8.6	8.6	8.6	12.8	4.0	16.8	16.0	3,366	190	249	
Nov	2.10	0.27	0.30	24.3	7.29	93.00	85.7		100.0	4.8	7.3	5.6	1.7	1.7	82.6	47.7	2.5	50.2	47.7	10,021	566	775	
Dec	-3.90	0.00	0.00	23.1	0.00	68.60	68.6		168.6	0.0		0.0	0.0	0.0	0.0	23.9	0.0	23.9	22.7	4,762	269	368	
<b>Total</b>		<b>35.1</b>				<b>945.9</b>	<b>359.6</b>				<b>557.8</b>	<b>429.0</b>	<b>128.8</b>	<b>157.3</b>	<b>236.4</b>	<b>233.4</b>	<b>252.0</b>	<b>485.4</b>	<b>461.2</b>	<b>97,828</b>	<b>5,473</b>	<b>6,502</b>	
<b>Total Recharge Volume</b>																					<b>11,974</b>		
<b>Total Enhanced Recharge Surplus (post-development volume - pre-development volume)</b>																					<b>-700</b>		

Notes: Precipitation and Temperature data from Environment Canada Climate Normals 1981-2010 for Fergus Shand Dam  
 Monthly water balance strategy as outlined in the document *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance (Thorntwaite and Mather, 1957)*  
 Monthly Heat Index (I) from Table 2 of *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*  
 Correction Factors from Table 6 of *Instructions and Tables for Computing Potential Evapotranspiration and the Water Balance*  
 Evaporation Factor for Impervious Surfaces = Average Annual Evapotranspiration for Impervious Surfaces (200mm/year) / Average Annual Evapotranspiration for Pervious Surfaces (558mm/year) = 0.36  
 Runoff Factor = [(Impervious Percentage of Site x Average Annual Runoff for Impervious Surfaces (745.9mm/year)) + (Pervious Silt Till Percentage of Site x Average Annual Runoff for Pervious Silt Till Surfaces (301.1 mm/year))] / Total Annual Recharge &



Ainley Farm Subdivision  
Township of Centre Wellington (Elora)  
Monthly Water Balance (Thorntwaite and Mather Method)  
Date: July 2022

**Catchment 1200 - Stormwater Management Pond 1 Infiltration Gallery**

**Infiltration Gallery No. 1**

Area = 540 m<sup>2</sup>  
Depth= 0.70 m  
Perforated Pipe Diameter= 0.450 m  
No. of Pipes= 4 - 6

Volume of Perforated Pipe = 100.52 cu m  
Volume of Clear Stone = 277.41 cu m  
Clear Stone Void Ratio= 0.33

**Total Storage Volume of Structure = 192.06 cu m**

A = contact area of structure = 540 sq m  
V = runoff volume to be infiltrated = 192.06 cu m  
P = percolation rate of native soils = 4.00 mm/h  
n = porosity of storage media (weighted) = 0.51  
T = retention time = Solve for T

T = (1000 x V) / (P x n x A) = 175.00 hours or 7.3 day draindown period

Contributing Area 1.14 ha  
Recharge Time 175.0 hours / 7.3 days  
Recharge Volume Potential 192.06 m<sup>3</sup>

Month	Total Runoff from Contributing Area (mm)	No. of days	Max Potential Recharge (m <sup>3</sup> )	Available Recharge (m <sup>3</sup> )	Enhanced Recharge (m <sup>3</sup> )
Jan	12.4	31	817	142	135
Feb	6.2	28	738	71	67
Mar	3.1	31	817	35	34
Apr	50.8	30	790	579	551
May	141.4	31	817	1,612	776
Jun	86.5	30	790	986	751
Jul	53.3	31	817	608	577
Aug	38.5	31	817	439	417
Sep	32.1	30	790	366	348
Oct	22.0	31	817	251	238
Nov	52.2	30	790	595	566
Dec	24.9	31	817	284	270
<b>Total</b>	<b>523.5</b>	<b>365</b>	<b>9,614</b>	<b>5,968</b>	<b>4,728</b>

**Ainley Farm Subdivision**  
**Township of Centre Wellington (Elora)**  
**Monthly Water Balance (Thorntwaite and Mather Method)**  
**Date: July 2022**

**Catchment 1400 - Park Infiltration Gallery**

**Infiltration Gallery No. 3**

Length = 79.05 m  
 Width = 10.00 m  
 Depth= 0.70 m  
 Perforated Pipe Diameter = 0.30 m  
 No. of Pipes= 1.00

Area of Material = 790.50 sq m

Volume of Perf. Pipe = 11.03  
 Volume of Clear Stone = 542.32 cu m  
 Clear Stone Void Ratio= 0.33

**Total Storage Volume of Structure = 191.80 cu m**

A = contact area of structure = 790.50 sq m  
 V = runoff volume to be infiltrated = 191.80 cu m  
 P = percolation rate of native soils = 4.00 mm/h  
 n = porosity of storage media (weighted) = 0.34  
 T = retention time = Solve for T

$T = (1000 \times V) / (P \times n \times A) = 176.67$  hours or 7.4 day draindown period

Contributing Area 0.62 ha  
 Recharge Time 176.7 hours / 7.4 days  
 Recharge Volume Potential 191.80 m<sup>3</sup>

Month	Total Runoff from Contributing Area (mm)	No. of days	Max Potential Recharge (m <sup>3</sup> )	Available Recharge (m <sup>3</sup> )	Enhanced Recharge (m <sup>3</sup> )
Jan	8.3	31	808	52	49
Feb	4.2	28	730	26	25
Mar	2.1	31	808	13	12
Apr	39.7	30	782	246	234
May	109.6	31	808	680	646
Jun	53.7	30	782	333	316
Jul	20.0	31	808	124	118
Aug	5.9	31	808	37	35
Sep	3.5	30	782	22	21
Oct	1.9	31	808	12	11
Nov	35.5	30	782	220	209
Dec	16.7	31	808	103	98
<b>Total</b>	<b>301.1</b>	<b>365</b>	<b>9,511</b>	<b>1,867</b>	<b>1,773</b>