



961 St. David Street North

Functional Servicing and Stormwater Management Report

Project Location:

961 St. David Street North, Fergus, ON

Prepared for:

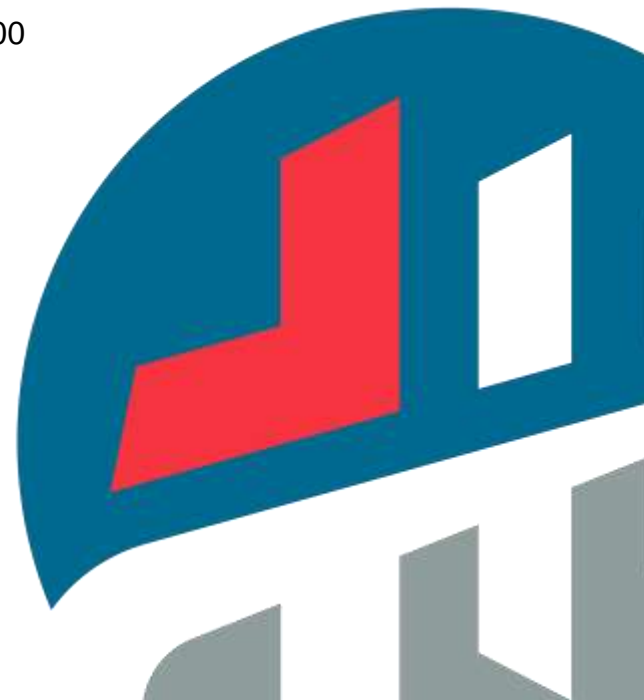
2687734 Ontario Inc.
766 Hespeler Road
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Prepared by:

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July 15, 2022

MTE File No.: 48650-100





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MTE Drawing No. C2.1	Appended Separately
Functional Site Servicing Plan	
MTE Drawing No. C2.2	Appended Separately

1.0 Introduction

MTE Consultants Inc. was retained by 2687734 Ontario Inc. to complete a Functional Servicing and Stormwater Management (FSSWM) Report for the in support of the Zoning By-Law Amendment and Vacant Lot Condominium applications for the proposed residential development to be constructed at 961 St. David Street North / King's Highway 6 (herein referred to as 'the Site') in the Township of Centre Wellington.

The purpose of this study is to support the Zoning By-Law Amendment and Vacant Land Condominium applications. This will be accomplished by reviewing the opportunities and constraints for the subject property with respect to servicing, grading, and stormwater management; reviewing the requirements of the reviewing agencies; describing the development concept; and demonstrating the functional serviceability of the property. Pending approval of the applications, detailed design of the Site will commence and be submitted to the Township of Centre Wellington and other governing agencies in support of the applications and Site Plan Approval.

1.1 Site Description

The Site encompasses an area of 1.402ha, and is currently comprised of a single detached dwelling, several accessory buildings, and an asphalt driveway off of 961 St. David Street North / King's Highway 6. The property is bounded to the west, south, and east by existing residential development, and to the northeast by St. David Street North / King's Highway 6. For the exact location of the Site refer to Figure 1.0.

The current zoning of the Site is Residential R1a Zone. A Zoning By-Law Amendment will be required to re-zone the Site to Residential R3 Zone to permit the proposed vacant land condominium development.

1.2 Proposed Development

The proposed development for the Site is a vacant land condominium, consisting of 13 single-detached houses, and 37 townhouse units, complete with a common element roadway and driveway entrance off of St. David Street North / King's Highway 6.



FIGURE 1.0

Date: MAR.16/22
Scale: 1:1500

**961 ST. DAVID ST. NORTH
LOCATION PLAN
FERGUS, ONTARIO**



Engineers, Scientists, Surveyors

Project No.: 48650-100

2.0 Existing Conditions

2.1 Topography

Existing topographic information for the Site was obtained from a detailed survey completed by MTE in February 2021. The Site's frontage along St. David Street North / King's Highway 6 has an approximately slope of 2% from west to east. There is an existing ditch along the south side of the highway along the Site's frontage, with an approximate slope of 1% from west to east. The road elevation is approximately 0.8m to 1.0m above the property line elevation along the Site's frontage. The northeast end of the Site is sloped towards St. David Street North / King's Highway 6, with high point elevations of 424.5mASL around the existing buildings, and a low point elevation of approximately 422.4mASL at the northeast corner of the Site. A portion of the middle of the property has an approximate slope of 1.5% towards the southeast property line, with a highpoint elevation of 424.5mASL near the center of the property, and a low point elevation of approximately 423.6mASL along the southeast property line. The majority of the southwest end of the Site is sloped towards the southwest property line, with a high point elevation of 424.5mASL near the approximate center of the property, and a low point elevation of 422.6mASL along the southwest property line.

2.2 Existing Servicing

The Site currently does not have any existing municipal water, storm, or sanitary services to the site. Existing servicing information within the surrounding right-of-ways was obtained from the topographic survey, and from plan and profile information provided by the Township of Centre Wellington.

2.2.1 Water

There is currently an existing 300mm diameter watermain on the far (north) side of St. David Street North / King's Highway 6. There are two existing municipal fire hydrants located on the north side of St. David Street North / King's Highway 6 near the Site. One is located approximately 94m to the north of the existing driveway entrance, and one is located approximately 69m to the south of the existing driveway entrance. A flow test was performed at the aforementioned hydrants by Classic Fire and Life Safety on June 21, 2022. Refer to Table 2.1 below for a summary of the flow test results.

Table 2.1 – Results of Flow Tests

Results of Flow Tests Completed June 21, 2022 by Classic Fire & Life Safety					
Test #	Outlet Inside Diameter (in.)	Number of Outlets	Pitot Pressure (PSI)	Residual Pressure (PSI)	Flow @ Residual (gal/min)
1	n/a	n/a	n/a	46	0
2	2.5	1	20	40	751
3	2.5	2	12 + 14	38	1,209

Refer to Appendix A for further details.

2.2.2 Sanitary

There is an existing 300mm diameter municipal sanitary sewer along the far (north) side of St. David Street North / King's Highway 6 which drains southeast, at a depth of approximately 5.7m to 6.6m below the centreline elevation of the road.

2.2.3 Storm

There is an existing 500mm diameter Corrugated Metal Pipe (CMP) culvert underneath the existing driveway entrance, draining southeast in the ditch along the Site's frontage with St. David Street North / King's Highway 6. The culvert underneath is at a depth of approximately 1m below the existing grade of the driveway.

2.3 Existing Soils Information

A geotechnical investigation was not available for this development at the time this report was published. However, a geotechnical investigation was completed by Golder Associates Ltd. for trenchless servicing crossing work underneath St. David Street North / King's Highway 6 that is proposed for the Site. This servicing work is detailed further in Section 3.2 of this report.

The geotechnical investigation by Golder Associates was completed in April 2021. Two boreholes were advanced to depths between 8.2m and 9.8m below existing grade to determine the underlying soil conditions at the proposed servicing crossing location. Borehole 20-1 was located near the northern corner of the Site, and Borehole 20-2 was located on the north edge of St. David Street North / King's Highway 6. The borehole logs revealed that the subsurface soil conditions are generally comprised of silty sand and sand deposits, underlain with a layer clayey silt.

At the time of borehole advancement, the groundwater level in Borehole 20-1 was measured at approximately 418.3mASL, and Borehole 20-2 was observed to be dry. However, these water levels were not considered to be representative of the stabilized groundwater conditions. Based on a previous geotechnical investigation completed nearby by LVM Inc. (2013), Golder determined that the groundwater level at the proposed servicing crossing location is expected to be approximately 422.5mASL.

2.4 Reviewing Agencies

2.4.1 Township of Centre Wellington

Functional grading, servicing and stormwater management designs as well as this Functional Servicing and Stormwater Management Report will be required for submission to the Township of Centre Wellington in support of the Zoning By-Law Amendment and Vacant Land Condominium applications. The Township will also be responsible for the review and approval of site plans, site grading, servicing, stormwater management, lighting and landscape design and ultimately issuing building permits.

2.4.2 Ministry of Transportation

St. David Street North is a part of Kings Highway 6, a provincially maintained highway. As such, the Ministry of Transportation (MTO) will be circulated on the Zoning By-Law Amendment and Site Plan Applications and will need to approve the site grading, servicing and stormwater management designs. An entrance permit will need to be obtained from the MTO for the new entrance off of St. David Street North / King's Highway 6.

2.4.3 Grand River Conservation Authority

The Site also falls within the area regulated by the Grand River Conservation Authority (GRCA). As such, the site engineering design will also be submitted to the GRCA for review and approval.

3.0 Proposed Grading and Servicing Strategy

Preliminary grading and servicing strategies have been developed based on the topographic survey, plan and profile information, and the Concept Plan provided by MHBC Ltd., dated May 6, 2022. Refer to the enclosed MTE Drawings C2.1 and C2.2 for details. These grading and servicing strategies will be further refined during detailed design during the Site Plan Approval process.

3.1 Proposed Grading

The proposed vacant land condominium development consists of 13 single-detached houses, and 37 townhouse units, complete with a common element roadway and driveway entrance off of St. David Street North / King's Highway 6. The proposed grading strategy will respect the existing grades along St. David Street North / King's Highway 6, and all other property boundaries. The majority of grading internal to the Site will involve directing stormwater runoff to the existing ditch along the south side of St. David Street North / King's Highway 6. Stormwater runoff will be conveyed to the existing ditch via an on-site storm sewer system, and via a rear-yard swale along the southeast property line. For the units along the south end of the Site, runoff from the rear of the units will continue to drain to the southwest property line at existing runoff rates. Refer to MTE Drawing C2.1 for an illustration of the functional grading design.

3.2 Proposed Servicing

The following sections provide details regarding the preliminary proposed water, sanitary and storm servicing for the proposed development. Refer to MTE Drawing C2.2 for an illustration of the functional servicing design.

Connection to the existing 300mm diameter municipal watermain and existing 300mm diameter municipal sanitary sewer along the north side of St. David Street North / King's Highway 6 will require trenchless installation of the services underneath the road. Detailed design and required supporting investigations have already been completed for the trenchless servicing crossing work, which have been accepted by the Township and the MTO. The proposed water service and the proposed sanitary service will both be 150mm diameter. Each will be extended from their respective existing mains to approximately 2m inside of the Site's property line. The proposed trenchless servicing crossing work is anticipated to be constructed in summer 2022.

3.2.1 Water

A 150mm diameter water service connection to the existing 300mm diameter municipal watermain along the north side of St. David Street North / King's Highway 6 will be required in order to service the proposed development. As previously described, this service will be brought underneath of St. David Street North / King's Highway 6 via trenchless installation. The 150mm diameter watermain will then be upsized to 250mm diameter at the property line, and extended in the common element roadway to service each unit. Each lot will be serviced off the proposed watermain with a 25mm diameter domestic water service connection. It is anticipated that two

on-site fire hydrants will also be required for the proposed development, as shown on the enclosed MTE Drawing C2.2.

Water Demand

Various guidelines and references exist for calculating the required water supply for firefighting purposes. In Ontario, there are two standards/guidelines that are most often referenced:

- Ontario Building Code (OBC) – provincial codes and guidelines published by the Ministry of Municipal Affairs and Housing for the Province of Ontario; and
- The Fire Underwriters Survey (FUS) – an insurance industry guideline.

Many municipalities in Ontario use both the OBC and the FUS fire flow requirements for assessing firefighting water supply requirements. Ideally, fire flow demands for new developments are calculated based on the FUS criteria; however, it is not always reasonable to expect that the local existing municipal infrastructure has the operational capacity to supply water at the rates prescribed in the FUS guidelines. As a result, at no time shall the available fire flow be less than that required by the Ontario Building Code.

The pressures and flows at the proposed private hydrant must be sufficient for firefighting conditions as established by the Ontario Building Code (2012). The minimum residual pressure permitted under firefighting conditions is 140.0kPa (20.3psi) per OBC 2012 A-3.2.5.7 3(b).

The buildings are proposed to be of wood frame construction (combustible construction). For the purposes of this analysis, the worst case scenario was determined to be the future townhouse building located on Units 28-32. This building has the largest allowable footprint, and in a firefighting scenario the hydrant furthest from the connection the municipal main will be utilized. The OBC and FUS requirements were calculated for this worst case scenario for reference and are shown in Table 3.1. Refer to Appendix A for detailed calculations.

Table 3.1 – Required Fire Flow

Building	OBC (L/s)	FUS (L/s)
Proposed Building	4,500	17,000

For this design, pressure and flow information for the existing 300mm diameter municipal watermain in the St. David Street North / King’s Highway 6 right-of-way was taken from the hydrant flow testing results as detailed in Section 2.2.1.

The minimum water supply flow rate for the proposed development is 283.0L/s (17,000L/min) based on the calculation method specified in the FUS. The residual pressures at the proposed hydrants are less than the minimum allowable pressure of 140kPa based on the FUS calculation method.

The minimum water supply flow rate for the proposed development is 75.0L/s (4,500L/min) based on the calculation method specified by the OBC. The residual pressure at the proposed hydrant was calculated to be 185kPa at a flow rate of 75.0L/s (4,500L/min), which is greater than the minimum allowable pressure of 140kPa per OBC 2012. Therefore, the proposed watermain configuration is expected to be sufficient. Based on a maximum day domestic demand of 0.93L/s and the fire flow demand of 75.0L/s, the total water demand for the Site is expected to be 75.9L/s.

The fire flow demand calculations completed in this report assume that the proposed buildings will have two above grade stories. Should the buildings increase to a height of 3-storeys as design progresses, fire walls may be required in order to meet previously described pressure and flow requirements set forth by the OBC.

3.2.2 Sanitary

A sanitary flow design sheet has been prepared to determine the flows anticipated to be generated by the proposed development. With the 13 units proposed for detached homes and 37 units proposed for townhouses, the resulting peak flow rate from the Site is expected to be 3.11L/s. Refer to Appendix A for the Sanitary Flow Design Sheet.

As previously described, a 150mm diameter sanitary service will be extended from the existing 300mm sanitary main on the north side of St. David Street North / King's Highway 6 to a private manhole just inside the Site property line. A 150mm diameter private sanitary sewer will be extended from this manhole throughout the common element area of the Site, servicing each of the proposed units.

The proposed 150mm diameter sanitary sewer has a minimum design slope of slope of 1.00%, corresponding to a full flow pipe capacity of 15.22/s. Therefore, the proposed 150mm diameter sanitary sewer has sufficient capacity for this development.

3.2.3 Storm

A private storm sewer system is proposed along the common element roadways within the proposed development. The storm sewer system, which will include several catchbasins, manholes, and catchbasin manholes, will convey runoff from the roadways, driveways, and landscaped areas through an OGS before discharging to the existing ditch along the south side of St. David Street North / King's Highway 6. A storm sewer design sheet for the last pipe run from the Site is included in Appendix A. The major overland flow route for the Site will be to the existing ditch along St. David Street North / King's Highway 6.

Specifications and details of these proposed servicing strategies will be further refined during detailed design.

4.0 Preliminary Storm Water Management Design

4.1 SWM Criteria

In the existing condition, stormwater runoff from the eastern end of the Site is directed to the existing ditch along St. David Street North / King's Highway 6. Stormwater runoff from the western end of the Site is directed to the southwest property line, where it subsequently sheet flows across the adjacent property before reaching Municipal Drain No. 4. A central portion of the Site drains to the adjacent property to the south.

The stormwater management design criteria for the Site, as established by the MTO and the Township of Centre Wellington, are as follows:

- i) Attenuation of the post-development peak flows for the 2-, 5-, 10-, 25-, 50-, and 100-year storm events to the pre-development (existing) peak flows;
- ii) Implementation of Enhanced (Level 1) water quality controls; and
- iii) Implementation of Erosion and Sediment Control Measures.

4.2 Water Quantity Control

In order to successfully complete the preliminary stormwater management design for the Site, the following specific tasks were undertaken:

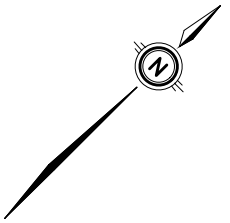
- i) Calculate the allowable runoff rates using MIDUSS NET;
- ii) Determine the percent impervious of the Site and catchment parameters for inclusion in MIDUSS NET modelling; and
- iii) Calculate post-development runoff hydrographs using MIDUSS NET.

The following table summarizes the catchments used in modelling the Site. The pre-development condition was separated into three catchments areas: the western portion of the Site that drains to the southwest property line, the central portion of the Site that drains to an adjacent property to the south, and the eastern portion of the Site that drains to St. David Street North / King's Highway 6. The post-development condition was separated into four catchment areas: the uncontrolled area directed to the southwest property line, the controlled area directed to St. David Street North / King's Highway 6, the uncontrolled area directed to rear-yard amended topsoil and subsequently to St. David Street North / King's Highway 6, and the uncontrolled area directed to St. David Street North / King's Highway 6. Figure 2.0 illustrates the limits of the pre-development catchment areas. Figure 3.0 illustrates the limits of the post-development catchment areas.




Table 4.1 – Catchment Parameters

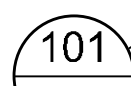
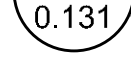
#	Catchment	Area (ha)	% Impervious	Pervious CN	Impervious CN	Slope (%)	Flow Length (m)
Pre-Development Catchment Areas							
101	Area to Southwest Property Line	0.734	0%	75	98	2.0	100
102	Area to Adjacent Property to the South	0.286	0%	75	98	1.0	40
103	Area to Highway 6	0.382	14%	75	98	5.0	70
Post-Development Catchment Areas							
201	Uncontrolled Area to Southwest Property Line	0.228	55%	75	98	2.0	15
202	Controlled Area to Highway 6	0.773	75%	75	98	2.0	15
203	Uncontrolled Area to Rear Yard Amended Topsoil, and Subsequently to Highway 6	0.352	25%	75	98	2.0	120
204	Uncontrolled Area to Highway 6	0.049	0%	75	98	2.0	20

As previously described, a geotechnical investigation was completed for the nearby proposed trenchless servicing work development by Golder. Based on the observed subsurface soil conditions, a conservative value of 75 was used for the pervious CN.



LEGEND

-  CATCHMENT 101
-  CATCHMENT 102
-  CATCHMENT 103

-  SUB-CATCHMENT NUMBER
-  AREA (ha.)

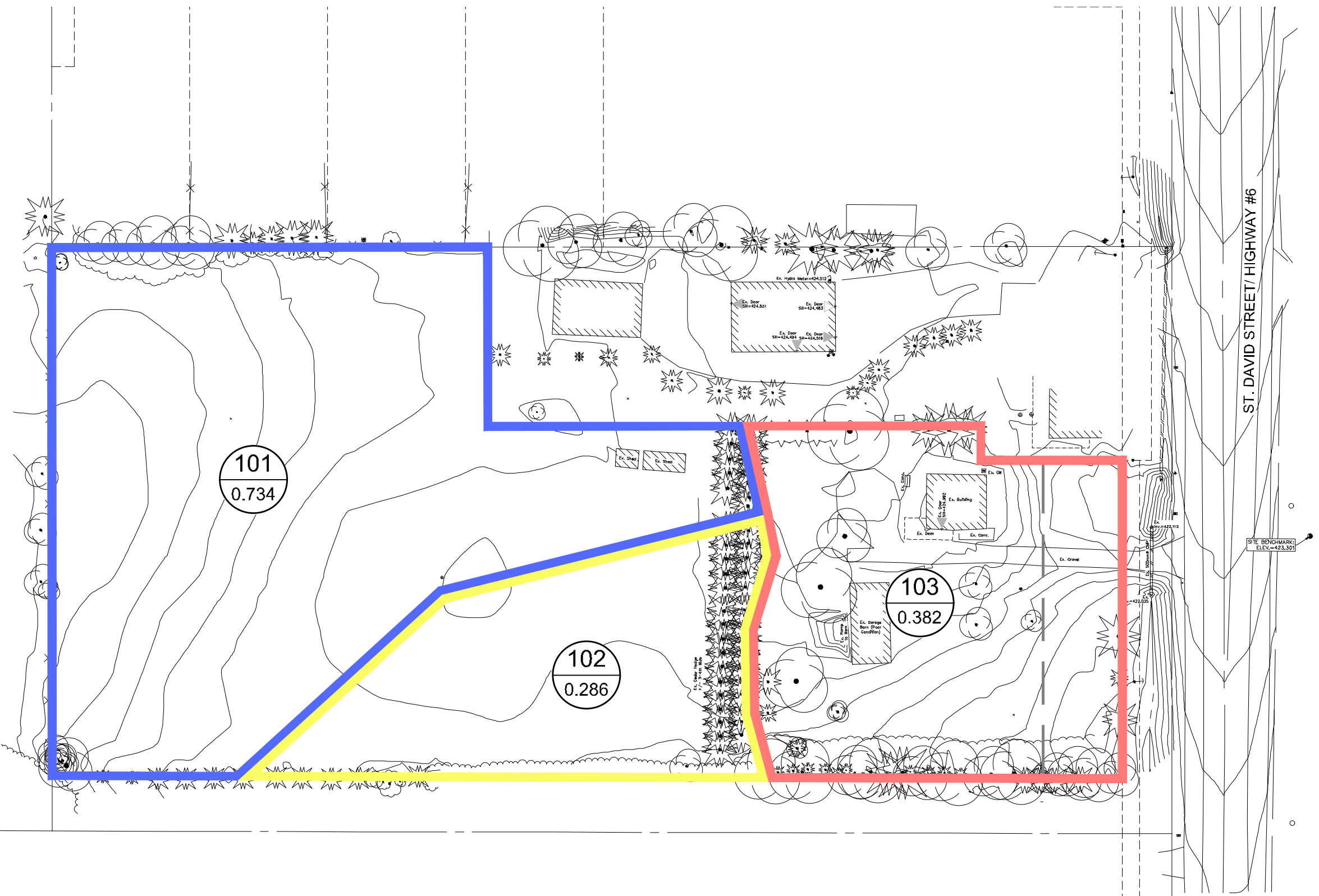


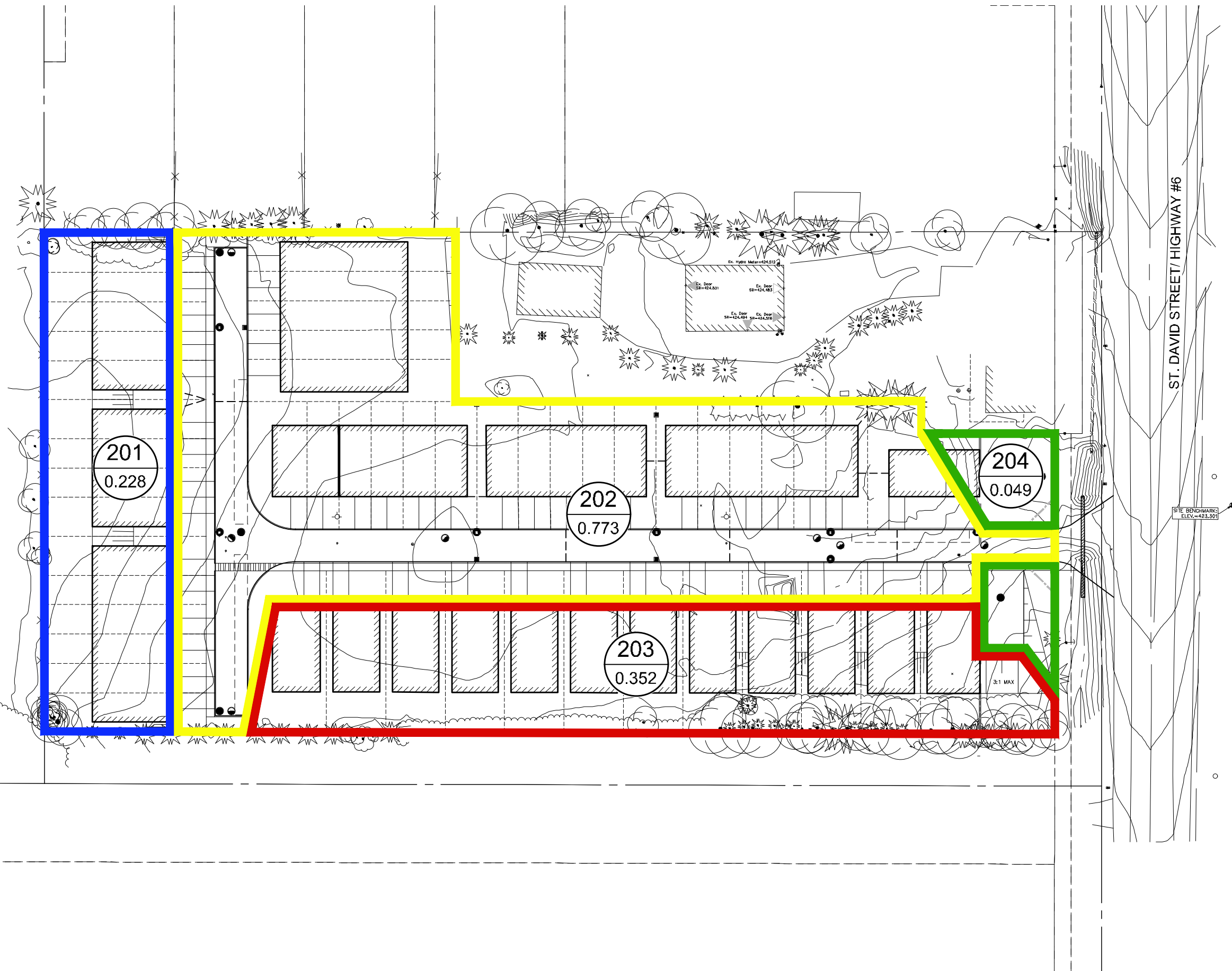
FIGURE 2.0 Date: JUN.10/22 Scale: 1:750

PRE-DEVELOPMENT CATCHMENT AREAS



Engineers, Scientists, Surveyors

Project No.: 48650-100



LEGEND

- CATCHMENT 201
- CATCHMENT 202
- CATCHMENT 203
- CATCHMENT 204

- 101 SUB-CATCHMENT NUMBER
- 0.131 AREA (ha.)

FIGURE 3.0 Date: JUN.10/22
Scale: 1:750

**POST DEVELOPMENT
CATCHMENT AREAS**



Engineers, Scientists, Surveyors

Project No.: 48650-100

In order to achieve the stormwater requirements for the Site, runoff generated within Catchment's 201 and 203 will be directed to a 3.5m wide, 0.5m deep layer of amended topsoil in the rear yards of the units. The amended topsoil will allow for increased infiltration and runoff volume reduction in these areas. Runoff generated in Catchment 202 will be directed controlled with a 75mm orifice tube. Storage volume will be provided by three underground storage tanks, and by surface ponding on the surface of the common element driveway. The following table summarizes the flows generated by the Site for each storm event. The post-development flow rates are subject to change during detailed design.

Table 4.2 – Summary of Flows (MIDUSS Modelling)

Modeling Condition	Pre-Development (m ³ /s)			Post-Development (m ³ /s)	
	Area to Southwest Property Line	Area to Adjacent Property to the South	Area to Highway 6	Area to Southwest Property Line	Area to Highway 6
2-Year Storm Event	0.004	0.002	0.011	0.000	0.016
5-Year Storm Event	0.011	0.006	0.016	0.000	0.018
10-Year Storm Event	0.018	0.009	0.020	0.000	0.020
25-Year Storm Event	0.029	0.014	0.026	0.004	0.022
50-Year Storm Event	0.038	0.019	0.032	0.009	0.023
100-Year Storm Event	0.052	0.024	0.040	0.019	0.025

With the installation of the orifice tube, the post-development runoff from the controlled portion of the Site for the 2-, 10-, 25-, 50- and 100-year storm events is controlled to 0.016m³/s, 0.017m³/s, 0.018m³/s, 0.019m³/s, 0.021m³/s, and 0.022m³/s, respectively.

The total post-development peak flows to St. David Street North / King's Highway 6 exceed pre-development levels for the 2- and 5-year storm events; however, a 75mm orifice tube is considered to be the minimum acceptable diameter, therefore additional reduction of the post-development peak flows is not considered to be feasible.

The maximum ponding depth in the common element driveway is 0.13m for the 100-year storm event. As above, the ponding values are subject to change at detailed design. Please refer to Appendix B for the MIDUSS outputs.

4.3 Water Quality Control

A Stormceptor Model EFO4 will be installed on the storm sewer system to provide water quality control for the Site. The chosen unit is expected to provide Enhanced (Level 1) water quality control. Refer to Appendix C for the sizing output from the Stormceptor Expert program. The Stormceptor will require regular annual maintenance to ensure it is operating properly. The owner may be required to enter into a maintenance agreement with a suitable contractor to complete this work. In addition, all the storm structures will have a 600mm sump.

4.4 Erosion and Sedimentation Control

Precautions will need to be taken during construction to limit erosion and sedimentation. Typically, the following measures are recommended during construction for erosion and sedimentation control:

- i) Erosion and sedimentation facilities are to be installed prior to any area grading operations;
- ii) All erosion control measures are to be inspected and monitored by the contractor and repairs are to be completed as required;
- iii) All materials and equipment used for the purpose of site preparation and project completion should be operated and stored in a manner that prevents any deleterious substance from leaving the site; and
- iv) To minimize the amount of mud being tracked onto the roadway, a mud mat should be installed at the primary construction entrance.

5.0 Conclusions

Based on the foregoing, it is concluded that:

- i) Municipal infrastructure for water and sanitary services is available along St. David Street North / King's Highway 6;
- ii) A fire flow analysis has been completed and demonstrates that adequate flow and pressure is available from the proposed on-site hydrants;
- iii) The water supply needs of the proposed development will not exceed what is available in the existing municipal water distribution system;
- iv) The proposed grading design will respect the natural topography of the Site to achieve a reasonable cut/fill balance where possible and match into existing grades along all property boundaries; and
- v) The SWM criteria can be satisfied with the implementation of on-site controls for water quantity and water quality.

Detailed grading and servicing designs and a detailed stormwater management design will be provided during detailed design in support of Site Plan Approval and Building Permits.

All of which is respectfully submitted,

MTE Consultants Inc.



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Appendix A

Design Sheets

PROJECT INFORMATION			
Project Name:	Fergus Flow Test	Const. Project #:	22-CAM-690-0568
Site Address:	961 St. David Street/Highway 6 Fergus	Design Project #:	2022-CFLS-359
City Contact:	Mike Mullen	Phone #:	519-501-7252
CFLS Contact:	Dean Wanders	Phone #:	905-514-7417
Technical Contact:	Andy Coghlin	Phone #:	519-476-0761

SITE INFORMATION

SITE MAP



Note: If the main is a dead end, the flowing hydrant shall be closest to the dead end

ITEMS TO LABEL ON MAP	HYDRANTS USED	MAIN SIZE
<input checked="" type="checkbox"/> Static / Residual & Flow Hydrants	<input checked="" type="checkbox"/> City Hydrant(s)	City: 6"
<input type="checkbox"/> Flow Direction (if the main is dead end)	<input type="checkbox"/> Site Hydrant(s)	Site:

SITE NOTES



FIRE +
LIFE
SAFETY

FLOW TEST REPORT

Form SD-003B RevDate: Nov 29, 2021

TEST INFORMATION

Minimum Required Flow:	NA	Min Ports:	2
CFLS Personnel Present:	Dean Wanders	Test Date:	2022-06-21
City / External Company:	Infrastructure Services	Test Time:	1:00pm

TEST EQUIPMENT

<input type="checkbox"/> Hose Monsters with built in Pitot	Hose length used:
<input type="checkbox"/> Hand held pitot gauge	<input checked="" type="checkbox"/> Pollard diffuser elbow with built in Pitot
<input type="checkbox"/> Other:	

TEST RESULTS

Number of Ports	Outlet Size (IN)	Discharge Coefficient	Pitot Reading (PSI)			Total Flow (GPM)	Static / Residual Pressure (PSI)
0 Ports	STATIC						46
1 Port	2.5	0.9	20			751	40
2 Ports	2.5	0.9	12	14		1,209	38
3 Ports	2.5	0.9				0	
4 Ports	2.5	0.9				0	
0 Ports	STATIC RE-CHECK						46

TEST NOTES

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HYDRAULIC ADJUSTMENTS (FOR OFFICE USE ONLY)

ADJUSTMENTS FOR HYDRAULIC GRADE LINE (HGL)

Reservoir HGL (m):		Site Elevation (m):	
Theoretical Static Head (PSI):	0	PSI to subtract from test pressures:	46

OTHER HYDRAULIC ADJUSTMENTS

Other adjustment as required by the City / AHJ:	
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**961 St. David Street
FIRE FLOW DEMANDS**

Kitchener, Ontario

Project #: 48650-100

Date: July 14, 2022

Date Printed: 7/14/2022

By: NGK

Development Information ¹								Fire Flow ²														Domestic Flow ^{3,4}						
								Ontario Building Code				Fire Underwriters Survey																
Node ID / Area ID / Building #	F.F.E. (m.a.s.l.)	Description	# of Units	Population # of people	Bldg Area (1 st Floor) m ²	Total Bldg Area m ²	Building Volume m ³	K	V	S _{tot}	Q	F	F	C	A	F	(2) Occupancy Reduction	(3) Sprinkler Protection	(4) Building Exposure	F	F	Fire Flow (Max OBC/FUS) L/s	MOE Guidelines L/s	Average Day L/s	Max Day L/s	Peak Hour L/s	Minimum Hour L/s	Max Day + Fire Flow L/s
Units 1-12, 50		Detached Houses	13	42	135	270	1,316	23	1,316	2.00	60,548	2,700	45	1.50	270	5,422	-15%	0%	75%	8,000	133	133	0.110	0.110	0.303	0.454	0.044	134
Units 13-18		Townhouses	6	15	504	1,008	3,024	23	3,024	2.00	139,104	4,500	75	1.50	1,008	10,477	-15%	0%	75%	16,000	267	267	0.038	0.038	0.105	0.157	0.015	268
Units 19-22		Townhouses	4	10	336	672	2,016	23	2,016	2.00	92,736	2,700	45	1.50	672	8,555	-15%	0%	75%	13,000	217	217	0.025	0.025	0.070	0.105	0.010	218
Units 23-27		Townhouses	5	12	423	846	2,538	23	2,538	2.00	116,748	3,600	60	1.50	846	9,598	-15%	0%	75%	14,000	233	233	0.032	0.032	0.087	0.131	0.013	234
Units 28-32		Townhouses	5	12	580	1,160	3,480	23	3,480	2.00	160,080	4,500	75	1.50	1,160	11,239	-15%	0%	75%	17,000	283	283	0.032	0.032	0.087	0.131	0.013	284
Units 33-38		Townhouses	6	15	486	972	2,916	23	2,916	2.00	134,136	3,600	60	1.50	972	10,288	-15%	0%	75%	15,000	250	250	0.038	0.038	0.105	0.157	0.015	251
Units 39-43		Townhouses	5	12	400	800	2,400	23	2,400	2.00	110,400	3,600	60	1.50	800	9,334	-15%	0%	75%	14,000	233	233	0.032	0.032	0.087	0.131	0.013	234
Units 44-49		Townhouses	6	15	480	960	2,880	23	2,880	2.00	132,480	3,600	60	1.50	960	10,225	-15%	0%	75%	15,000	250	250	0.038	0.038	0.105	0.157	0.015	251
TOTALS FOR SITE			50	133				Max Fire Flow = 75					Max Fire Flow = 283					283	0.35	0.35	0.95	1.43	0.14	284				
Sum of Maximum Day Flows + Largest Fire Flow (L/s) = 284																												

Assumptions:

1 Number of units are based on the Conceptual Plan by MHBC Ltd., dated March 23, 2022

2 Residential population is calculated using a Persons Per Unit (PPU) count taken from "Region of Waterloo Water and Wastewater Monitoring Report" (WWWMR) (Region of Waterloo, June 2021)

Residential = 3.25 PPU for Single and Semi-Detached houses

Residential = 2.44 PPU for Townhouses

3 All buildings are classified as occupancy group C (Residential Occupancy)

4 Average Daily Demands for each building are based on "Tri City Water Distribution Master Plan Final Report" by AECOM, Dated May 2009 :

Residential = 225 L/cap/day

5 Peaking Factors based on "Design Guidelines for Drinking-Water Systems" (MOE, 2008):

Average Day = 1

Maximum Day = 2.75

Peak Hour = 4.13

Minimum Hour = 0.4



**961 St. David Street
FIRE FLOW ANALYSIS**

Fergus, Ontario

Project Number: 48650-100

Date: June 3, 2022

Design By: NGK

File: Q:\48650\100\Water\Obsolete\48650-100_Site Fire Flow Analysis with test results.xlsx

CALCULATION OF RESIDUAL PRESSURE AT ON-SITE HYDRANT

1. Boundary Conditions (Based on Fire Flow Test Results):

	Metric	Imperial	
P0 - Starting Pressure	32.35 m	46.0 psi	
P1 - Pressure at Q1	26.72 m	38.0 psi	
Q1 - From Fire Flow Test	4577 L/min	1209 U.S. gal/min	From: Hydrant Flow Test Results
Q2 - Required Flow	4500 L/min	1189 U.S. gal/min	From: Water Demand calculations by MTE
P-loss 1	5.63 m	8 psi	
P-loss 2	5.45 m	8 psi	
P2 - Residual Pressure	26.90 m	38 psi	

2. Friction Losses Through Water Service:

	Metric	Imperial
<i>Hazen-Williams Equation</i>		
C _{hw} = Pipe Friction Factor	150	150
k = conversion factor	10.675	4.727
n = constant	1.852	1.852
m = constant	4.8704	4.8704
Q = Flow	4500 L/min	
Q = Flow	0.075 m ³ /s	1189 U.S. gal/min
d = Pipe Diameter (1)	150 mm	5.91 in
	0.15 m	
d = Pipe Diameter (2)	250 mm	9.84 in
	0.25 m	
p = Loss/Length (1)	0.0846 m/m	0.0367 psi/ft
p = Loss/Length (2)	0.0070 m/m	0.0030 psi/ft
Length (1)	36 m	118 ft
Length (2)	132 m	433 ft
Loss	3.98 m	5.7 psi
	39 kPa	

3. Friction Losses Through Apurtenances:


Apurtenances	Number	K	Velocity m/s	Head Loss m	Total Loss m	psi
<i>Site</i>						
Valve - 150mm dia.	3	0.120	4.244	0.110	0.331	0.470
Expander - 150mm to 250mm dia.	1	2.127	4.244	1.953	1.953	2.777
45° Bend - 150mm dia.	1	0.240	1.528	0.029	0.029	0.041
45° Bend - 250mm dia.	1	0.224	1.528	0.027	0.027	0.038
250mm dia. Tee (through)	1	0.280	1.528	0.033	0.033	0.047
150mm dia. Tee (branch)	2	0.900	4.244	0.826	1.653	2.350
Total Minor Losses					4.024	5.722

4. Elevation - Elevational differences from existing hydrant to proposed hydrant

	Metric	Imperial
Elevation at Boundary (i.e. Residual Hydrant):	424.00 m	1391 ft
Elevation at Site Hydrant:	424.00 m	1391 ft
Elevation Difference = Loss/Gain	0 m	0.0 psi

ANALYSIS SUMMARY

Total Losses	8.000 m		
	78.48 kPa	11.4 psi	
Residual Pressure after Losses	18.90 m		
	185 kPa	26.9 psi	PASS
<i>Allowable Residual Pressure</i>	<i>140 kPa</i>	<i>20.3 psi</i>	

961 St. David Street Fergus, Ontario (TOWNSHIP OF CENTRE WELLINGTON)				SANITARY SEWER DESIGN SHEET ENGINEERING AND PUBLIC WORKS								Design Parameters																				
Project Number: 48650-100 Date: May 12, 2022 Design By: NGK Checked By: JPL File: L:\CORPORATE\Administration\Staff Phone List.xlsx												Drainage Area Plan No: n/a												Average Daily Flow Mannings "n" 0.013 Residential ¹ 0.00521 L/s/c Commercial 0.95 L/s/ha Industrial 0.40 L/s/ha Inst. / School 0.25 L/s/ha Min. Velocity 0.6 m/sec Max. Velocity 3.0 m/sec Residential Harmon Peaking Factor (F) $F = 1 + 14/(4 + P^{0.5})$ Commercial Peaking Factor = 2.5 Residential Areas Infiltration ¹ 0.15 L/s/ha								
LOCATION				RESIDENTIAL AREAS AND POPULATION						SCHOOL, INSTITUTIONAL			COMMERCIAL			INDUSTRIAL			INFILTRATION			DESIGN										
STREET	AREA NO.	MANHOLE LOCATION		AREA	No. UNITS @	No. UNITS @	POPUL.	CUMUL POPUL.	PEAK FACTOR "F"	PEAK RES. FLOW	HECTARES AND FLOW OF EACH ZONING									TOTALS- C-I FLOW	AREA	CUMUL AREA	INFIL FLOW	TOTAL VOLUME FLOW	LENGTH	SLOPE	PIPE SIZE	CAPACITY	FULL FLOW VELOCITY			
		FROM MH	TO MH		3.25	2.44					0.25 L/s/ha	0.95 L/s/ha	0.40 L/s/ha	AREA	CUMUL AREA	PEAK FLOW	AREA	CUMUL AREA	PEAK FLOW											AREA	CUMUL AREA	PEAK FLOW
		ha	ha		ha	ha					ha	ha	L/sec	ha	ha	L/sec	ha	ha	L/sec											L/sec	ha	ha
Proposed development				1.402	13.00	37.00	0.133	0.133	4.208032	2.9046									1.402	1.402	0.2103	3.1149	50.0	0.90	150	14.4406	0.818					

NOTES
1. Residential domestic flow of 450L/capita/day (equal to 0.00521 L/s/capita), and infiltration rate of 0.15L/ha taken from the Township of Centre Wellington's Municipal Servicing Standards, 2004

961 St. David Street
 Fergus, Ontario
 Township of Centre Wellington

Project Number: 48650-100
 Date: May 10, 2022
 Design By: NGK
 Checked By: JPL
 File: Q:\48650\100\Storm\Storm Sewer Design Sheet Kitchener Rev10.xlsx

STORM SEWER DESIGN SHEET

ENGINEERING AND PUBLIC WORKS

Drainage Area Plan No:

Design Parameters		
5 YEAR STORM		
Q=kAIC, k=0.00278	Manning's "n"	0.013
Intensity (I) = a/(tc+b) ²	Min. Velocity	0.800 m/s
a = 500	Max. Velocity	6.000 m/s
b = 0.24		
c = 0.6877		



LOCATION				STORMWATER FLOW							DESIGN							
STREET	AREA NUMBER	MANHOLE LOCATION		AREA (A)	RUNOFF COEFF. (C)	A x C	CUMUL. A x C	CONCENTRATION TIME		RAIN INTENSITY (I)	FLOW (Q)	PIPE SIZE	LENGTH	SLOPE	CAPACITY	FULL FLOW VELOCITY		PIPE FULL
		FROM MH	TO MH					TOTAL	IN PIPE							mm/hr	L/s	
				ha	ha		ha	min	min	mm/hr	L/s	mm	m	%	L/s	m/s	%	
Controlled Area				0.773				5-year flow taken from MIDUSS model			17.00000	250	10.0	0.50	42.04989	0.8566	40.43	

Appendix B

MIDUSS Output

2yr Pre

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1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 2yr pre TCW.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/4/2022 at 4:24:28 PM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 31 TIME PARAMETERS"
15 " 5.000 Time Step"
16 " 180.000 Max. Storm length"
17 " 1500.000 Max. Hydrograph"
18 " 32 STORM Chicago storm"
19 " 1 Chicago storm"
20 " 375.000 Coefficient A"
21 " 0.240 Constant B"
22 " 0.689 Exponent C"
23 " 0.400 Fraction R"
24 " 180.000 Duration"
25 " 1.000 Time step multiplier"
26 " Maximum intensity 119.788 mm/hr"
27 " Total depth 31.396 mm"
28 " 6 002hyd Hydrograph extension used in this file"
29 " 33 CATCHMENT 101"
30 " 1 Triangular SCS"
31 " 1 Equal length"
32 " 1 SCS method"
33 " 101 Area Draining to the West"
34 " 0.000 % Impervious"
35 " 0.734 Total Area"
36 " 100.000 Flow length"
37 " 2.000 Overland Slope"
38 " 0.734 Pervious Area"
39 " 100.000 Pervious length"
40 " 2.000 Pervious slope"
41 " 0.000 Impervious Area"
42 " 100.000 Impervious length"
43 " 2.000 Impervious slope"
44 " 0.250 Pervious Manning 'n'"
45 " 75.000 Pervious SCS Curve No."
46 " 0.156 Pervious Runoff coefficient"
47 " 0.100 Pervious Ia/S coefficient"
48 " 8.467 Pervious Initial abstraction"
49 " 0.015 Impervious Manning 'n'"
50 " 98.000 Impervious SCS Curve No."
51 " 0.000 Impervious Runoff coefficient"
52 " 0.100 Impervious Ia/S coefficient"
53 " 0.518 Impervious Initial abstraction"
54 " 0.004 0.000 0.000 0.000 c.m/sec"
55 " Catchment 101 Pervious Impervious Total Area "
56 " Surface Area 0.734 0.000 0.734 hectare"
57 " Time of concentration 56.110 4.409 56.110 minutes"
58 " Time to Centroid 181.077 97.331 181.077 minutes"
59 " Rainfall depth 31.396 31.396 31.396 mm"
60 " Rainfall volume 230.44 0.00 230.44 c.m"
61 " Rainfall losses 26.511 5.151 26.511 mm"
62 " Runoff depth 4.885 26.245 4.885 mm"
63 " Runoff volume 35.86 0.00 35.86 c.m"
64 " Runoff coefficient 0.156 0.000 0.156 "
65 " Maximum flow 0.004 0.000 0.004 c.m/sec"
66 " 40 HYDROGRAPH Start - New Tributary"
67 " 2 Start - New Tributary"
68 " 0.004 0.000 0.000 0.000"
69 " 33 CATCHMENT 102"
70 " 1 Triangular SCS"
71 " 1 Equal length"
72 " 1 SCS method"
73 " 102 Area Draining to South"
74 " 0.000 % Impervious"
75 " 0.286 Total Area"
76 " 40.000 Flow length"
77 " 1.000 Overland Slope"
78 " 0.286 Pervious Area"
79 " 40.000 Pervious length"
80 " 1.000 Pervious slope"
81 " 0.000 Impervious Area"
82 " 40.000 Impervious length"
83 " 1.000 Impervious slope"
84 " 0.250 Pervious Manning 'n'"
85 " 75.000 Pervious SCS Curve No."
86 " 0.156 Pervious Runoff coefficient"
87 " 0.100 Pervious Ia/S coefficient"
88 " 8.467 Pervious Initial abstraction"
89 " 0.015 Impervious Manning 'n'"
90 " 98.000 Impervious SCS Curve No."
91 " 0.000 Impervious Runoff coefficient"
92 " 0.100 Impervious Ia/S coefficient"
93 " 0.518 Impervious Initial abstraction"
94 " 0.002 0.000 0.000 0.000 c.m/sec"
95 " Catchment 102 Pervious Impervious Total Area "
96 " Surface Area 0.286 0.000 0.286 hectare"
97 " Time of concentration 39.864 3.132 39.864 minutes"
98 " Time to Centroid 160.020 95.246 160.020 minutes"
99 " Rainfall depth 31.396 31.396 31.396 mm"
100 " Rainfall volume 89.79 0.00 89.79 c.m"
101 " Rainfall losses 26.511 5.390 26.511 mm"
102 " Runoff depth 4.885 26.006 4.885 mm"
103 " Runoff volume 13.97 0.00 13.97 c.m"
104 " Runoff coefficient 0.156 0.000 0.156 "
105 " Maximum flow 0.002 0.000 0.002 c.m/sec"
106 " 40 HYDROGRAPH Start - New Tributary"
107 " 2 Start - New Tributary"
108 " 0.002 0.000 0.000 0.000"
109 " 33 CATCHMENT 103"
110 " 1 Triangular SCS"
111 " 1 Equal length"
112 " 1 SCS method"
113 " 103 Area Draining to Highway 6"
114 " 14.000 % Impervious"
115 " 0.382 Total Area"
116 " 70.000 Flow length"
117 " 5.000 Overland Slope"
118 " 0.329 Pervious Area"
119 " 70.000 Pervious length"
120 " 5.000 Pervious slope"
121 " 0.053 Impervious Area"
122 " 70.000 Impervious length"
123 " 5.000 Impervious slope"
124 " 0.250 Pervious Manning 'n'"
125 " 75.000 Pervious SCS Curve No."
126 " 0.156 Pervious Runoff coefficient"
127 " 0.100 Pervious Ia/S coefficient"
128 " 8.467 Pervious Initial abstraction"

```

129	"	0.015	Impervious Manning 'n'"					
130	"	98.000	Impervious SCS Curve No."					
131	"	0.834	Impervious Runoff coefficient"					
132	"	0.100	Impervious Ia/S coefficient"					
133	"	0.518	Impervious Initial abstraction"					
134	"		0.011 0.000 0.000 0.000 c.m/sec"					
135	"		Catchment 103	Pervious	Impervious	Total Area	"	
136	"		Surface Area	0.329	0.053	0.382	hectare"	
137	"		Time of concentration	34.413	2.704	19.636	minutes"	
138	"		Time to Centroid	152.959	94.431	125.684	minutes"	
139	"		Rainfall depth	31.396	31.396	31.396	mm"	
140	"		Rainfall volume	103.14	16.79	119.93	c.m"	
141	"		Rainfall losses	26.512	5.214	23.530	mm"	
142	"		Runoff depth	4.884	26.182	7.866	mm"	
143	"		Runoff volume	16.05	14.00	30.05	c.m"	
144	"		Runoff coefficient	0.156	0.834	0.251	"	
145	"		Maximum flow	0.003	0.011	0.011	c.m/sec"	
146								

5yr Pre

1	"	MIDUSS Output ----->"	65	"	Maximum flow 0.011 0.000 0.011 c.m/sec"
2	"	MIDUSS version Version 2.25 rev. 473"	66	" 40	HYDROGRAPH Start - New Tributary"
3	"	MIDUSS created Sunday, February 7, 2010"	67	"	2 Start - New Tributary"
4	"	10 Units used: ie METRIC"	68	"	0.011 0.000 0.000 0.000"
5	"	Job folder: Q:\48650\100\SWM\TCW versions"	69	" 33	CATCHMENT 102"
6	"	Output filename: 5yr pre.Out"	70	"	1 Triangular SCS"
7	"	Licensee name: A"	71	"	1 Equal length"
8	"	Company "	72	"	1 SCS method"
9	"	Date & Time last used: 5/4/2022 at 4:11:08 PM"	73	"	102 Area Draining to South"
10	" 31	TIME PARAMETERS"	74	"	0.000 % Impervious"
11	"	5.000 Time Step"	75	"	0.286 Total Area"
12	"	180.000 Max. Storm length"	76	"	40.000 Flow length"
13	"	1500.000 Max. Hydrograph"	77	"	1.000 Overland Slope"
14	" 31	TIME PARAMETERS"	78	"	0.286 Pervious Area"
15	"	5.000 Time Step"	79	"	40.000 Pervious length"
16	"	180.000 Max. Storm length"	80	"	1.000 Pervious slope"
17	"	1500.000 Max. Hydrograph"	81	"	0.000 Impervious Area"
18	" 32	STORM Chicago storm"	82	"	40.000 Impervious length"
19	"	1 Chicago storm"	83	"	1.000 Impervious slope"
20	"	500.000 Coefficient A"	84	"	0.250 Pervious Manning 'n'"
21	"	0.240 Constant B"	85	"	75.000 Pervious SCS Curve No."
22	"	0.688 Exponent C"	86	"	0.227 Pervious Runoff coefficient"
23	"	0.400 Fraction R"	87	"	0.100 Pervious Ia/S coefficient"
24	"	180.000 Duration"	88	"	8.467 Pervious Initial abstraction"
25	"	1.000 Time step multiplier"	89	"	0.015 Impervious Manning 'n'"
26	"	Maximum intensity 160.061 mm/hr"	90	"	98.000 Impervious SCS Curve No."
27	"	Total depth 42.145 mm"	91	"	0.000 Impervious Runoff coefficient"
28	"	6 005hyd Hydrograph extension used in this file"	92	"	0.100 Impervious Ia/S coefficient"
29	" 33	CATCHMENT 101"	93	"	0.518 Impervious Initial abstraction"
30	"	1 Triangular SCS"	94	"	0.006 0.000 0.000 0.000 c.m/sec"
31	"	1 Equal length"	95	"	Catchment 102 Pervious Impervious Total Area "
32	"	1 SCS method"	96	"	Surface Area 0.286 0.000 0.286 hectare"
33	"	101 Area Draining to the West"	97	"	Time of concentration 28.179 2.752 28.178 minutes"
34	"	0.000 % Impervious"	98	"	Time to Centroid 144.328 93.539 144.328 minutes"
35	"	0.734 Total Area"	99	"	Rainfall depth 42.145 42.145 42.145 mm"
36	"	100.000 Flow length"	100	"	Rainfall volume 120.53 0.00 120.53 c.m"
37	"	2.000 Overland Slope"	101	"	Rainfall losses 32.569 5.505 32.569 mm"
38	"	0.734 Pervious Area"	102	"	Runoff depth 9.576 36.640 9.576 mm"
39	"	100.000 Pervious length"	103	"	Runoff volume 27.39 0.00 27.39 c.m"
40	"	2.000 Pervious slope"	104	"	Runoff coefficient 0.227 0.000 0.227 "
41	"	0.000 Impervious Area"	105	"	Maximum flow 0.006 0.000 0.006 c.m/sec"
42	"	100.000 Impervious length"	106	" 40	HYDROGRAPH Start - New Tributary"
43	"	2.000 Impervious slope"	107	"	2 Start - New Tributary"
44	"	0.250 Pervious Manning 'n'"	108	"	0.006 0.000 0.000 0.000"
45	"	75.000 Pervious SCS Curve No."	109	" 33	CATCHMENT 103"
46	"	0.227 Pervious Runoff coefficient"	110	"	1 Triangular SCS"
47	"	0.100 Pervious Ia/S coefficient"	111	"	1 Equal length"
48	"	8.467 Pervious Initial abstraction"	112	"	1 SCS method"
49	"	0.015 Impervious Manning 'n'"	113	"	103 Area Draining to Highway 6"
50	"	98.000 Impervious SCS Curve No."	114	"	14.000 % Impervious"
51	"	0.000 Impervious Runoff coefficient"	115	"	0.382 Total Area"
52	"	0.100 Impervious Ia/S coefficient"	116	"	70.000 Flow length"
53	"	0.518 Impervious Initial abstraction"	117	"	5.000 Overland Slope"
54	"	0.011 0.000 0.000 0.000 c.m/sec"	118	"	0.329 Pervious Area"
55	"	Catchment 101 Pervious Impervious Total Area "	119	"	70.000 Pervious length"
56	"	Surface Area 0.734 0.000 0.734 hectare"	120	"	5.000 Pervious slope"
57	"	Time of concentration 39.662 3.873 39.662 minutes"	121	"	0.053 Impervious Area"
58	"	Time to Centroid 160.316 95.565 160.316 minutes"	122	"	70.000 Impervious length"
59	"	Rainfall depth 42.145 42.145 42.145 mm"	123	"	5.000 Impervious slope"
60	"	Rainfall volume 309.34 0.00 309.34 c.m"	124	"	0.250 Pervious Manning 'n'"
61	"	Rainfall losses 32.561 5.881 32.561 mm"	125	"	75.000 Pervious SCS Curve No."
62	"	Runoff depth 9.583 36.263 9.583 mm"	126	"	0.227 Pervious Runoff coefficient"
63	"	Runoff volume 70.34 0.00 70.34 c.m"	127	"	0.100 Pervious Ia/S coefficient"
64	"	Runoff coefficient 0.227 0.000 0.227 "	128	"	8.467 Pervious Initial abstraction"

```

129 "      0.015  Impervious Manning 'n'"
130 "      98.000 Impervious SCS Curve No."
131 "      0.871  Impervious Runoff coefficient"
132 "      0.100  Impervious Ia/S coefficient"
133 "      0.518  Impervious Initial abstraction"
134 "          0.016   0.000   0.000   0.000 c.m/sec"
135 "      Catchment 103      Pervious  Impervious  Total Area  "
136 "      Surface Area      0.329    0.053    0.382    hectare"
137 "      Time of concentration  24.325  2.376    15.892    minutes"
138 "      Time to Centroid    138.963  92.887   121.261    minutes"
139 "      Rainfall depth      42.145  42.145   42.145    mm"
140 "      Rainfall volume     138.45  22.54    160.99    c.m"
141 "      Rainfall losses     32.564  5.426    28.765    mm"
142 "      Runoff depth        9.581   36.719   13.380    mm"
143 "      Runoff volume       31.47   19.64    51.11    c.m"
144 "      Runoff coefficient   0.227   0.871    0.317    "
145 "      Maximum flow        0.007   0.015    0.016    c.m/sec"
146 " 38  START/RE-START TOTALS  "
147 "      3  Runoff Totals on EXIT"
148 "      Total Catchment area          0.000  hectare"
149 "      Total Impervious area          0.000  hectare"
150 "      Total % impervious            0.000"
151 " 19  EXIT"
152

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10yr Pre

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1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 10yr pre TCW.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/4/2022 at 4:26:04 PM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 595.000 Coefficient A"
17 " 0.360 Constant B"
18 " 0.691 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 186.431 mm/hr"
23 " Total depth 49.226 mm"
24 " 6 010hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 101"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 101 Area Draining to the West"
30 " 0.000 % Impervious"
31 " 0.734 Total Area"
32 " 100.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.734 Pervious Area"
35 " 100.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.000 Impervious Area"
38 " 100.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.269 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.000 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.018 0.000 0.000 0.000 c.m/sec"
51 " Catchment 101 Pervious Impervious Total Area "
52 " Surface Area 0.734 0.000 0.734 hectare"
53 " Time of concentration 34.073 3.625 34.073 minutes"
54 " Time to Centroid 151.986 94.644 151.986 minutes"
55 " Rainfall depth 49.226 49.226 49.226 mm"
56 " Rainfall volume 361.32 0.00 361.32 c.m"
57 " Rainfall losses 35.986 6.385 35.986 mm"
58 " Runoff depth 13.240 42.841 13.240 mm"
59 " Runoff volume 97.18 0.00 97.18 c.m"
60 " Runoff coefficient 0.269 0.000 0.269 "
61 " Maximum flow 0.018 0.000 0.018 c.m/sec"
62 " 40 HYDROGRAPH Start - New Tributary"
63 " 2 Start - New Tributary"
64 " 0.018 0.000 0.000 0.000"

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65 " 33 CATCHMENT 102"
66 " 1 Triangular SCS"
67 " 1 Equal length"
68 " 1 SCS method"
69 " 102 Area Draining to South"
70 " 0.000 % Impervious"
71 " 0.286 Total Area"
72 " 40.000 Flow length"
73 " 1.000 Overland Slope"
74 " 0.286 Pervious Area"
75 " 40.000 Pervious length"
76 " 1.000 Pervious slope"
77 " 0.000 Impervious Area"
78 " 40.000 Impervious length"
79 " 1.000 Impervious slope"
80 " 0.250 Pervious Manning 'n'"
81 " 75.000 Pervious SCS Curve No."
82 " 0.269 Pervious Runoff coefficient"
83 " 0.100 Pervious Ia/S coefficient"
84 " 8.467 Pervious Initial abstraction"
85 " 0.015 Impervious Manning 'n'"
86 " 98.000 Impervious SCS Curve No."
87 " 0.000 Impervious Runoff coefficient"
88 " 0.100 Impervious Ia/S coefficient"
89 " 0.518 Impervious Initial abstraction"
90 " 0.009 0.000 0.000 0.000 c.m/sec"
91 " Catchment 102 Pervious Impervious Total Area "
92 " Surface Area 0.286 0.000 0.286 hectare"
93 " Time of concentration 24.208 2.576 24.208 minutes"
94 " Time to Centroid 137.952 92.639 137.952 minutes"
95 " Rainfall depth 49.226 49.226 49.226 mm"
96 " Rainfall volume 140.79 0.00 140.79 c.m"
97 " Rainfall losses 35.984 5.590 35.984 mm"
98 " Runoff depth 13.242 43.636 13.242 mm"
99 " Runoff volume 37.87 0.00 37.87 c.m"
100 " Runoff coefficient 0.269 0.000 0.269 "
101 " Maximum flow 0.009 0.000 0.009 c.m/sec"
102 " 40 HYDROGRAPH Start - New Tributary"
103 " 2 Start - New Tributary"
104 " 0.009 0.000 0.000 0.000"
105 " 33 CATCHMENT 103"
106 " 1 Triangular SCS"
107 " 1 Equal length"
108 " 1 SCS method"
109 " 103 Area Draining to Highway 6"
110 " 14.000 % Impervious"
111 " 0.382 Total Area"
112 " 70.000 Flow length"
113 " 5.000 Overland Slope"
114 " 0.329 Pervious Area"
115 " 70.000 Pervious length"
116 " 5.000 Pervious slope"
117 " 0.053 Impervious Area"
118 " 70.000 Impervious length"
119 " 5.000 Impervious slope"
120 " 0.250 Pervious Manning 'n'"
121 " 75.000 Pervious SCS Curve No."
122 " 0.269 Pervious Runoff coefficient"
123 " 0.100 Pervious Ia/S coefficient"
124 " 8.467 Pervious Initial abstraction"
125 " 0.015 Impervious Manning 'n'"
126 " 98.000 Impervious SCS Curve No."
127 " 0.886 Impervious Runoff coefficient"
128 " 0.100 Impervious Ia/S coefficient"

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129	"	0.518	Impervious Initial abstraction"				
130	"	0.020	0.000	0.000	0.000	c.m/sec"	
131	"	Catchment 103	Pervious	Impervious	Total Area	"	
132	"	Surface Area	0.329	0.053	0.382	hectare"	
133	"	Time of concentration	20.897	2.224	14.379	minutes"	
134	"	Time to Centroid	133.227	92.111	118.875	minutes"	
135	"	Rainfall depth	49.226	49.226	49.226	mm"	
136	"	Rainfall volume	161.72	26.33	188.04	c.m"	
137	"	Rainfall losses	35.985	5.608	31.733	mm"	
138	"	Runoff depth	13.241	43.618	17.493	mm"	
139	"	Runoff volume	43.50	23.33	66.82	c.m"	
140	"	Runoff coefficient	0.269	0.886	0.355	"	
141	"	Maximum flow	0.011	0.019	0.020	c.m/sec"	
142	"						

25yr Pre

1	"	MIDUSS Output ----->"	65	"	Rainfall losses	39.869	6.386	39.869	mm"
2	"	MIDUSS version	66	"	Runoff depth	18.484	51.966	18.484	mm"
3	"	MIDUSS created	67	"	Runoff volume	135.67	0.00	135.67	c.m"
4	"	10 Units used:	68	"	Runoff coefficient	0.317	0.000	0.317	"
5	"	Job folder:	69	"	Maximum flow	0.029	0.000	0.029	c.m/sec"
6	"	Output filename:	70	"	40 HYDROGRAPH Start - New Tributary"				
7	"	Licensee name:	71	"	2 Start - New Tributary"				
8	"	Company	72	"	0.029 0.000 0.000 0.000"				
9	"	Date & Time last used:	73	"	33 CATCHMENT 102"				
10	"	31 TIME PARAMETERS"	74	"	1 Triangular SCS"				
11	"	5.000 Time Step"	75	"	1 Equal length"				
12	"	180.000 Max. Storm length"	76	"	1 SCS method"				
13	"	1500.000 Max. Hydrograph"	77	"	102 Area Draining to South"				
14	"	31 TIME PARAMETERS"	78	"	0.000 % Impervious"				
15	"	5.000 Time Step"	79	"	0.286 Total Area"				
16	"	180.000 Max. Storm length"	80	"	40.000 Flow length"				
17	"	1500.000 Max. Hydrograph"	81	"	1.000 Overland Slope"				
18	"	31 TIME PARAMETERS"	82	"	0.286 Pervious Area"				
19	"	5.000 Time Step"	83	"	40.000 Pervious length"				
20	"	180.000 Max. Storm length"	84	"	1.000 Pervious slope"				
21	"	1500.000 Max. Hydrograph"	85	"	0.000 Impervious Area"				
22	"	32 STORM Chicago storm"	86	"	40.000 Impervious length"				
23	"	1 Chicago storm"	87	"	1.000 Impervious slope"				
24	"	702.000 Coefficient A"	88	"	0.250 Pervious Manning 'n'"				
25	"	0.350 Constant B"	89	"	75.000 Pervious SCS Curve No."				
26	"	0.690 Exponent C"	90	"	0.317 Pervious Runoff coefficient"				
27	"	0.400 Fraction R"	91	"	0.100 Pervious Ia/S coefficient"				
28	"	180.000 Duration"	92	"	8.467 Pervious Initial abstraction"				
29	"	1.000 Time step multiplier"	93	"	0.015 Impervious Manning 'n'"				
30	"	Maximum intensity	94	"	98.000 Impervious SCS Curve No."				
31	"	220.574 mm/hr"	95	"	0.000 Impervious Runoff coefficient"				
32	"	Total depth	96	"	0.100 Impervious Ia/S coefficient"				
33	"	58.353 mm"	97	"	0.518 Impervious Initial abstraction"				
34	"	6 025hyd Hydrograph extension used in this file"	98	"	0.014 0.000 0.000 0.000 c.m/sec"				
35	"	33 CATCHMENT 101"	99	"	Catchment 102 Pervious Impervious Total Area "				
36	"	1 Triangular SCS"	100	"	Surface Area	0.286	0.000	0.286	hectare"
37	"	1 Equal length"	101	"	Time of concentration	20.799	2.398	20.799	minutes"
38	"	1 SCS method"	102	"	Time to Centroid	132.350	91.879	132.350	minutes"
39	"	101 Area Draining to the West"	103	"	Rainfall depth	58.353	58.353	58.353	mm"
40	"	0.000 % Impervious"	104	"	Rainfall volume	166.89	0.00	166.89	c.m"
41	"	0.734 Total Area"	105	"	Rainfall losses	39.869	5.724	39.869	mm"
42	"	100.000 Flow length"	106	"	Runoff depth	18.483	52.629	18.483	mm"
43	"	2.000 Overland Slope"	107	"	Runoff volume	52.86	0.00	52.86	c.m"
44	"	0.734 Pervious Area"	108	"	Runoff coefficient	0.317	0.000	0.317	"
45	"	100.000 Pervious length"	109	"	Maximum flow	0.014	0.000	0.014	c.m/sec"
46	"	2.000 Pervious slope"	110	"	40 HYDROGRAPH Start - New Tributary"				
47	"	0.000 Impervious Area"	111	"	2 Start - New Tributary"				
48	"	100.000 Impervious length"	112	"	0.014 0.000 0.000 0.000"				
49	"	2.000 Impervious slope"	113	"	33 CATCHMENT 103"				
50	"	0.250 Pervious Manning 'n'"	114	"	1 Triangular SCS"				
51	"	75.000 Pervious SCS Curve No."	115	"	1 Equal length"				
52	"	0.317 Pervious Runoff coefficient"	116	"	1 SCS method"				
53	"	0.100 Pervious Ia/S coefficient"	117	"	103 Area Draining to Highway 6"				
54	"	8.467 Pervious Initial abstraction"	118	"	14.000 % Impervious"				
55	"	0.015 Impervious Manning 'n'"	119	"	0.382 Total Area"				
56	"	98.000 Impervious SCS Curve No."	120	"	70.000 Flow length"				
57	"	0.000 Impervious Runoff coefficient"	121	"	5.000 Overland Slope"				
58	"	0.100 Impervious Ia/S coefficient"	122	"	0.329 Pervious Area"				
59	"	0.518 Impervious Initial abstraction"	123	"	70.000 Pervious length"				
60	"	0.029 0.000 0.000 0.000 c.m/sec"	124	"	5.000 Pervious slope"				
61	"	Catchment 101 Pervious Impervious Total Area "	125	"	0.053 Impervious Area"				
62	"	Surface Area	126	"	70.000 Impervious length"				
63	"	0.734 0.000 0.734 hectare"	127	"	5.000 Impervious slope"				
64	"	Time of concentration	128	"	0.250 Pervious Manning 'n'"				
		29.275 3.375 29.275 minutes"							
		Time to Centroid							
		144.683 93.660 144.683 minutes"							
		Rainfall depth							
		58.353 58.353 58.353 mm"							
		Rainfall volume							
		428.31 0.00 428.31 c.m"							

129	"	75.000	Pervious SCS Curve No."				
130	"	0.317	Pervious Runoff coefficient"				
131	"	0.100	Pervious Ia/S coefficient"				
132	"	8.467	Pervious Initial abstraction"				
133	"	0.015	Impervious Manning 'n'"				
134	"	98.000	Impervious SCS Curve No."				
135	"	0.898	Impervious Runoff coefficient"				
136	"	0.100	Impervious Ia/S coefficient"				
137	"	0.518	Impervious Initial abstraction"				
138	"		0.026	0.000	0.000	0.000	c.m/sec"
139	"		Catchment 103	Pervious	Impervious	Total Area	"
140	"		Surface Area	0.329	0.053	0.382	hectare"
141	"		Time of concentration	17.954	2.070	12.938	minutes"
142	"		Time to Centroid	128.213	91.378	116.580	minutes"
143	"		Rainfall depth	58.353	58.353	58.353	mm"
144	"		Rainfall volume	191.70	31.21	222.91	c.m"
145	"		Rainfall losses	39.864	5.931	35.114	mm"
146	"		Runoff depth	18.488	52.422	23.239	mm"
147	"		Runoff volume	60.74	28.04	88.77	c.m"
148	"		Runoff coefficient	0.317	0.898	0.398	"
149	"		Maximum flow	0.018	0.023	0.026	c.m/sec"
150							

50yr Pre

1	"	MIDUSS Output ----->"	65	"	Rainfall losses	42.373	6.372	42.373	mm"
2	"	MIDUSS version	66	"	Runoff depth	22.629	58.631	22.629	mm"
3	"	MIDUSS created	67	"	Runoff volume	166.10	0.00	166.10	c.m"
4	"	10 Units used:	68	"	Runoff coefficient	0.348	0.000	0.348	"
5	"	Job folder:	69	"	Maximum flow	0.038	0.000	0.038	c.m/sec"
6	"	Output filename:	70	"	40 HYDROGRAPH Start - New Tributary"				
7	"	Licensee name:	71	"	2 Start - New Tributary"				
8	"	Company	72	"	0.038	0.000	0.000	0.000"	
9	"	Date & Time last used:	73	"	33 CATCHMENT 102"				
10	"	31 TIME PARAMETERS"	74	"	1 Triangular SCS"				
11	"	5.000 Time Step"	75	"	1 Equal length"				
12	"	180.000 Max. Storm length"	76	"	1 SCS method"				
13	"	1500.000 Max. Hydrograph"	77	"	102 Area Draining to South"				
14	"	31 TIME PARAMETERS"	78	"	0.000 % Impervious"				
15	"	5.000 Time Step"	79	"	0.286 Total Area"				
16	"	180.000 Max. Storm length"	80	"	40.000 Flow length"				
17	"	1500.000 Max. Hydrograph"	81	"	1.000 Overland Slope"				
18	"	31 TIME PARAMETERS"	82	"	0.286 Pervious Area"				
19	"	5.000 Time Step"	83	"	40.000 Pervious length"				
20	"	180.000 Max. Storm length"	84	"	1.000 Pervious slope"				
21	"	1500.000 Max. Hydrograph"	85	"	0.000 Impervious Area"				
22	"	32 STORM Chicago storm"	86	"	40.000 Impervious length"				
23	"	1 Chicago storm"	87	"	1.000 Impervious slope"				
24	"	780.000 Coefficient A"	88	"	0.250 Pervious Manning 'n'"				
25	"	0.360 Constant B"	89	"	75.000 Pervious SCS Curve No."				
26	"	0.690 Exponent C"	90	"	0.348 Pervious Runoff coefficient"				
27	"	0.400 Fraction R"	91	"	0.100 Pervious Ia/S coefficient"				
28	"	180.000 Duration"	92	"	8.467 Pervious Initial abstraction"				
29	"	1.000 Time step multiplier"	93	"	0.015 Impervious Manning 'n'"				
30	"	Maximum intensity	94	"	98.000 Impervious SCS Curve No."				
31	"	244.972 mm/hr"	95	"	0.000 Impervious Runoff coefficient"				
32	"	65.003 mm"	96	"	0.100 Impervious Ia/S coefficient"				
33	"	6 050hyd Hydrograph extension used in this file"	97	"	0.518 Impervious Initial abstraction"				
34	"	33 CATCHMENT 101"	98	"	0.019	0.000	0.000	0.000	c.m/sec"
35	"	1 Triangular SCS"	99	"	Catchment 102 Pervious				Total Area "
36	"	1 Equal length"	100	"	Surface Area	0.286	0.000	0.286	hectare"
37	"	1 SCS method"	101	"	Time of concentration	19.024	2.294	19.024	minutes"
38	"	101 Area Draining to the West"	102	"	Time to Centroid	129.314	91.460	129.314	minutes"
39	"	0.000 % Impervious"	103	"	Rainfall depth	65.003	65.003	65.003	mm"
40	"	0.734 Total Area"	104	"	Rainfall volume	185.91	0.00	185.91	c.m"
41	"	100.000 Flow length"	105	"	Rainfall losses	42.413	5.828	42.413	mm"
42	"	2.000 Overland Slope"	106	"	Runoff depth	22.589	59.174	22.589	mm"
43	"	0.734 Pervious Area"	107	"	Runoff volume	64.61	0.00	64.61	c.m"
44	"	100.000 Pervious length"	108	"	Runoff coefficient	0.348	0.000	0.348	"
45	"	2.000 Pervious slope"	109	"	Maximum flow	0.019	0.000	0.019	c.m/sec"
46	"	0.000 Impervious Area"	110	"	40 HYDROGRAPH Start - New Tributary"				
47	"	100.000 Impervious length"	111	"	2 Start - New Tributary"				
48	"	2.000 Impervious slope"	112	"	0.019	0.000	0.000	0.000"	
49	"	0.250 Pervious Manning 'n'"	113	"	33 CATCHMENT 103"				
50	"	75.000 Pervious SCS Curve No."	114	"	1 Triangular SCS"				
51	"	0.348 Pervious Runoff coefficient"	115	"	1 Equal length"				
52	"	0.100 Pervious Ia/S coefficient"	116	"	1 SCS method"				
53	"	8.467 Pervious Initial abstraction"	117	"	103 Area Draining to Highway 6"				
54	"	0.015 Impervious Manning 'n'"	118	"	14.000 % Impervious"				
55	"	98.000 Impervious SCS Curve No."	119	"	0.382 Total Area"				
56	"	0.000 Impervious Runoff coefficient"	120	"	70.000 Flow length"				
57	"	0.100 Impervious Ia/S coefficient"	121	"	5.000 Overland Slope"				
58	"	0.518 Impervious Initial abstraction"	122	"	0.329 Pervious Area"				
59	"	0.038 0.000 0.000 0.000 c.m/sec"	123	"	70.000 Pervious length"				
60	"	Catchment 101 Pervious	124	"	5.000 Pervious slope"				
61	"	Surface Area	125	"	0.053 Impervious Area"				
62	"	0.734 3.229 26.777 minutes"	126	"	70.000 Impervious length"				
63	"	Time to Centroid	127	"	5.000 Impervious slope"				
64	"	140.702 93.078 140.702 minutes"	128	"	0.250 Pervious Manning 'n'"				
		Rainfall depth							
		65.003 65.003 65.003 mm"							
		Rainfall volume							
		477.12 0.00 477.12 c.m"							

129	"	75.000	Pervious SCS Curve No."				
130	"	0.347	Pervious Runoff coefficient"				
131	"	0.100	Pervious Ia/S coefficient"				
132	"	8.467	Pervious Initial abstraction"				
133	"	0.015	Impervious Manning 'n'"				
134	"	98.000	Impervious SCS Curve No."				
135	"	0.906	Impervious Runoff coefficient"				
136	"	0.100	Impervious Ia/S coefficient"				
137	"	0.518	Impervious Initial abstraction"				
138	"		0.032	0.000	0.000	0.000	c.m/sec"
139	"		Catchment 103	Pervious	Impervious	Total Area	"
140	"		Surface Area	0.329	0.053	0.382	hectare"
141	"		Time of concentration	16.422	1.980	12.119	minutes"
142	"		Time to Centroid	125.533	90.934	115.222	minutes"
143	"		Rainfall depth	65.003	65.003	65.003	mm"
144	"		Rainfall volume	213.55	34.76	248.31	c.m"
145	"		Rainfall losses	42.423	6.120	37.340	mm"
146	"		Runoff depth	22.580	58.883	27.662	mm"
147	"		Runoff volume	74.18	31.49	105.67	c.m"
148	"		Runoff coefficient	0.347	0.906	0.426	"
149	"		Maximum flow	0.026	0.026	0.032	c.m/sec"
150							

100yr Pre

1	"	MIDUSS Output ----->"	65	"	Rainfall losses	44.722	6.351	44.722	mm"
2	"	MIDUSS version	66	"	Runoff depth	27.106	65.478	27.106	mm"
3	"	MIDUSS created	67	"	Runoff volume	198.96	0.00	198.96	c.m"
4	"	Units used:	68	"	Runoff coefficient	0.377	0.000	0.377	"
5	"	Job folder:	69	"	Maximum flow	0.052	0.000	0.052	c.m/sec"
6	"	Output filename:	70	"	40	HYDROGRAPH Start - New Tributary"			
7	"	Licensee name:	71	"	2	Start - New Tributary"			
8	"	Company	72	"		0.052	0.000	0.000	
9	"	Date & Time last used:	73	"	33	CATCHMENT 102"			
10	"	31	74	"	1	Triangular SCS"			
11	"	5.000	75	"	1	Equal length"			
12	"	180.000	76	"	1	SCS method"			
13	"	1500.000	77	"	102	Area Draining to South"			
14	"	31	78	"	0.000	% Impervious"			
15	"	5.000	79	"	0.286	Total Area"			
16	"	180.000	80	"	40.000	Flow length"			
17	"	1500.000	81	"	1.000	Overland Slope"			
18	"	31	82	"	0.286	Pervious Area"			
19	"	5.000	83	"	40.000	Pervious length"			
20	"	180.000	84	"	1.000	Pervious slope"			
21	"	1500.000	85	"	0.000	Impervious Area"			
22	"	32	86	"	40.000	Impervious length"			
23	"	1	87	"	1.000	Impervious slope"			
24	"	851.000	88	"	0.250	Pervious Manning 'n'"			
25	"	0.290	89	"	75.000	Pervious SCS Curve No."			
26	"	0.687	90	"	0.377	Pervious Runoff coefficient"			
27	"	0.400	91	"	0.100	Pervious Ia/S coefficient"			
28	"	180.000	92	"	8.467	Pervious Initial abstraction"			
29	"	1.000	93	"	0.015	Impervious Manning 'n'"			
30	"	Maximum intensity	94	"	98.000	Impervious SCS Curve No."			
31	"	Total depth	95	"	0.000	Impervious Runoff coefficient"			
32	"	6	96	"	0.100	Impervious Ia/S coefficient"			
33	"	33	97	"	0.518	Impervious Initial abstraction"			
34	"	1	98	"		0.024	0.000	0.000	0.000 c.m/sec"
35	"	1	99	"		Catchment 102	Pervious	Impervious	Total Area "
36	"	1	100	"		Surface Area	0.286	0.000	0.286
37	"	101	101	"		Time of concentration	17.560	2.200	17.560
38	"	0.000	102	"		Time to Centroid	126.777	91.112	126.777
39	"	0.734	103	"		Rainfall depth	71.828	71.828	71.828
40	"	100.000	104	"		Rainfall volume	205.43	0.00	205.43
41	"	2.000	105	"		Rainfall losses	44.716	5.983	44.716
42	"	0.734	106	"		Runoff depth	27.112	65.845	27.112
43	"	100.000	107	"		Runoff volume	77.54	0.00	77.54
44	"	2.000	108	"		Runoff coefficient	0.377	0.000	0.377
45	"	0.000	109	"		Maximum flow	0.024	0.000	0.024
46	"	100.000	110	"	40	HYDROGRAPH Start - New Tributary"			
47	"	2.000	111	"	2	Start - New Tributary"			
48	"	0.250	112	"		0.024	0.000	0.000	0.000
49	"	75.000	113	"	33	CATCHMENT 103"			
50	"	0.377	114	"	1	Triangular SCS"			
51	"	0.100	115	"	1	Equal length"			
52	"	8.467	116	"	1	SCS method"			
53	"	0.015	117	"	103	Area Draining to Highway 6"			
54	"	98.000	118	"	14.000	% Impervious"			
55	"	0.000	119	"	0.382	Total Area"			
56	"	0.100	120	"	70.000	Flow length"			
57	"	0.518	121	"	5.000	Overland Slope"			
58	"	0.052	122	"	0.329	Pervious Area"			
59	"		123	"	70.000	Pervious length"			
60	"		124	"	5.000	Pervious slope"			
61	"		125	"	0.053	Impervious Area"			
62	"		126	"	70.000	Impervious length"			
63	"		127	"	5.000	Impervious slope"			
64	"		128	"	0.250	Pervious Manning 'n'"			

129	"	75.000	Pervious SCS Curve No."				
130	"	0.377	Pervious Runoff coefficient"				
131	"	0.100	Pervious Ia/S coefficient"				
132	"	8.467	Pervious Initial abstraction"				
133	"	0.015	Impervious Manning 'n'"				
134	"	98.000	Impervious SCS Curve No."				
135	"	0.912	Impervious Runoff coefficient"				
136	"	0.100	Impervious Ia/S coefficient"				
137	"	0.518	Impervious Initial abstraction"				
138	"		0.040	0.000	0.000	0.000	c.m/sec"
139	"		Catchment 103	Pervious	Impervious	Total Area	"
140	"		Surface Area	0.329	0.053	0.382	hectare"
141	"		Time of concentration	15.158	1.899	11.413	minutes"
142	"		Time to Centroid	123.231	90.553	114.001	minutes"
143	"		Rainfall depth	71.828	71.828	71.828	mm"
144	"		Rainfall volume	235.97	38.41	274.38	c.m"
145	"		Rainfall losses	44.733	6.312	39.354	mm"
146	"		Runoff depth	27.095	65.517	32.474	mm"
147	"		Runoff volume	89.01	35.04	124.05	c.m"
148	"		Runoff coefficient	0.377	0.912	0.452	"
149	"		Maximum flow	0.033	0.029	0.040	c.m/sec"
150							

2yr Post

```

1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 2yr post.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/10/2022 at 10:36:02 AM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 375.000 Coefficient A"
17 " 0.240 Constant B"
18 " 0.689 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 119.788 mm/hr"
23 " Total depth 31.396 mm"
24 " 6 002hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 201"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 201 Uncontrolled to Southwest Property Line"
30 " 55.000 % Impervious"
31 " 0.228 Total Area"
32 " 15.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.103 Pervious Area"
35 " 15.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.125 Impervious Area"
38 " 15.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.156 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.825 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.030 0.000 0.000 0.000 c.m/sec"
51 " Catchment 201 Pervious Impervious Total Area "
52 " Surface Area 0.103 0.125 0.228 hectare"
53 " Time of concentration 17.976 1.412 3.627 minutes"
54 " Time to Centroid 131.637 92.372 97.622 minutes"
55 " Rainfall depth 31.396 31.396 31.396 mm"
56 " Rainfall volume 32.21 39.37 71.58 c.m"
57 " Rainfall losses 26.512 5.505 14.958 mm"
58 " Runoff depth 4.883 25.891 16.437 mm"
59 " Runoff volume 5.01 32.47 37.48 c.m"
60 " Runoff coefficient 0.156 0.825 0.524 "
61 " Maximum flow 0.001 0.030 0.030 c.m/sec"
62 " 40 HYDROGRAPH Add Runoff "
63 " 4 Add Runoff "
64 " 0.030 0.030 0.000 0.000"

```

```

65 " 57 TRENCH Design d/s of 201"
66 " 0.030 Peak inflow"
67 " 37.477 Hydrograph volume"
68 " 424.500 Ground elevation"
69 " 423.500 Downstream trench invert"
70 " 0.500 Trench height"
71 " 422.500 Water table elevation"
72 " 3.500 Trench top width"
73 " 3.500 Trench bottom width"
74 " 40.000 Voids ratio (%)"
75 " 20.000 Hydraulic conductivity"
76 " 0.000 Trench gradient (%)"
77 " 85.000 Trench length"
78 " 1.000 Include base width"
79 " 21. Number of stages"
80 " Level Discharge Volume"
81 " 423.500 0.000 0.0"
82 " 423.550 0.000 5.9"
83 " 423.600 0.000 11.9"
84 " 423.650 0.000 17.8"
85 " 423.700 0.000 23.8"
86 " 423.750 0.000 29.8"
87 " 423.800 0.000 35.7"
88 " 423.850 0.000 41.7"
89 " 423.900 0.000 47.6"
90 " 423.950 0.000 53.6"
91 " 424.000 1.458 59.5"
92 " 424.050 4.123 59.6"
93 " 424.100 7.577 59.6"
94 " 424.150 11.664 59.7"
95 " 424.200 16.303 59.7"
96 " 424.250 21.430 59.8"
97 " 424.300 27.003 59.8"
98 " 424.350 32.994 59.9"
99 " 424.400 39.369 60.0"
100 " 424.450 46.112 60.0"
101 " 424.500 53.197 60.1"
102 " 1. WEIRS"
103 " Crest Weir Crest Left Right"
104 " elevation coefficient breadth sideslope sideslope"
105 " 423.950 0.900 85.000 0.000 0.000"
106 " 1. MANHOLE"
107 " Access"
108 " diameter"
109 " 1.200"
110 " Peak outflow 0.000 c.m/sec"
111 " Outflow volume 0.002 c.m"
112 " Peak exfiltration 0.002 c.m/sec"
113 " Exfiltration volume 37.465 c.m"
114 " Maximum level 423.693 metre"
115 " Maximum storage 22.941 c.m"
116 " Centroidal lag 3.895 hours"
117 " Infiltration area 2 sides 32.773 sq.metre"
118 " Infiltration Base area 297.500 sq.metre"
119 " 0.030 0.030 0.000 0.002 c.m/sec"
120 " 40 HYDROGRAPH Combine 1"
121 " 6 Combine "
122 " 1 Node #"
123 " Total Site"
124 " Maximum flow 0.000 c.m/sec"
125 " Hydrograph volume 0.002 c.m"
126 " 0.030 0.030 0.000 0.000"
127 " 40 HYDROGRAPH Start - New Tributary"
128 " 2 Start - New Tributary"

```

129 " " 0.030 0.000 0.000 0.000"

130 " 33 CATCHMENT 202"

131 " 1 Triangular SCS"

132 " 1 Equal length"

133 " 1 SCS method"

134 " 202 Controlled Area to Highway 6"

135 " 75.000 % Impervious"

136 " 0.773 Total Area"

137 " 15.000 Flow length"

138 " 2.000 Overland Slope"

139 " 0.193 Pervious Area"

140 " 15.000 Pervious length"

141 " 2.000 Pervious slope"

142 " 0.580 Impervious Area"

143 " 15.000 Impervious length"

144 " 2.000 Impervious slope"

145 " 0.250 Pervious Manning 'n' "

146 " 75.000 Pervious SCS Curve No. "

147 " 0.156 Pervious Runoff coefficient"

148 " 0.100 Pervious Ia/S coefficient"

149 " 8.467 Pervious Initial abstraction"

150 " 0.015 Impervious Manning 'n' "

151 " 98.000 Impervious SCS Curve No. "

152 " 0.825 Impervious Runoff coefficient"

153 " 0.100 Impervious Ia/S coefficient"

154 " 0.518 Impervious Initial abstraction"

155 " " 0.137 0.000 0.000 0.000 c.m/sec"

156 " Catchment 202 Pervious Impervious Total Area "

157 " Surface Area 0.193 0.580 0.773 hectare"

158 " Time of concentration 17.976 1.412 2.392 minutes"

159 " Time to Centroid 131.637 92.372 94.695 minutes"

160 " Rainfall depth 31.396 31.396 31.396 mm"

161 " Rainfall volume 60.67 182.02 242.69 c.m"

162 " Rainfall losses 26.512 5.505 10.757 mm"

163 " Runoff depth 4.883 25.891 20.639 mm"

164 " Runoff volume 9.44 150.10 159.54 c.m"

165 " Runoff coefficient 0.156 0.825 0.657 "

166 " Maximum flow 0.002 0.137 0.137 c.m/sec"

167 " 40 HYDROGRAPH Add Runoff "

168 " 4 Add Runoff "

169 " " 0.137 0.137 0.000 0.000"

170 " 54 POND DESIGN"

171 " 0.137 Current peak flow c.m/sec"

172 " 0.180 Target outflow c.m/sec"

173 " 159.5 Hydrograph volume c.m"

174 " 13. Number of stages"

175 " 422.046 Minimum water level metre"

176 " 424.050 Maximum water level metre"

177 " 422.046 Starting water level metre"

178 " 0 Keep Design Data: 1 = True; 0 = False"

179 " Level Discharge Volume"

180 " 422.046 0.000 0.000"

181 " 422.350 0.00809 1.01E-05"

182 " 422.600 0.01139 12.800"

183 " 422.800 0.01346 23.000"

184 " 423.000 0.01526 61.200"

185 " 423.260 0.01731 110.900"

186 " 423.500 0.01901 187.100"

187 " 423.660 0.02007 216.500"

188 " 423.830 0.02113 226.000"

189 " 423.880 0.02143 228.700"

190 " 423.930 0.02173 241.000"

191 " 423.980 0.02203 264.900"

192 " 424.050 0.05083 264.900"

193 " 1. WEIRS"

194 " Crest Weir Crest Left Right"

195 " elevation coefficient breadth sideslope sideslope"

196 " 423.980 0.900 1.000 0.000 0.000"

197 " 1. ORIFICES"

198 " Orifice Orifice Orifice Number of"

199 " invert coefficient diameter orifices"

200 " 422.046 0.820 0.0750 1.000"

201 " Peak outflow 0.016 c.m/sec"

202 " Maximum level 423.053 metre"

203 " Maximum storage 71.277 c.m"

204 " Centroidal lag 2.375 hours"

205 " 0.137 0.137 0.016 0.000 c.m/sec"

206 " 40 HYDROGRAPH Combine 2"

207 " 6 Combine "

208 " 2 Node #"

209 " Total to Highway 6"

210 " Maximum flow 0.016 c.m/sec"

211 " Hydrograph volume 159.111 c.m"

212 " 0.137 0.137 0.016 0.016"

213 " 40 HYDROGRAPH Start - New Tributary"

214 " 2 Start - New Tributary"

215 " 0.137 0.000 0.016 0.016"

216 " 33 CATCHMENT 203"

217 " 1 Triangular SCS"

218 " 1 Equal length"

219 " 1 SCS method"

220 " 203 Uncontrolled to Rear Yard Amended Topsoil then to Hwy 6"

221 " 25.000 % Impervious"

222 " 0.352 Total Area"

223 " 120.000 Flow length"

224 " 2.000 Overland Slope"

225 " 0.264 Pervious Area"

226 " 120.000 Pervious length"

227 " 2.000 Pervious slope"

228 " 0.088 Impervious Area"

229 " 120.000 Impervious length"

230 " 2.000 Impervious slope"

231 " 0.250 Pervious Manning 'n' "

232 " 75.000 Pervious SCS Curve No. "

233 " 0.156 Pervious Runoff coefficient"

234 " 0.100 Pervious Ia/S coefficient"

235 " 8.467 Pervious Initial abstraction"

236 " 0.015 Impervious Manning 'n' "

237 " 98.000 Impervious SCS Curve No. "

238 " 0.839 Impervious Runoff coefficient"

239 " 0.100 Impervious Ia/S coefficient"

240 " 0.518 Impervious Initial abstraction"

241 " " 0.018 0.000 0.016 0.016 c.m/sec"

242 " Catchment 203 Pervious Impervious Total Area "

243 " Surface Area 0.264 0.088 0.352 hectare"

244 " Time of concentration 62.596 4.918 25.535 minutes"

245 " Time to Centroid 189.486 98.108 130.770 minutes"

246 " Rainfall depth 31.396 31.396 31.396 mm"

247 " Rainfall volume 82.88 27.63 110.51 c.m"

248 " Rainfall losses 26.510 5.049 21.145 mm"

249 " Runoff depth 4.885 26.347 10.251 mm"

250 " Runoff volume 12.90 23.19 36.08 c.m"

251 " Runoff coefficient 0.156 0.839 0.326 "

252 " Maximum flow 0.001 0.018 0.018 c.m/sec"

253 " 40 HYDROGRAPH Add Runoff "

254 " 4 Add Runoff "

255 " " 0.018 0.018 0.016 0.016"

256 " 57 TRENCH Design d/s of 203"

257	"	0.018	Peak inflow"						
258	"	36.082	Hydrograph volume"						
259	"	424.500	Ground elevation"						
260	"	423.500	Downstream trench invert"						
261	"	0.500	Trench height"						
262	"	422.500	Water table elevation"						
263	"	3.500	Trench top width"						
264	"	3.500	Trench bottom width"						
265	"	40.000	Void ratio (%)"						
266	"	20.000	Hydraulic conductivity"						
267	"	0.000	Trench gradient (%)"						
268	"	130.000	Trench length"						
269	"	1.000	Include base width"						
270	"	21.	Number of stages"						
271	"		Level Discharge	Volume"					
272	"	423.500	0.000	0.000	0.0"				
273	"	423.550	0.000	9.1"					
274	"	423.600	0.000	18.2"					
275	"	423.650	0.000	27.3"					
276	"	423.700	0.000	36.4"					
277	"	423.750	0.000	45.5"					
278	"	423.800	0.000	54.6"					
279	"	423.850	0.000	63.7"					
280	"	423.900	0.000	72.8"					
281	"	423.950	0.000	81.9"					
282	"	424.000	0.017	91.0"					
283	"	424.050	0.049	91.1"					
284	"	424.100	0.089	91.1"					
285	"	424.150	0.137	91.2"					
286	"	424.200	0.192	91.2"					
287	"	424.250	0.252	91.3"					
288	"	424.300	0.318	91.3"					
289	"	424.350	0.388	91.4"					
290	"	424.400	0.463	91.5"					
291	"	424.450	0.543	91.5"					
292	"	424.500	0.626	91.6"					
293	"	1.	WEIRS"						
294	"		Crest Weir	Crest	Left	Right			
295	"		elevation coefficie	breadth	sideslope	sideslope"			
296	"	423.950	0.900	1.000	0.000	0.000"			
297	"	1.	MANHOLE"						
298	"		Access"						
299	"		diameter"						
300	"	1.200"							
301	"		Peak outflow	0.000	c.m/sec"				
302	"		Outflow volume	0.001	c.m"				
303	"		Peak exfiltration	0.003	c.m/sec"				
304	"		Exfiltration volume	36.070	c.m"				
305	"		Maximum level	423.571	metre"				
306	"		Maximum storage	12.943	c.m"				
307	"		Centroidal lag	3.302	hours"				
308	"		Infiltration area 2 sides	18.488	sq.metre"				
309	"		Infiltration Base area	455.000	sq.metre"				
310	"	0.018	0.018	0.000	0.003	c.m/sec"			
311	"	40	HYDROGRAPH Combine	2"					
312	"	6	Combine "						
313	"	2	Node #"						
314	"		Total to Highway 6"						
315	"		Maximum flow	0.016	c.m/sec"				
316	"		Hydrograph volume	159.112	c.m"				
317	"	0.018	0.018	0.000	0.016"				
318	"	40	HYDROGRAPH Start - New Tributary"						
319	"	2	Start - New Tributary"						
320	"	0.018	0.000	0.000	0.016"				

321	"	33	CATCHMENT 204"						
322	"	1	Triangular SCS"						
323	"	1	Equal length"						
324	"	1	SCS method"						
325	"	204	Uncontrolled to Highway 6"						
326	"	0.000	% Impervious"						
327	"	0.049	Total Area"						
328	"	20.000	Flow length"						
329	"	2.000	Overland Slope"						
330	"	0.049	Pervious Area"						
331	"	20.000	Pervious length"						
332	"	2.000	Pervious slope"						
333	"	0.000	Impervious Area"						
334	"	20.000	Impervious length"						
335	"	2.000	Impervious slope"						
336	"	0.250	Pervious Manning 'n'"						
337	"	75.000	Pervious SCS Curve No."						
338	"	0.156	Pervious Runoff coefficient"						
339	"	0.100	Pervious Ia/S coefficient"						
340	"	8.467	Pervious Initial abstraction"						
341	"	0.015	Impervious Manning 'n'"						
342	"	98.000	Impervious SCS Curve No."						
343	"	0.000	Impervious Runoff coefficient"						
344	"	0.100	Impervious Ia/S coefficient"						
345	"	0.518	Impervious Initial abstraction"						
346	"	0.001	0.000	0.000	0.016	c.m/sec"			
347	"		Catchment 204	Pervious	Impervious	Total Area			
348	"		Surface Area	0.049	0.000	0.049	hectare"		
349	"		Time of concentration	21.363	1.678	21.363	minutes"		
350	"		Time to Centroid	136.032	92.661	136.032	minutes"		
351	"		Rainfall depth	31.396	31.396	31.396	mm"		
352	"		Rainfall volume	15.38	0.00	15.38	c.m"		
353	"		Rainfall losses	26.513	5.389	26.513	mm"		
354	"		Runoff depth	4.883	26.006	4.883	mm"		
355	"		Runoff volume	2.39	0.00	2.39	c.m"		
356	"		Runoff coefficient	0.156	0.000	0.156	"		
357	"		Maximum flow	0.001	0.000	0.001	c.m/sec"		
358	"	40	HYDROGRAPH Add Runoff "						
359	"	4	Add Runoff "						
360	"		0.001	0.001	0.000	0.016"			
361	"	40	HYDROGRAPH Copy to Outflow"						
362	"	8	Copy to Outflow"						
363	"		0.001	0.001	0.001	0.016"			
364	"	40	HYDROGRAPH Combine	2"					
365	"	6	Combine "						
366	"	2	Node #"						
367	"		Total to Highway 6"						
368	"		Maximum flow		0.016	c.m/sec"			
369	"		Hydrograph volume		161.505	c.m"			
370	"		0.001	0.001	0.001	0.016"			
371	"	40	HYDROGRAPH Confluence	2"					
372	"	7	Confluence "						
373	"	2	Node #"						
374	"		Total to Highway 6"						
375	"		Maximum flow		0.016	c.m/sec"			
376	"		Hydrograph volume		161.505	c.m"			
377	"		0.001	0.016	0.001	0.000"			
378	"	40	HYDROGRAPH Copy to Outflow"						
379	"	8	Copy to Outflow"						
380	"		0.001	0.016	0.016	0.000"			
381	"	40	HYDROGRAPH Combine	1"					
382	"	6	Combine "						
383	"	1	Node #"						
384	"		Total Site"						

```

385 "      Maximum flow          0.016  c.m/sec"
386 "      Hydrograph volume     161.507  c.m"
387 "      0.001    0.016    0.016    0.016"
388 " 40 HYDROGRAPH Confluence 1"
389 " 7 Confluence "
390 " 1 Node #"
391 "      Total Site"
392 "      Maximum flow          0.016  c.m/sec"
393 "      Hydrograph volume     161.507  c.m"
394 "      0.001    0.016    0.016    0.000"
395 " 38 START/RE-START TOTALS 1"
396 " 3 Runoff Totals on EXIT"
397 "      Total Catchment area          1.402  hectare"
398 "      Total Impervious area         0.793  hectare"
399 "      Total % impervious          56.573"
400 " 19 EXIT"
401

```


5yr Post

```

1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 5yr post.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/10/2022 at 10:34:16 AM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 500.000 Coefficient A"
17 " 0.240 Constant B"
18 " 0.688 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 160.061 mm/hr"
23 " Total depth 42.145 mm"
24 " 6 005hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 201"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 201 Uncontrolled to Southwest Property Line"
30 " 55.000 % Impervious"
31 " 0.228 Total Area"
32 " 15.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.103 Pervious Area"
35 " 15.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.125 Impervious Area"
38 " 15.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.227 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.855 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.043 0.000 0.000 0.000 c.m/sec"
51 " Catchment 201 Pervious Impervious Total Area "
52 " Surface Area 0.103 0.125 0.228 hectare"
53 " Time of concentration 12.707 1.241 3.285 minutes"
54 " Time to Centroid 122.816 91.016 96.684 minutes"
55 " Rainfall depth 42.145 42.145 42.145 mm"
56 " Rainfall volume 43.24 52.85 96.09 c.m"
57 " Rainfall losses 32.593 6.117 18.031 mm"
58 " Runoff depth 9.552 36.027 24.113 mm"
59 " Runoff volume 9.80 45.18 54.98 c.m"
60 " Runoff coefficient 0.227 0.855 0.572 "
61 " Maximum flow 0.003 0.042 0.043 c.m/sec"
62 " 40 HYDROGRAPH Add Runoff "
63 " 4 Add Runoff "
64 " 0.043 0.043 0.000 0.000"

```

```

65 " 57 TRENCH Design d/s of 201"
66 " 0.043 Peak inflow"
67 " 54.978 Hydrograph volume"
68 " 424.500 Ground elevation"
69 " 423.500 Downstream trench invert"
70 " 0.500 Trench height"
71 " 422.500 Water table elevation"
72 " 3.500 Trench top width"
73 " 3.500 Trench bottom width"
74 " 40.000 Voids ratio (%)"
75 " 20.000 Hydraulic conductivity"
76 " 0.000 Trench gradient (%)"
77 " 85.000 Trench length"
78 " 1.000 Include base width"
79 " 21. Number of stages"
80 " Level Discharge Volume"
81 " 423.500 0.000 0.0"
82 " 423.550 0.000 5.9"
83 " 423.600 0.000 11.9"
84 " 423.650 0.000 17.8"
85 " 423.700 0.000 23.8"
86 " 423.750 0.000 29.8"
87 " 423.800 0.000 35.7"
88 " 423.850 0.000 41.7"
89 " 423.900 0.000 47.6"
90 " 423.950 0.000 53.6"
91 " 424.000 1.458 59.5"
92 " 424.050 4.123 59.6"
93 " 424.100 7.577 59.6"
94 " 424.150 11.664 59.7"
95 " 424.200 16.303 59.7"
96 " 424.250 21.430 59.8"
97 " 424.300 27.003 59.8"
98 " 424.350 32.994 59.9"
99 " 424.400 39.369 60.0"
100 " 424.450 46.112 60.0"
101 " 424.500 53.197 60.1"
102 " 1. WEIRS"
103 " Crest Weir Crest Left Right"
104 " elevation coefficient breadth sideslope sideslope"
105 " 423.950 0.900 85.000 0.000 0.000"
106 " 1. MANHOLE"
107 " Access"
108 " diameter"
109 " 1.200"
110 " Peak outflow 0.000 c.m/sec"
111 " Outflow volume 0.003 c.m"
112 " Peak exfiltration 0.002 c.m/sec"
113 " Exfiltration volume 54.965 c.m"
114 " Maximum level 423.815 metre"
115 " Maximum storage 37.466 c.m"
116 " Centroidal lag 4.819 hours"
117 " Infiltration area 2 sides 53.519 sq.metre"
118 " Infiltration Base area 297.500 sq.metre"
119 " 0.043 0.043 0.000 0.002 c.m/sec"
120 " 40 HYDROGRAPH Combine 1"
121 " 6 Combine "
122 " 1 Node #"
123 " Total Site"
124 " Maximum flow 0.000 c.m/sec"
125 " Hydrograph volume 0.003 c.m"
126 " 0.043 0.043 0.000 0.000"
127 " 40 HYDROGRAPH Start - New Tributary"
128 " 2 Start - New Tributary"

```



```

385 " Maximum flow 0.018 c.m/sec"
386 " Hydrograph volume 232.059 c.m"
387 " 0.002 0.018 0.018 0.018"
388 " 40 HYDROGRAPH Confluence 1"
389 " 7 Confluence "
390 " 1 Node #"
391 " Total Site"
392 " Maximum flow 0.018 c.m/sec"
393 " Hydrograph volume 232.059 c.m"
394 " 0.002 0.018 0.018 0.000"
395 " 38 START/RE-START TOTALS 1"
396 " 3 Runoff Totals on EXIT"
397 " Total Catchment area 1.402 hectare"
398 " Total Impervious area 0.793 hectare"
399 " Total % impervious 56.573"
400 " 19 EXIT"
401

```

10yr Post

```

1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 10yr post.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/10/2022 at 10:31:38 AM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 595.000 Coefficient A"
17 " 0.360 Constant B"
18 " 0.691 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 186.431 mm/hr"
23 " Total depth 49.226 mm"
24 " 6 010hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 201"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 201 Uncontrolled to Southwest Property Line"
30 " 55.000 % Impervious"
31 " 0.228 Total Area"
32 " 15.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.103 Pervious Area"
35 " 15.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.125 Impervious Area"
38 " 15.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.268 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.867 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.052 0.000 0.000 0.000 c.m/sec"
51 " Catchment 201 Pervious Impervious Total Area "
52 " Surface Area 0.103 0.125 0.228 hectare"
53 " Time of concentration 10.916 1.162 3.132 minutes"
54 " Time to Centroid 119.039 90.250 96.065 minutes"
55 " Rainfall depth 49.226 49.226 49.226 mm"
56 " Rainfall volume 50.51 61.73 112.24 c.m"
57 " Rainfall losses 36.015 6.525 19.795 mm"
58 " Runoff depth 13.211 42.701 29.430 mm"
59 " Runoff volume 13.55 53.55 67.10 c.m"
60 " Runoff coefficient 0.268 0.867 0.598 "
61 " Maximum flow 0.005 0.050 0.052 c.m/sec"
62 " 40 HYDROGRAPH Add Runoff "
63 " 4 Add Runoff "
64 " 0.052 0.052 0.000 0.000"

```

```

65 " 57 TRENCH Design d/s of 201"
66 " 0.052 Peak inflow"
67 " 67.101 Hydrograph volume"
68 " 424.500 Ground elevation"
69 " 423.500 Downstream trench invert"
70 " 0.500 Trench height"
71 " 422.500 Water table elevation"
72 " 3.500 Trench top width"
73 " 3.500 Trench bottom width"
74 " 40.000 Voids ratio (%)"
75 " 20.000 Hydraulic conductivity"
76 " 0.000 Trench gradient (%)"
77 " 85.000 Trench length"
78 " 1.000 Include base width"
79 " 21. Number of stages"
80 " Level Discharge Volume"
81 " 423.500 0.000 0.0"
82 " 423.550 0.000 5.9"
83 " 423.600 0.000 11.9"
84 " 423.650 0.000 17.8"
85 " 423.700 0.000 23.8"
86 " 423.750 0.000 29.8"
87 " 423.800 0.000 35.7"
88 " 423.850 0.000 41.7"
89 " 423.900 0.000 47.6"
90 " 423.950 0.000 53.6"
91 " 424.000 1.458 59.5"
92 " 424.050 4.123 59.6"
93 " 424.100 7.577 59.6"
94 " 424.150 11.664 59.7"
95 " 424.200 16.303 59.7"
96 " 424.250 21.430 59.8"
97 " 424.300 27.003 59.8"
98 " 424.350 32.994 59.9"
99 " 424.400 39.369 60.0"
100 " 424.450 46.112 60.0"
101 " 424.500 53.197 60.1"
102 " 1. WEIRS"
103 " Crest Weir Crest Left Right"
104 " elevation coefficient breadth sideslope sideslope"
105 " 423.950 0.900 85.000 0.000 0.000"
106 " 1. MANHOLE"
107 " Access"
108 " diameter"
109 " 1.200"
110 " Peak outflow 0.000 c.m/sec"
111 " Outflow volume 0.003 c.m"
112 " Peak exfiltration 0.002 c.m/sec"
113 " Exfiltration volume 67.096 c.m"
114 " Maximum level 423.903 metre"
115 " Maximum storage 47.942 c.m"
116 " Centroidal lag 5.404 hours"
117 " Infiltration area 2 sides 68.487 sq.metre"
118 " Infiltration Base area 297.500 sq.metre"
119 " 0.052 0.052 0.000 0.002 c.m/sec"
120 " 40 HYDROGRAPH Combine 1"
121 " 6 Combine "
122 " 1 Node #"
123 " Total Site"
124 " Maximum flow 0.000 c.m/sec"
125 " Hydrograph volume 0.003 c.m"
126 " 0.052 0.052 0.000 0.000"
127 " 40 HYDROGRAPH Start - New Tributary"
128 " 2 Start - New Tributary"

```

129	"	0.052	0.000	0.000	0.000"	193	"	1.	WEIRS"					
130	"	33	CATCHMENT 202"			194	"		Crest	Weir	Crest	Left	Right"	
131	"	1	Triangular SCS"			195	"		elevation	coefficient	breadth	sideslope	sideslope"	
132	"	1	Equal length"			196	"		423.980	0.900	1.000	0.000	0.000"	
133	"	1	SCS method"			197	"	1.	ORIFICES"					
134	"	202	Controlled Area to Highway 6"			198	"		Orifice	Orifice	Orifice	Number of		
135	"	75.000	% Impervious"			199	"		invert	coefficient	diameter	orifices"		
136	"	0.773	Total Area"			200	"		422.046	0.820	0.0750	1.000"		
137	"	15.000	Flow length"			201	"		Peak outflow		0.018	c.m/sec"		
138	"	2.000	Overland Slope"			202	"		Maximum level		423.374	metre"		
139	"	0.193	Pervious Area"			203	"		Maximum storage		146.966	c.m"		
140	"	15.000	Pervious length"			204	"		Centroidal lag		2.998	hours"		
141	"	2.000	Pervious slope"			205	"		0.235	0.235	0.018	0.000	c.m/sec"	
142	"	0.580	Impervious Area"			206	"	40	HYDROGRAPH	Combine	2"			
143	"	15.000	Impervious length"			207	"	6	Combine	"				
144	"	2.000	Impervious slope"			208	"	2	Node #"					
145	"	0.250	Pervious Manning 'n'"			209	"		Total to Highway 6"					
146	"	75.000	Pervious SCS Curve No."			210	"		Maximum flow		0.018	c.m/sec"		
147	"	0.268	Pervious Runoff coefficient"			211	"		Hydrograph volume		272.718	c.m"		
148	"	0.100	Pervious Ia/S coefficient"			212	"		0.235	0.235	0.018	0.018"		
149	"	8.467	Pervious Initial abstraction"			213	"	40	HYDROGRAPH Start - New Tributary"					
150	"	0.015	Impervious Manning 'n'"			214	"	2	Start - New Tributary"					
151	"	98.000	Impervious SCS Curve No."			215	"		0.235	0.000	0.018	0.018"		
152	"	0.867	Impervious Runoff coefficient"			216	"	33	CATCHMENT 203"					
153	"	0.100	Impervious Ia/S coefficient"			217	"	1	Triangular SCS"					
154	"	0.518	Impervious Initial abstraction"			218	"	1	Equal length"					
155	"	0.235	0.000	0.000	0.000 c.m/sec"	219	"	1	SCS method"					
156	"	Catchment 202	Pervious	Impervious	Total Area "	220	"	203	Uncontrolled to Rear Yard Amended Topsoil then to Hwy 6"					
157	"	Surface Area	0.193	0.580	0.773 hectare"	221	"	25.000	% Impervious"					
158	"	Time of concentration	10.916	1.162	2.073 minutes"	222	"	0.352	Total Area"					
159	"	Time to Centroid	119.039	90.250	92.941 minutes"	223	"	120.000	Flow length"					
160	"	Rainfall depth	49.226	49.226	49.226 mm"	224	"	2.000	Overland Slope"					
161	"	Rainfall volume	95.13	285.39	380.52 c.m"	225	"	0.264	Pervious Area"					
162	"	Rainfall losses	36.015	6.525	13.898 mm"	226	"	120.000	Pervious length"					
163	"	Runoff depth	13.211	42.701	35.328 mm"	227	"	2.000	Pervious slope"					
164	"	Runoff volume	25.53	247.56	273.09 c.m"	228	"	0.088	Impervious Area"					
165	"	Runoff coefficient	0.268	0.867	0.718 "	229	"	120.000	Impervious length"					
166	"	Maximum flow	0.010	0.232	0.235 c.m/sec"	230	"	2.000	Impervious slope"					
167	"	40	HYDROGRAPH Add Runoff "			231	"	0.250	Pervious Manning 'n'"					
168	"	4	Add Runoff "			232	"	75.000	Pervious SCS Curve No."					
169	"	0.235	0.235	0.000	0.000"	233	"	0.269	Pervious Runoff coefficient"					
170	"	54	POND DESIGN"			234	"	0.100	Pervious Ia/S coefficient"					
171	"	0.235	Current peak flow	c.m/sec"		235	"	8.467	Pervious Initial abstraction"					
172	"	0.180	Target outflow	c.m/sec"		236	"	0.015	Impervious Manning 'n'"					
173	"	273.1	Hydrograph volume	c.m"		237	"	98.000	Impervious SCS Curve No."					
174	"	13.	Number of stages"			238	"	0.881	Impervious Runoff coefficient"					
175	"	422.046	Minimum water level	metre"		239	"	0.100	Impervious Ia/S coefficient"					
176	"	424.050	Maximum water level	metre"		240	"	0.518	Impervious Initial abstraction"					
177	"	422.046	Starting water level	metre"		241	"	0.029	0.000	0.018	0.018	c.m/sec"		
178	"	0	Keep Design Data: 1 = True; 0 = False"			242	"	Catchment 203	Pervious	Impervious	Total Area "			
179	"		Level Discharge	Volume"		243	"	Surface Area	0.264	0.088	0.352 hectare"			
180	"	422.046	0.000	0.000"		244	"	Time of concentration	38.012	4.045	20.284 minutes"			
181	"	422.350	0.00809	1.01E-05"		245	"	Time to Centroid	157.603	95.239	125.054 minutes"			
182	"	422.600	0.01139	12.800"		246	"	Rainfall depth	49.226	49.226	49.226 mm"			
183	"	422.800	0.01346	23.000"		247	"	Rainfall volume	129.96	43.32	173.28 c.m"			
184	"	423.000	0.01526	61.200"		248	"	Rainfall losses	35.991	5.881	28.463 mm"			
185	"	423.260	0.01731	110.900"		249	"	Runoff depth	13.235	43.345	20.763 mm"			
186	"	423.500	0.01901	187.100"		250	"	Runoff volume	34.94	38.14	73.08 c.m"			
187	"	423.660	0.02007	216.500"		251	"	Runoff coefficient	0.269	0.881	0.422 "			
188	"	423.830	0.02113	226.000"		252	"	Maximum flow	0.006	0.028	0.029 c.m/sec"			
189	"	423.880	0.02143	228.700"		253	"	40	HYDROGRAPH Add Runoff "					
190	"	423.930	0.02173	241.000"		254	"	4	Add Runoff "					
191	"	423.980	0.02203	264.900"		255	"	0.029	0.029	0.018	0.018"			
192	"	424.050	0.05083	264.900"		256	"	57	TRENCH Design d/s of 203"					


```

385 " Maximum flow 0.020 c.m/sec"
386 " Hydrograph volume 279.182 c.m"
387 " 0.002 0.020 0.020 0.020"
388 " 40 HYDROGRAPH Confluence 1"
389 " 7 Confluence "
390 " 1 Node #"
391 " Total Site"
392 " Maximum flow 0.020 c.m/sec"
393 " Hydrograph volume 279.182 c.m"
394 " 0.002 0.020 0.020 0.000"
395 " 38 START/RE-START TOTALS 1"
396 " 3 Runoff Totals on EXIT"
397 " Total Catchment area 1.402 hectare"
398 " Total Impervious area 0.793 hectare"
399 " Total % impervious 56.573"
400 " 19 EXIT"
401

```


25yr Post

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1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 25yr post.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/10/2022 at 10:28:05 AM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 702.000 Coefficient A"
17 " 0.350 Constant B"
18 " 0.690 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 220.574 mm/hr"
23 " Total depth 58.353 mm"
24 " 6 025hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 201"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 201 Uncontrolled to Southwest Property Line"
30 " 55.000 % Impervious"
31 " 0.228 Total Area"
32 " 15.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.103 Pervious Area"
35 " 15.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.125 Impervious Area"
38 " 15.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.316 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.879 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.064 0.000 0.000 0.000 c.m/sec"
51 " Catchment 201 Pervious Impervious Total Area "
52 " Surface Area 0.103 0.125 0.228 hectare"
53 " Time of concentration 9.379 1.081 2.966 minutes"
54 " Time to Centroid 115.782 89.548 95.508 minutes"
55 " Rainfall depth 58.353 58.353 58.353 mm"
56 " Rainfall volume 59.87 73.17 133.04 c.m"
57 " Rainfall losses 39.929 7.077 21.861 mm"
58 " Runoff depth 18.424 51.276 36.492 mm"
59 " Runoff volume 18.90 64.30 83.20 c.m"
60 " Runoff coefficient 0.316 0.879 0.625 "
61 " Maximum flow 0.008 0.061 0.064 c.m/sec"
62 " 40 HYDROGRAPH Add Runoff "
63 " 4 Add Runoff "
64 " 0.064 0.064 0.000 0.000"

```

```

65 " 57 TRENCH Design d/s of 201"
66 " 0.064 Peak inflow"
67 " 83.202 Hydrograph volume"
68 " 424.500 Ground elevation"
69 " 423.500 Downstream trench invert"
70 " 0.500 Trench height"
71 " 422.500 Water table elevation"
72 " 3.500 Trench top width"
73 " 3.500 Trench bottom width"
74 " 40.000 Voids ratio (%)"
75 " 20.000 Hydraulic conductivity"
76 " 0.000 Trench gradient (%)"
77 " 85.000 Trench length"
78 " 1.000 Include base width"
79 " 21. Number of stages"
80 " Level Discharge Volume"
81 " 423.500 0.000 0.0"
82 " 423.550 0.000 5.9"
83 " 423.600 0.000 11.9"
84 " 423.650 0.000 17.8"
85 " 423.700 0.000 23.8"
86 " 423.750 0.000 29.8"
87 " 423.800 0.000 35.7"
88 " 423.850 0.000 41.7"
89 " 423.900 0.000 47.6"
90 " 423.950 0.000 53.6"
91 " 424.000 1.458 59.5"
92 " 424.050 4.123 59.6"
93 " 424.100 7.577 59.6"
94 " 424.150 11.664 59.7"
95 " 424.200 16.303 59.7"
96 " 424.250 21.430 59.8"
97 " 424.300 27.003 59.8"
98 " 424.350 32.994 59.9"
99 " 424.400 39.369 60.0"
100 " 424.450 46.112 60.0"
101 " 424.500 53.197 60.1"
102 " 1. WEIRS"
103 " Crest Weir Crest Left Right"
104 " elevation coefficient breadth sideslope sideslope"
105 " 423.950 0.900 85.000 0.000 0.000"
106 " 1. MANHOLE"
107 " Access"
108 " diameter"
109 " 1.200"
110 " Peak outflow 0.004 c.m/sec"
111 " Outflow volume 8.290 c.m"
112 " Peak exfiltration 0.003 c.m/sec"
113 " Exfiltration volume 74.391 c.m"
114 " Maximum level 423.950 metre"
115 " Maximum storage 53.585 c.m"
116 " Centroidal lag 2.262 hours"
117 " Infiltration area 2 sides 76.549 sq.metre"
118 " Infiltration Base area 297.500 sq.metre"
119 " 0.064 0.064 0.004 0.003 c.m/sec"
120 " 40 HYDROGRAPH Combine 1"
121 " 6 Combine "
122 " 1 Node #"
123 " Total Site"
124 " Maximum flow 0.004 c.m/sec"
125 " Hydrograph volume 8.290 c.m"
126 " 0.064 0.064 0.004 0.004"
127 " 40 HYDROGRAPH Start - New Tributary"
128 " 2 Start - New Tributary"

```

129 " 0.064 0.000 0.004 0.004"

130 " 33 CATCHMENT 202"

131 " 1 Triangular SCS"

132 " 1 Equal length"

133 " 1 SCS method"

134 " 202 Controlled Area to Highway 6"

135 " 75.000 % Impervious"

136 " 0.773 Total Area"

137 " 15.000 Flow length"

138 " 2.000 Overland Slope"

139 " 0.193 Pervious Area"

140 " 15.000 Pervious length"

141 " 2.000 Pervious slope"

142 " 0.580 Impervious Area"

143 " 15.000 Impervious length"

144 " 2.000 Impervious slope"

145 " 0.250 Pervious Manning 'n'"

146 " 75.000 Pervious SCS Curve No."

147 " 0.316 Pervious Runoff coefficient"

148 " 0.100 Pervious Ia/S coefficient"

149 " 8.467 Pervious Initial abstraction"

150 " 0.015 Impervious Manning 'n'"

151 " 98.000 Impervious SCS Curve No."

152 " 0.879 Impervious Runoff coefficient"

153 " 0.100 Impervious Ia/S coefficient"

154 " 0.518 Impervious Initial abstraction"

155 " 0.286 0.000 0.004 0.004 c.m/sec"

Catchment 202	Pervious	Impervious	Total Area	
Surface Area	0.193	0.580	0.773	hectare"
Time of concentration	9.379	1.081	1.969	minutes"
Time to Centroid	115.782	89.548	92.354	minutes"
Rainfall depth	58.353	58.353	58.353	mm"
Rainfall volume	112.77	338.30	451.07	c.m"
Rainfall losses	39.929	7.077	15.290	mm"
Runoff depth	18.424	51.276	43.063	mm"
Runoff volume	35.60	297.27	332.87	c.m"
Runoff coefficient	0.316	0.879	0.738	"
Maximum flow	0.015	0.281	0.286	c.m/sec"

156 " Catchment 202 Pervious Impervious Total Area "

157 " Surface Area 0.193 0.580 0.773 hectare"

158 " Time of concentration 9.379 1.081 1.969 minutes"

159 " Time to Centroid 115.782 89.548 92.354 minutes"

160 " Rainfall depth 58.353 58.353 58.353 mm"

161 " Rainfall volume 112.77 338.30 451.07 c.m"

162 " Rainfall losses 39.929 7.077 15.290 mm"

163 " Runoff depth 18.424 51.276 43.063 mm"

164 " Runoff volume 35.60 297.27 332.87 c.m"

165 " Runoff coefficient 0.316 0.879 0.738 "

166 " Maximum flow 0.015 0.281 0.286 c.m/sec"

167 " 40 HYDROGRAPH Add Runoff "

168 " 4 Add Runoff "

169 " 0.286 0.286 0.004 0.004"

170 " 54 POND DESIGN"

Current peak flow	c.m/sec"
0.286	
Target outflow	c.m/sec"
0.180	
Hydrograph volume	c.m"
332.9	
Number of stages	
13	
Minimum water level	metre"
422.046	
Maximum water level	metre"
424.050	
Starting water level	metre"
422.046	
0	
Keep Design Data: 1 = True; 0 = False"	
Level Discharge	Volume"
422.046	0.000
422.350	0.00809
422.600	0.01139
422.800	0.01346
423.000	0.01526
423.260	0.01731
423.500	0.01901
423.660	0.02007
423.830	0.02113
423.880	0.02143
423.930	0.02173
423.980	0.02203
424.050	0.05083

193 " 1. WEIRS"

194 " Crest Weir Crest Left Right"

195 " elevation coefficie breadth sideslope sideslope"

196 " 423.980 0.900 1.000 0.000 0.000"

197 " 1. ORIFICES"

198 " Orifice Orifice Orifice Number of"

199 " invert coefficie diameter orifices"

200 " 422.046 0.820 0.0750 1.000"

201 " Peak outflow 0.019 c.m/sec"

202 " Maximum level 423.521 metre"

203 " Maximum storage 190.964 c.m"

204 " Centroidal lag 3.315 hours"

205 " 0.286 0.286 0.019 0.004 c.m/sec"

206 " 40 HYDROGRAPH Combine 2"

207 " 6 Combine "

208 " 2 Node #"

209 " Total to Highway 6"

210 " Maximum flow 0.019 c.m/sec"

211 " Hydrograph volume 332.443 c.m"

212 " 0.286 0.286 0.019 0.019"

213 " 40 HYDROGRAPH Start - New Tributary"

214 " 2 Start - New Tributary"

215 " 0.286 0.000 0.019 0.019"

216 " 33 CATCHMENT 203"

217 " 1 Triangular SCS"

218 " 1 Equal length"

219 " 1 SCS method"

220 " 203 Uncontrolled to Rear Yard Amended Topsoil then to Hwy 6"

221 " 25.000 % Impervious"

222 " 0.352 Total Area"

223 " 120.000 Flow length"

224 " 2.000 Overland Slope"

225 " 0.264 Pervious Area"

226 " 120.000 Pervious length"

227 " 2.000 Pervious slope"

228 " 0.088 Impervious Area"

229 " 120.000 Impervious length"

230 " 2.000 Impervious slope"

231 " 0.250 Pervious Manning 'n'"

232 " 75.000 Pervious SCS Curve No."

233 " 0.317 Pervious Runoff coefficient"

234 " 0.100 Pervious Ia/S coefficient"

235 " 8.467 Pervious Initial abstraction"

236 " 0.015 Impervious Manning 'n'"

237 " 98.000 Impervious SCS Curve No."

238 " 0.888 Impervious Runoff coefficient"

239 " 0.100 Impervious Ia/S coefficient"

240 " 0.518 Impervious Initial abstraction"

241 " 0.036 0.000 0.019 0.019 c.m/sec"

Catchment 203	Pervious	Impervious	Total Area	
Surface Area	0.264	0.088	0.352	hectare"
Time of concentration	32.659	3.765	18.703	minutes"
Time to Centroid	149.614	94.370	122.931	minutes"
Rainfall depth	58.353	58.353	58.353	mm"
Rainfall volume	154.05	51.35	205.40	c.m"
Rainfall losses	39.863	6.529	31.529	mm"
Runoff depth	18.490	51.824	26.824	mm"
Runoff volume	48.81	45.61	94.42	c.m"
Runoff coefficient	0.317	0.888	0.460	"
Maximum flow	0.010	0.033	0.036	c.m/sec"

242 " Catchment 203 Pervious Impervious Total Area "

243 " Surface Area 0.264 0.088 0.352 hectare"

244 " Time of concentration 32.659 3.765 18.703 minutes"

245 " Time to Centroid 149.614 94.370 122.931 minutes"

246 " Rainfall depth 58.353 58.353 58.353 mm"

247 " Rainfall volume 154.05 51.35 205.40 c.m"

248 " Rainfall losses 39.863 6.529 31.529 mm"

249 " Runoff depth 18.490 51.824 26.824 mm"

250 " Runoff volume 48.81 45.61 94.42 c.m"

251 " Runoff coefficient 0.317 0.888 0.460 "

252 " Maximum flow 0.010 0.033 0.036 c.m/sec"

253 " 40 HYDROGRAPH Add Runoff "

254 " 4 Add Runoff "

255 " 0.036 0.036 0.019 0.019"

256 " 57 TRENCH Design d/s of 203"

257	"	0.036	Peak inflow"			
258	"	94.419	Hydrograph volume"			
259	"	424.500	Ground elevation"			
260	"	423.500	Downstream trench invert"			
261	"	0.500	Trench height"			
262	"	422.500	Water table elevation"			
263	"	3.500	Trench top width"			
264	"	3.500	Trench bottom width"			
265	"	40.000	Void ratio (%)"			
266	"	20.000	Hydraulic conductivity"			
267	"	0.000	Trench gradient (%)"			
268	"	130.000	Trench length"			
269	"	1.000	Include base width"			
270	"	21.	Number of stages"			
271	"		Level Discharge	Volume"		
272	"	423.500	0.000	0.00		
273	"	423.550	0.000	9.1"		
274	"	423.600	0.000	18.2"		
275	"	423.650	0.000	27.3"		
276	"	423.700	0.000	36.4"		
277	"	423.750	0.000	45.5"		
278	"	423.800	0.000	54.6"		
279	"	423.850	0.000	63.7"		
280	"	423.900	0.000	72.8"		
281	"	423.950	0.000	81.9"		
282	"	424.000	0.017	91.0"		
283	"	424.050	0.049	91.1"		
284	"	424.100	0.089	91.1"		
285	"	424.150	0.137	91.2"		
286	"	424.200	0.192	91.2"		
287	"	424.250	0.252	91.3"		
288	"	424.300	0.318	91.3"		
289	"	424.350	0.388	91.4"		
290	"	424.400	0.463	91.5"		
291	"	424.450	0.543	91.5"		
292	"	424.500	0.626	91.6"		
293	"	1.	WEIRS"			
294	"		Crest Weir	Crest	Left	Right"
295	"		elevation coefficie	breadth	sideslope	sideslope"
296	"		423.950 0.900	1.000	0.000	0.000"
297	"	1.	MANHOLE"			
298	"		Access"			
299	"		diameter"			
300	"		1.200"			
301	"		Peak outflow	0.000		c.m/sec"
302	"		Outflow volume	0.003		c.m"
303	"		Peak exfiltration	0.003		c.m/sec"
304	"		Exfiltration volume	94.460		c.m"
305	"		Maximum level	423.822		metre"
306	"		Maximum storage	58.668		c.m"
307	"		Centroidal lag	5.373		hours"
308	"		Infiltration area 2 sides	83.813		sq.metre"
309	"		Infiltration Base area	455.000		sq.metre"
310	"		0.036 0.036	0.000	0.003	c.m/sec"
311	"	40	HYDROGRAPH Combine	2"		
312	"		6 Combine "			
313	"		2 Node #"			
314	"		Total to Highway 6"			
315	"		Maximum flow	0.019		c.m/sec"
316	"		Hydrograph volume	332.446		c.m"
317	"		0.036 0.036	0.000	0.019"	
318	"	40	HYDROGRAPH Start - New Tributary"			
319	"		2 Start - New Tributary"			
320	"		0.036 0.000	0.000	0.019"	

321	"	33	CATCHMENT 204"			
322	"		1 Triangular SCS"			
323	"		1 Equal length"			
324	"		1 SCS method"			
325	"		204 Uncontrolled to Highway 6"			
326	"		0.000 % Impervious"			
327	"		0.049 Total Area"			
328	"		20.000 Flow length"			
329	"		2.000 Overland Slope"			
330	"		0.049 Pervious Area"			
331	"		20.000 Pervious length"			
332	"		2.000 Pervious slope"			
333	"		0.000 Impervious Area"			
334	"		20.000 Impervious length"			
335	"		2.000 Impervious slope"			
336	"		0.250 Pervious Manning 'n'"			
337	"		75.000 Pervious SCS Curve No."			
338	"		0.317 Pervious Runoff coefficient"			
339	"		0.100 Pervious Ia/S coefficient"			
340	"		8.467 Pervious Initial abstraction"			
341	"		0.015 Impervious Manning 'n'"			
342	"		98.000 Impervious SCS Curve No."			
343	"		0.000 Impervious Runoff coefficient"			
344	"		0.100 Impervious Ia/S coefficient"			
345	"		0.518 Impervious Initial abstraction"			
346	"		0.004 0.000 0.000		0.019	c.m/sec"
347	"		Catchment 204 Pervious	Impervious	Total Area	"
348	"		Surface Area	0.049	0.000	0.049 hectare"
349	"		Time of concentration	11.146	1.285	11.146 minutes"
350	"		Time to Centroid	118.302	90.077	118.302 minutes"
351	"		Rainfall depth	58.353	58.353	58.353 mm"
352	"		Rainfall volume	28.59	0.00	28.59 c.m"
353	"		Rainfall losses	39.879	6.582	39.879 mm"
354	"		Runoff depth	18.473	51.771	18.473 mm"
355	"		Runoff volume	9.05	0.00	9.05 c.m"
356	"		Runoff coefficient	0.317	0.000	0.317 "
357	"		Maximum flow	0.004	0.000	0.004 c.m/sec"
358	"	40	HYDROGRAPH Add Runoff "			
359	"		4 Add Runoff "			
360	"		0.004 0.004	0.000	0.019"	
361	"	40	HYDROGRAPH Copy to Outflow"			
362	"		8 Copy to Outflow"			
363	"		0.004 0.004	0.004	0.019"	
364	"	40	HYDROGRAPH Combine 2"			
365	"		6 Combine "			
366	"		2 Node #"			
367	"		Total to Highway 6"			
368	"		Maximum flow		0.022	c.m/sec"
369	"		Hydrograph volume		341.498	c.m"
370	"		0.004 0.004	0.004	0.022"	
371	"	40	HYDROGRAPH Confluence 2"			
372	"		7 Confluence "			
373	"		2 Node #"			
374	"		Total to Highway 6"			
375	"		Maximum flow		0.022	c.m/sec"
376	"		Hydrograph volume		341.498	c.m"
377	"		0.004 0.022	0.004	0.000"	
378	"	40	HYDROGRAPH Copy to Outflow"			
379	"		8 Copy to Outflow"			
380	"		0.004 0.022	0.022	0.000"	
381	"	40	HYDROGRAPH Combine 1"			
382	"		6 Combine "			
383	"		1 Node #"			
384	"		Total Site"			

```

385 "      Maximum flow           0.025  c.m/sec"
386 "      Hydrograph volume      349.789  c.m"
387 "      0.004    0.022    0.022    0.025"
388 " 40  HYDROGRAPH Confluence  1"
389 "      7 Confluence "
390 "      1 Node #"
391 "      Total Site"
392 "      Maximum flow           0.025  c.m/sec"
393 "      Hydrograph volume      349.789  c.m"
394 "      0.004    0.025    0.022    0.000"
395 " 38  START/RE-START TOTALS 1"
396 "      3 Runoff Totals on EXIT"
397 "      Total Catchment area          1.402  hectare"
398 "      Total Impervious area         0.793  hectare"
399 "      Total % impervious          56.573"
400 " 19  EXIT"
401

```

50yr Post

```

1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 50yr post.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/10/2022 at 10:24:48 AM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 780.000 Coefficient A"
17 " 0.360 Constant B"
18 " 0.690 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 244.972 mm/hr"
23 " Total depth 65.003 mm"
24 " 6 050hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 201"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 201 Uncontrolled to Southwest Property Line"
30 " 55.000 % Impervious"
31 " 0.228 Total Area"
32 " 15.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.103 Pervious Area"
35 " 15.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.125 Impervious Area"
38 " 15.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.347 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.885 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.072 0.000 0.000 0.000 c.m/sec"
51 " Catchment 201 Pervious Impervious Total Area "
52 " Surface Area 0.103 0.125 0.228 hectare"
53 " Time of concentration 8.579 1.034 2.868 minutes"
54 " Time to Centroid 113.921 89.143 95.166 minutes"
55 " Rainfall depth 65.003 65.003 65.003 mm"
56 " Rainfall volume 66.69 81.51 148.21 c.m"
57 " Rainfall losses 42.421 7.474 23.200 mm"
58 " Runoff depth 22.581 57.528 41.802 mm"
59 " Runoff volume 23.17 72.14 95.31 c.m"
60 " Runoff coefficient 0.347 0.885 0.643 "
61 " Maximum flow 0.011 0.068 0.072 c.m/sec"
62 " 40 HYDROGRAPH Add Runoff "
63 " 4 Add Runoff "
64 " 0.072 0.072 0.000 0.000"

```

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65 " 57 TRENCH Design d/s of 201"
66 " 0.072 Peak inflow"
67 " 95.309 Hydrograph volume"
68 " 424.500 Ground elevation"
69 " 423.500 Downstream trench invert"
70 " 0.500 Trench height"
71 " 422.500 Water table elevation"
72 " 3.500 Trench top width"
73 " 3.500 Trench bottom width"
74 " 40.000 Voids ratio (%)"
75 " 20.000 Hydraulic conductivity"
76 " 0.000 Trench gradient (%)"
77 " 85.000 Trench length"
78 " 1.000 Include base width"
79 " 21. Number of stages"
80 " Level Discharge Volume"
81 " 423.500 0.000 0.0"
82 " 423.550 0.000 5.9"
83 " 423.600 0.000 11.9"
84 " 423.650 0.000 17.8"
85 " 423.700 0.000 23.8"
86 " 423.750 0.000 29.8"
87 " 423.800 0.000 35.7"
88 " 423.850 0.000 41.7"
89 " 423.900 0.000 47.6"
90 " 423.950 0.000 53.6"
91 " 424.000 1.458 59.5"
92 " 424.050 4.123 59.6"
93 " 424.100 7.577 59.6"
94 " 424.150 11.664 59.7"
95 " 424.200 16.303 59.7"
96 " 424.250 21.430 59.8"
97 " 424.300 27.003 59.8"
98 " 424.350 32.994 59.9"
99 " 424.400 39.369 60.0"
100 " 424.450 46.112 60.0"
101 " 424.500 53.197 60.1"
102 " 1. WEIRS"
103 " Crest Weir Crest Left Right"
104 " elevation coefficient breadth sideslope sideslope"
105 " 423.950 0.900 85.000 0.000 0.000"
106 " 1. MANHOLE"
107 " Access"
108 " diameter"
109 " 1.200"
110 " Peak outflow 0.009 c.m/sec"
111 " Outflow volume 18.482 c.m"
112 " Peak exfiltration 0.003 c.m/sec"
113 " Exfiltration volume 74.997 c.m"
114 " Maximum level 423.951 metre"
115 " Maximum storage 53.630 c.m"
116 " Centroidal lag 2.042 hours"
117 " Infiltration area 2 sides 76.616 sq.metre"
118 " Infiltration Base area 297.500 sq.metre"
119 " 0.072 0.072 0.009 0.003 c.m/sec"
120 " 40 HYDROGRAPH Combine 1"
121 " 6 Combine "
122 " 1 Node #"
123 " Total Site"
124 " Maximum flow 0.009 c.m/sec"
125 " Hydrograph volume 18.482 c.m"
126 " 0.072 0.072 0.009 0.009"
127 " 40 HYDROGRAPH Start - New Tributary"
128 " 2 Start - New Tributary"

```

129 " 0.072 0.000 0.009 0.009"

130 " 33 CATCHMENT 202"

131 " 1 Triangular SCS"

132 " 1 Equal length"

133 " 1 SCS method"

134 " 202 Controlled Area to Highway 6"

135 " 75.000 % Impervious"

136 " 0.773 Total Area"

137 " 15.000 Flow length"

138 " 2.000 Overland Slope"

139 " 0.193 Pervious Area"

140 " 15.000 Pervious length"

141 " 2.000 Pervious slope"

142 " 0.580 Impervious Area"

143 " 15.000 Impervious length"

144 " 2.000 Impervious slope"

145 " 0.250 Pervious Manning 'n' "

146 " 75.000 Pervious SCS Curve No. "

147 " 0.347 Pervious Runoff coefficient"

148 " 0.100 Pervious Ia/S coefficient"

149 " 8.467 Pervious Initial abstraction"

150 " 0.015 Impervious Manning 'n' "

151 " 98.000 Impervious SCS Curve No. "

152 " 0.885 Impervious Runoff coefficient"

153 " 0.100 Impervious Ia/S coefficient"

154 " 0.518 Impervious Initial abstraction"

155 " 0.323 0.000 0.009 0.009 c.m/sec"

Catchment 202	Pervious	Impervious	Total Area	"
Surface Area	0.193	0.580	0.773	hectare
Time of concentration	8.579	1.034	1.907	minutes
Time to Centroid	113.921	89.143	92.010	minutes
Rainfall depth	65.003	65.003	65.003	mm
Rainfall volume	125.62	376.85	502.47	c.m
Rainfall losses	42.421	7.474	16.211	mm
Runoff depth	22.581	57.528	48.791	mm
Runoff volume	43.64	333.52	377.16	c.m
Runoff coefficient	0.347	0.885	0.751	"
Maximum flow	0.021	0.315	0.323	c.m/sec"

156 " Catchment 202 Pervious Impervious Total Area "

157 " Surface Area 0.193 0.580 0.773 hectare"

158 " Time of concentration 8.579 1.034 1.907 minutes"

159 " Time to Centroid 113.921 89.143 92.010 minutes"

160 " Rainfall depth 65.003 65.003 65.003 mm"

161 " Rainfall volume 125.62 376.85 502.47 c.m"

162 " Rainfall losses 42.421 7.474 16.211 mm"

163 " Runoff depth 22.581 57.528 48.791 mm"

164 " Runoff volume 43.64 333.52 377.16 c.m"

165 " Runoff coefficient 0.347 0.885 0.751 "

166 " Maximum flow 0.021 0.315 0.323 c.m/sec"

167 " 40 HYDROGRAPH Add Runoff "

168 " 4 Add Runoff "

169 " 0.323 0.323 0.009 0.009"

170 " 54 POND DESIGN"

Current peak flow	c.m/sec"
0.323	
Target outflow	c.m/sec"
0.180	
Hydrograph volume	c.m"
377.2	
Number of stages	"
13.	
Minimum water level	metre"
422.046	
Maximum water level	metre"
424.050	
Starting water level	metre"
422.046	
0	
Keep Design Data:	1 = True; 0 = False"
1	
Level Discharge	Volume"
422.046	0.000
422.350	0.00809
422.600	0.01139
422.800	0.01346
423.000	0.01526
423.260	0.01731
423.500	0.01901
423.660	0.02007
423.830	0.02113
423.880	0.02143
423.930	0.02173
423.980	0.02203
424.050	0.05083

193 " 1. WEIRS"

194 " Crest Weir Crest Left Right"

195 " elevation coefficie breadth sideslope sideslope"

196 " 423.980 0.900 1.000 0.000 0.000"

197 " 1. ORIFICES"

198 " Orifice Orifice Orifice Number of"

199 " invert coefficie diameter orifices"

200 " 422.046 0.820 0.0750 1.000"

201 " Peak outflow 0.021 c.m/sec"

202 " Maximum level 423.783 metre"

203 " Maximum storage 223.395 c.m"

204 " Centroidal lag 3.511 hours"

205 " 0.323 0.323 0.021 0.009 c.m/sec"

206 " 40 HYDROGRAPH Combine 2"

207 " 6 Combine "

208 " 2 Node #"

209 " Total to Highway 6"

210 " Maximum flow 0.021 c.m/sec"

211 " Hydrograph volume 377.033 c.m"

212 " 0.323 0.323 0.021 0.021"

213 " 40 HYDROGRAPH Start - New Tributary"

214 " 2 Start - New Tributary"

215 " 0.323 0.000 0.021 0.021"

216 " 33 CATCHMENT 203"

217 " 1 Triangular SCS"

218 " 1 Equal length"

219 " 1 SCS method"

220 " 203 Uncontrolled to Rear Yard Amended Topsoil then to Hwy 6"

221 " 25.000 % Impervious"

222 " 0.352 Total Area"

223 " 120.000 Flow length"

224 " 2.000 Overland Slope"

225 " 0.264 Pervious Area"

226 " 120.000 Pervious length"

227 " 2.000 Pervious slope"

228 " 0.088 Impervious Area"

229 " 120.000 Impervious length"

230 " 2.000 Impervious slope"

231 " 0.250 Pervious Manning 'n' "

232 " 75.000 Pervious SCS Curve No. "

233 " 0.348 Pervious Runoff coefficient"

234 " 0.100 Pervious Ia/S coefficient"

235 " 8.467 Pervious Initial abstraction"

236 " 0.015 Impervious Manning 'n' "

237 " 98.000 Impervious SCS Curve No. "

238 " 0.894 Impervious Runoff coefficient"

239 " 0.100 Impervious Ia/S coefficient"

240 " 0.518 Impervious Initial abstraction"

241 " 0.041 0.000 0.021 0.021 c.m/sec"

Catchment 203	Pervious	Impervious	Total Area	"
Surface Area	0.264	0.088	0.352	hectare"
Time of concentration	29.872	3.602	17.758	minutes"
Time to Centroid	145.259	93.828	121.542	minutes"
Rainfall depth	65.003	65.003	65.003	mm"
Rainfall volume	171.61	57.20	228.81	c.m"
Rainfall losses	42.369	6.895	33.500	mm"
Runoff depth	22.633	58.108	31.502	mm"
Runoff volume	59.75	51.13	110.89	c.m"
Runoff coefficient	0.348	0.894	0.485	"
Maximum flow	0.013	0.037	0.041	c.m/sec"

242 " Catchment 203 Pervious Impervious Total Area "

243 " Surface Area 0.264 0.088 0.352 hectare"

244 " Time of concentration 29.872 3.602 17.758 minutes"

245 " Time to Centroid 145.259 93.828 121.542 minutes"

246 " Rainfall depth 65.003 65.003 65.003 mm"

247 " Rainfall volume 171.61 57.20 228.81 c.m"

248 " Rainfall losses 42.369 6.895 33.500 mm"

249 " Runoff depth 22.633 58.108 31.502 mm"

250 " Runoff volume 59.75 51.13 110.89 c.m"

251 " Runoff coefficient 0.348 0.894 0.485 "

252 " Maximum flow 0.013 0.037 0.041 c.m/sec"

253 " 40 HYDROGRAPH Add Runoff "

254 " 4 Add Runoff "

255 " 0.041 0.041 0.021 0.021"

256 " 57 TRENCH Design d/s of 203"

257	"	0.041	Peak inflow"		
258	"	110.887	Hydrograph volume"		
259	"	424.500	Ground elevation"		
260	"	423.500	Downstream trench invert"		
261	"	0.500	Trench height"		
262	"	422.500	Water table elevation"		
263	"	3.500	Trench top width"		
264	"	3.500	Trench bottom width"		
265	"	40.000	Void ratio (%)"		
266	"	20.000	Hydraulic conductivity"		
267	"	0.000	Trench gradient (%)"		
268	"	130.000	Trench length"		
269	"	1.000	Include base width"		
270	"	21.	Number of stages"		
271	"		Level Discharge	Volume"	
272	"	423.500	0.000	0.000	0.0"
273	"	423.550	0.000	9.1"	
274	"	423.600	0.000	18.2"	
275	"	423.650	0.000	27.3"	
276	"	423.700	0.000	36.4"	
277	"	423.750	0.000	45.5"	
278	"	423.800	0.000	54.6"	
279	"	423.850	0.000	63.7"	
280	"	423.900	0.000	72.8"	
281	"	423.950	0.000	81.9"	
282	"	424.000	0.017	91.0"	
283	"	424.050	0.049	91.1"	
284	"	424.100	0.089	91.1"	
285	"	424.150	0.137	91.2"	
286	"	424.200	0.192	91.2"	
287	"	424.250	0.252	91.3"	
288	"	424.300	0.318	91.3"	
289	"	424.350	0.388	91.4"	
290	"	424.400	0.463	91.5"	
291	"	424.450	0.543	91.5"	
292	"	424.500	0.626	91.6"	
293	"	1.	WEIRS"		
294	"		Crest Weir Crest Left Right"		
295	"	elevation coefficie	breadth sideslope sideslope"		
296	"	423.950 0.900	1.000 0.000 0.000"		
297	"	1.	MANHOLE"		
298	"		Access"		
299	"		diameter"		
300	"	1.200"			
301	"		Peak outflow	0.000	c.m/sec"
302	"		Outflow volume	0.003	c.m"
303	"		Peak exfiltration	0.004	c.m/sec"
304	"		Exfiltration volume	110.885	c.m"
305	"		Maximum level	423.899	metre"
306	"		Maximum storage	72.605	c.m"
307	"		Centroidal lag	5.894	hours"
308	"		Infiltration area 2 sides	103.721	sq.metre"
309	"		Infiltration Base area	455.000	sq.metre"
310	"	0.041	0.041	0.000	0.004 c.m/sec"
311	"	40	HYDROGRAPH Combine 2"		
312	"	6	Combine "		
313	"	2	Node #"		
314	"		Total to Highway 6"		
315	"		Maximum flow	0.021	c.m/sec"
316	"		Hydrograph volume	377.036	c.m"
317	"	0.041	0.041	0.000	0.021"
318	"	40	HYDROGRAPH Start - New Tributary"		
319	"	2	Start - New Tributary"		
320	"	0.041	0.000	0.000	0.021"

321	"	33	CATCHMENT 204"		
322	"	1	Triangular SCS"		
323	"	1	Equal length"		
324	"	1	SCS method"		
325	"	204	Uncontrolled to Highway 6"		
326	"	0.000	% Impervious"		
327	"	0.049	Total Area"		
328	"	20.000	Flow length"		
329	"	2.000	Overland Slope"		
330	"	0.049	Pervious Area"		
331	"	20.000	Pervious length"		
332	"	2.000	Pervious slope"		
333	"	0.000	Impervious Area"		
334	"	20.000	Impervious length"		
335	"	2.000	Impervious slope"		
336	"	0.250	Pervious Manning 'n'"		
337	"	75.000	Pervious SCS Curve No."		
338	"	0.346	Pervious Runoff coefficient"		
339	"	0.100	Pervious Ia/S coefficient"		
340	"	8.467	Pervious Initial abstraction"		
341	"	0.015	Impervious Manning 'n'"		
342	"	98.000	Impervious SCS Curve No."		
343	"	0.000	Impervious Runoff coefficient"		
344	"	0.100	Impervious Ia/S coefficient"		
345	"	0.518	Impervious Initial abstraction"		
346	"	0.005	0.000	0.000	0.021 c.m/sec"
347	"		Catchment 204	Pervious	Impervious Total Area "
348	"		Surface Area	0.049	0.000 0.049 hectare"
349	"		Time of concentration	10.195	1.229 10.195 minutes"
350	"		Time to Centroid	116.372	89.660 116.372 minutes"
351	"		Rainfall depth	65.003	65.003 65.003 mm"
352	"		Rainfall volume	31.85	0.00 31.85 c.m"
353	"		Rainfall losses	42.484	6.915 42.484 mm"
354	"		Runoff depth	22.519	58.088 22.519 mm"
355	"		Runoff volume	11.03	0.00 11.03 c.m"
356	"		Runoff coefficient	0.346	0.000 0.346 "
357	"		Maximum flow	0.005	0.000 0.005 c.m/sec"
358	"	40	HYDROGRAPH Add Runoff "		
359	"	4	Add Runoff "		
360	"		0.005	0.005	0.000 0.021"
361	"	40	HYDROGRAPH Copy to Outflow"		
362	"	8	Copy to Outflow"		
363	"		0.005	0.005	0.005 0.021"
364	"	40	HYDROGRAPH Combine 2"		
365	"	6	Combine "		
366	"	2	Node #"		
367	"		Total to Highway 6"		
368	"		Maximum flow		0.023 c.m/sec"
369	"		Hydrograph volume	388.070	c.m"
370	"		0.005	0.005	0.005 0.023"
371	"	40	HYDROGRAPH Confluence 2"		
372	"	7	Confluence "		
373	"	2	Node #"		
374	"		Total to Highway 6"		
375	"		Maximum flow		0.023 c.m/sec"
376	"		Hydrograph volume	388.070	c.m"
377	"		0.005	0.023	0.005 0.000"
378	"	40	HYDROGRAPH Copy to Outflow"		
379	"	8	Copy to Outflow"		
380	"		0.005	0.023	0.023 0.000"
381	"	40	HYDROGRAPH Combine 1"		
382	"	6	Combine "		
383	"	1	Node #"		
384	"		Total Site"		

```

385 "      Maximum flow           0.031  c.m/sec"
386 "      Hydrograph volume      406.553 c.m"
387 "      0.005    0.023    0.023    0.031"
388 " 40 HYDROGRAPH Confluence 1"
389 " 7 Confluence "
390 " 1 Node #"
391 "      Total Site"
392 "      Maximum flow           0.031  c.m/sec"
393 "      Hydrograph volume      406.553 c.m"
394 "      0.005    0.031    0.023    0.000"
395 " 38 START/RE-START TOTALS 1"
396 " 3 Runoff Totals on EXIT"
397 "      Total Catchment area          1.402  hectare"
398 "      Total Impervious area         0.793  hectare"
399 "      Total % impervious          56.573"
400 " 19 EXIT"
401

```


100yr Post

```

1 " MIDUSS Output ----->"
2 " MIDUSS version Version 2.25 rev. 473"
3 " MIDUSS created Sunday, February 7, 2010"
4 " 10 Units used: ie METRIC"
5 " Job folder: Q:\48650\100\SWM"
6 " Output filename: 100yr post.Out"
7 " Licensee name: A"
8 " Company "
9 " Date & Time last used: 5/10/2022 at 10:17:16 AM"
10 " 31 TIME PARAMETERS"
11 " 5.000 Time Step"
12 " 180.000 Max. Storm length"
13 " 1500.000 Max. Hydrograph"
14 " 32 STORM Chicago storm"
15 " 1 Chicago storm"
16 " 851.000 Coefficient A"
17 " 0.290 Constant B"
18 " 0.687 Exponent C"
19 " 0.400 Fraction R"
20 " 180.000 Duration"
21 " 1.000 Time step multiplier"
22 " Maximum intensity 270.966 mm/hr"
23 " Total depth 71.978 mm"
24 " 6 100hyd Hydrograph extension used in this file"
25 " 33 CATCHMENT 201"
26 " 1 Triangular SCS"
27 " 1 Equal length"
28 " 1 SCS method"
29 " 201 Uncontrolled to Southwest Property Line"
30 " 55.000 % Impervious"
31 " 0.228 Total Area"
32 " 15.000 Flow length"
33 " 2.000 Overland Slope"
34 " 0.103 Pervious Area"
35 " 15.000 Pervious length"
36 " 2.000 Pervious slope"
37 " 0.125 Impervious Area"
38 " 15.000 Impervious length"
39 " 2.000 Impervious slope"
40 " 0.250 Pervious Manning 'n'"
41 " 75.000 Pervious SCS Curve No."
42 " 0.377 Pervious Runoff coefficient"
43 " 0.100 Pervious Ia/S coefficient"
44 " 8.467 Pervious Initial abstraction"
45 " 0.015 Impervious Manning 'n'"
46 " 98.000 Impervious SCS Curve No."
47 " 0.890 Impervious Runoff coefficient"
48 " 0.100 Impervious Ia/S coefficient"
49 " 0.518 Impervious Initial abstraction"
50 " 0.080 0.000 0.000 0.000 c.m/sec"
51 " Catchment 201 Pervious Impervious Total Area "
52 " Surface Area 0.103 0.125 0.228 hectare"
53 " Time of concentration 7.910 0.992 2.773 minutes"
54 " Time to Centroid 112.515 88.822 94.922 minutes"
55 " Rainfall depth 71.978 71.978 71.978 mm"
56 " Rainfall volume 73.85 90.26 164.11 c.m"
57 " Rainfall losses 44.823 7.891 24.511 mm"
58 " Runoff depth 27.155 64.087 47.467 mm"
59 " Runoff volume 27.86 80.36 108.23 c.m"
60 " Runoff coefficient 0.377 0.890 0.659 "
61 " Maximum flow 0.016 0.076 0.080 c.m/sec"
62 " 40 HYDROGRAPH Add Runoff "
63 " 4 Add Runoff "
64 " 0.080 0.080 0.000 0.000"

```

```

65 " 57 TRENCH Design d/s of 201"
66 " 0.080 Peak inflow"
67 " 108.225 Hydrograph volume"
68 " 424.500 Ground elevation"
69 " 423.500 Downstream trench invert"
70 " 0.500 Trench height"
71 " 422.500 Water table elevation"
72 " 3.500 Trench top width"
73 " 3.500 Trench bottom width"
74 " 40.000 Voids ratio (%)"
75 " 20.000 Hydraulic conductivity"
76 " 0.000 Trench gradient (%)"
77 " 85.000 Trench length"
78 " 1.000 Include base width"
79 " 21. Number of stages"
80 " Level Discharge Volume"
81 " 423.500 0.000 0.0"
82 " 423.550 0.000 5.9"
83 " 423.600 0.000 11.9"
84 " 423.650 0.000 17.8"
85 " 423.700 0.000 23.8"
86 " 423.750 0.000 29.8"
87 " 423.800 0.000 35.7"
88 " 423.850 0.000 41.7"
89 " 423.900 0.000 47.6"
90 " 423.950 0.000 53.6"
91 " 424.000 1.458 59.5"
92 " 424.050 4.123 59.6"
93 " 424.100 7.577 59.6"
94 " 424.150 11.664 59.7"
95 " 424.200 16.303 59.7"
96 " 424.250 21.430 59.8"
97 " 424.300 27.003 59.8"
98 " 424.350 32.994 59.9"
99 " 424.400 39.369 60.0"
100 " 424.450 46.112 60.0"
101 " 424.500 53.197 60.1"
102 " 1. WEIRS"
103 " Crest Weir Crest Left Right"
104 " elevation coefficient breadth sideslope sideslope"
105 " 423.950 0.900 85.000 0.000 0.000"
106 " 1. MANHOLE"
107 " Access"
108 " diameter"
109 " 1.200"
110 " Peak outflow 0.019 c.m/sec"
111 " Outflow volume 33.303 c.m"
112 " Peak exfiltration 0.003 c.m/sec"
113 " Exfiltration volume 75.532 c.m"
114 " Maximum level 423.951 metre"
115 " Maximum storage 53.686 c.m"
116 " Centroidal lag 1.879 hours"
117 " Infiltration area 2 sides 76.694 sq.metre"
118 " Infiltration Base area 297.500 sq.metre"
119 " 0.080 0.080 0.019 0.003 c.m/sec"
120 " 40 HYDROGRAPH Combine 1"
121 " 6 Combine "
122 " 1 Node #"
123 " Total Site"
124 " Maximum flow 0.019 c.m/sec"
125 " Hydrograph volume 33.303 c.m"
126 " 0.080 0.080 0.019 0.019"
127 " 40 HYDROGRAPH Start - New Tributary"
128 " 2 Start - New Tributary"

```


257 " 0.046 Peak inflow"
 258 " 129.149 Hydrograph volume"
 259 " 424.500 Ground elevation"
 260 " 423.500 Downstream trench invert"
 261 " 0.500 Trench height"
 262 " 422.500 Water table elevation"
 263 " 3.500 Trench top width"
 264 " 3.500 Trench bottom width"
 265 " 40.000 Voids ratio (%) "
 266 " 20.000 Hydraulic conductivity"
 267 " 0.000 Trench gradient (%) "
 268 " 130.000 Trench length"
 269 " 1.000 Include base width"
 270 " 21. Number of stages"
 271 " Level Discharge Volume"
 272 " 423.500 0.000 0.0"
 273 " 423.550 0.000 9.1"
 274 " 423.600 0.000 18.2"
 275 " 423.650 0.000 27.3"
 276 " 423.700 0.000 36.4"
 277 " 423.750 0.000 45.5"
 278 " 423.800 0.000 54.6"
 279 " 423.850 0.000 63.7"
 280 " 423.900 0.000 72.8"
 281 " 423.950 0.000 81.9"
 282 " 424.000 0.017 91.0"
 283 " 424.050 0.049 91.1"
 284 " 424.100 0.089 91.1"
 285 " 424.150 0.137 91.2"
 286 " 424.200 0.192 91.2"
 287 " 424.250 0.252 91.3"
 288 " 424.300 0.318 91.3"
 289 " 424.350 0.388 91.4"
 290 " 424.400 0.463 91.5"
 291 " 424.450 0.543 91.5"
 292 " 424.500 0.626 91.6"
 293 " 1. WEIRS"
 294 " Crest Weir Crest Left Right "
 295 " elevation coefficie breadth sideslope sideslope"
 296 " 423.950 0.900 1.000 0.000 0.000"
 297 " 1. MANHOLE"
 298 " Access"
 299 " diameter"
 300 " 1.200"
 301 " Peak outflow 0.003 c.m/sec"
 302 " Outflow volume 6.189 c.m"
 303 " Peak exfiltration 0.004 c.m/sec"
 304 " Exfiltration volume 122.938 c.m"
 305 " Maximum level 423.958 metre"
 306 " Maximum storage 83.437 c.m"
 307 " Centroidal lag 3.049 hours"
 308 " Infiltration area 2 sides 119.193 sq.metre"
 309 " Infiltration Base area 455.000 sq.metre"
 310 " 0.046 0.046 0.003 0.004 c.m/sec"
 311 " 40 HYDROGRAPH Combine 2"
 312 " 6 Combine "
 313 " 2 Node # "
 314 " Total to Highway 6"
 315 " Maximum flow 0.025 c.m/sec"
 316 " Hydrograph volume 431.238 c.m"
 317 " 0.046 0.046 0.003 0.025"
 318 " 40 HYDROGRAPH Start - New Tributary"
 319 " 2 Start - New Tributary"
 320 " 0.046 0.000 0.003 0.025"

321 " 33 CATCHMENT 204"
 322 " 1 Triangular SCS"
 323 " 1 Equal length"
 324 " 1 SCS method"
 325 " 204 Uncontrolled to Highway 6"
 326 " 0.000 % Impervious"
 327 " 0.049 Total Area"
 328 " 20.000 Flow length"
 329 " 2.000 Overland Slope"
 330 " 0.049 Pervious Area"
 331 " 20.000 Pervious length"
 332 " 2.000 Pervious slope"
 333 " 0.000 Impervious Area"
 334 " 20.000 Impervious length"
 335 " 2.000 Impervious slope"
 336 " 0.250 Pervious Manning 'n"
 337 " 75.000 Pervious SCS Curve No."
 338 " 0.377 Pervious Runoff coefficient"
 339 " 0.100 Pervious Ia/S coefficient"
 340 " 8.467 Pervious Initial abstraction"
 341 " 0.015 Impervious Manning 'n"
 342 " 98.000 Impervious SCS Curve No."
 343 " 0.000 Impervious Runoff coefficient"
 344 " 0.100 Impervious Ia/S coefficient"
 345 " 0.518 Impervious Initial abstraction"
 346 " 0.006 0.000 0.003 0.025 c.m/sec"
 347 " Catchment 204 Pervious Impervious Total Area "
 348 " Surface Area 0.049 0.000 0.049 hectare"
 349 " Time of concentration 9.400 1.178 9.400 minutes"
 350 " Time to Centroid 114.691 89.317 114.691 minutes"
 351 " Rainfall depth 71.978 71.978 71.978 mm"
 352 " Rainfall volume 35.27 0.00 35.27 c.m"
 353 " Rainfall losses 44.863 7.301 44.863 mm"
 354 " Runoff depth 27.115 64.677 27.115 mm"
 355 " Runoff volume 13.29 0.00 13.29 c.m"
 356 " Runoff coefficient 0.377 0.000 0.377 "
 357 " Maximum flow 0.006 0.000 0.006 c.m/sec"
 358 " 40 HYDROGRAPH Add Runoff "
 359 " 4 Add Runoff "
 360 " 0.006 0.006 0.003 0.025"
 361 " 40 HYDROGRAPH Copy to Outflow"
 362 " 8 Copy to Outflow"
 363 " 0.006 0.006 0.006 0.025"
 364 " 40 HYDROGRAPH Combine 2"
 365 " 6 Combine "
 366 " 2 Node # "
 367 " Total to Highway 6"
 368 " Maximum flow 0.025 c.m/sec"
 369 " Hydrograph volume 444.525 c.m"
 370 " 0.006 0.006 0.006 0.025"
 371 " 40 HYDROGRAPH Confluence 2"
 372 " 7 Confluence "
 373 " 2 Node # "
 374 " Total to Highway 6"
 375 " Maximum flow 0.025 c.m/sec"
 376 " Hydrograph volume 444.525 c.m"
 377 " 0.006 0.025 0.006 0.000"
 378 " 40 HYDROGRAPH Copy to Outflow"
 379 " 8 Copy to Outflow"
 380 " 0.006 0.025 0.025 0.000"
 381 " 40 HYDROGRAPH Combine 1"
 382 " 6 Combine "
 383 " 1 Node # "
 384 " Total Site"

```
385 " Maximum flow 0.044 c.m/sec"
386 " Hydrograph volume 477.828 c.m"
387 " 0.006 0.025 0.025 0.044"
388 " 40 HYDROGRAPH Confluence 1"
389 " 7 Confluence "
390 " 1 Node #"
391 " Total Site"
392 " Maximum flow 0.044 c.m/sec"
393 " Hydrograph volume 477.828 c.m"
394 " 0.006 0.044 0.025 0.000"
395
```

Appendix C

Stormceptor Sizing

Stormceptor® EF Sizing Report

**STORMCEPTOR®
ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION**

05/10/2022

Province:	Ontario
City:	Fergus
Nearest Rainfall Station:	WATERLOO WELLINGTON AP
Climate Station Id:	6149387
Years of Rainfall Data:	34

Project Name:	961 St. David Street North
Project Number:	48650-100
Designer Name:	Nathan Katerberg
Designer Company:	MTE Consultants Inc
Designer Email:	nkaterberg@mte85.com
Designer Phone:	519-743-6500
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	961 St. David Street North
------------	----------------------------

Drainage Area (ha):	0.77
---------------------	------

% Imperviousness:	75.00
-------------------	-------

Runoff Coefficient 'c': 0.75

Particle Size Distribution:	Fine
-----------------------------	------

Target TSS Removal (%):	80.0
-------------------------	------

Required Water Quality Runoff Volume Capture (%):	90.00
---	-------

Estimated Water Quality Flow Rate (L/s):	21.88
--	-------

Oil / Fuel Spill Risk Site?	Yes
-----------------------------	-----

Upstream Flow Control?	Yes
------------------------	-----

Upstream Orifice Control Flow Rate to Stormceptor (L/s):	22.00
--	-------

Peak Conveyance (maximum) Flow Rate (L/s):	
--	--

Site Sediment Transport Rate (kg/ha/yr):	
--	--

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	80
EFO6	89
EFO8	95
EFO10	97
EFO12	99

Recommended Stormceptor EFO Model: EFO4
Estimated Net Annual Sediment (TSS) Load Reduction (%): 80
Water Quality Runoff Volume Capture (%): > 90



Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor® EF Sizing Report

Upstream Flow Controlled Results

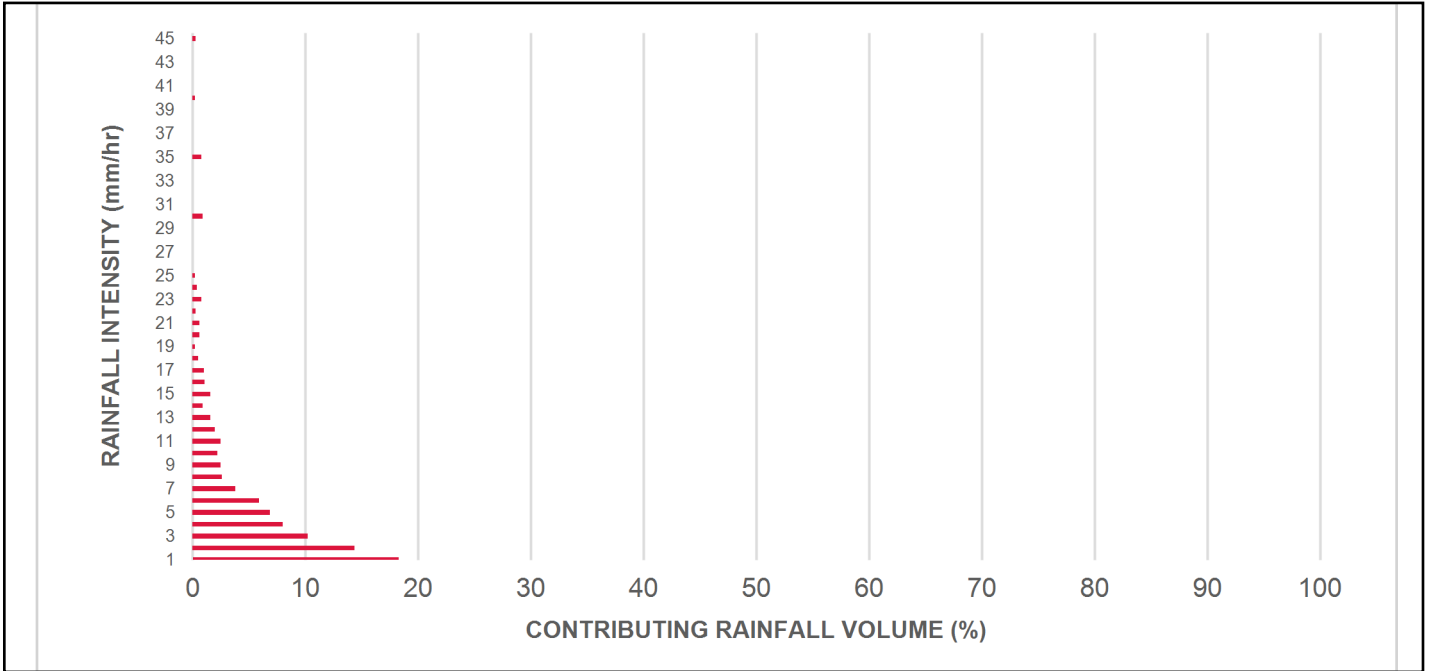
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m ²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	0.80	48.0	40.0	100	8.5	8.5
1	18.3	26.8	1.61	96.0	80.0	98	18.0	26.5
2	14.4	41.3	3.21	193.0	161.0	88	12.7	39.3
3	10.2	51.5	4.82	289.0	241.0	81	8.3	47.6
4	8.0	59.5	6.42	385.0	321.0	78	6.2	53.8
5	6.9	66.4	8.03	482.0	401.0	74	5.1	58.9
6	5.9	72.3	9.63	578.0	482.0	70	4.1	63.0
7	3.8	76.1	11.24	674.0	562.0	66	2.5	65.5
8	2.6	78.7	12.84	771.0	642.0	64	1.7	67.2
9	2.5	81.1	14.45	867.0	722.0	64	1.6	68.8
10	2.2	83.3	16.05	963.0	803.0	63	1.4	70.1
11	2.5	85.8	17.66	1060.0	883.0	62	1.6	71.7
12	2.0	87.8	19.27	1156.0	963.0	62	1.2	72.9
13	12.2	100.0	20.87	1252.0	1044.0	61	7.4	80.3
14	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
15	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
16	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
17	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
18	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
19	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
20	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
21	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
22	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
23	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
24	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
25	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
30	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
35	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
40	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
45	0.0	100.0	22.00	1320.0	1100.0	59	0.0	80.3
Estimated Net Annual Sediment (TSS) Load Reduction =								80 %

Climate Station ID: 6149387 Years of Rainfall Data: 34

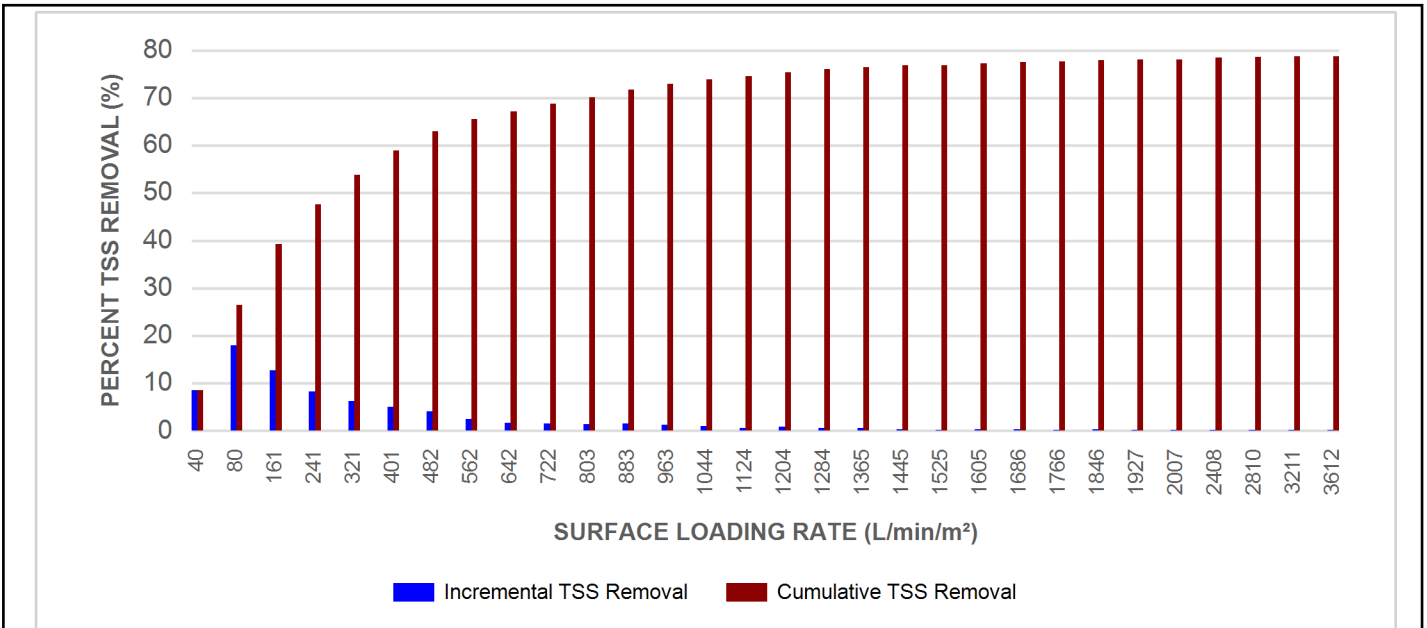


Stormceptor® EF Sizing Report

RAINFALL DATA FROM WATERLOO WELLINGTON AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

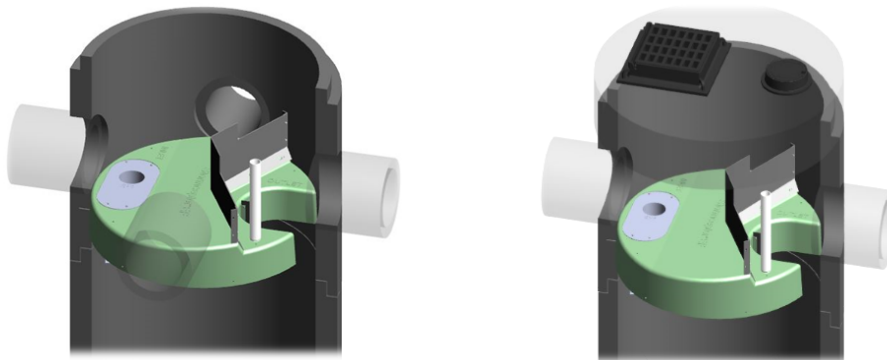
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

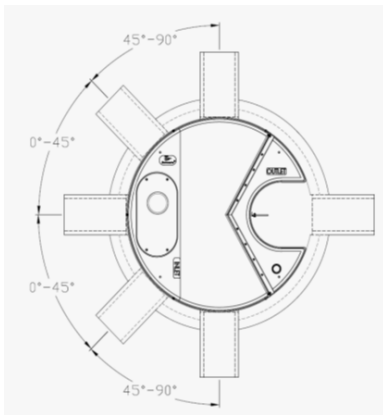
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



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INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



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remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

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assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.