



Preliminary Servicing & Stormwater Management Report for:

Ainley Farm Subdivision
Township of Centre Wellington (Elora)

GMBP File: 411009
October 2017

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Appendix B	Groundwater Elevation Monitoring – CMT Engineering Inc. (October 12, 2012)
Appendix C	Stormwater Management Analysis

**PRELIMINARY SERVICING &
STORMWATER MANAGEMENT REPORT
AINLEY FARM SUBDIVISION
TOWNSHIP OF CENTRE WELLINGTON (ELORA)
October 30, 2017
Our File: 411009**

1.0 INTRODUCTION

In support of the Draft Plan of Subdivision Application for Part of Lots 17 and 18, Concession 12 in the Township of Centre Wellington (Geographic Township of Nichol) herein after referred to as the Ainley Farm Subdivision, GM BluePlan Engineering Limited have prepared this report to address the preliminary servicing and stormwater management requirements for the site.

The servicing and stormwater management techniques were derived from the recommendations presented in the following reports:

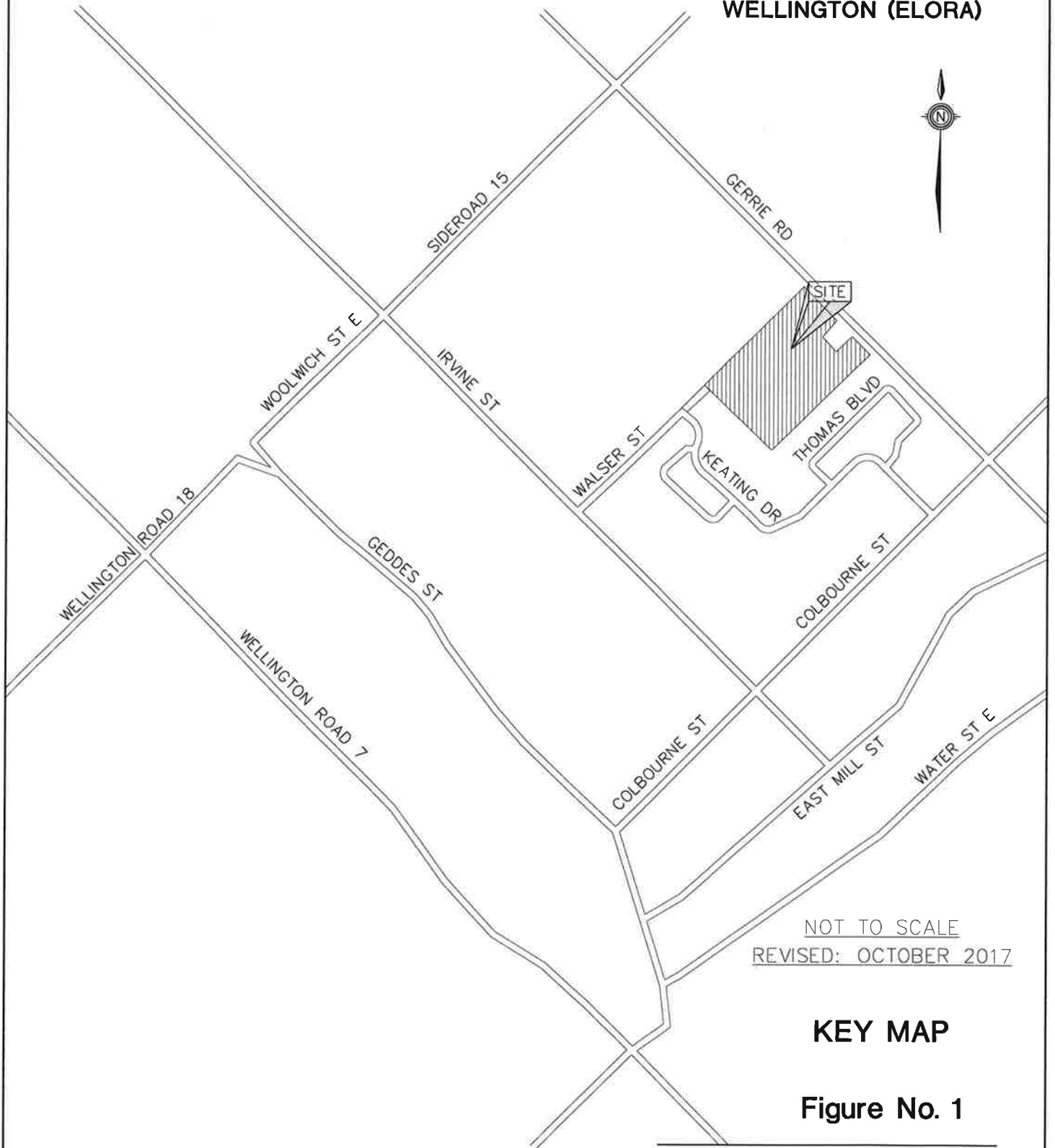
- Stormwater Management Plan for The North Valley Subdivision (Cambridge Engineering and Planning Consultants Limited, January 1994),
- Design Report, Ville Lora Downs North Subdivision, Phase III (Gamsby and Mannerow Limited, July 2004),
- Design Report, Ville Lora Downs Subdivision, Stage VI (Gamsby and Mannerow Limited, April 1998),
- Preliminary Geotechnical Investigation completed CMT Engineering Inc. (March 29, 2006), and
- Environmental Impact Study completed by North-South Environmental Inc. (June 2006).

Together, these reports form the overview for the development of these lands while maintaining the adjacent natural features.

2.0 LOCATION

Figure 1 shows the location of the Ainley Farm Subdivision and the surrounding area. The 21.46-hectare site is bound by existing agricultural and future development lands to the north, Gerrie Road to the east, existing residential lands to the south (Ville Lora Downs Subdivision, Phase V and Phase VI) and existing wetland and residential lands to the west (Ville Lora Downs North Subdivision, Phase III).

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TOWNSHIP OF CENTRE
WELLINGTON (ELORA)**



NOT TO SCALE
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KEY MAP

Figure No. 1



3.0 EXISTING CONDITIONS

3.1 LAND USE

The site is currently comprised of agricultural fields and a natural heritage feature consisting of a woodlot and wetland. The existing site features are shown on the General Plans (GM BluePlan Engineering Limited Drawing No. 1 and No. 2).

3.2 TOPOGRAPHY

The topography throughout the Ainley Farm Subdivision is undulating and consists of rolling slopes with gradients ranging from 0.5% to 20%. Original ground elevations on site range from approximately 410.0m to approximately 416.0m. The northeastern portion of the site generally drains in a northeast direction towards Gerrie Road. The remainder of the site generally drains in a southwest direction towards the existing wetland, ultimately discharging to the existing channel located immediately south of the wetland. The northwestern portion of the site, adjacent to the existing Walser Street right-of-way, drains in a southerly direction towards Walser Street.

3.3 SOILS

The predominant surface soil type on the site is Harriston Loam (Soil Survey of Wellington County Report No. 35). Harriston Loam has a hydrologic soil classification of BC and generally has good drainage characteristics.

The Preliminary Geotechnical Investigation by CMT Engineering Inc. (March 2006) established the characteristics of the underlying soils. The boreholes identified the underlying soils as topsoil overlying organic silt, silt or sandy silt, silt till or sandy silt till, sand or silty sand and clayey silt. The results of the geotechnical investigation are included in Appendix 'A'.

3.4 GROUNDWATER

As part of the Preliminary Geotechnical Investigation (CMT Engineering Inc., March 2006), groundwater observation wells were installed. Groundwater elevation measurements have been collected on a monthly basis from March 2006 to the present, to establish seasonally high groundwater elevations. To date, the monitoring has established that groundwater levels vary seasonally. Lower groundwater elevations have been observed in the late summer and fall. The highest groundwater elevations have been observed during the spring snow melt.

From the groundwater elevation measurements and the Preliminary Geotechnical Investigation, the seasonally high groundwater level is estimated to range from approximately 0.10m to 1.10m below the original ground surface.

Based on the underlying native soils and the high groundwater elevations across the site, the use of infiltration structures for recharge is not recommended for this site. The results of the groundwater elevation measurements have been included in Appendix 'B'.

4.0 PROPOSED DEVELOPMENT

The Draft Plan of Subdivision, prepared by Black, Shoemaker, Robinson & Donaldson Limited (January 23, 2017) (Figure 2), illustrates the proposed lot fabric, internal roads, park block, and open space areas and stormwater management blocks.

Access to the 21.46-hectare development will be provided via Gerrie Road and the extension of Walser Street.

Within the development, there are 122 single family lots, one (1) multi-family/on-street townhouses block, one (1) apartment block, one (1) open space block, one (1) park block and three (3) stormwater management blocks.

In addition, four (4) future single detached lots will be created on the north side of Walser Avenue through the extension of Walser Avenue into the Ainsley Farm property.

4.1 SITE GRADING

The site layout and internal road network for the Ainley Farm Subdivision are shown on the General Plans (GM BluePlan Engineering Limited Drawing No. 1 and No. 2). The grade and elevation of the internal streets are controlled by the existing centre line elevations of Walser Street and Gerrie Road, the major overland flow route to the stormwater management facilities and the elevation of the existing sanitary sewers on Walser Street and Keating Drive.

The site has been graded to match the existing elevations along the property boundary of the adjacent lands. Minor grading on the adjacent lands located along the north boundary of the site is required. The adjacent lands along the north boundary of the site are owned by the Developer (James Keating Construction (2004) Limited).

4.2 STREETS

All streets will be constructed with a minimum grade of 0.5% and a maximum grade of 8.0% as per Township of Centre Wellington standards. An urban road cross-section (20 m right-of-way width), with concrete curb and gutter will be provided for Street No.1, 2, 3, 4 and the extension of Walser Street, as per Township of Centre Wellington Standard Drawing STD R1.

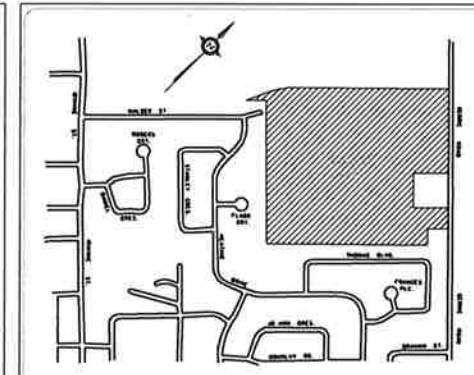
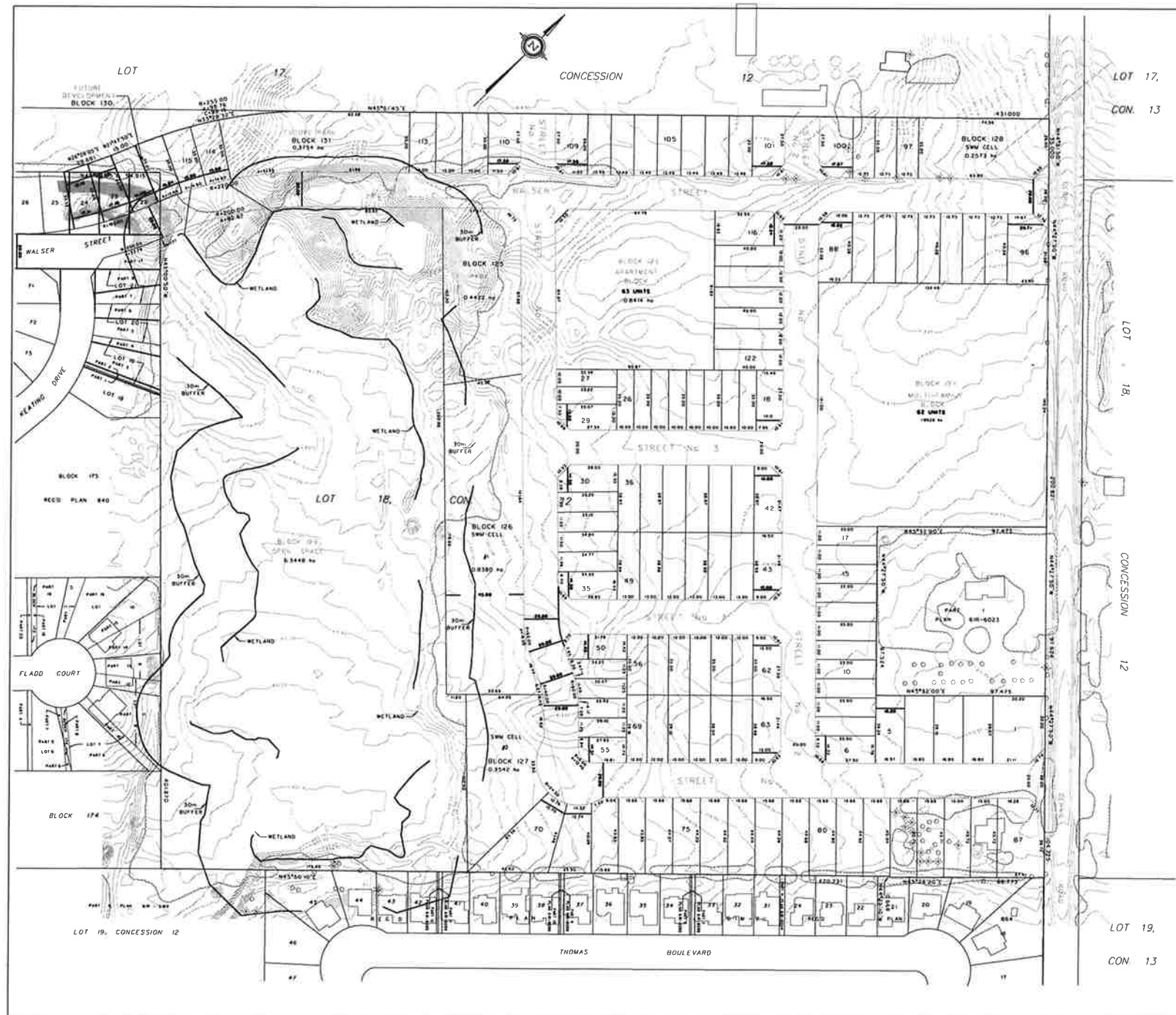
Concrete sidewalks (1.5 metre wide) will be constructed along both sides of the Walser Street extension and Street No. 1, 2, 3 and 4.

4.3 WATER SUPPLY

As part of the Ville Lora Downs North Subdivision Phase III, a 200mm diameter watermain was terminated at the easterly limit of Walser Street. There is currently no watermain on Gerrie Road across the frontage of the Ainley Farm Subdivision.

Water supply for the Ainley Farm Subdivision will be provided via the extension of a 200mm diameter watermain, along the Walser Street extension, Street No. 2 and a portion of Street No. 1. A 150mm diameter watermain will also be extended along the remainder of Street No. 1, Street No. 3, and Street No. 4.

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 TOWNSHIP OF CENTRE
 WELLINGTON (ELORA)



KEY PLAN N.T.S.

ADDITIONAL INFORMATION REQUIRED UNDER SECTION 51 (19) OF THE PLANNING ACT

BY (W) (S) AS SHOWN
 (M) SEE SCHEDULE
 (A) (B) AS SHOWN
 (C) MUNICIPAL WATER SUPPLY
 (D) HARRISTON LOAM
 (E) AS SHOWN
 (F) MUNICIPAL STORM AND SANITARY SEWERS
 (G) NONE KNOWN

OWNERS CERTIFICATE

WE HEREBY AUTHORIZE BLACK, SHOEMAKER, ROBINSON, AND DONALDSON LIMITED, ONTARIO LAND SURVEYORS, URBAN AND RURAL PLANNERS TO SUBMIT THIS DRAFT PLAN OF PROPOSED SUBDIVISION

DATE: _____ JAMES HEATING CONSTRUCTION (2004) LTD.

SURVEYOR'S CERTIFICATE

I HEREBY CERTIFY THAT THE BOUNDARIES OF THE LANDS TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE ACCURATELY AND CORRECTLY SHOWN.

DATE: _____ JIM O. ROBINSON
 ONTARIO LAND SURVEYOR

METRIC

DISTANCES SHOWN ON THIS PLAN ARE IN METRES AND CAN BE CONVERTED TO FEET BY DIVIDING BY 0.3048

RELEVANT INFORMATION

LOTS/BLOCKS	LAND USE	UNITS	AREAS (ha)
1879 - 19 HE	SINGLE-DETACHED RESIDENTIAL	182	6.4338
BLOCK 120	APARTMENT BLDG	42	0.6410
BLOCK 121	ON-STREET THERMOPILES	42	0.6410
BLOCK 122	PARK	1	0.0157
BLOCKS 123, 124 & 125	STORMWATER MANAGEMENT	1,000	1.4255
BLOCK 126	OPEN SPACE	1	0.2000
STREETS	ROADS	-	0.4258
BLOCK 127	OPEN SPACE	1	0.2000
BLOCK 128	FUTURE DEVELOPMENT	-	0.2714
TOTAL		251	21.4803 ha

NOTES

ELEVATIONS AND RELEVANT INFORMATION TAKEN FROM

DRAFT PLAN OF SUBDIVISION

OF PART OF LOTS 17 & 18, CONCESSION 12

TOWNSHIP OF CENTRE WELLINGTON
 (GEOGRAPHIC TOWNSHIP OF NICHOL)
 COUNTY OF WELLINGTON

SCALE 1:1000

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DATE: JANUARY 23, 2017
 DRAWN BY: M3
 PROJECT: 04-SUB-16

NOT TO SCALE
 REVISED: OCTOBER 2017

DRAFT PLAN OF SUBDIVISION

Figure No. 2



4.4 SANITARY SEWER

During the municipal servicing of the Ville Lora Downs North Subdivision Phase III, a 200mm diameter sanitary sewer was designed, approved and constructed on Walser Street. The existing 200mm diameter sanitary sewer is currently terminated at the easterly limit of Walser Street. As part of the Ville Lora Downs Subdivision, Phase VI, a 200mm diameter sanitary sewer was also designed, approved and constructed on Keating Drive. There are currently no sanitary sewers on Gerrie Road across the frontage of the Ainley Farm.

Sanitary service for the Ainley Farm Subdivision will be provided via connections to both the existing 200mm diameter sanitary sewer on Walser Street and the existing 200mm diameter sanitary sewer on Keating Drive.

The extension of a 200mm diameter sanitary sewer along the Walser Street extension will service the lots fronting on to Walser Street, as well as a portion of the lots fronting onto Street No. 2. The extension of a 200mm diameter sanitary sewer on easement through Drimmie Part to Street No. 1, from the existing 200mm diameter sanitary sewer on Keating Drive, will service the remainder of the subdivision (Street No. 2, Street No. 3 and Street No. 4).

4.5 STORM SEWER

The storm sewer system for the Ainley Farm Subdivision will be sized to convey the 5-year design storm event and the storm sewer system will discharge to the two (2) proposed stormwater management facilities or to the existing storm sewer on Walser Street.

The storm sewers on Street No. 1, Street No. 3, Street No. 4, a portion of Street No. 2 and a portion of the Walser Street extension will discharge to the proposed Stormwater Management Facility No. 1 located to the east of the existing wetland.

The storm sewers on the remainder of Street No. 2, along with a portion of the Walser Street extension, will discharge to the proposed Stormwater Management Facility No. 2 located to the west of Gerrie Road.

The storm sewers on the remainder of the Walser Street extension will discharge directly to the existing storm sewer system on Walser Street, ultimately discharging to the existing storm sewers on Keating Drive.

4.6 DEWATERING

Dewatering may be required during the installation of sanitary sewer, storm sewer and watermain. A Permit to Take Water (PTTW) or an Environmental Activity and Sector Registry (EASR) from the Ministry of Environment and Climate Change (MOECC) will be required if dewatering activities will involve the removal of more than 400,000 litres of groundwater per day from the site.

If dewatering activities are required during the installation of sewers and watermain, all discharge will be directed to the interim sediment control pond prior to discharge from the site.

As part of the area grading of the site, the interim stormwater management facility will be constructed and will act as an interim sedimentation control pond for the remainder of the municipal servicing and home building construction. This will prevent sediment from being discharged to the wetland. Upon build-out, accumulated sediment will be collected and removed from the interim sediment control pond.

4.7 FOUNDATION DRAINAGE

As per the Township of Centre Wellington municipal standards, foundation drainage will be provided via sump pits and sump pumps in each residential unit, ultimately discharging via individual storm sewer lateral connections to the storm sewer system located within the municipal right-of-way.

4.8 STORMWATER MANAGEMENT

Details of the stormwater management system for the Ainley Farm Subdivision are discussed in detail in Section 5.0.

4.9 WATER BUDGET

Based on average water budget values for this area, the average annual precipitation is estimated to be 925 mm. The potential for evapotranspiration for this area is estimated to be 555 mm for the silt till and 495 mm for sand and gravel. Therefore, 370 mm and 430 mm remain available for infiltration and runoff from the silt till and sand and gravel, respectively.

From the Preliminary Geotechnical Investigation (CMT Engineering Inc., March 26, 2006), the surficial deposits across the majority of the site are described as native silt tills, with some sandy silt tills. As there are no areas of consistent sandy soils across the site, the characteristics of the silt tills will be used to develop the water budget analysis across the site.

The recharge rate for the native silt tills is estimated to be 125 mm and therefore, the runoff is estimated to be 245 mm.

Based on the annual infiltration rates, the existing annual average groundwater recharge occurring within the 21.46-hectare site, and 1.24 hectares of external areas discharging to the site, is estimated to be 28,273 m³. Under post-development conditions, the annual average groundwater recharge occurring on-site and within the external areas is estimated to be 17,951 m³. The groundwater recharge has been identified in Table No.1.

Under existing conditions the annual average runoff from the site and external areas is estimated to be 56,009 m³. As a result of the proposed development the impervious area (rooftop and paved surfaces) of the site increases, the annual potential evapotranspiration for impervious surfaces decreases to 200 mm and the runoff from the site increases. The runoff from the site and external areas under post-development conditions is estimated to be 95,065 m³ per year.

The estimated existing and post-development recharge and runoff volumes for the Ainley Farm Subdivision are detailed in Table 1. The estimations take into account the surficial geology, which is comprised mainly of glacial tills. The net recharge values are for the uppermost overburden aquifer.

In summary, the estimated recharge and runoff volume for the Ainley Farm Subdivision are as follows:

Table No. 2: Summary of Recharge and Runoff Volume

	Existing Condition	Post-Development Condition	Percent Change
Total Estimated Recharge	28,273 m ³	17,951 m ³	-37%
Total Estimated Runoff	56,009 m ³	95,065 m ³	70%

Table 1: Water Budget - Existing and Post Development Conditions

Ainley Farm Subdivision

	Existing Conditions											
	10-11		40		Total To Wetland	20-21		Total To Gerrie Road and Grand River	30-31		Total To Walsler Street	Existing
	Impervious	Pervious	Impervious	Pervious		Impervious	Pervious		Impervious	Pervious		Total
Annual Precipitation (mm)	925	925	925	925		925	925		925	925		
Annual Evapotranspiration (mm) - S&G	200	495	200	495		200	495		200	495		
Available for Recharge & Runoff (mm) - S&G	725	430	725	430		725	430		725	430		
Annual Evapotranspiration (mm) - Till	200	555	200	555		200	555		200	555		
Available for Recharge & Runoff (mm) - Till	725	370	725	370		725	370		725	370		
Annual Precipitation <= 5 year storm	680	680	680	680		680	680		680	680		
Total Area (ha)	7.89		6.34			7.47			1.00			
Area (ha)	0.00	7.89	0.00	6.34	14.23	0.08	7.39	7.47	0.00	1.00	1.00	22.70
Area (m ²)	0	78,900	0	63,400	142,300	820	73,880	74,700	0	10,000	10,000	227,000
Geology:												
S&G area (ha)	0.00	0.00	0.00	0.00	-	0.00	0.00	-	0.00	0.00	-	-
Till area (ha)	0.00	7.89	0.00	6.34	14.23	0.08	7.39	7.47	0.00	1.00	1.00	22.70
Annual Infiltration:												
Pervious: S&G areas (@ 380 mm/year)	-	0	-	0	-	-	0	-	-	0	-	-
Till areas (@ 125 mm/year)	-	9,863	-	7,925	17,788	-	9,235	9,235	-	1,250	1,250	28,273
Impervious: S&G areas (@ 0 mm/year)	0	-	0	-	-	0	-	-	0	-	-	-
Till areas (@ 0 mm/year)	0	-	0	-	-	0	-	-	0	-	-	-
Total Annual Infiltration (m ³ /year)	0	9,863	0	7,925	17,788	0	9,235	9,235	0	1,250	1,250	28,273
Annual Runoff:												
Pervious: S&G areas (@ 50 mm/year)	-	0	-	0	-	-	0	-	-	0	-	-
Till areas (@ 245 mm/year)	-	19,331	-	15,533	34,864	-	18,101	18,101	-	2,450	2,450	55,414
Impervious: S&G areas (@ 725 mm/year)	0	-	0	-	-	0	-	-	0	-	-	-
Till areas (@ 725 mm/year)	0	-	0	-	-	595	-	595	0	-	-	595
Total Annual Runoff (m ³ /year)	0	19,331	0	15,533	34,864	595	18,101	18,695	0	2,450	2,450	56,009
Summary:												
Runoff (m ³ /year)	0	19,331	0	15,533	34,864	595	18,101	18,695	0	2,450	2,450	56,009
Natural recharge (m ³ /year)	0	9,863	0	7,925	17,788	0	9,235	9,235	0	1,250	1,250	28,273
Total recharge (m ³ /year)	-	9,863	-	7,925	17,788	-	9,235	9,235	-	1,250	1,250	28,273
Net Runoff from Site (m ³ /year)	0	19,331	0	15,533	34,864	595	18,101	18,695	0	2,450	2,450	56,009
Total Recharge (m³/year)		9,863		7,925	17,788		9,235	9,235		1,250	1,250	28,273
Discharge from Site (m³/year)		19,331		15,533	34,864		18,695	18,695		2,450	2,450	56,009

Table 1: Water Budget - Existing and Post Development Conditions

Ainley Farm Subdivision

	Post Development Conditions																						Post-Dev Total
	1000		1100		4000		Total To Wetland	2100		2200		2300		Total To Gerrie Road and Grand River	3100		3200		3300		Total To Walser Street		
	Impervious	Pervious	Impervious	Pervious	Impervious	Pervious		Impervious	Pervious	Impervious	Pervious	Impervious	Pervious		Impervious	Pervious	Impervious	Pervious	Impervious	Pervious		Impervious	
Annual Precipitation (mm)	925	925	925	925	925	925		925	925	925	925	925	925		925	925	925	925	925	925			
Annual Evapotranspiration (mm) - S&G	200	495	200	495	200	495		200	495	200	495	200	495		200	495	200	495	200	495			
Available for Recharge & Runoff (mm) - S&G	725	430	725	430	725	430		725	430	725	430	725	430		725	430	725	430	725	430			
Annual Evapotranspiration (mm) - Till	200	555	200	555	200	555		200	555	200	555	200	555		200	555	200	555	200	555			
Available for Recharge & Runoff (mm) - Till	725	370	725	370	725	370		725	370	725	370	725	370		725	370	725	370	725	370			
Annual Precipitation <= 5 year storm	680	680	680	680	680	680		680	680	680	680	680	680		680	680	680	680	680	680			
Total Area (ha)	11.28		0.47		6.34			2.18		0.91		0.47			0.40		0.35		0.22				
Area (ha)	5.64	5.64	0.00	0.47	0.00	6.34	18.09	1.31	0.87	0.68	0.23	0.05	0.42	3.56	0.24	0.16	0.21	0.14	0.13	0.09	0.97		
Area (m ²)	56,400	56,400	0	4,700	0	63,400	180,900	13,080	8,720	6,825	2,275	470	4,230	35,600	2,400	1,600	2,100	1,400	1,320	880	9,700		
Geology:																							
S&G area (ha)	0.00	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	0.00	-	0.00	0.00	0.00	0.00	0.00	0.00	-		
Till area (ha)	5.64	5.64	0.00	0.47	0.00	6.34	18.09	1.31	0.87	0.68	0.23	0.05	0.42	3.56	0.24	0.16	0.21	0.14	0.13	0.09	0.97		
Annual Infiltration:																							
Pervious: S&G areas (@ 380 mm/year)	-	0	-	0	-	0	-	-	0	-	0	-	0	-	-	0	-	0	-	0	-		
Till areas (@ 125 mm/year)	-	7,050	-	588	-	7,925	15,563	-	1,090	-	284	-	529	1,903	-	200	-	175	-	110	485		
Impervious: S&G areas (@ 0 mm/year)	0	-	0	-	0	-	-	0	-	0	-	0	-	-	0	-	0	-	0	-	-		
Till areas (@ 0 mm/year)	0	-	0	-	0	-	-	0	-	0	-	0	-	-	0	-	0	-	0	-	-		
Total Annual Infiltration (m ³ /year)	0	7,050	0	588	0	7,925	15,563	0	1,090	0	284	0	529	1,903	0	200	0	175	0	110	485		
Annual Runoff:																							
Pervious: S&G areas (@ 50 mm/year)	-	0	-	0	-	0	-	-	0	-	0	-	0	-	-	0	-	0	-	0	-		
Till areas (@ 245 mm/year)	-	13,818	-	1,152	-	15,533	30,503	-	2,136	-	557	-	1,036	3,730	-	392	-	343	-	216	951		
Impervious: S&G areas (@ 725 mm/year)	0	-	0	-	0	-	-	0	-	0	-	0	-	-	0	-	0	-	0	-	-		
Till areas (@ 725 mm/year)	40,890	-	0	-	0	-	40,890	9,483	-	4,948	-	341	-	14,772	1,740	-	1,523	-	957	-	4,220		
Total Annual Runoff (m ³ /year)	40,890	13,818	0	1,152	0	15,533	71,393	9,483	2,136	4,948	557	341	1,036	18,502	1,740	392	1,523	343	957	216	5,170		
Summary:																							
Runoff (m ³ /year)	40,890	13,818	0	1,152	0	15,533	71,393	9,483	2,136	4,948	557	341	1,036	18,502	1,740	392	1,523	343	957	216	5,170		
Natural recharge (m ³ /year)	0	7,050	0	588	0	7,925	15,563	0	1,090	0	284	0	529	1,903	0	200	0	175	0	110	485		
Total recharge (m ³ /year)	0	7,050	0	588	-	7,925	15,563	0	1,090	0	284	0	529	1,903	0	200	0	175	0	110	485		
Net Runoff from Site (m ³ /year)	40,890	13,818	0	1,152	0	15,533	71,393	9,483	2,136	4,948	557	341	1,036	18,502	1,740	392	1,523	343	957	216	5,170		
Total Recharge (m³/year)	7,050		588		7,925		15,563	1,090		284		529		1,903	200		175		110		485		
Discharge from Site (m³/year)	54,708		1,152		15,533		71,393	11,619		5,506		1,377		18,502	2,132		1,866		1,173		5,170		

Table 1: Water Budget - Existing and Post Development Conditions

Ainley Farm Subdivision

	Totals (Entire Site)			Totals (Wetland)			Totals (To Gerrie Road and Grand River)			Totals (To Walser Street)		
	Existing	Post-Dev		Existing	Post-Dev		Existing	Post-Dev		Existing	Post-Dev	
	Total	Total	% Difference	Total	Total	% Difference	Total	Total	% Difference	Total	Total	% Difference
Annual Precipitation (mm)												
Annual Evapotranspiration (mm) - S&G												
Available for Recharge & Runoff (mm) - S&G												
Annual Evapotranspiration (mm) - Till												
Available for Recharge & Runoff (mm) - Till												
Annual Precipitation <= 5 year storm												
Total Area (ha)												
Area (ha)	22.70	22.62	0%	14.23	18.09	27%	7.47	3.56	-52%	1.00	0.97	-3%
Area (m ²)	227,000			0			63,400			10,000		
Geology:												
S&G area (ha)	-	-		-	-		-	-		-	-	
Till area (ha)	22.70	22.62	0%	14.23	18.09	27%	7.47	3.56	-52%	1.00	0.97	-3%
Annual Infiltration:												
Pervious: S&G areas (@ 380 mm/year)	-	-		-	-		0	-		-	-	
Till areas (@ 125 mm/year)	28,273	17,951	-37%	17,788	15,563	-13%	9,235	1,903	-79%	1,250	485	-61%
Impervious: S&G areas (@ 0 mm/year)	-	-		-	-		-	-		-	-	
Till areas (@ 0 mm/year)	-	-		-	-		-	-		-	-	
Total Annual Infiltration (m ³ /year)	28,273	17,951	-37%	17,788	15,563	-13%	9,235	1,903	-79%	1,250	485	-61%
Annual Runoff:												
Pervious: S&G areas (@ 50 mm/year)	-	-		-	-		-	-		-	-	
Till areas (@ 245 mm/year)	55,414	35,183	-37%	34,864	30,503	-13%	18,101	3,730	-79%	2,450	951	-61%
Impervious: S&G areas (@ 725 mm/year)	-	-		-	-		-	-		-	-	
Till areas (@ 725 mm/year)	595	59,881		-	0		595	14,772		-	4,220	
Total Annual Runoff (m ³ /year)	56,009	95,065	70%	34,864	71,393	105%	18,695	18,502	-1%	2,450	5,170	111%
Summary:												
Runoff (m ³ /year)	56,009	95,065	70%	34,864	71,393	105%	18,695	18,502	-1%	2,450	5,170	111%
Natural recharge (m ³ /year)	28,273	17,951	-37%	17,788	15,563	-13%	9,235	1,903	-79%	1,250	485	-61%
Total recharge (m ³ /year)	28,273	17,951	-37%	17,788	15,563	-13%	9,235	1,903	-79%	1,250	485	-61%
Net Runoff from Site (m ³ /year)	56,009	95,065	70%	34,864	71,393	105%	18,695	18,502	-1%	2,450	5,170	111%
Total Recharge (m³/year)	28,273	17,951	-37%	17,788	15,563	-13%	9,235	1,903	-79%	1,250	485	-61%
Discharge from Site (m³/year)	56,009	95,065	70%	34,864	71,393	105%	18,695	18,502	-1%	2,450	5,170	111%

5.0 STORMWATER MANAGEMENT SYSTEM

The objectives of the stormwater management plan are as follows:

- a) Provide Enhanced (80% Total Suspended Solids) water quality control prior to discharge to the existing wetland and to an existing tributary of the Grand River.
- b) Provide quantity control for the full range of design storms to attenuate post-development runoff to the existing condition level.
- c) Route the Regional Storm to minimize flood damage.

5.1 STORMWATER MANAGEMENT CRITERIA

The studies, policies and guidelines used to develop the stormwater management plan for this development were as follows:

- 1) Stormwater Management Planning and Design Manual, 2003
- 2) Design Principles for Stormwater Management Facilities, 1996
- 3) The Interim Stormwater Quality Control Guidelines, 1991
- 4) The Stormwater Quality Best Management Practices Manual, 1991
- 5) The MTO Drainage Management Technical Guidelines, 1989
- 6) The Ontario Urban Design Guidelines, 1987

The method used to evaluate and design the stormwater management plan was as follows:

A three-hour duration rainfall event was used to generate the mass rainfall data required for the 2, 5, 10, 25, 50 and 100-year design storms. The Fergus Shand Dam Chicago parameters and the total depth of rainfall for each storm are as follows:

Table No. 3: Chicago Rainfall Distribution Parameters

	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year
a =	695.047	1459.072	2327.596	3701.648	5089.418	6933.019
b =	6.387	13.690	19.500	25.500	30.000	34.699
c =	0.793	0.850	0.894	0.937	0.967	0.998
r =	0.38	0.38	0.38	0.38	0.38	0.38
Duration = (minutes)	180	180	180	180	180	180
Rainfall Depth = (mm)	33.014	49.792	61.359	75.581	86.737	97.921

The SCS infiltration method was used in the runoff calculations. The CN parameters used in the MIDUSS modelling are as follows:

Table No. 4: SCS Curve Number Parameters

	IMPERVIOUS AREAS	PERVIOUS AREAS
Residential	98	78
Agricultural	98	74
Wetland/Forest	98	50

The hydrologic model MIDUSS was used to create the runoff hydrographs and to route the flows through the storage structures.

5.2 STORMWATER MANAGEMENT APPROACH

In line with current practices and guidelines, the stormwater management plan for the Ainley Farm Subdivision is a “treatment train” to attenuate post-development flows and to provide Enhanced (80% total suspended solids removal) water quality control treatment prior to discharge from the site. The “treatment train” will include a combination of lot level, conveyance and end-of-pipe best management practices.

Lot level controls will simply consist of directing roof leaders to grassed areas and grassed swales.

Conveyance controls will include the use of storm sewers, grassed swales, four (4) oil/grit separator structures for Stormwater Management Facility No.1 and Stormwater Management Facility No.2.

End-of-pipe controls will be provided by two (2) extended detention stormwater management facilities designed to attenuate post-development runoff prior to discharge from the site. Runoff generated from Stormwater Management Facility No.1 will discharge to the existing wetland, ultimately discharging to the existing swale in Drimmie Park and the existing storm sewers on Keating Drive. Stormwater Management Facility No.2 will discharge to the roadside ditch along Gerrie Road, ultimately discharging to a tributary of the Grand River.

A small portion of runoff from the westerly portion of Walser Street will discharge uncontrolled to the existing storm sewer system on Walser Street.

Major storm flows from the development will sheetflow overland via the municipal right-of-ways to either Stormwater Management Facility No.1 or Stormwater Management Facility No.2.

This combination of lot-level, conveyance and end-of-pipe controls will control the release of the runoff from the site.

5.3 STORMWATER MANAGEMENT PLAN

The best management practices (BMP's) in the Stormwater Management Planning and Design Manual (2003) were screened. Those found to be applicable to this development are discussed in the following sections.

5.3.1 LOT LEVEL CONTROLS

Stormwater management practices recommended to provide lot level control on this site are as follows:

a) Roof Drainage to Ground Surface

The driveways and front yards will drain to the street. The roof and rear yard will generally drain to the rear of the lot with exception for lots with back to front drainage.

The roof runoff will be filtered across the grassed surface and some will infiltrate. The runoff for any event large enough to generate flow to the swale system will be adequately filtered by the grass enroute.

b) Rear Yard Swales

The lots will be graded to current Township of Centre Wellington Standards. Where practical, the length of the rear lot swales between catch basins will be increased to extend the contact time with the grassed surfaces.

To promote infiltration on the lots and in the swales, it is recommended that the average depth of graded topsoil be 300 mm.

c) Lot Level Infiltration Systems

The Stormwater Management Practices and Planning Manual (2003), recommends that infiltration structures be installed in soils having a hydraulic conductivity greater than or equal to 15 mm/hour (4.2×10^{-4} cm/s) and where a 1 metre minimum separation from the seasonally high groundwater level can be provided.

The soils on the site have high silt content and thus a low hydraulic conductivity, estimated to be in the order of 1×10^{-4} cm/s. Seasonally high groundwater levels on the site range from 0.10 metres to 1.10 metres below the ground surface, therefore making the 1 metre separation from high groundwater level difficult to achieve.

To demonstrate the infeasibility of including lot-level infiltration within the stormwater management design, we have considered Lot 65. The rear yard and rooftop catchment area of Lot 65 is approximately 0.03 hectares in size and will have an average imperviousness of 50%. Under a "first flush" (2-year) design storm, this lot would generate approximately 5.16 m^3 of runoff.

An infiltration trench, 6 metres long by 3 metre wide by 1 metre deep, constructed in this lot would have an effective contact area of 18 m^2 . Based a hydraulic conductivity of 1.0×10^{-4} cm/s, the estimated rate of recharge is $1.8 \times 10^{-5} \text{ m}^3/\text{s}$.

Therefore, the estimated "drain down" time for a rear lot infiltration structure would be approximately 80 hours. The Stormwater Management Planning and Design Manual (2003) recommend that infiltration structures drain within 24 hours.

Based on the estimated "drain down" time of 80 hours and the high groundwater levels, it is our opinion that infiltration systems are **not** feasible and should **not** be incorporated as part of the development.

5.3.2 CONVEYANCE CONTROLS

The storm conveyance system for the development will consist of grassed swales, storm sewers, major overland channel and four (4) oil/grit separator structures. Conveyance controls will be achieved through the regular maintenance of the grassed swales, storm sewers, major overland channel, forebay and oil/grit separator structures as part of the Township's annual maintenance program. Maintenance requirements will include the annual removal of accumulated sediments and debris from manholes, catch basins, and oil/grit separator structures.

5.3.3 END-OF-PIPE CONTROLS

a) Existing Conditions

Under existing conditions, the majority of the site is utilized for agricultural purposes. For hydrologic modelling purposes, the 21.46-hectare site and 1.24 hectares of external areas was modelled as seven (7) catchments. These catchments are shown on the Existing Conditions Storm Drainage Area Plan (Figure 3).

Catchment 10 (7.76 hectares, 0% impervious) consists primarily of agricultural lands and an existing residential lot.

Catchment 11 (0.13 hectares, 0% impervious) represents the external lands, which consists primarily of agricultural lands of an existing residential lot.

Runoff generated from Catchment 10 and 11 currently sheetflows overland in an east to west direction, ultimately discharging to the existing wetland.

Catchment 20 (6.65 hectares, 0% impervious) consists primarily of agricultural lands and an existing residential lot.

Catchment 21 (0.82 hectares, 0% impervious) represents external lands consisting of an undeveloped residential lot.

Runoff generated from Catchment 20 and 21 currently sheetflows overland to the existing roadside ditch along Gerrie Road and ultimately to a tributary of the Grand River.

Catchment 30 (0.78 hectares, 0% impervious) represents the external lands, which consists primarily of an existing wetland.

Catchment 31 (0.22 hectares, 0% impervious) represents the external lands, which consists primarily of an existing wetland and agricultural lands.

Runoff generated from Catchment 30 and 31 currently sheetflows overland, ultimately discharging to the existing wetland.

Catchment 40 (6.34 hectares, 0% impervious) represents the south-westerly portion of the site, consisting of a natural heritage feature (wetland and woodlot).

Runoff generated from Catchment 40 currently sheetflows overland in an east to west direction, discharging to an existing swale in Drimmie Park and ultimately the existing storm sewer system on Keating Drive.

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LEGEND

----- DRAINAGE AREA BOUNDARY

200	CATCHMENT NUMBER
0.30 80	% IMPERVIOUS
	AREA IN HECTARES

----- EXTERNAL LANDS

SCALE = 1:3000
 REVISED: OCTOBER 2017

EXISTING CONDITIONS
 STORM DRAINAGE
 AREA PLAN

Figure No. 3

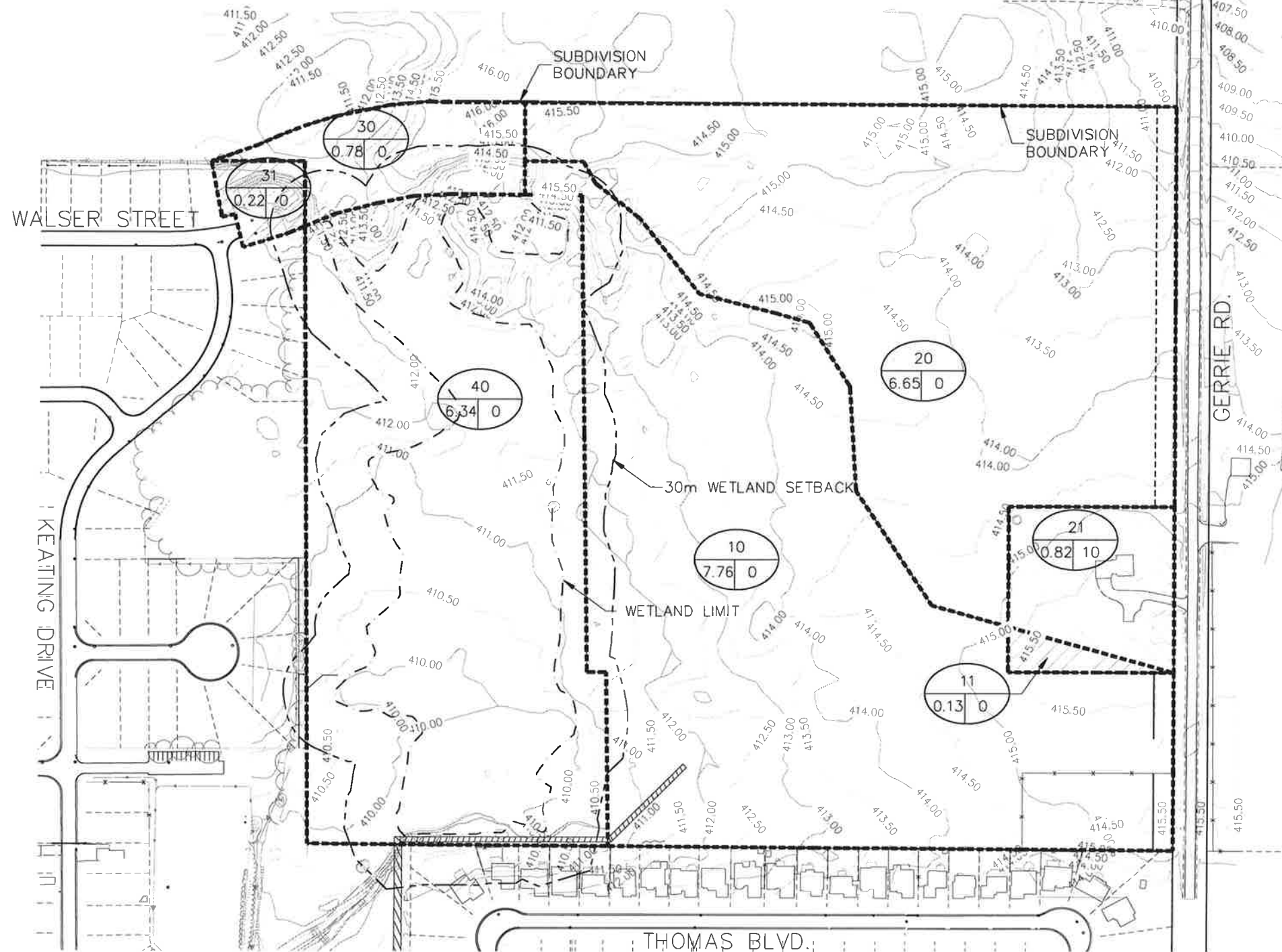


Table No. 5 gives the results of the ponding in the existing wetland.

Table No. 5: Wetland (Existing Condition Flow Rates and Runoff Volumes)

	Available Capacity			Actual Capacity Used			Drawdown Time (hr)**
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
Wetland Bottom	0.000	0.0	409.63	---	---	---	---
2-Year	---	---	---	0.091	39	409.65	7.4
5-Year	---	---	---	0.315	135	409.70	6.9
10-Year	---	---	---	0.536	231	409.75	6.9
25-Year	---	---	---	0.736	556	409.80	6.7
50-Year	---	---	---	0.935	881	409.84	6.5
100-Year	---	---	---	1.157	1242	409.89	6.0
Weir	1.345	9,472.3	410.50	---	---	---	---
Regional Storm	---	---	---	1.422	1675	409.95	52.1
Overflow	1.885	15,057.7	410.75	---	---	---	---

**Drawdown time obtained from the hydrologic modeling software MIDUSS

Table No. 6 gives the results of the existing condition drainage channel routing downstream of the existing wetland.

Table No. 6: Wetland (Existing Condition Drainage Channel Downstream of Wetland – Section 1 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.091	0.159	0.535
5-Year	---	---	---	0.315	0.254	0.730
10-Year	---	---	---	0.536	0.310	0.834
25-Year	---	---	---	0.736	0.349	0.903
50-Year	---	---	---	0.935	0.381	0.958
100-Year	---	---	---	1.157	0.413	1.011
Regional Storm	---	---	---	1.422	0.446	1.064
Top of Bank	10.655	0.95	1.602	---	---	---

Table No. 7 gives the results of the existing condition drainage channel routing downstream of the existing wetland.

Table No. 7: Wetland (Existing Condition Drainage Channel Downstream of Wetland – Section 2 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.091	0.081	0.504
5-Year	---	---	---	0.313	0.164	0.766
10-Year	---	---	---	0.534	0.221	0.907
25-Year	---	---	---	0.735	0.264	1.000
50-Year	---	---	---	0.934	0.301	1.074
100-Year	---	---	---	1.153	0.336	1.142
Regional Storm	---	---	---	1.415	0.375	1.212
Top of Bank	9.246	0.95	1.966	---	---	---

Table No. 8 summarizes the existing condition flow rates and runoff volumes from the site for the full range of design storm events.

Table No. 8: Existing Condition Flow Rates and Runoff Volumes

	CATCHMENTS										
	30	31	Total to Walser	10	11	40	Total to Ex. Wetland	20	21	Total to Tributary of Grand River	Total from Site
2 year											
Flow Rate (m ³ /s)	0.006	0.002	0.008	0.044	0.001	0.054	0.093	0.038	0.016	0.043	0.144
Runoff Volume (m ³)	39.9	11.3	51.2	397.3	6.7	324.5	728.5	340.5	60.4	400.8	1180.5
5 year											
Flow Rate (m ³ /s)	0.021	0.007	0.028	0.157	0.004	0.186	0.331	0.135	0.030	0.152	0.511
Runoff Volume (m ³)	100.1	28.2	128.3	995.9	16.7	813.0	1825.6	853.4	130.5	983.9	2937.8
10 year											
Flow Rate (m ³ /s)	0.036	0.013	0.048	0.273	0.008	0.320	0.517	0.234	0.049	0.263	0.828
Runoff Volume (m ³)	151.3	42.7	193.9	1505.1	25.2	1229.4	2759.8	1289.8	188.3	1478.1	4431.8
25 year											
Flow Rate (m ³ /s)	0.059	0.020	0.078	0.454	0.012	0.520	0.940	0.389	0.078	0.435	1.453
Runoff Volume (m ³)	222.18	62.6	284.8	2210.7	37.0	1805.2	4052.8	1894.4	266.6	2161.0	6498.6
50 year											
Flow Rate (m ³ /s)	0.080	0.027	0.105	0.618	0.016	0.703	1.261	0.530	0.103	0.592	1.958
Runoff Volume (m ³)	282.5	79.6	362.1	2811.2	47.1	2295.0	5153	2409.1	332.7	2741.8	8256.9
100 year											
Flow Rate (m ³ /s)	0.102	0.034	0.134	0.801	0.020	0.877	1.614	0.687	0.134	0.763	2.511
Runoff Volume (m ³)	346.3	97.5	443.8	3447.0	57.6	2814.3	6318.9	2953.9	401.7	3355.6	10118.3
Regional											
Flow Rate (m ³ /s)	0.087	0.023	0.110	0.881	0.014	0.688	1.582	0.755	0.087	0.840	2.532
Runoff Volume (m ³)	1598.8	453.3	2052.2	15780.0	267.9	12943.0	28991.5	13523.0	1719.5	15243.0	46286.7

b) Allowable Release Rates

In order to maintain the existing condition drainage pattern, the allowable release rates have been determined by the existing conditions release rates. Under post-development conditions, runoff generated from the site will be attenuated to the existing condition level. Therefore, the allowable release rates from the site under post-development conditions are outlined in Table No. 9.

Table No. 9: Allowable Release Rates

Allowable Release Rate	2-Year	5-Year	10-Year	25-Year	50-Year	100-Year	Regional
To Ex. Wetland	0.093 m ³ /s	0.331 m ³ /s	0.517 m ³ /s	0.940 m ³ /s	1.261 m ³ /s	1.614 m ³ /s	1.582 m ³ /s
To Tributary of Grand River	0.043 m ³ /s	0.152 m ³ /s	0.263 m ³ /s	0.435 m ³ /s	0.592 m ³ /s	0.763 m ³ /s	0.840 m ³ /s
To Walser	0.008 m ³ /s	0.028 m ³ /s	0.048 m ³ /s	0.078 m ³ /s	0.105 m ³ /s	0.134 m ³ /s	0.110 m ³ /s
Total	0.144 m³/s	0.511 m³/s	0.828 m³/s	1.453 m³/s	1.958 m³/s	2.511 m³/s	2.532 m³/s

c) Post-Development Conditions

Under post-development conditions, the existing drainage patterns of the site will be maintained. Post-development flows from the site will be attenuated to existing condition levels through the use of two (2) stormwater management facilities. Stormwater Management Facility No. 1 will outlet to the existing wetland. Stormwater Management Facility No. 2 will outlet to the existing roadside ditch along Gerrie Road and ultimately a tributary of the Grand River.

For the post-development condition analysis, the 21.46-hectare site and 1.24 hectares of external areas was modelled as nine (9) drainage catchments. These catchments are shown on the Post-Development Storm Drainage Area Plan (Figure No. 4).

Catchment 1000 (11.28-hectares, 50% Impervious) represents the southwest portion of development, including Street No. 1, a portion of Street 2, Street 3, Street 4 and Stormwater Management Facility No. 1. Major and minor storm runoff generated from Catchment 1000 will be directed to Stormwater Management Facility No. 1.

Catchment 1100 (0.47-hectares, 0% Impervious) represents a portion of external lands including existing residential lot. Major and minor storm runoff generated from Catchment 1100 will be directed to Stormwater Management Facility No. 1.

Quantity control for minor and major stormwater runoff generated from Catchment 1000 and 1100 will be provided by Stormwater Management Facility No. 1. A multi-staged outlet structure consisting of a 130 mm diameter knockout, two ditch inlet catch basin structures, complete with 300 mm diameter orifice plate and 200 mm diameter orifice plate and a 20.0 metre wide overflow weir will attenuate runoff generated from Catchment 1000 and 1100 prior to discharge to the existing wetland.

Quality control treatment (80% TSS removal) for runoff generated from Catchment 1000 and 1100 will be provided by three (3) oil/grit separator structures. The first oil/grit separator structure (Stormceptor STC 9000 or approved equivalent) will be located at the northerly inlet to Stormwater Management Facility No. 1 (Street 3). The second oil/grit separator structure (Stormceptor STC 6000 or approved equivalent) will be located at the central inlet to Stormwater Management Facility

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LEGEND

--- DRAINAGE AREA BOUNDARY

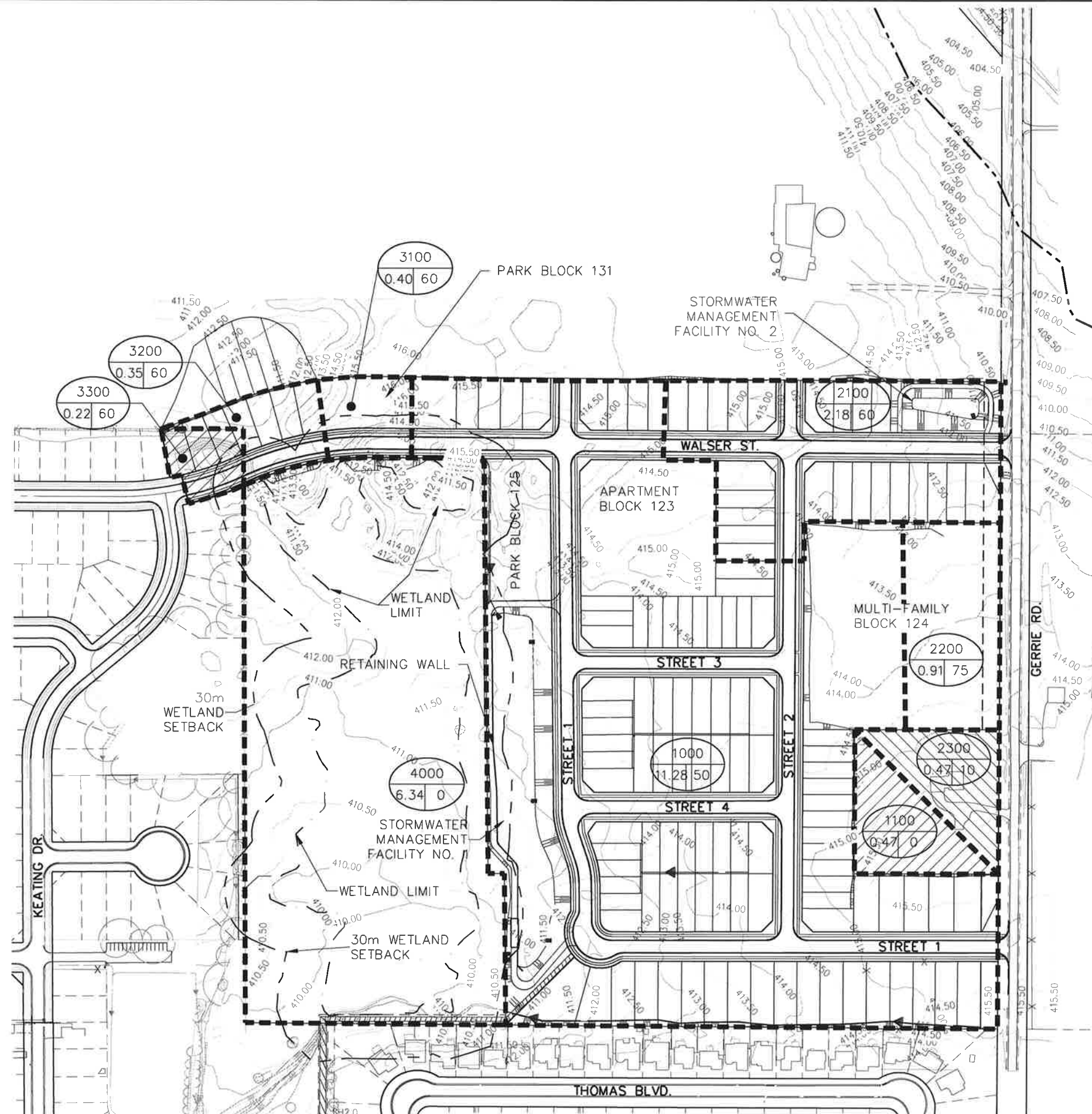
CATCHMENT NUMBER
 % IMPERVIOUS
 AREA IN HECTARES

EXTERNAL LANDS

SCALE = 1:3000
 REVISED: OCTOBER 2017

POST-DEVELOPMENT
 STORM DRAINAGE
 AREA PLAN

Figure No. 4



No. 1 (Street 4). The third oil/grit separator structure (Stormceptor STC 4000 or approved equivalent) will be located at the southerly inlet to Stormwater Management Facility No. 1 (Street 1). **Catchment 2100 (2.18-hectares, 60% Impervious)** represents the north portion of development, including the remainder of Street 2, and a portion of Walser Street. Major and minor storm runoff generated from Catchment 2100 will be directed to Stormwater Management Facility No. 2.

Quantity and quality control for minor and major stormwater runoff generated from Catchment 2100 will be provided by Stormwater Management Facility No. 2. A multi-staged outlet structure consisting of a 150 mm diameter knockout, a 450 mm diameter outlet pipe and a 10.0 metre long overflow weir will attenuate runoff generated from Catchment 2100 prior to discharge to the existing roadside ditch along Gerrie Road and ultimately a tributary of the Grand River.

Quality control treatment (80% TSS removal) for runoff generated from Catchment 2100 will be provided by one (1) oil/grit separator structure. The oil/grit separator structure (Stormceptor STC 4000 or approved equivalent) will be located at the inlet to Stormwater Management Facility No. 2.

Catchment 2200 (0.91-hectares, 75% Impervious) represents the multi-family residential block. Runoff generated from Catchment 2200 will discharge to the roadside ditch along Gerrie Road, and ultimately a tributary of the Grand River. At such time as development of Catchment 2200 proceeds, a privately owned and operated on-site quality and quantity control stormwater management facility will be required to attenuate stormwater runoff to the existing condition level, prior to discharge to the existing roadside ditch along Gerrie Road.

The privately owned and operated on-site stormwater management facility will be designed, reviewed and approved as part of the site plan approval process for the development block. The on-site stormwater management controls which may be utilized include, but are not limited to, a stormwater management facility (i.e. SWM pond), rooftop storage, parking lot ponding (to a maximum depth of 0.3m), below grade storage (i.e. clear stone storage, superpipe storage, etc.) and oil/grit separators.

Catchment 2300 (0.47-hectares, 10% Impervious) represents the remainder of the existing residential lot on Gerrie Road. Major and minor storm runoff generated from Catchment 2300 will be directed to the existing roadside ditch on Gerrie Road, ultimately discharging to a tributary of the Grand River.

Catchment 3100 (0.40-hectares, 60% Impervious) represents a portion of Walser Street and future Park Block 123. Minor storm runoff generated from Catchment 3100 will be directed to Stormwater Management Facility No. 2. Major storm runoff generated from Catchment 3100 will sheetflow overland to the existing Walser Street right-of-way.

Catchment 3200 (0.35-hectares, 60% Impervious) represents four (4) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3200 will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3200 will sheetflow overland to the existing Walser Street right-of-way.

Catchment 3300 (0.22-hectares, 60% Impervious) represents three (3) new single family lots and a portion of Walser Street that form part of the Ville Lora Downs North Phase III development. Minor storm runoff generated from Catchment 3300 will be directed to the existing storm sewers on Walser Street. Major storm runoff generated from Catchment 3300 will discharge overland to the existing Walser Street right-of-way.

Quality and quantity control for stormwater runoff generated from Catchments 3200 and 3300 will be provided by the existing stormwater management facilities approved and constructed as part of the Villa Lora Downs North Phase II development.

Catchment 4000 (6.34 hectares, 0% impervious) represents the remainder of the site, which is a natural heritage feature consisting of a woodlot and wetland area. Runoff generated from Catchment 4000 will continue to sheetflow overland, ultimately discharging to the existing swale in Drimmie Park and the existing storm sewers on Keating Drive.

Table No. 10 lists the uncontrolled flow rate and runoff volumes generated from each catchment area shown on Figure No. 4, for the 2, 5, 10, 25, 50 and 100-year design storm events and the Regional storm.

Table No. 10: Post-Development Uncontrolled Flow Rate and Runoff Volume

	CATCHMENTS													
	1000	1100	4000	To Ex. Wetland	2100	2200	2300	3100 (minor)	To Roadside Ditch	3100 (major)	3200	3300	To Walsler Street	Total
2 year														
Flow Rate (m ³ /s)	1.014	0.009	0.054	1.064	0.437	0.129	0.010	0.043	0.576	0.00	0.038	0.024	0.061	1.700
Runoff Volume (m ³)	1945.6	32.2	324.5	2379.6	420.1	203.6	41.9	77.31	987.3	0.00	67.7	42.5	110.2	3477.1
5 year														
Flow Rate (m ³ /s)	1.493	0.027	0.186	1.592	0.597	0.174	0.023	0.062	0.788	0.00	0.054	0.034	0.088	2.468
Runoff Volume (m ³)	3375.6	74.59	813.0	4394.2	1254.8	334.6	87.8	131.0	1677.1	0.00	114.6	72.0	186.6	6257.9
10 year														
Flow Rate (m ³ /s)	1.840	0.042	0.320	1.979	0.706	0.204	0.036	0.075	0.933	0.013	0.066	0.041	0.120	3.031
Runoff Volume (m ³)	4431.7	109.3	1229.4	5933.93	1630.5	428.7	124.5	170.0	2183.7	6.5	148.7	93.5	248.7	8366.4
25 year														
Flow Rate (m ³ /s)	2.321	0.064	0.520	2.537	0.854	0.243	0.055	0.092	1.130	0.030	0.081	0.051	0.162	3.829
Runoff Volume (m ³)	5580.3	156.6	1805.2	7946.0	2107.7	546.8	173.6	219.5	2828.1	20.8	192.1	120.7	333.7	11107.8
50 Year														
Flow Rate (m ³ /s)	2.726	0.081	0.703	3.024	0.964	0.272	0.073	0.107	1.277	0.045	0.093	0.059	0.197	4.498
Runoff Volume (m ³)	6876.2	196.2	2295.1	9590.3	2490.3	640.8	214.2	259.2	3345.3	36.4	226.8	142.6	405.7	11341.3
100 Year														
Flow Rate (m ³ /s)	3.124	0.099	0.877	3.571	1.088	0.302	0.090	0.121	1.445	0.059	0.106	0.066	0.230	5.198
Runoff Volume (m ³)	7984.7	237.1	2814.3	11282.1	2881.0	736.1	256.9	299.4	3874.0	53.3	262.0	164.7	480.0	15636.1
Regional Storm														
Flow Rate (m ³ /s)	1.367	0.054	0.688	2.083	0.465	0.110	0.051	0.049	0.051	0.000	0.043	0.027	0.070	2.770
Runoff Volume (m ³)	25716.0	1010.2	12943.0	40591.6	8956.6	2150.2	1036.4	922.4	1036.4	0.0	807.1	507.3	1314.4	54049.4

Table No. 11 compares the routing results through the proposed Stormwater Management Facility No. 1.

**Table No. 11: Catchment 1000 & 1100 – Stormwater Management Facility No. 1
 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Pond Bottom / 130mm Knockout	0.00	0.0	411.00	---	---	---
2 year	---	---	---	0.017	1,908.2	411.31
CB Lip 1 Elevation	0.026	3,804.5	411.60	---	---	---
5 year	---	---	---	0.024	3,379.6	411.54
10 year	---	---	---	0.092	4,142.3	411.65
CB Lip 2 Elevation	0.163	4,806.0	411.75	---	---	---
25 year	---	---	---	0.220	5,041.1	411.79
Weir	0.252	5,485.0	411.85	---	---	---
50 year	---	---	---	0.464	5,588.4	411.87
100 year	---	---	---	0.917	5,809.4	411.91
Regional Storm	---	---	---	1.322	6,057.8	411.96
Top of Bank	1.912	6,295.5	412.00	---	---	---

Table No. 12 compares the routing results through the proposed Stormwater Management Facility No. 2.

**Table No. 12: Catchment 2100 – Stormwater Management Facility No. 2
 Available Stage/Storage/Discharge**

CONTROL	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Permanent Pool /150mm Knockout	0.00	0.0	410.65	---	---	---
2 year	---	---	---	0.028	252.82	411.06
CB Lip Elevation	0.291	446.3	411.30	---	---	---
5 year	---	---	---	0.103	382.70	411.23
10 year	---	---	---	0.195	413.67	411.26
Weir	0.384	806.9	411.65	---	---	---
25 year	---	---	---	0.294	456.53	411.31
50 year	---	---	---	0.316	523.93	411.38
100 year	---	---	---	0.338	609.58	411.58
Regional Storm	---	---	---	0.240	432.77	411.29
Top of bank	2.818	1,195.4	411.95	---	---	---

Table No. 13 gives the results of the post-development ponding occurring in the wetland, which discharges to the existing channel located immediately south of the existing wetland.

Table No. 13: Wetland (Post-Development Condition Flow Rates and Runoff Volumes)

	Available Capacity			Actual Capacity Used			Drawdown Time (hr)**
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
Wetland Bottom	0.000	0.0	409.63	---	---	---	---
2-Year	---	---	---	0.067	29	409.65	86.3
5-Year	---	---	---	0.195	84	409.67	106.2
10-Year	---	---	---	0.319	137	409.70	112.5
25-Year	---	---	---	0.555	257	409.75	113.6
50-Year	---	---	---	0.706	503	409.79	114.3
100-Year	---	---	---	1.104	1159	409.88	114.3
Weir	1.345	9,472.3	410.50	---	---	---	---
Regional Storm	---	---	---	1.869	2369	410.03	160.2
Overflow	1.885	15,057.7	410.75	---	---	---	---

**Drawdown time obtained from the hydrologic modeling software MIDUSS

Table No. 14 gives the results of the post-development condition drainage channel routing downstream of the existing wetland.

Table No. 14: Wetland (Post-Development Condition Drainage Channel Downstream of Wetland – Section 1 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.067	0.142	0.496
5-Year	---	---	---	0.195	0.212	0.648
10-Year	---	---	---	0.319	0.255	0.732
25-Year	---	---	---	0.555	0.314	0.841
50-Year	---	---	---	0.706	0.343	0.893
100-Year	---	---	---	1.104	0.406	0.999
Regional Storm	---	---	---	1.869	0.495	1.139
Top of Bank	10.655	0.95	1.602	---	---	---

Table No. 15 gives the results of the post-development condition drainage channel routing downstream of the existing wetland.

Table No. 15: Wetland (Post-Development Condition Drainage Channel Downstream of Wetland – Section 2 of 2)

	Channel Design Capacity			Actual Channel Capacity Used		
	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s	Peak Flow m ³ /s	Average Channel Depth m	Velocity m/s
2-Year	---	---	---	0.067	0.067	0.452
5-Year	---	---	---	0.194	0.125	0.654
10-Year	---	---	---	0.316	0.165	0.768
25-Year	---	---	---	0.554	0.226	0.917
50-Year	---	---	---	0.705	0.258	0.987
100-Year	---	---	---	1.103	0.329	1.128
Regional Storm	---	---	---	1.862	0.432	1.310
Top of Bank	9.246	0.95	1.966	---	---	---

Table No. 16 summarizes the post-development flow rates from the site.

Table No. 16: Summary of Post-Development Flow Rates from the Site

CATCHMENT	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	Regional Storm
Catchments 1000 & 1100 & 3100 (minor) (controlled)	0.017 m ³ /s	0.024 m ³ /s	0.108 m ³ /s	0.220 m ³ /s	0.464 m ³ /s	0.917 m ³ /s	1.322 m ³ /s
Catchment 4000 (uncontrolled)	0.054 m ³ /s	0.186 m ³ /s	0.320 m ³ /s	0.520 m ³ /s	0.703 m ³ /s	0.877 m ³ /s	0.688 m ³ /s
Total to Ex. Wetland	0.069 m³/s	0.206 m³/s	0.343 m³/s	0.615 m³/s	0.864 m³/s	1.561 m³/s	1.936 m³/s
Catchments 2100 (controlled)	0.028 m ³ /s	0.103 m ³ /s	0.195 m ³ /s	0.294 m ³ /s	0.316 m ³ /s	0.338 m ³ /s	0.240 m ³ /s
Catchment 2200 (uncontrolled)	0.014 m ³ /s	0.042 m ³ /s	0.067 m ³ /s	0.105 m ³ /s	0.138 m ³ /s	0.172 m ³ /s	0.098 m ³ /s
Catchment 2300 (uncontrolled)	0.010 m ³ /s	0.023 m ³ /s	0.036 m ³ /s	0.055 m ³ /s	0.073 m ³ /s	0.090 m ³ /s	0.051 m ³ /s
Total to Tributary of Grand River	0.048 m³/s	0.162 m³/s	0.298 m³/s	0.452 m³/s	0.523 m³/s	0.598 m³/s	0.389 m³/s
Catchment 3100 (major) & 3200 & 3300 (uncontrolled)	0.061 m ³ /s	0.088 m ³ /s	0.120 m ³ /s	0.162 m ³ /s	0.197 m ³ /s	0.230 m ³ /s	0.070 m ³ /s
Total to Walser Street	0.061 m³/s	0.088 m³/s	0.120 m³/s	0.162 m³/s	0.197 m³/s	0.230 m³/s	0.070 m³/s
Total from Site	0.116 m³/s	0.349 m³/s	0.566 m³/s	0.897 m³/s	1.128 m³/s	1.488 m³/s	2.277 m³/s

The following table compares the allowable release rates to the post-development flow rates for the site.

Table No. 17: Comparison of Allowable Release Rates and Post-Development Conditions Flow Rates

DESIGN STORM	To Ex. Wetland		To Tributary of Grand River		To Walser Street		Total from Site	
	Allowable Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)	Allowable Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)	Allowable Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)	Allowable Release Rate (m ³ /s)	Post Flow Rate (m ³ /s)
2 year	0.093	0.069	0.043	0.048	0.008	0.061	0.144	0.116
5 year	0.331	0.206	0.152	0.162	0.028	0.088	0.511	0.349
10 year	0.517	0.343	0.263	0.298	0.048	0.120	0.828	0.566
25 year	0.940	0.615	0.435	0.452	0.078	0.162	1.453	0.897
50 Year	1.261	0.864	0.592	0.523	0.105	0.197	1.958	1.128
100 Year	1.614	1.561	0.763	0.593	0.134	0.230	2.511	1.488
Regional	1.582	1.936	0.840	0.389	0.110	0.070	2.532	2.277

Therefore, the post-development runoff generated from the site will be attenuated to the less than the allowable release rates for the full range of design storm events.

5.3.4 MINOR / MAJOR DRAINAGE SYSTEM

Minor storm drainage will be conveyed to the proposed stormwater management facilities and the existing storm sewers on Walser Street via storm sewers with the capacity to convey the 5-year design storm event.

The major storm runoff generated from Street 1, and a portion of Street 2, Street 3 and Street 4 will discharge to the proposed stormwater management facility located east of the existing wetland (Stormwater Management Facility No. 1), which outlets to the existing wetland, ultimately discharging to the existing storm sewers on Keating Drive.

The major storm runoff generated from the remainder of Street 2 and a portion of the Walser Street extension will discharge to the proposed stormwater management facility (Stormwater Management Facility No. 2), ultimately discharging to a tributary of the Grand River.

The major storm runoff generated from the remainder of the Walser Street extension will discharge directly to the existing Walser Street Right-of-Way, ultimately discharging to the Keating Drive Right-of-Way.

Preliminary analysis indicates that the municipal right-of-way has the capacity to convey the runoff from a major design storm event.

The major design storm drainage patterns expected for the Ainley Farm Subdivision are shown on Figure 5.

411009
AINLEY FARM SUBDIVISION
TOWNSHIP OF CENTRE
WELLINGTON (ELORA)



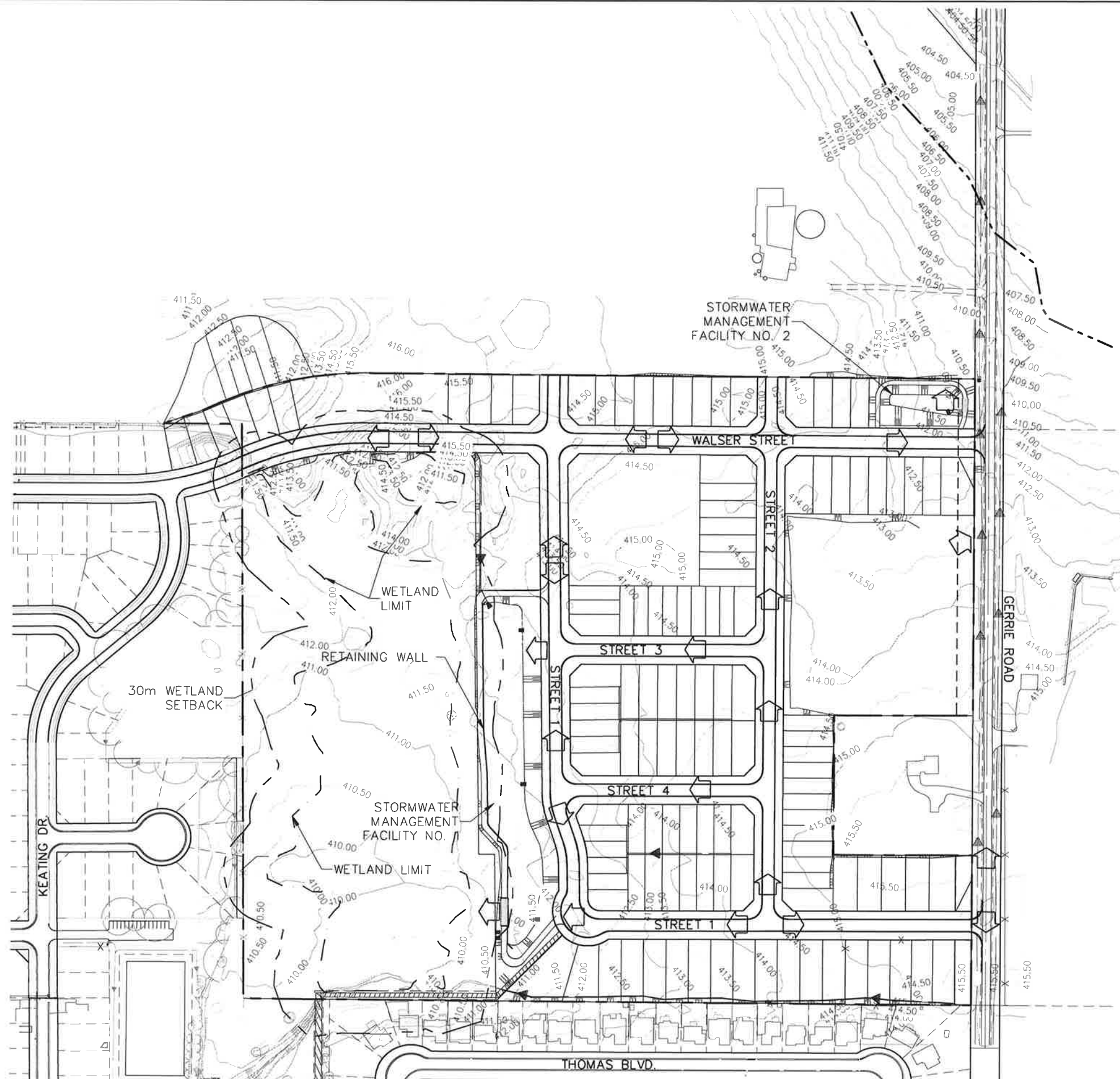
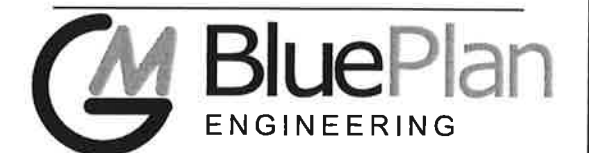
LEGEND

 MAJOR OVERLAND FLOW

SCALE = 1:3000
REVISED: OCTOBER 2017

MAJOR STORM
DRAINAGE PATTERN
PLAN

Figure No. 5



6.0 SEDIMENT AND EROSION CONTROL PLAN

A silt fence will be installed along the property boundary. The silt fence will serve to minimize the opportunity for water borne sediments to be transported from the site to the adjacent properties.

Temporary straw bale check dams will be installed in rear yard swales after the initial grading has been completed to slow the flow rates and promote the settlement of water borne sediments before they reach the silt fences and stormwater management facilities.

Upon completion of the grading, any area not subject to active construction within 30 days will be top soiled and seeded as per OPSS 572.

Once catch basins have been installed, the grates will be wrapped in filter cloth. This feature will be maintained until all building and landscaping has been completed.

Inspection and maintenance of all silt fencing and sediment and erosion controls will start after installation is complete. These features will be inspected on a weekly basis or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the controls found to need repair.

Once construction and landscaping within the limits of the subdivision has been substantially completed (75% house building construction is complete), the silt fence will be removed, any accumulated sediment will be collected and the area will be restored.

After construction of the subdivision, erosion and sediment transport will be minimal.

7.0 MAINTENANCE PLAN

A two-phase maintenance plan is recommended. Phase I will address the short-term more intensive maintenance necessary during and immediately after construction. Once all landscaping has been completed, maintenance will shift to Phase II.

As outlined in the section on Sediment and Erosion Control, Phase I will include weekly inspection of all sediment and erosion control devices plus "as needed" inspection after significant rainfall, with the repair of any damaged works and collection of captured sediment.

Phase II will be the maintenance carried out by the Township of Centre Wellington after all construction has been completed. This work will involve a yearly visual inspection of the stormwater management facilities and catch basins to determine the amount of sediment accumulation. Sediment should be removed as required and the recommended vegetation replanted.

8.0 CONCLUSIONS

From the foregoing analysis, the following conclusions are drawn:

- Water supply for the Ainley Farm Subdivision will be provided via the extension of a 200 mm diameter watermain along the Walser Street extension, Street No. 2 and a portion of Street No. 1. A 150 mm diameter watermain will be extended along the remainder of Street No. 1, Street No. 3, and Street No. 4.
- Sanitary service for the proposed lots along the Walser Street extension and a portion of Street No. 2 will be provided by the extension of a 200 mm diameter sanitary sewer from the existing 200 mm diameter sanitary sewer on Walser Street. Sanitary service for the remainder of the site will be provided by the extension of a 200 mm diameter sanitary sewer on easement from the existing 200 mm diameter sanitary sewer on Keating Drive.
- Storm sewers will be designed to convey the 5-year design storm event and will discharge to the two (2) stormwater management facilities and the existing storm sewer on Walser Street.
- Major storm runoff will be conveyed within the limits of the street right-of-ways to the two (2) stormwater management facilities and the existing Walser Street right-of-way.
- As per the Township of Centre Wellington municipal standards, foundation drainage will be collected in sump pits in each residential unit and pumped to the storm sewer system located within the municipal right-of-way.
- Quantity control for runoff generated from the development will be provided by two (2) stormwater management facilities,
- Quality control for runoff generated from the development will be provided by four (4) oil/grit separators (Stormceptor or approved equivalent).
- Installation of infiltration systems in the rear yard areas is not feasible due to the low hydraulic conductivity of the native underlying silty tills and the presence of high groundwater levels.
- During the construction phase, the erosion control measures will minimize the transport of sediment off-site during the construction period.

All of which is respectfully submitted.

GM BLUEPLAN ENGINEERING LIMITED

Per:



Sarah Primmer, P.Eng.

SP/

Encl.





APPENDIX A
PRELIMINARY GEOTECHNICAL INVESTIGATION
CMT ENGINEERING INC.
MARCH 29, 2006

PRELIMINARY GEOTECHNICAL
INVESTIGATION

AINLEY SUBDIVISION
TOWNSHIP OF CENTRE WELLINGTON
VILLAGE OF ELORA, ONTARIO

CMT Project 06-004

Prepared For:

Gamsby and Mannerow Limited

March 29, 2006





CMT Engineering Inc.
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March 29, 2006

06-004.R01

Gamsby and Mannerow Limited
255 Woodlawn Road West, Suite 210
Guelph, Ontario
N1H 8J1

Attention: Mr. Glenn Anderson, C.E.T.

Dear Sir:

**Re: Preliminary Geotechnical Investigation
Ainley Subdivision
Township of Centre Wellington
Village of Elora, Ontario**

As requested, CMT Engineering Inc. conducted a subsoil investigation at the above-referenced site, and we are pleased to present the enclosed report.

We trust that this information meets your present requirements and we thank you for this opportunity to have been of service. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

A handwritten signature in black ink, appearing to read 'Robert Koopmans', written over a series of horizontal, wavy lines that serve as a background for the signature.

Robert Koopmans, P.Eng.

ks

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Drawing 1 - Site Plan Showing Borehole Locations

Appendix A - Borehole Logs 101 to 108

Appendix B - Cross-Sections

Appendix C - Grain Size Analyses

Appendix D - Laboratory Proctor Test

Appendix E - Well Record

1.0 INTRODUCTION

The services of CMT Engineering Inc. were retained by Mr. Glenn Anderson of Gamsby and Mannerow Limited to carry out a subsurface investigation for the proposed Ainley Subdivision in the Township of Centre Wellington (Village of Elora).

It is our understanding that single-family and multi-family residences are proposed to be built on the eastern two-thirds of the property. The western one-third of the property is proposed for open space and may contain a storm water management pond.

The purpose of the investigation was to determine the subsurface soil profile, the water levels in the boreholes and provide recommendations with respect to site grading, bearing capacity for house foundations, trench excavations, bedding and backfilling for service pipes, site dewatering, road construction, pavement design recommendations and soil hydraulic conductivity for storm water management design.

2.0 SITE CONDITIONS

The geotechnical investigation was conducted on Part Lot 18, Concession 12 of the Township of Centre Wellington. In general, the eastern two-thirds of the property is currently farm land, while the western one-third is treed. The site topography undulates slightly and the ground surface elevation drops towards the southwest corner of the property.

3.0 FIELD AND LABORATORY PROCEDURES

On January 25, 2006, a track-mount CME 55 drillrig operated by Aardvark Drilling Inc. was used to drill eight (8) boreholes (referenced as Boreholes 101 to 108) to depths of between 3.5 m (11.5 ft) and 5.0 m (16.4 ft) below the existing ground surface elevation. Standard penetration tests were conducted at 0.76 m (2.5 ft) intervals to depths of 3.0 m (10.0 ft) and at 1.5 m (5.0 ft) intervals below 3.0 m (10.0 ft) in all boreholes. Monitoring wells were installed in all eight boreholes to determine the presence and depth of the groundwater table.

Technical staff from CMT Engineering Inc. observed the drilling operation and collected and logged the recovered soil samples. Soil samples taken from Borehole 102 (3.05 to 3.51 m), Borehole 103 (4.57 to 5.03 m), Borehole 105 (2.29 to 2.74 m) and Borehole 107 (2.29 to 2.74 m) were placed in marked sample bags for grain size analyses (refer to Appendix C for laboratory test results). A bulk sample from Borehole 103 (1.5 to 2.0 m) was submitted for laboratory Proctor testing (refer to Appendix D for the laboratory test results). A small portion of each sample was placed in a sealed marked jar for moisture content determinations.

Gamsby and Mannerow Limited surveyed the ground surface elevations for all boreholes, as well as the tops of the monitoring wells.

Drawing 1 shows the site plan with all of the borehole locations.

4.0 SUBSOIL CONDITIONS

The soil conditions at the borehole locations are summarized briefly below, while a more detailed stratigraphic description is provided in the borehole logs in Appendix A. Cross-section profiles through Boreholes 101-103-102-104 and Boreholes 105-106-107-108 are provided in Appendix B.

4.1 Topsoil

Dark brown silt topsoil was found at the top of all eight boreholes. The topsoil was frozen at the time of the investigation. The thickness of the topsoil ranged from 30 mm to 60 mm (average 42 mm).

4.2 Organic Silt

Organic silt was found underlying the topsoil in Borehole 103. The organic silt was saturated, loose and brown with some sand and occasional topsoil nodules. The moisture content of the organic silt was 83.5% and the N-count was 8 blows per 0.30 m.

4.3 Silt or Sandy Silt

Silt was found underlying the sandy silt in Borehole 105. The silt was very moist, loose and brown with some clay, trace sand and trace gravel. The moisture content of the silt was 18.5% and the N-count was 8 blows per 0.30 m.

Sandy silt was found underlying the topsoil in Borehole 102. In general, the sandy silt was moist, compact and brown with a trace of clay and a trace of gravel. The moisture content was 10.4% and the N-count was 12 blows per 0.30 m.

4.4 Silt Till or Sandy Silt Till

Glacial till comprising silt or sandy silt was found in all boreholes (101 to 108). In general, the silt till was moist, compact to very dense and brown with trace to some sand,

gravel and clay. The moisture content ranged from 6.8% to 20.4% (average 11.7%) and the N-count ranged from 14 to 100 blows per 0.30 m (average 37 blows per 0.30 m). The sandy silt fill was generally moist, compact to very dense and brown with trace to some sand, gravel and clay. The moisture content ranged from 7.6% to 26.7% (average 13.4%) and the N-count ranged from 7 to 100 blows per 0.30 m (average 36 blows per 0.30 m).

4.5 Sand or Silty Sand

Sand was found in Boreholes 101, 102, 105 and 107. In general, the sand was very moist to wet, compact and brown with trace silt and/or trace gravel. The moisture content ranged from 14.3% to 20.0% (average 17.2%) and the N-count ranged from 7 to 22 blows per 0.30 m (average 15 blows per 0.30 m).

Silty sand was found in Boreholes 101, 103, 104, 105, 106 and 108. In general, the silty sand was wet to saturated, compact and brown with occasional trace gravel. The moisture content ranged from 10.5% to 27.7% (average 20.8%) and the N-count ranged from 1 to 25 blows per 0.30 m (average 10 blows per 0.30 m).

4.6 Clayey Silt

A localized layer of clayey silt was found in Borehole 107. The clayey silt was moist, compact and brown with trace sand and trace gravel. The moisture content was 13.6% and the N-count was 11 blows per 0.30 m.

4.7 Groundwater Conditions

Monitoring wells were installed in all boreholes. The monitoring wells were constructed utilizing 50 mm Schedule 40 PVC pipe with a 3 m long slot 10 screen surrounded by the sand filter comprising #3 industrial sand. The boreholes were backfilled with 3/8" bentonite holeplug from the top of the sand filter to the existing ground surface. For protection and security purposes, locking steel protective covers were installed on all of the monitoring wells.

A copy of the well record has been included in Appendix E. It is a requirement of Regulation 903 of the Ontario Water Resources Act that the monitoring well installations be abandoned within 180 days after they are no longer in use.

At the time of writing, the static water levels in the monitoring wells had been read on February 8, 2006, February 20, 2006, March 9, 2006, March 25, 2006 and March 29, 2006. A summary showing the ground surface, borehole bottom and water level elevations for Boreholes 101 to 108 are provided below:

Borehole No.	Ground Surface Elevation (m)	Elevation of Borehole Bottom (m)	Elevation of Water Table (m)				
			(F) - Frozen				
			Feb 8, 2006	Feb 20, 2006	Mar 9, 2006	Mar 25, 2006	Mar 29, 2006
101	413.64	408.64	413.07	413.11	412.83	412.96	--
102	414.37	409.37	411.57	411.96	411.91	412.48	--
103	414.89	409.89	412.65	412.98	412.88	412.77	--
104	410.93	407.43	410.36	410.60	410.17	410.66	--
105	414.05	409.28	414.05	414.07 (F)	414.15 (F)	414.15 (F)	414.68
106	410.91	405.94	410.67	410.86 (F)	410.93 (F)	410.75	--
107	409.58	406.08	409.43	409.06 (F)	409.12 (F)	409.41	--
108	410.32	406.82	409.06	409.21	408.82	409.01	--

Due to the close proximity of the groundwater to the ground surface, some of the monitoring wells were frozen at the time of the water level readings.

The groundwater levels will be measured on a monthly basis in an effort to try and establish extreme (high and low) groundwater elevations.

5.0 DISCUSSION

It is our understanding that the property owner is proposing to develop a residential subdivision on the property investigated. The subdivision will be fully serviced with municipal sewers and water supply. A storm water management facility is proposed to be constructed in the western portion of the site.

5.1 Site Grading

Prior to the commencement of any site grading, all topsoil and organic silt soils (Borehole 103) must be removed from the proposed building envelopes (including extended zone of influence areas), road allowance and driveways.

Due to the high water table and isolated wet surface conditions, it may be necessary to utilize an excavator during topsoil stripping to minimize over-excavation as a result of soil disturbance from heavy construction traffic.

At this time, the proposed founding elevations for the residences are not available. However, it would appear that some cut and fill operations will be required to level the building site.

Prior to any placement of structural fill, the subgrade for the building envelope must be prepared large enough to accommodate a 1:1 slope commencing at a distance of 1.0 m beyond the outside edge of the proposed foundation down to approved native founding soils.

Soils approved for use as structural fill must be placed in loose lifts not exceeding 0.3 m (1 ft) in depth for granular soils and 0.2 m (8") in depth for fine grained (silt and clay) soils and compacted using adequate heavy vibratory padfoot compaction equipment to a minimum of 98% standard Proctor maximum dry density (SPMDD). The approved structural fill materials must be free of frozen materials, organics or other deleterious materials and must not contain particles exceeding 150 mm (6") in diameter. The soils must be at moisture contents suitable to achieve the specified compaction.

A laboratory Proctor moisture-density test was performed on a bulk sample of the silt till from Borehole 103 (depth 1.5 to 2.0 m). The results of the laboratory Proctor test indicate that the optimum moisture content of the sample is 8.3%. Since the insitu moisture contents of the split spoon sample of silt till ranged from 6.8% to 20.4% (average 11.7%), it should be anticipated that the majority of the silt till will require air-drying in order to achieve the specified compaction during construction.

The fine grained soils encountered in the geotechnical investigation are highly susceptible to strength losses if subjected to frequent disturbance by construction traffic. Therefore, it is recommended to minimize construction traffic on subgrade soils.

It would be recommended that the site grading and underground service installation be undertaken during drier warm weather conditions in order to minimize dewatering operations, eliminate frost problems and most importantly improve the placement and compaction of structural fill and backfill materials. Proper compaction and backfilling operations are imperative in order to provide adequate support for structures, service pipes, driveway and roadways.

If site grading and site servicing is undertaken during cold or wet weather conditions, projected overall costs would be anticipated to be higher and the project would be expected to take longer to complete.

5.2 Site Dewatering

Based on this geotechnical investigation and similar high water tables encountered during the construction of the neighbouring Ville Lora Downs Subdivision, water concerns should be anticipated for this project. Static water levels measured in the monitoring wells suggest that perched groundwater can be expected at the locations of Boreholes 101, 102, 103, 105 and 106 which were advanced within the proposed residential development area. The water appears to be surface water that has perched on top of the relatively impermeable sandy silt till, sandy silt and silt till soils. Furthermore, artesian conditions can also be expected at the locations of Boreholes 101, 102 and 103. The artesian water appears to be located between the upper sandy silt till and lower silt till in Borehole 101, between the upper and lower silt tills in Borehole 102 and below the silt till layer in Borehole 103.

Provisions for site dewatering should be part of the site development and construction process. Normally, it would be recommended that well points be installed in order to dewater the site so that site services and residential foundations could be installed. However, based on past experience, the installation of a well point dewatering system by qualified contractors can be very expensive and not necessarily guaranteed. It is probably most cost-effective to install a series of inverted drainage pipes in advance of the service (sanitary, storm and water) trench excavations and also at the locations of the manholes. Water pumps should be utilized to pump water from the inverted pipes on a continuous basis in order to keep the water table drawn down below the excavation level. Temporary drainage trenches should be constructed to remove the site water to a storm water retention pond (or reasonable alternative). The removal of considerable amounts of fine soil particles from the pumping operation can be anticipated. As such, the drainage trenches, storm water pond, pumps and hoses will most likely require regular cleanout. It might be cost-effective in regard to road construction and house construction to investigate the possibility of installing a permanent deep drainage system to lower the water table in the immediate area. Caution would be necessary with this option, since it could affect wells and building structures on adjacent properties.

The dewatering conditions may improve if work is conducted during the drier summer months as well as following the installation of the services.

5.3 Excavations

The anticipated sanitary, storm and water pipe invert elevations are all expected to be well below the water table and therefore site dewatering will be required (see Section 5.2 above). Based on observations from the neighbouring Ville Lora Downs Subdivision, the water levels in the summer are generally lower and therefore dewatering requirements

may be less. However, the anticipated effects of the artesian water conditions are still expected to be of concern.

All excavations must be carried out in accordance with Ontario Regulation 213/91 (Reg 213/91) of the Occupational Health and Safety Act and Regulations for Construction Projects.

Type 2 Soils: The native glacial till soils would be classified as Type 2 soils under Reg 213/91 and must be sloped to within 1.2 m of the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical. Where excavations expose glacial till soils underlain by wet sand or silt soils, the recommendations for Type 4 soils below must be adhered to.

Type 3 Soils: The native sand and silt soils in an unsaturated condition (above the water table) would be classified as Type 3 soils under Reg 213/91 and must be sloped from the bottom of the excavation at a minimum gradient of 1 horizontal to 1 vertical.

Type 4 Soils: All native sand or silt soils in a saturated condition (below the water table) would be classified as Type 4 soils under Reg 213/91. Excavations that expose the Type 2 and Type 3 soils noted above but are underlain by saturated sand or silt soils must be treated as Type 4 soils as well. Type 4 soils must be sloped at a minimum gradient of 3 horizontal to 1 vertical. The loose wet condition of the Type 4 soils makes them very susceptible to sloughing and slope failure during excavation.

If it is not practical to excavate according to the above requirements, then a trench box system (designed in accordance with the Ontario Health and Safety Act Regulations) may be utilized.

It should be noted that some of the native glacial till soils become very dense with depth (N-values in excess of 100 blows per 0.30 m) and may prove difficult to excavate with conventional excavating equipment. It is also imperative that when the very dense soils are utilized for backfilling of service trenches, the material must be broken down (pulverized) to minimize voids and reduce the potential for settlement.

5.4 Service Pipe Bedding

The native soils are generally considered to be suitable for indirect support of the proposed service pipes. Where water inflow is a concern and the soil conditions are not suitable to support the pipe, then 80 mm to 120 mm (3" to 5") river stone (or equivalent) with a 150 mm (6") layer of 19 mm clear stone should be used to create an adequate supporting base for the pipe.

Pipe embedment and backfill for flexible pipes should be undertaken in accordance with OPSD-802.010. Pipe embedment, cover and backfill for rigid pipes should be undertaken in accordance with OPSD-802.030 or OPSD-802.031. Trenching, backfilling and compaction with respect to storm sewer pipe installations should comply with OPSS 514.

Flexible Pipes: The pipe bedding should be shaped to receive the bottom of the pipe. If necessary, pipe culvert frost treatment should be undertaken in accordance with OPSD-803.030 and OPSD-803.031. The trench excavations should be symmetrical with respect to the centreline of the pipe. The granular material placed under the haunches of the pipe must be compacted to 95% SPMDD prior to the continued placement and compaction of the embedment material. The homogeneous granular material used for embedment should be placed and compacted uniformly around the pipe. Should wet conditions be encountered at the base of the trench, then the pipe should consist of 19 mm clear stone (meeting OPS Specifications). Normally, it would be advisable to wrap the clear stone with geotextile to prevent fine soils from entering the clear stone and thereby creating voids around the pipe. In wet conditions, this is not possible to do and generally not necessary since most of the void spaces are quickly filled with fine soils as water (with suspended fine soils) rapidly enters the excavation. It is imperative that the newly installed pipe be backfilled as soon as possible in order to prevent the potential for pipe uplift. This can occur due to buoyancy, as water enters the excavation. It is also advisable to check the elevation of the installed pipe at regular intervals to ensure that uplift has not occurred. Protection against heavy construction equipment should be undertaken in accordance with OPSD-808.010.

Rigid Pipes: In general, the pipe installation recommendations for rigid pipes are the same as those for flexible pipes except that the minimum depth of bedding below a rigid pipe should be $0.15 D$ (where D is the pipe diameter). In no case should this dimension be less than 150 mm or greater than 300 mm.

5.5 Trench Backfill

Native backfill material can be used to fill the trench from 12" (30 cm) above the pipe to the subgrade elevation provided that the material is free of organics, not frozen and is not overly wet (above the optimum moisture).

Based on the existing water table, the moisture contents determined from soil samples that were taken during the geotechnical investigation, and the laboratory Proctor test (see Appendix C), it can be assumed that most soils will be too wet to enable proper compaction. As such, these soils should be allowed to drain and air-dry as long as possible before backfilling.

If wet or frozen soils are used for backfill purposes, proper compaction of the backfill will not be possible and settlement of the trenches can be expected. Site assessments will be required to determine what options can be undertaken to construct a suitable road base. These options may include subexcavating and increasing the thickness of the granular subbase, the possible use of high strength geotextiles, or a combination of both.

5.6 Sensitivity of Subsoils

The silty nature of many of the soils encountered in the boreholes can make them highly susceptible to strength losses and will prove difficult to place and compact if they become overly wet as a result of inclement weather or water seepage. If the soils become overly wet and disturbed, they may become unsuitable for reuse and require subexcavation. As such, the following is recommended:

- provide proper measures for adequate drainage during construction
- use a smooth-lipped bucket while excavating to the subgrade elevation to reduce disturbance
- minimize construction traffic traveling over the subgrade soils

5.7 Road Construction and Pavement Design

In order to achieve a suitable subgrade for the construction of the pavement structure, the following recommendations are provided:

- a) If necessary, maintain the site dewatering system during preparation of the road subgrade. Once the road subgrade is completed, the drainage pipes should be removed or cut off at the subgrade elevation and infilled with lean concrete or a bentonite slurry.
- b) The design subgrade for the road should be proof-rolled using heavy rubber-tire equipment, such as a grader. Compactive effort should be applied and compaction tests should be undertaken. Areas requiring fill to achieve the subgrade elevation should be treated as indicated above prior to placement of any additional fill. The subgrade should be evaluated to determine if subexcavation and additional Granular 'B' will be required or if the installation of a reinforcing geotextile will be necessary.
- c) The road subgrade should be cut to grade using a smooth-lipped bucket. The subgrade should be graded smooth (with no depressions) and sloped at a minimum of 2%. Construction traffic should not be allowed onto the prepared road subgrade. Construction traffic should travel only on the Granular 'B' subbase. It may be necessary to temporarily

increase the thickness of the Granular 'B' during road construction to accommodate the truck traffic.

d) It is recommended that 100 mm diameter perforated subdrains fitted with a filter sock be installed along each curb line to collect and redirect water beneath the pavement surface. It is suggested that the subdrains be installed in a 0.3 m (1 ft) by 0.3 m (1 ft) trench and placed approximately 50 mm (2") from the trench bottom. In drier conditions, the perforated subdrain with a factory-installed filter sock can be installed in Granular 'A' bedding. In wet conditions, 19 mm clear stone wrapped completely in non-woven geotextile (such as Terrafix 270R or equivalent) is recommended. Rapid drainage of the pavement structure is critical to ensure long-term performance of the road.

Based on the anticipated loading and considering that the subsoils contain frost-susceptible soils, the following pavement design is recommended for the proposed roads:

Material	Recommended Thickness
Asphaltic Concrete	HL3 - 40 mm (1.5") HL4 or HL8 - 50 mm (2.0")
Granular 'A' Base	150 mm (6.0")
Granular 'B' Subbase	450 mm (18.0")

The granular subbase materials should be compacted to 100% SPMDD. Asphaltic concrete should be supplied, placed and compacted to 97% Marshall bulk relative density in accordance with OPSS 1150 and OPSS 310.

The pavement should be designed to ensure that water will not pond on the pavement surface. If the surface asphalt is not placed in a reasonable time following the placement of the binder asphalt, it is recommended that the catch basin lids be lowered or apertures provided to allow the surface water to drain rather than accumulating around the catch basins.

5.8 Bearing Capacity / Settlement

The proposed residential buildings may be supported on conventional spread and pier footings provided they are founded on undisturbed native soils at or below the elevations listed in the following table or structural fill prepared as detailed in Section 5.1 of this report:

Borehole No.	Existing Ground Surface Elevation (m)	Highest Recommended Footing Elevation (m)	Soil Type
101	413.64	413.01	sandy silt till
102	414.37	412.70	silt till
103	414.89	413.59	silt till
105	414.05	411.65	sandy silt till
106	410.91	408.51	silt till

It is ideally recommended that foundations be constructed above the water table. The native founding soils and structural fill in a drained condition would be considered suitable to support foundations designed with a safe net allowable bearing capacity of 150 kPa. It is anticipated that the water table may be within one footing width below the founding elevation. Therefore, a safe net allowable bearing capacity of 75 kPa should be used for design purposes.

With respect to the bearing capacities as determined above, total and differential settlements are estimated to be within the generally acceptable limits of 25 mm (1") and 19 mm (3/4") respectively.

A minimum of 1.2 m (4 ft) of soil cover above the footing grade must be provided for frost protection.

5.9 Residential Drainage Considerations

If high water conditions continue to exist during the construction of the residential foundations, and the foundations are constructed near or below the water table, then the following will be required:

- a granular drainage layer and sump pump will be required as per Section 9.14.4 of the current Ontario Building Code

- slab-on-grades constructed where groundwater levels may cause hydrostatic pressure must be designed to resist such pressures
- slab-on-grade and exterior walls must be waterproofed

If foundation construction occurs above the high water table, then conventional construction methods can be utilized.

5.10 Potential Storm Water Management Facility

Boreholes 104, 107 and 108 were all drilled in the open space area (west side of property) where a storm water management facility is proposed. In general, Borehole 104 has silt till underlain by silty sand. It would appear that artesian conditions are present in the silty sand layer below the more impermeable silt till layer. Based on the monitoring well readings, the water level fluctuates to just below the ground surface elevation.

In general, Borehole 107 has sand underlain by clayey silt and lower sandy silt till. Artesian conditions may be present in the sandy silt till below the more impermeable clayey silt layer. Based on the monitoring well readings, the water level was just below the ground surface elevation. The upper sand layer has a high moisture content due to the infiltration of surface water, which is in turn impeded by the lower clayey silt layer.

In general, Borehole 108 has silty sand underlain by sandy silt till. Artesian conditions may be present in the lower portion of the sandy silt layer below the more impermeable silty sand layer (higher density, lower moisture). Based on the monitoring well readings, the water level has fluctuated to within approximately 1.0 m of the ground surface elevation.

Based on the results of the geotechnical investigation, it can be concluded that the soil and groundwater conditions in the area of Boreholes 104, 107 and 108 are unsuitable for an inground storm water management facility.

6.0 SITE INSPECTIONS

Site grading, dewatering, trench excavations, backfilling and compaction of the service pipes should be supervised by qualified geotechnical personnel to ensure that a suitable subbase is prepared, proper backfill materials are used and that the specified compaction is achieved.

The construction of the pavement structure should also be supervised by qualified personnel to ensure that suitable materials are used and that the specified compaction is achieved. It is also

recommended that the residential foundation excavations be examined to ensure that the bearing capacity of the soil is suitable to support the structures.

CMT Engineering Inc. would be pleased to provide inspection, testing and consulting services for this project.

7.0 LIMITATIONS OF THE INVESTIGATION

This investigation was conducted to determine the subsurface conditions for this project and the comments are based on the information gathered at the borehole locations only. It is therefore assumed that the borehole information is representative of the subsoil conditions across the site. Should any conditions at the site be encountered which differ from those found at the borehole locations, we request that we be notified immediately.

This report is intended solely for the client named. Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties.

We trust that this report meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Respectfully submitted,

Robert Koopmans, P.Eng.
Consulting Engineer



Tim Salter, C.E.T.



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Base plan provided by:



QUALITY AND PROFESSIONALITY
CONSULTING SERVICES FOR LAND AND
SURVEYING



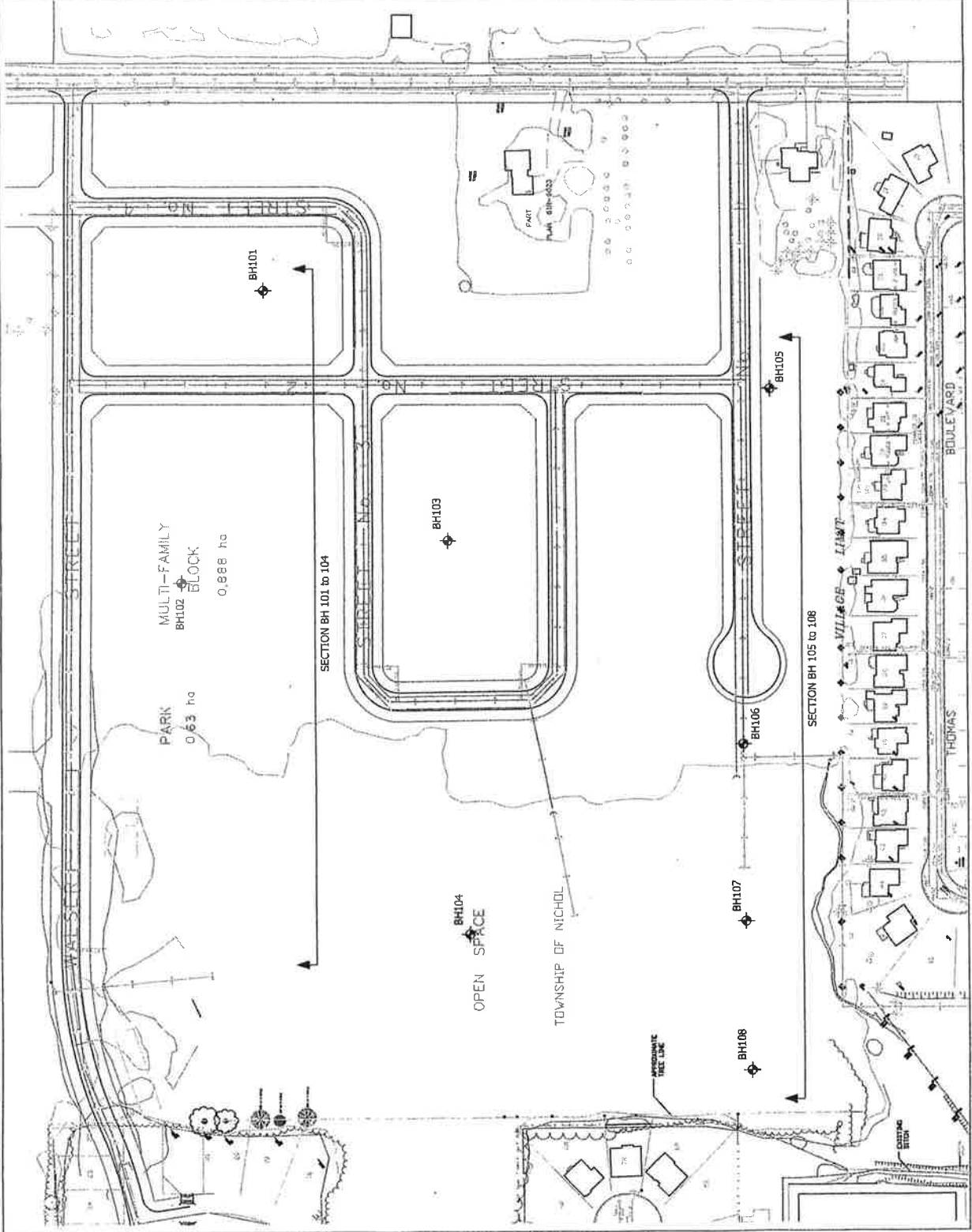
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BOREHOLE LOCATION

PLAN
Ainley Subdivision
Township of Centre
Wellington
Village of Elora, ON

Project: 06-004	Drawing: 2
Date: Mar. 2006	Sheet: 1
Scale: 1:2000	



APPENDIX A

**BOREHOLE LOGS
Boreholes 101 to 108**

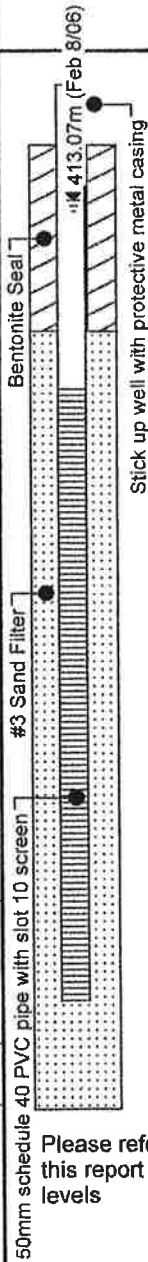
BOREHOLE 101

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 413.64m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer
								kPa
0					Ground Surface (m) 413.64			
0					Topsoil 0.00			
1					Dark brown silt, frozen 413.31			
2					Sandy Silt Till 0.33			
3	1	SS	1		Compact brown sandy silt till, some clay, some gravel, moist		9.7	22
4								
5							20.7	20
6	2	SS	2		Sand 411.84			
7					Compact brown sand, very moist to wet 411.54			
8					Silty Sand 2.10			
9	3	SS	3		Compact brown silty sand, wet		24.9	14
10								
11	4	SS	4		spoon sank through silty sand under weight of rods		27.7	1
12								
13	4				Sand 409.64			
14					Dense brown sand, trace silt, wet 4.00			
15							20.4	40
16	5	SS	5		Silt Till 408.94			
17					Dense brown silt till, some sand, gravel and clay, moist 4.70			
18					408.64			
19					5.00			
					End of Borehole			



Please refer to section 4.7 of this report for additional water levels



BOREHOLE 102

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 414.37m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer	
								kPa	SPT (N)
								100 300 10 30 50 70 90 Blows/0.3 m	
0					Ground Surface (m) 414.37				
0					Topsoil				
1					Dark brown silt, frozen 414.07				
1					Sandy Silt 0.30				
2					Compact mottled brown sandy silt till, trace clay, trace gravel, moist				
3	SS		1					10.4	12
4					413.00				
5					Silt Till 1.37				
6	SS		2		Compact brown silt till, trace sand, trace gravel, moist			17.6	15
7									
8	SS		3					4.8	30
9					Sand 2.60				
10					Compact brown sand, trace silt, trace gravel, moist to very moist				
11	SS		4					20.0	22
12									
13					410.37				
14					Silt Till 4.00				
15					Dense grey silt till, trace sand gravel and clay, moist				
16	SS		5					7.7	53
17					409.37				
17					End of Borehole 5.00				
18									
19									

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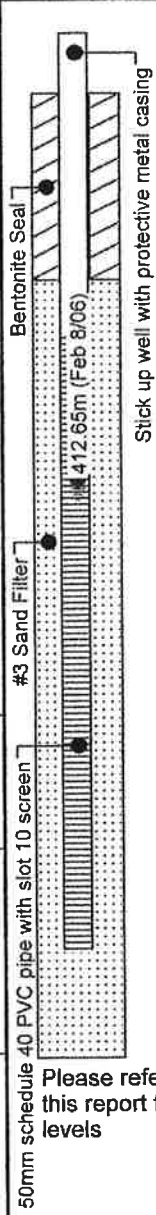
BOREHOLE 103

Date Drilled: Jan. 24, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 414.89m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 414.89			
0					Topsoil Dark brown silt, frozen 0.00			
1					414.34			
2					Organic Silt Loose brown organic silt, some sand, occasional topsoil nodule, very moist 0.55			
3	SS		1		413.89			
4					Silt Till Compact brown silt till, trace sand, trace gravel, moist, becoming dense with depth 1.00			
5							13.6	20
6	AS		2					
7								
8	SS		3				9.2	32
9								
10								
11	SS		4		411.54		13.1	37
12					Silty Sand Dense brown silty sand, wet 3.35			
13					410.89			
14					Becoming loose, trace clay, saturated 4.00			
15								
16	SS		5		409.89		23.1	6
17					End of Borehole 5.00			
18								
19								



Please refer to section 4.7 of this report for additional water levels

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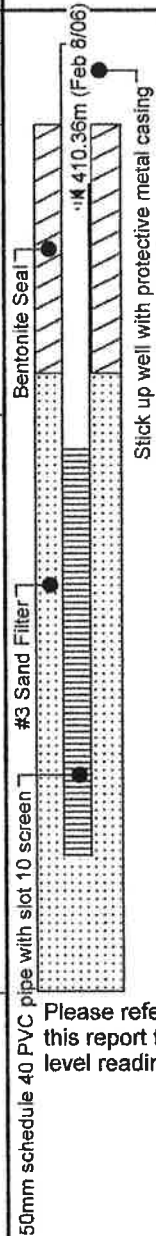
BOREHOLE 104

Date Drilled: Jan. 24, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 410.93m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer kPa SPT (N) Blows/0.3 m
0					Ground Surface (m) 410.93			
0.00					<i>Topsail</i> Dark brown silt, frozen			
1					410.63			
1.30					<i>Silt Till</i> Compact dark brown silt till, some clay, trace sand, trace gravel, moist			
3	SS		1				10.2	14
4					409.56			
5					<i>Silty Sand</i> Compact brown silty sand, saturated			
5.137								
6	SS		2				23.0	14
7								
8	SS		3				17.8	8
9								
10								
10.3								
11	SS		4				25.9	10
12					407.43			
12.350					End of Borehole			
13								
14								



Please refer to section 4.7 of this report for additional water level readings



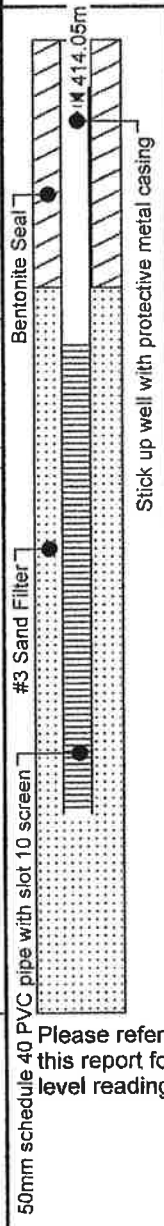
BOREHOLE 105

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 414.05m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Inst'n Date (Feb 8/06)	Moisture Content %		Pocket Penetrometer	
							Wp [---X---] Wl	SPT (N)	kPa	Blows/0.3 m
							10 20 30 40	10 30 50 70 90	100 300	
0					Ground Surface (m) 414.05					
0					Topsoil 0.00					
1					Dark brown silt, frozen 413.67					
2					Silty Sand 0.38					
2					Loose red-brown silty sand, moist					
3	SS		1					18.5		4
4										
5					Silt 1.37					
5					Loose brown silt, some clay, trace sand, trace gravel, very moist 412.68			18.5		8
6	SS		2							
7					Sand 2.00					
7					wet sand seam at 2.0m depth					
8	SS		3		Sandy Silt Till			7.7		26
8					Very dense brown sandy silt till, some clay, trace gravel, moist					
9										
10	SS		4					8.1		100
11										
12										
13										
14										
15	SS		5					7.6		100
16					End of Borehole 4.77					
17										
18										
19										



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BOREHOLE 106

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 410.91m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation 50mm schedule 40 PVC pipe with slot 10 screen #3 Sand Filter Bentonite Seal Stick up well with protective metal casing (Feb 8/06)	Moisture Content %		Pocket Penetrometer						
							Wp [---X---] Wl	SPT (N)	kPa	Blows/0.3 m					
10							10	20	30	40	10	30	50	70	90
0					Ground Surface (m) 410.91										
0					Topsoil Dark brown silt, frozen	0.00									
1					410.59										
1					Silty Sand Loose mottled grey silty sand, trace gravel, moist	0.32									
3	SS		1								24.0				9
4															
5											12.1				9
6	SS		2												
7					408.81										
7					Sandy Silt Till Dense to very dense grey sandy silt till, trace sand, trace gravel, moist	2.10									
8	SS		3								6.8				33
9															
10															
10	SS		4								8.3				28
11															
12															
13															
14															
15															
15	SS		5								9.0				100
16					405.94										
16					End of Borehole	4.97									
17															
18															
19															

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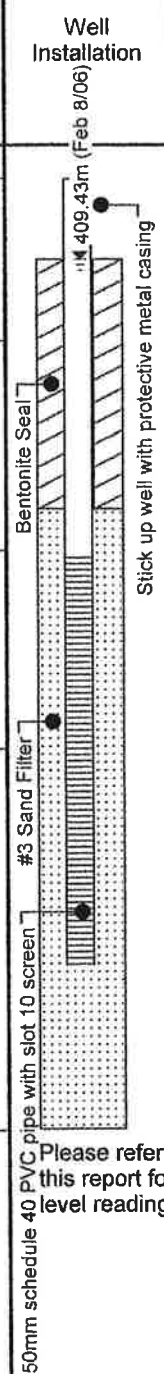
BOREHOLE 107

Date Drilled: Jan. 24, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 409.58m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content % Wp [---X---] Wl	Pocket Penetrometer
								kPa
0					Ground Surface (m) 409.58			
0					Topsoil Dark brown silt, frozen			
2					408.98			
2.60					Sand Loose red-brown sand, some silt, moist to wet			
3	SS		1				14.3	7
4					408.21			
4.137					Clayey Silt Compact brown clayey silt, trace sand, trace gravel, moist			
5	SS		2				13.6	11
6					407.48			
6.210					Sandy Silt Till Compact to loose brown sandy silt till, trace clay, saturated			
7								
7.210	AS		3				19.1	25
8								
8.3								
9								
9.3	SS		4				17.3	7
10								
10.350					End of Borehole			
11								
11.350								
12								
12.4								
13								
14								



Please refer to section 4.7 of this report for additional water level readings

CMT ENGINEERING INC.
 1011 Industrial Crescent, Unit 1
 St. Clemente, Ontario N0B 2M0
 phone 519-699-5775 fax 519-699-4664
 www.cmlinc.net



BOREHOLE 108

Date Drilled: Jan. 25, 2006
Rig: CME 55
Contractor: Aardvark
Drilling Method: HSA

Elevation: 410.32m
Logged by: CD

Project No.: 06-004
Project: Ainley Subdivision
 Township of Centre Wellington
Location: Elora

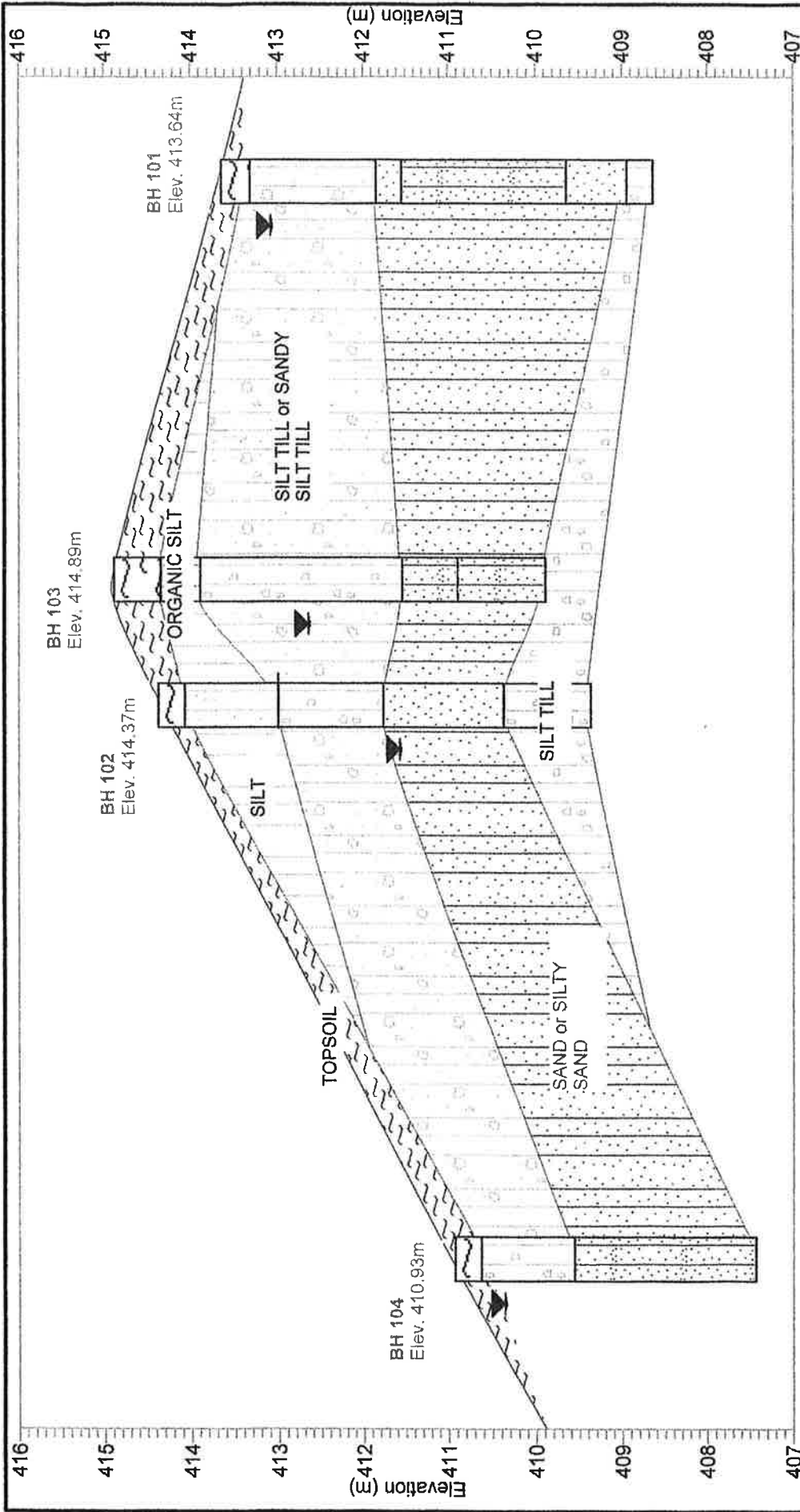
Depth (ft/m)	Sample Type	Recovery (%)	Sample Number	Symbols	SOIL DESCRIPTION	Well Installation	Moisture Content %		Pocket Penetrometer			
							Wp [---X---] Wl		kPa	SPT (N)		
								100	300			
								Blows/0.3 m				
								10	30	50	70	90
0					Ground Surface (m) 410.32							
0					Topsoil Dark brown silt, frozen							
0					409.74							
1					Silty Sand Compact mottled brown silty sand, trace gravel, moist							
1	SS		1		408.95		10.5		25			
2					Sandy Silt Till Dense to loose brown sandy silt till, wet							
2	SS		2		408.95		16.8		38			
3												
3	SS		3				19.9		18			
4												
4	SS		4				26.7		7			
5					406.82							
5					End of Borehole 3.50							
6												
6												
7												
7												
8												
8												
9												
9												
10												
10												
11												
11												
12												
12												
13												
13												
14												

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 1011 Industrial Crescent, Unit 1
 St. Clements, Ontario N0B 2M0
 phone 519-699-5775 fax 519-699-4664
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Please refer to section 4.7 of this report for additional water level readings

APPENDIX B
CROSS-SECTIONS



1011 Industrial Cres.,
Unit 1
St. Clements, Ontario

Project: AINLEY SUBDIVISION

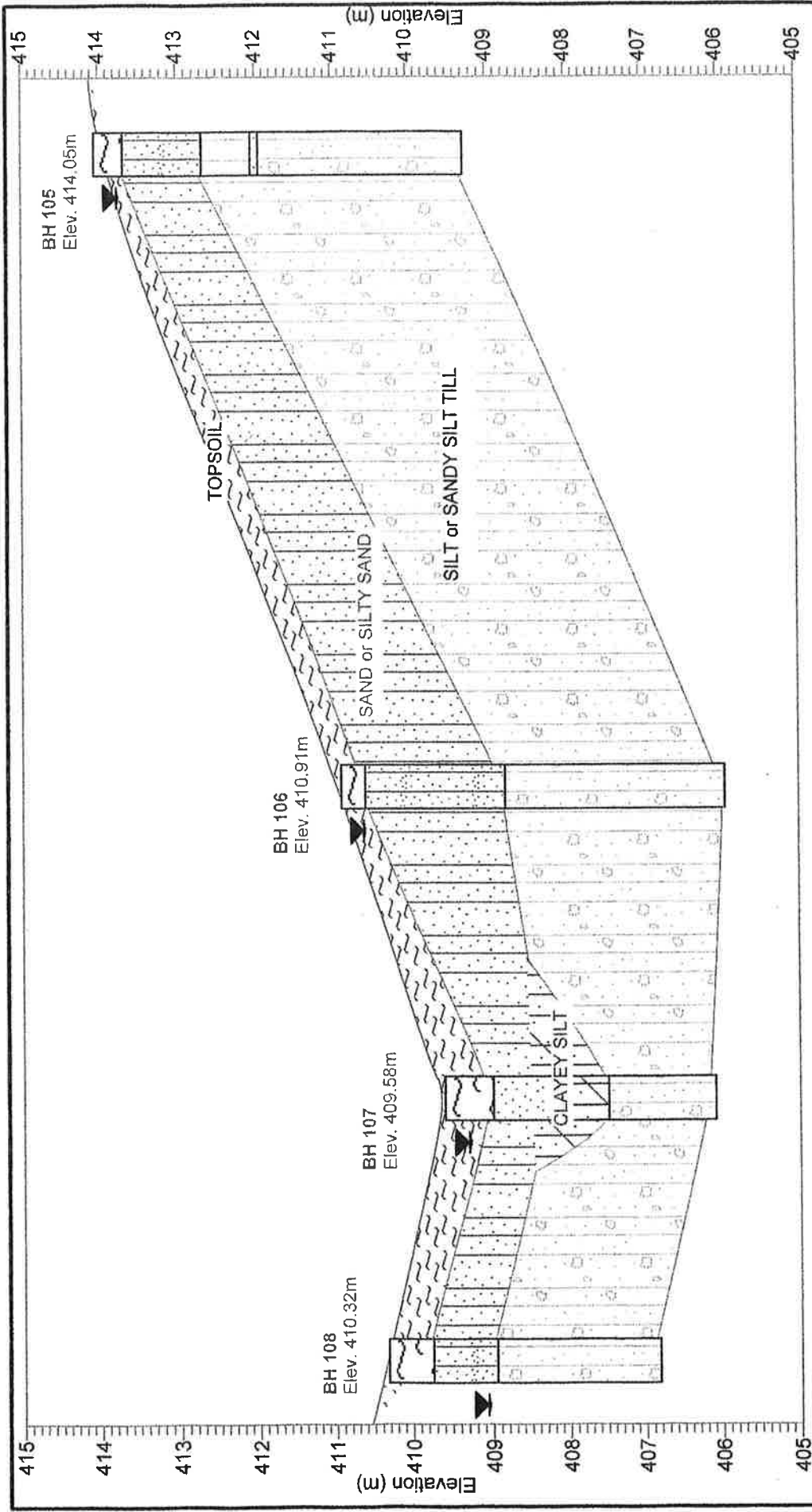
Project Number: 06-004

Location: Elora, Ontario

Drawn By: JS

Date: February, 2006

NOTE: Water levels shown were measured on Feb. 8, 2006. Please refer to Section 4.7 of this report for additional water levels.



NOTE: Water levels shown were measured on Feb. 8, 2006. Please refer to Section 4.7 of this report for additional water levels.

Project: ANLEY SUBDIVISION
 Project Number: 06-004
 Location: Elora, Ontario
 Drawn By: JS
 Date: February 2006

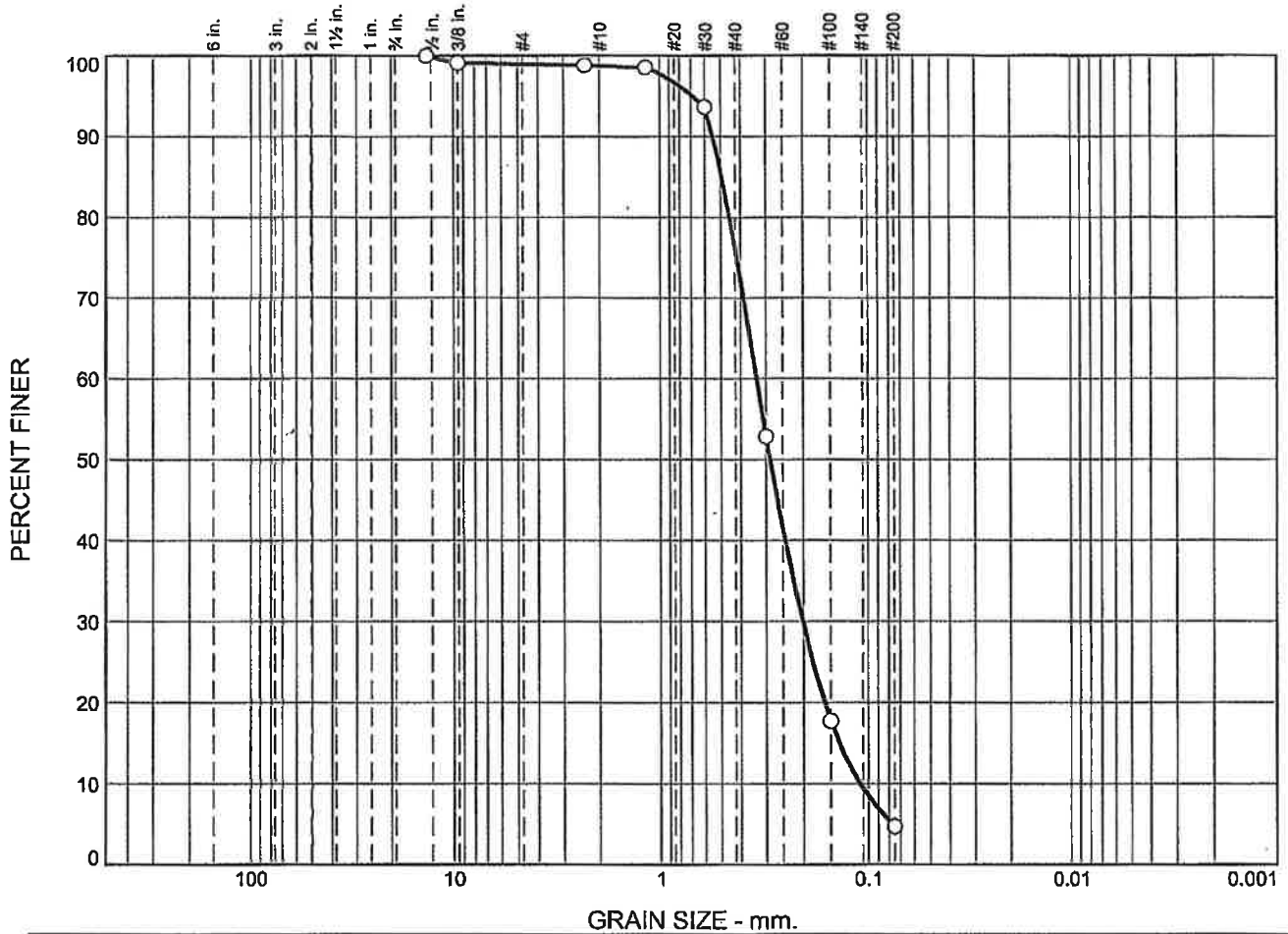


1011 Industrial Cres.,
 Unit 1
 St. Clements, Ontario

APPENDIX C

GRAIN SIZE ANALYSES

Particle Size Distribution Report

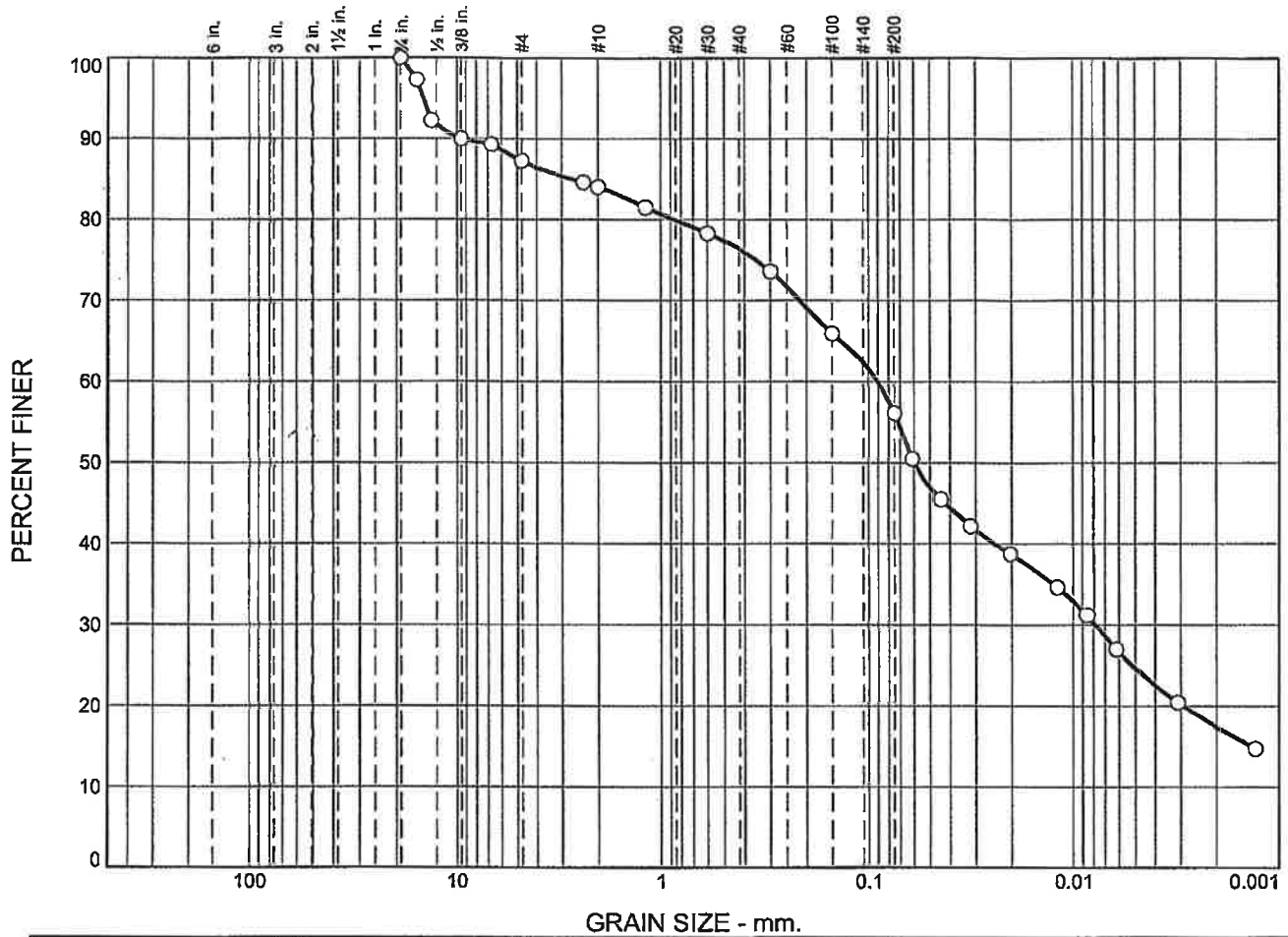


	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	1.0	0.2	22.7	71.4	4.7	

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH102 - SS4	1	3.05-3.51m	Sand, trace silt, trace gravel	SP
				Tested by CMT - January 27, 2006	

CMT Engineering Inc. St. Clements, ON	Client: Ainley Subdivision Project: Township of Centre Wellington Elora, Ontario Project No.: 06-004
--	--

Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	12.8	3.2	7.6	20.3	38.7	17.4

SOIL DATA

SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH105 - SS3	1	2.29-2.74m	Sandy silt, some clay, some gravel	ML
Tested by CMT - January 27, 2006					

CMT Engineering Inc.

St. Clements, ON

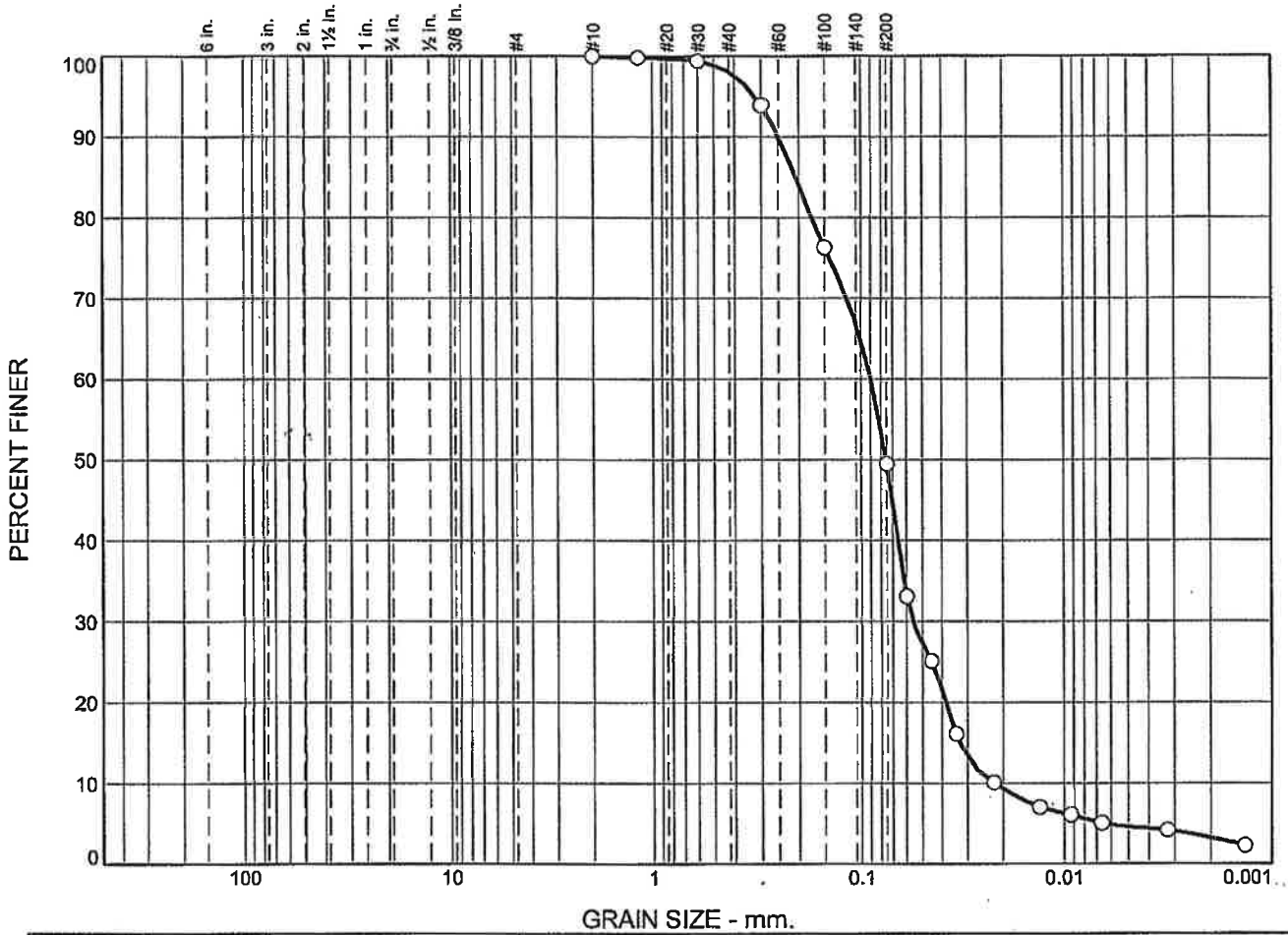
Client: Ainley Subdivision

Project: Township of Centre Wellington
Elora, Ontario

Project No.: 06-004

Figure 3

Particle Size Distribution Report



	% Cobbles	% Gravel		% Sand			% Fines	
		Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
○	0.0	0.0	0.0	0.0	1.9	48.6	46.3	3.2

SOIL DATA					
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	Material Description	USCS
○	BH107 - SS3	1	2.29-2.74m	Sand and silt, trace clay	SM
Tested by CMT - January 27, 2006					

CMT Engineering Inc.

St. Clements, ON

Client: Ainley Subdivision

Project: Township of Centre Wellington
Elora, Ontario

Project No.: 06-004

Figure 4

APPENDIX D
LABORATORY PROCTOR TEST

CMT ENGINEERING INC.

LABORATORY PROCTOR TEST

PROJECT NO.: 06-004

PROJECT: Ainley Subdivision

PROJECT LOCATION: Township of Centre Wellington (Elora)

SAMPLED FROM: Borehole 103, BS, depth 1.5 to 2.0 m

DATE SAMPLED/BY: January 25, 2006 by C.D. of CMT Inc.

DATE TESTED/BY: January 26, 2006 by J.S. of CMT Inc.

SOIL TYPE: silt till

REMARKS:

TEST STANDARD:

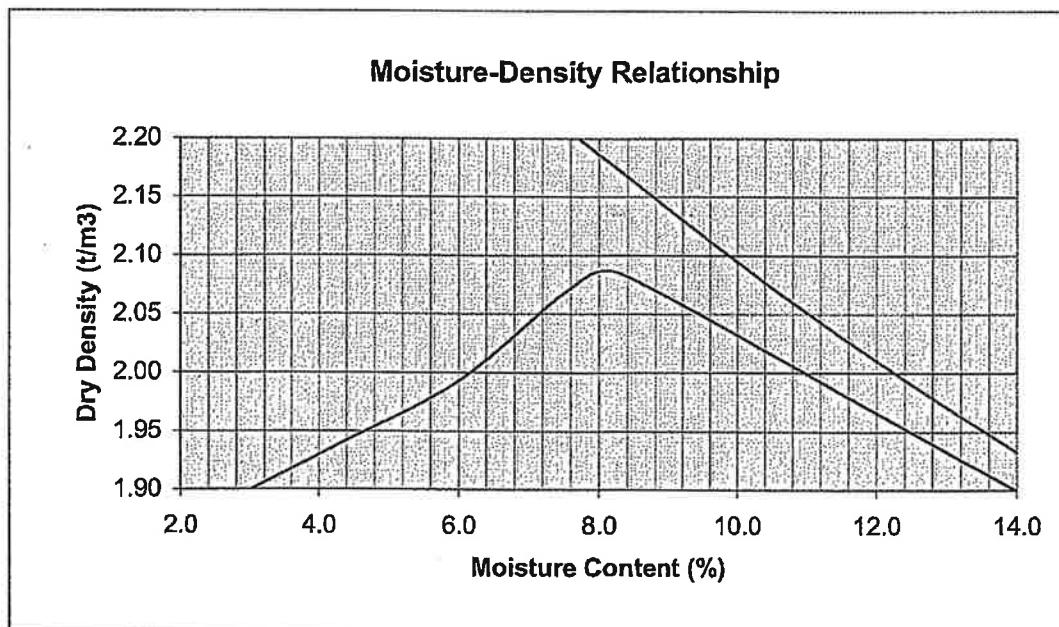
ASTM D698

PROCEDURE - A B C

ASSUMED SPECIFIC GRAVITY: 2.65

MAXIMUM DRY DENSITY: 2.085 t/m³

OPTIMUM MOISTURE CONTENT: 8.3%



APPENDIX E

WELL RECORD

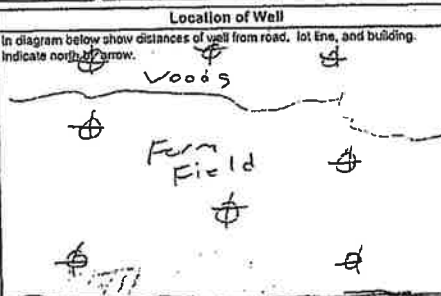
Instructions for Completing Form

- For use in the Province of Ontario only. This document is a permanent legal document. Please retain for future reference.
- All Sections must be completed in full to avoid delays in processing. Further instructions and explanations are available on the back of this form.
- Questions regarding completing this application can be directed to the Water Well Management Coordinator at 416-235-8203.
- All metre measurements shall be reported to 1/10th of a metre.
- Please print clearly in blue or black ink only.

Well Owner's Information and Location of Well Information						Ministry Use Only		
First Name James		Last Name Keating		Mailing Address (Street Number/Name, RR, Lot, Concession) Matheson St.		MUN	CON	LOT
County/District/Municipality		Township/City/Town/Village Elera		Province Ontario	Postal Code	Telephone Number (include area code)		
Address of Well Location (County/District/Municipality) Gerric Rd.				Township Center Wellington	Lot 17-18	Concession 12		
RR#/Street Number/Name				City/Town/Village Elera	Site/Compartment/Block/Tract etc.			
GPS Reading	NAD 83	Zone	Eastings N258214	Northings	Unit Make/Model	Mode of Operation: <input type="checkbox"/> Undifferentiated <input type="checkbox"/> Averaged <input type="checkbox"/> Differentiated, specify		

Log of Overburden and Bedrock Materials (see instructions)

General Colour	Most common material	Other Materials	General Description	Depth From	Metres To
Brown	SILT TILL			0	1.5
Brown	SILT SAND			1.5	4.5

Hole Diameter Depth Metres Diameter Centimetres From To 0 4.5 72		Construction Record Inside diam centimetres Material Well thickness centimetres Depth From Metres To 4 <input type="checkbox"/> Steel <input type="checkbox"/> Fibreglass <input checked="" type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized 0 1.5 <input type="checkbox"/> Steel <input type="checkbox"/> Fibreglass <input type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized <input type="checkbox"/> Steel <input type="checkbox"/> Fibreglass <input type="checkbox"/> Plastic <input type="checkbox"/> Concrete <input type="checkbox"/> Galvanized Screen Outside diam 4 Slot No. 1.5 4.5 <input type="checkbox"/> Galvanized No Casing or Screen <input type="checkbox"/> Open hole				Test of Well Yield Pumping test method Draw Down Time min Water Level Metres Recovery Time min Water Level Metres Pump intake set at (metres) Static Level Pumping rate (litres/min) 1 1 Duration of pumping hrs + min 2 2 Final water level and of pumping metres 3 3 Recommended pump type <input type="checkbox"/> Shallow <input type="checkbox"/> Deep Recommended pump depth metres 4 4 Recommended pump rate (litres/min) 10 10 15 15 If flowing give rate (litres/min) 20 20 25 25 If pumping discontinued, give reason. 30 30 40 40 50 50 60 60			
Water Record Water found at Metres Kind of Water <input type="checkbox"/> m <input type="checkbox"/> Fresh <input type="checkbox"/> Sulphur <input type="checkbox"/> Gas <input type="checkbox"/> Salty <input type="checkbox"/> Minerals <input type="checkbox"/> Other: <input type="checkbox"/> m <input type="checkbox"/> Fresh <input type="checkbox"/> Sulphur <input type="checkbox"/> Gas <input type="checkbox"/> Salty <input type="checkbox"/> Minerals <input type="checkbox"/> Other: <input type="checkbox"/> m <input type="checkbox"/> Fresh <input type="checkbox"/> Sulphur <input type="checkbox"/> Gas <input type="checkbox"/> Salty <input type="checkbox"/> Minerals <input type="checkbox"/> Other: After test of well yield, water was <input type="checkbox"/> Clear and sediment free <input type="checkbox"/> Other, specify Chlorinated <input type="checkbox"/> Yes <input type="checkbox"/> No		Plugging and Sealing Record <input type="checkbox"/> Annular space <input type="checkbox"/> Abandonment Depth set at - Metres Material and type (benionite slurry, neat cement slurry) etc. Volume Placed (cubic metres) From To 0 1 Bentonite Pellets 100 Lts 1 4.5 #3 Well Sand 300 Lts				Location of Well In diagram below show distances of well from road, lot line, and building. Indicate north by arrow.  Gerric Rd			
Method of Construction <input type="checkbox"/> Cable Tool <input type="checkbox"/> Rotary (air) <input type="checkbox"/> Diamond <input type="checkbox"/> Digging <input checked="" type="checkbox"/> Rotary (conventional) <input type="checkbox"/> Air percussion <input type="checkbox"/> Jetting <input type="checkbox"/> Other <input type="checkbox"/> Rotary (reverse) <input type="checkbox"/> Boring <input type="checkbox"/> Drilling		Water Use <input type="checkbox"/> Domestic <input type="checkbox"/> Industrial <input type="checkbox"/> Public Supply <input type="checkbox"/> Other <input type="checkbox"/> Stock <input type="checkbox"/> Commercial <input type="checkbox"/> Not used <input type="checkbox"/> Irrigation <input type="checkbox"/> Municipal <input type="checkbox"/> Cooling & air conditioning				Final Status of Well <input type="checkbox"/> Water Supply <input type="checkbox"/> Recharge well <input type="checkbox"/> Unfinished <input type="checkbox"/> Abandoned, (Other) <input checked="" type="checkbox"/> Observation well <input type="checkbox"/> Abandoned, insufficient supply <input type="checkbox"/> Dewatering <input type="checkbox"/> Test Hole <input type="checkbox"/> Abandoned, poor quality <input type="checkbox"/> Replacement well			
Well Contractor/Technician Information Name of Well Contractor Andrew Dilling Inc Business Address (street name, number, city etc.) 25 Lewis Rd. Suelph ON N1H 1E9 Name of Well Technician (last name, first name) Henry Matheson Signature of Technician/Contractor [Signature]		Well Contractor's Licence No. 7738 Well Technician's Licence No. 7-3305 Date Submitted 7006 10 21 01				Ministry Use Only Data Source (Contractor) Date Received 7006 10 21 01 Date of Inspection 7006 10 21 01 Remarks Well Record Number			



APPENDIX B
GROUNDWATER ELEVATION MONITORING
CMT ENGINEERING INC.
OCTOBER 12, 2012



CMT Engineering Inc.
CONSULTING ENGINEERS
1011 Industrial Crescent, Unit 1
St. Clements, Ontario N0B 2M0
Tel: 519-699-5775
Fax: 519-699-4664
www.cmtinc.net



(411009)

July 4, 2013

06-004.L71

Gamsby and Mannerow Limited
650 Woodlawn Road West
Block C, Unit 1
Guelph, Ontario
N1K 1B8

Dear Sir:

**Re: Groundwater Monitoring
Ainley Subdivision
Elora, Ontario**

Attached is a current summary of the water level measurements for the above-referenced site. The graphs have been amended to include total monthly precipitation as recorded at the Environment Canada Fergus MOE Weather Station.

I trust this information meets with your present requirements. Should you have any questions, please do not hesitate to contact our office.

Yours very truly,

A handwritten signature in black ink, appearing to read 'Tim Salter'.

Tim Salter, C.E.T.

ks

Encl - Water Level Measurements

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Feb 8/06	Water Elevation (m) Feb 20/06	Water Elevation (m) Mar 9/06	Water Elevation (m) Mar 25/06	Water Elevation (m) Mar 29/06	Water Elevation (m) Apr 28/06	Water Elevation (m) June 6/06	Water Elevation (m) July 8/06	Water Elevation (m) Aug 7/06	Water Elevation (m) Sept 7/06
101	413.64	413.07	413.11	412.83	412.96	--	412.94	412.59	411.70	411.34	411.00
102	414.37	411.57	411.96	411.91	412.48	--	412.43	412.12	411.78	411.43	411.14
103	414.89	412.65	412.98	412.88	412.77	--	412.76	411.55	410.95	410.43	410.31
104	410.93	410.36	410.60	410.17	410.66	--	410.69	410.15	409.15	408.71	408.52
105	414.05	414.05	414.07 (F)	414.15 (F)	414.15 (F)	414.68	413.44	412.86	412.27	412.06	411.59
106	410.91	410.67	410.86 (F)	410.93 (F)	410.75	--	410.54	410.36	409.93	409.89	409.39
107	409.58	409.43	409.06 (F)	409.12 (F)	409.41	--	409.42	409.03	408.11	408.00	407.63
108	410.32	409.06	409.21	408.82	409.01	--	408.99	408.43	407.94	407.76	407.11

*(F) = Frozen

WATER LEVEL MEASUREMENTS**AINLEY SUBDIVISION
ELORA, ONTARIO**

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Oct 6/06	Water Elevation (m) Nov 11/06	Water Elevation (m) Dec 7/06	Water Elevation (m) Jan 9/07	Water Elevation (m) Feb 12/07	Water Elevation (m) Mar 8/07	Water Elevation (m) Apr 10/07	Water Elevation (m) May 12/07	Water Elevation (m) June 11/07	Water Elevation (m) July 11/07
101	413.64	410.83	412.67	412.97	413.03	412.11	411.61	413.02	412.75	411.87	411.42
102	414.37	411.00	411.25	411.71	411.99	411.69	411.45	412.14	411.96	411.67	411.38
103	414.89	410.36	411.10	411.91	412.27	411.05	410.66	412.50	411.59	410.97	410.54
104	410.93	408.71	409.13	409.45	409.65	409.12	408.96	409.78	409.39	409.24	408.92
105	414.05	411.95	413.94	413.71	413.90	412.95	412.65	413.72	413.29	412.51	412.11
106	410.91	410.13	410.74	410.57	410.59	410.28	410.22	410.55	410.50	410.07	409.59
107	409.58	408.28	408.94	409.11	409.16	408.60	408.43	409.25	409.01	408.24	407.71
108	410.32	407.62	408.21	408.48	408.57	408.07	407.93	408.69	408.36	407.96	407.57

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Aug 15/07	Water Elevation (m) Sept 13/07	Water Elevation (m) Oct 12/07	Water Elevation (m) Nov 8/07	Water Elevation (m) Dec 11/07	Water Elevation (m) Jan 15/08	Water Elevation (m) Feb 12/08	Water Elevation (m) Mar 8/08	Water Elevation (m) Apr 13/08	Water Elevation (m) May 8/08
101	413.64	411.01	410.72	410.50	410.35	410.33	412.73	412.92	413.00	413.19	413.05
102	414.37	411.13	410.98	410.87	410.81	410.77	411.26	411.53	411.80	412.92	412.46
103	414.89	410.34	410.36	410.36	410.36	410.36	411.29	411.74	412.03	413.40	412.50
104	410.93	408.73	408.61	408.57	408.58	408.76	409.70	409.66	409.81	410.70	410.35
105	414.05	411.67	411.34	411.22	411.22	411.22	414.00	414.11	414.10 frozen	413.99	413.71
106	410.91	409.22	408.99	408.89	408.89	409.44	410.63	410.61	410.64	410.77	410.69
107	409.58	407.49	407.40	407.43	407.52	407.91	409.20	409.05	409.10	409.52	409.35
108	410.32	407.36	407.37	407.37	407.37	407.52	408.52	408.50	408.57	409.19	408.89

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) June 10/08	Water Elevation (m) July 8/08	Water Elevation (m) Aug 1/08	Water Elevation (m) Sept 10/08	Water Elevation (m) Oct 8/08	Water Elevation (m) Nov 17/08	Water Elevation (m) Dec 17/08	Water Elevation (m) Jan 23/09	Water Elevation (m) Feb 20/09	Water Elevation (m) Mar 18/09
101	413.64	412.58	412.60	412.55	411.67	410.86	413.08	413.135	412.725	412.960	413.053
102	414.37	412.13	412.06	411.95	411.57	410.30	411.52	411.979	412.147	412.419	412.772
103	414.89	411.53	411.60	411.56	410.86	410.64	412.14	412.746	411.796	412.600	413.046
104	410.93	409.86	409.71	409.76	409.34	408.18	409.98	410.183	409.902	410.238	410.670
105	414.05	412.96	412.87	412.93	412.26	411.60	413.99	413.969	413.221	414.066 (F)	414.070 (F)
106	410.91	410.83	410.24	410.36	410.28	409.04	410.78	410.67	410.567	411.010 (F)	410.963 (F)
107	409.58	409.00	408.72	408.86	408.40	407.36	409.24	409.249	409.009	409.246	409.502
108	410.32	408.33	408.20	408.29	407.95	406.82	408.51	408.662	408.359	408.651	408.948

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Apr 21/09	Water Elevation (m) May 21/09	Water Elevation (m) June 26/09	Water Elevation (m) July 22/09	Water Elevation (m) Aug 27/09	Water Elevation (m) Sept 29/09	Water Elevation (m) Oct 30/09	Water Elevation (m) Dec 7/09	Water Elevation (m) Jan 5/10	Water Elevation (m) Feb 9/10
101	413.64	412.95	412.77	411.93	411.54	411.27	411.16	411.72	412.26	412.04	411.55
102	414.37	412.79	412.43	411.93	411.63	411.28	411.11	411.11	411.15	411.19	411.06
103	414.89	412.88	412.03	411.17	410.80	410.44	410.36	410.57	410.91	410.83	410.47
104	410.93	410.45	410.11	409.61	409.35	409.15	409.04	409.23	409.41	409.36	409.19
105	414.05	413.45	412.98	412.21	411.93	412.05	412.12	412.97	413.25	413.05	412.72
106	410.91	410.55	410.43	410.08	409.87	409.90	410.08	410.33	410.54	410.33	410.22
107	409.58	409.35	409.12	408.62	408.13	407.97	407.97	408.47	408.85	408.81	408.60
108	410.32	408.86	408.51	408.08	407.84	407.66	407.52	407.79	408.01	408.05	407.93

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Mar 2/10	Water Elevation (m) Apr 17/10	Water Elevation (m) May 11/10	Water Elevation (m) June 1/10	Water Elevation (m) June 29/10	Water Elevation (m) Aug 5/10	Water Elevation (m) Sept 22/10	Water Elevation (m) Oct 22/10	Water Elevation (m) Nov 9/10	Water Elevation (m) Dec 6/10
101	413.64	411.31	412.70	412.87	412.31	412.92	411.66	410.97	410.83	410.84	411.37
102	414.37	411.01	411.60	411.65	411.64	411.73	411.43	411.09	410.58	410.89	410.93
103	414.89	410.37	411.53	411.73	411.21	411.80	410.76	dry	dry	dry	dry
104	410.93	409.14	409.82	410.09	409.51	409.90	409.17	408.85	408.86	408.89	409.20
105	414.05	412.47	413.22	413.36	412.67	413.37	412.16	411.49	411.75	412.25	413.30
106	410.91	410.12	410.49	410.55	410.15	410.53	409.90	409.50	409.60	409.75	410.33
107	409.58	408.38	409.12	409.34	408.69	409.23	408.06	407.69	407.80	407.88	408.72
108	410.32	407.81	408.43	408.70	408.16	408.42	407.78	407.41	407.44	407.51	407.59

*(F) = Frozen

WATER LEVEL MEASUREMENTSAINLEY SUBDIVISION
ELORA, ONTARIO

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Jan 11/11	Water Elevation (m) Feb 19/11	Water Elevation (m) Mar 31/11	Water Elevation (m) July 19/11	Water Elevation (m) Sept 30/11	Water Elevation (m) Dec 7/11	Water Elevation (m) Feb 10/12	Water Elevation (m) Apr 4/12	Water Elevation (m) June 27/12	Water Elevation (m) Aug 1/12
101	413.64	412.24	412.62	413.09	412.33	411.51	413.21	412.91	412.67	411.22	410.84
102	414.37	411.10	411.09	dry	412.09	411.22	412.23	412.16	412.07	411.28	411.05
103	414.89	410.76	411.02	dry	411.42	410.49	413.25	412.28	411.73	410.45	dry
104	410.93	409.27	409.20	410.05	409.54	409.24	410.45	409.95	409.86	409.06	408.73
105	414.05	413.52	413.93	413.86	412.53	412.83	413.93	413.53	412.99	411.75	411.30
106	410.91	410.37	410.61	410.66	410.09	410.23	410.73	410.52	410.46	409.64	409.17
107	409.58	408.85	408.70	409.29	408.21	408.18	409.44	409.14	409.10	407.93	407.52
108	410.32	408.02	407.89	408.66	407.92	407.73	408.92	408.53	408.38	407.69	dry

*(F) = Frozen

WATER LEVEL MEASUREMENTS

**AINLEY SUBDIVISION
ELORA, ONTARIO**

Borehole No.	Ground Surface Elevation (m)	Water Elevation (m) Oct 11/12	Water Elevation (m) Dec 11/12	Water Elevation (m) Mar 9/13	Water Elevation (m) May 3/13	Water Elevation (m) July 3/13	Water Elevation (m)	Water Elevation (m)	Water Elevation (m)	Water Elevation (m)
101	413.64	410.48	412.25	412.19	412.94	412.74				
102	414.37	410.81	411.18	411.42	412.52	412.08				
103	414.89	dry	410.87	411.00	412.71	411.74				
104	410.93	408.69	409.36	408.32	410.21	409.94				
105	414.05	dry	413.48	413.12	413.38	412.89				
106	410.91	409.26	410.53	410.40	410.55	410.44				
107	409.58	407.61	408.91	408.84	409.32	409.21				
108	410.32	dry	418.10	408.09	408.74	408.50				

*(F) = Frozen



APPENDIX C
STORMWATER MANAGEMENT ANALYSIS

```

" MIDUSS Output ----->"
" MIDUSS version Version 2.25 rev. 473"
" MIDUSS created Sunday, February 07, 2010"
" 10 Units used: ie METRIC"
" Job folder: W:\Kitchener\411-2011\411009\Design Data\
" Modelling Files\2017-01-02\Existing\2017-03-07"
" Output filename: 411009_2YR EX_MARCH 2017.out"
" Licensee name: gmbp"
" Company Hewlett-Packard Company"
" Date & Time last used: 3/8/2017 at 8:17:24 AM"
31 TIME PARAMETERS"
" 5.000 Time Step"
" 180.000 Max. Storm length"
" 3600.000 Max. Hydrograph"
32 STORM Chicago storm"
" 1 Chicago storm"
" 695.050 Coefficient A"
" 6.387 Constant B"
" 0.793 Exponent C"
" 0.380 Fraction R"
" 180.000 Duration"
" 1.000 Time step multiplier"
" Maximum intensity 93.293 mm/hr"
" Total depth 33.014 mm"
" 6 002hyd Hydrograph extension used in this file"
33 CATCHMENT 30"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 30 Catchment 30"
" 0.000 % Impervious"
" 0.780 Total Area"
" 75.000 Flow length"
" 2.000 Overland Slope"
" 0.780 Pervious Area"
" 75.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 75.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.155 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.006 0.000 0.000 0.000 c.m/sec"
" Catchment 30 Pervious Impervious Total Area "
" Surface Area 0.780 0.000 0.780 hectare"
" Time of concentration 51.506 4.129 51.506 minutes"
" Time to Centroid 164.128 92.147 164.128 minutes"
" Rainfall depth 33.014 33.014 33.014 mm"
" Rainfall volume 257.51 0.00 257.51 c.m"
" Rainfall losses 27.894 5.340 27.894 mm"
" Runoff depth 5.120 27.674 5.120 mm"
" Runoff volume 39.93 0.00 39.93 c.m"
" Runoff coefficient 0.155 0.000 0.155 "
" Maximum flow 0.006 0.000 0.006 c.m/sec"
40 HYDROGRAPH Add Runoff "

```

```

"      4  Add Runoff "
"      0.006      0.006      0.000      0.000"
" 33    CATCHMENT 31"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"     31  Catchment 31"
"     0.000 % Impervious"
"     0.220 Total Area"
"    40.000 Flow length"
"     2.000 Overland Slope"
"     0.220 Pervious Area"
"    40.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    40.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.155 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.002      0.006      0.000      0.000 c.m/sec"
"      Catchment 31      Pervious      Impervious      Total Area "
"      Surface Area      0.220      0.000      0.220      hectare"
"      Time of concentration      35.323      2.832      35.323      minutes"
"      Time to Centroid      144.986      90.217      144.986      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      72.63      0.00      72.63      c.m"
"      Rainfall losses      27.897      5.467      27.897      mm"
"      Runoff depth      5.117      27.547      5.117      mm"
"      Runoff volume      11.26      0.00      11.26      c.m"
"      Runoff coefficient      0.155      0.000      0.155      "
"      Maximum flow      0.002      0.000      0.002      c.m/sec"
" 40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"           0.002      0.008      0.000      0.000"
" 40    HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"           0.002      0.008      0.008      0.000"
" 40    HYDROGRAPH Combine 2"
"      6  Combine "
"      2  Node #"
"      To Walser Street"
"      Maximum flow      0.008      c.m/sec"
"      Hydrograph volume      51.192      c.m"
"           0.002      0.008      0.008      0.008"
" 40    HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"           0.002      0.000      0.008      0.008"
" 33    CATCHMENT 10"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"     10  Catchment 10"
"     0.000 % Impervious"
"     7.760 Total Area"
"    150.000 Flow length"

```

"	2.000	Overland Slope"				
"	7.760	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.155	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.044	0.000	0.008	0.008 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	78.068	6.258	78.068	minutes"
"		Time to Centroid	195.540	95.197	195.540	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	2561.88	0.00	2561.88	c.m"
"		Rainfall losses	27.894	5.228	27.894	mm"
"		Runoff depth	5.120	27.786	5.120	mm"
"		Runoff volume	397.31	0.00	397.31	c.m"
"		Runoff coefficient	0.155	0.000	0.155	"
"		Maximum flow	0.044	0.000	0.044	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"		4 Add Runoff "				
"		0.044	0.044	0.008	0.008"	
"	33	CATCHMENT 11"				
"		1 Triangular SCS"				
"		1 Equal length"				
"		1 SCS method"				
"		11 Catchment 11"				
"		0.000 % Impervious"				
"		0.130 Total Area"				
"		40.000 Flow length"				
"		2.000 Overland Slope"				
"		0.130 Pervious Area"				
"		40.000 Pervious length"				
"		2.000 Pervious slope"				
"		0.000 Impervious Area"				
"		40.000 Impervious length"				
"		2.000 Impervious slope"				
"		0.250 Pervious Manning 'n'"				
"		74.000 Pervious SCS Curve No."				
"		0.155 Pervious Runoff coefficient"				
"		0.100 Pervious Ia/S coefficient"				
"		8.924 Pervious Initial abstraction"				
"		0.015 Impervious Manning 'n'"				
"		98.000 Impervious SCS Curve No."				
"		0.000 Impervious Runoff coefficient"				
"		0.100 Impervious Ia/S coefficient"				
"		0.518 Impervious Initial abstraction"				
"		0.001	0.044	0.008	0.008 c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"
"		Surface Area	0.130	0.000	0.130	hectare"
"		Time of concentration	35.323	2.832	35.323	minutes"
"		Time to Centroid	144.986	90.217	144.986	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"

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"		Rainfall volume	42.92	0.00	42.92	c.m"
"		Rainfall losses	27.897	5.467	27.897	mm"
"		Runoff depth	5.117	27.547	5.117	mm"
"		Runoff volume	6.65	0.00	6.65	c.m"
"		Runoff coefficient	0.155	0.000	0.155	"
"		Maximum flow	0.001	0.000	0.001	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"	"	4 Add Runoff "				
"	"		0.001	0.045	0.008	0.008"
"	33	CATCHMENT 40"				
"	"	1 Triangular SCS"				
"	"	1 Equal length"				
"	"	1 SCS method"				
"	"	40 Catchment 40"				
"	"	0.000 % Impervious"				
"	"	6.340 Total Area"				
"	"	60.000 Flow length"				
"	"	2.000 Overland Slope"				
"	"	6.340 Pervious Area"				
"	"	60.000 Pervious length"				
"	"	2.000 Pervious slope"				
"	"	0.000 Impervious Area"				
"	"	60.000 Impervious length"				
"	"	2.000 Impervious slope"				
"	"	0.250 Pervious Manning 'n'"				
"	"	74.000 Pervious SCS Curve No."				
"	"	0.155 Pervious Runoff coefficient"				
"	"	0.100 Pervious Ia/S coefficient"				
"	"	8.924 Pervious Initial abstraction"				
"	"	0.015 Impervious Manning 'n'"				
"	"	98.000 Impervious SCS Curve No."				
"	"	0.000 Impervious Runoff coefficient"				
"	"	0.100 Impervious Ia/S coefficient"				
"	"	0.518 Impervious Initial abstraction"				
"	"		0.054	0.045	0.008	0.008 c.m/sec"
"	"	Catchment 40	Pervious	Impervious	Total Area	"
"	"	Surface Area	6.340	0.000	6.340	hectare"
"	"	Time of concentration	45.052	3.611	45.051	minutes"
"	"	Time to Centroid	156.495	91.497	156.495	minutes"
"	"	Rainfall depth	33.014	33.014	33.014	mm"
"	"	Rainfall volume	2093.08	0.00	2093.09	c.m"
"	"	Rainfall losses	27.895	5.642	27.895	mm"
"	"	Runoff depth	5.119	27.372	5.119	mm"
"	"	Runoff volume	324.53	0.00	324.53	c.m"
"	"	Runoff coefficient	0.155	0.000	0.155	"
"	"	Maximum flow	0.054	0.000	0.054	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"	"	4 Add Runoff "				
"	"		0.054	0.093	0.008	0.008"
"	54	POND DESIGN"				
"	"	0.093 Current peak flow	c.m/sec"			
"	"	0.050 Target outflow	c.m/sec"			
"	"	728.5 Hydrograph volume	c.m"			
"	"	6. Number of stages"				
"	"	409.630 Minimum water level	metre"			
"	"	410.750 Maximum water level	metre"			
"	"	409.630 Starting water level	metre"			
"	"	0 Keep Design Data: 1 = True; 0 = False"				
"	"	Level Discharge	Volume"			
"	"	409.630	0.000	0.000"		
"	"	409.750	0.5400	232.250"		
"	"	410.000	1.632	2017.880"		
"	"	410.250	3.737	5148.940"		

```

"           410.500      1.345  9472.330"
"           410.750      1.885  15057.74"
"           Peak outflow                0.091    c.m/sec"
"           Maximum level                409.650  metre"
"           Maximum storage              39.152    c.m"
"           Centroidal lag              3.081    hours"
"           0.054      0.093      0.091      0.008 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.054      0.091      0.091      0.008"
" 52 CHANNEL DESIGN"
"     0.091 Current peak flow    c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     0.000 Basewidth    metre"
"     7.410 Left bank slope"
"     6.000 Right bank slope"
"     0.950 Channel depth    metre"
"     1.040 Gradient    %"
"           Depth of flow                0.159    metre"
"           Velocity                    0.535    m/sec"
"           Channel capacity            10.655    c.m/sec"
"           Critical depth              0.130    metre"
" 53 ROUTE Channel Route 72"
"     72.40 Channel Route 72 Reach length (metre)"
"     0.460 X-factor <= 0.5"
"    101.445 K-lag (seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 X-factor <= 0.5"
"    30.000 K-lag (seconds)"
"     0.500 Beta weighting factor"
"   100.000 Routing time step (seconds)"
"           1 No. of sub-reaches"
"           Peak outflow                0.091    c.m/sec"
"           0.054      0.091      0.091      0.008 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.054      0.091      0.091      0.008"
" 52 CHANNEL DESIGN"
"     0.091 Current peak flow    c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     2.000 Basewidth    metre"
"     2.950 Left bank slope"
"     3.000 Right bank slope"
"     0.950 Channel depth    metre"
"     1.040 Gradient    %"
"           Depth of flow                0.081    metre"
"           Velocity                    0.504    m/sec"
"           Channel capacity            9.246    c.m/sec"
"           Critical depth              0.058    metre"
" 53 ROUTE Channel Route 40"
"     39.80 Channel Route 40 Reach length (metre)"
"     0.443 X-factor <= 0.5"
"    59.188 K-lag (seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 X-factor <= 0.5"
"    30.000 K-lag (seconds)"
"     0.500 Beta weighting factor"
"   60.000 Routing time step (seconds)"
"           1 No. of sub-reaches"
"           Peak outflow                0.091    c.m/sec"
"           0.054      0.091      0.091      0.008 c.m/sec"

```

411009_2YR EX_MARCH 2017

```

" 40 HYDROGRAPH Next link "
" 5 Next link "
" 0.054 0.091 0.091 0.008"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.054 0.091 0.091 0.008"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.091 c.m/sec"
" Hydrograph volume 728.490 c.m"
" 0.054 0.091 0.091 0.091"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.054 0.000 0.091 0.091"
" 33 CATCHMENT 20"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 20 Catchment 20"
" 0.000 % Impervious"
" 6.650 Total Area"
" 150.000 Flow length"
" 2.000 Overland Slope"
" 6.650 Pervious Area"
" 150.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 150.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.155 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.038 0.000 0.091 0.091 c.m/sec"
" Catchment 20 Pervious Impervious Total Area "
" Surface Area 6.650 0.000 6.650 hectare"
" Time of concentration 78.068 6.258 78.068 minutes"
" Time to Centroid 195.540 95.197 195.539 minutes"
" Rainfall depth 33.014 33.014 33.014 mm"
" Rainfall volume 2195.43 0.00 2195.43 c.m"
" Rainfall losses 27.894 5.228 27.894 mm"
" Runoff depth 5.120 27.786 5.120 mm"
" Runoff volume 340.48 0.00 340.48 c.m"
" Runoff coefficient 0.155 0.000 0.155 "
" Maximum flow 0.038 0.000 0.038 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.038 0.038 0.091 0.091"
" 33 CATCHMENT 21"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 21 Catchment 20"
" 10.000 % Impervious"
" 0.820 Total Area"

```

```

" 40.000 Flow length"
" 2.000 Overland Slope"
" 0.738 Pervious Area"
" 40.000 Pervious length"
" 2.000 Pervious slope"
" 0.082 Impervious Area"
" 40.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.155 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.834 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.016 0.038 0.091 0.091 c.m/sec"
" Catchment 21 Pervious Impervious Total Area "
" Surface Area 0.738 0.082 0.820 hectare"
" Time of concentration 35.323 2.832 23.162 minutes"
" Time to Centroid 144.986 90.217 124.487 minutes"
" Rainfall depth 33.014 33.014 33.014 mm"
" Rainfall volume 243.64 27.07 270.71 c.m"
" Rainfall losses 27.897 5.467 25.654 mm"
" Runoff depth 5.117 27.547 7.360 mm"
" Runoff volume 37.76 22.59 60.35 c.m"
" Runoff coefficient 0.155 0.834 0.223 "
" Maximum flow 0.007 0.015 0.016 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.016 0.043 0.091 0.091"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.016 0.043 0.043 0.091"
" 64 SHOW TABLE"
" 2 Flow hydrograph"
" 4 Inflow Hydrograph"
" Maximum flow 0.043 c.m/sec"
" Hydrograph volume 400.831 c.m"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.134 c.m/sec"
" Hydrograph volume 1129.320 c.m"
" 0.016 0.043 0.043 0.134"
" 38 START/RE-START TOTALS 21"
" 3 Runoff Totals on EXIT"
" Total Catchment area 22.700 hectare"
" Total Impervious area 0.082 hectare"
" Total % impervious 0.361"
" 19 EXIT"

```



```

" MIDUSS Output ----->"
" MIDUSS version Version 2.25 rev. 473"
" MIDUSS created Sunday, February 07, 2010"
" 10 Units used: ie METRIC"
" Job folder: w:\Kitchener\411-2011\411009\Design Data\
" Modelling Files\2017-01-02\Existing\2017-03-07"
" Output filename: 411009_5YR EX_MARCH 2017.out"
" Licensee name: gmbp"
" Company Hewlett-Packard Company"
" Date & Time last used: 3/8/2017 at 8:22:34 AM"

```

```

" 31 TIME PARAMETERS"
" 5.000 Time Step"
" 180.000 Max. Storm length"
" 3600.000 Max. Hydrograph"
" 32 STORM Chicago storm"
" 1 Chicago storm"
" 1459.072 Coefficient A"
" 13.690 Constant B"
" 0.850 Exponent C"
" 0.380 Fraction R"
" 180.000 Duration"
" 1.000 Time step multiplier"

```

```

" Maximum intensity 113.586 mm/hr"
" Total depth 49.792 mm"
" 6 005hyd Hydrograph extension used in this file"

```

```

" 33 CATCHMENT 30"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 30 Catchment 30"
" 0.000 % Impervious"
" 0.780 Total Area"
" 75.000 Flow length"
" 2.000 overland Slope"
" 0.780 Pervious Area"
" 75.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 75.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.258 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"

```

	0.021	0.000	0.000	0.000 c.m/sec"	
Catchment 30	Pervious	Impervious	Total Area		"
Surface Area	0.780	0.000	0.780		hectare"
Time of concentration	36.283	3.738	36.283		minutes"
Time to Centroid	141.285	90.242	141.285		minutes"
Rainfall depth	49.792	49.792	49.792		mm"
Rainfall volume	388.37	0.00	388.38		c.m"
Rainfall losses	36.959	6.081	36.959		mm"
Runoff depth	12.833	43.710	12.833		mm"
Runoff volume	100.10	0.00	100.10		c.m"
Runoff coefficient	0.258	0.000	0.258		"
Maximum flow	0.021	0.000	0.021		c.m/sec"

```

" 40 HYDROGRAPH Add Runoff "

```

```

"      4  Add Runoff "
"      0.021      0.021      0.000      0.000"
" 33  CATCHMENT 31"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      31  Catchment 31"
"      0.000  % Impervious"
"      0.220  Total Area"
"     40.000  Flow length"
"      2.000  Overland slope"
"      0.220  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.258  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.007      0.021      0.000      0.000 c.m/sec"
"      Catchment 31      Pervious      Impervious      Total Area "
"      Surface Area      0.220      0.000      0.220      hectare"
"      Time of concentration 24.883      2.563      24.883      minutes"
"      Time to Centroid      128.082      88.517      128.082      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      109.54      0.00      109.54      c.m"
"      Rainfall losses      36.970      6.066      36.969      mm"
"      Runoff depth      12.822      43.726      12.822      mm"
"      Runoff volume      28.21      0.00      28.21      c.m"
"      Runoff coefficient      0.258      0.000      0.258      "
"      Maximum flow      0.007      0.000      0.007      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.007      0.028      0.000      0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.007      0.028      0.028      0.000"
" 40  HYDROGRAPH Combine 2"
"      6  Combine "
"      2  Node #"
"      To Walser Street"
"      Maximum flow      0.028      c.m/sec"
"      Hydrograph volume      128.306      c.m"
"      0.007      0.028      0.028      0.028"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.007      0.000      0.028      0.028"
" 33  CATCHMENT 10"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      10  Catchment 10"
"      0.000  % Impervious"
"      7.760  Total Area"
"     150.000  Flow length"

```

"	2.000	Overland Slope"				
"	7.760	Pervious Area"				
"	150.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.000	Impervious Area"				
"	150.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	74.000	Pervious SCS Curve No."				
"	0.258	Pervious Runoff coefficient"				
"	0.100	Pervious Ia/S coefficient"				
"	8.924	Pervious Initial abstraction"				
"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.000	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.157	0.000	0.028	0.028 c.m/sec"	
"		Catchment 10	Pervious	Impervious	Total Area	"
"		Surface Area	7.760	0.000	7.760	hectare"
"		Time of concentration	54.995	5.665	54.994	minutes"
"		Time to Centroid	162.955	92.780	162.955	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	3863.83	0.00	3863.84	c.m"
"		Rainfall losses	36.958	5.466	36.958	mm"
"		Runoff depth	12.834	44.325	12.834	mm"
"		Runoff volume	995.89	0.00	995.90	c.m"
"		Runoff coefficient	0.258	0.000	0.258	"
"		Maximum flow	0.157	0.000	0.157	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"		4 Add Runoff "				
"		0.157	0.157	0.028	0.028"	
"	33	CATCHMENT 11"				
"		1 Triangular SCS"				
"		1 Equal length"				
"		1 SCS method"				
"		11 Catchment 11"				
"		0.000 % Impervious"				
"		0.130 Total Area"				
"		40.000 Flow length"				
"		2.000 Overland Slope"				
"		0.130 Pervious Area"				
"		40.000 Pervious length"				
"		2.000 Pervious slope"				
"		0.000 Impervious Area"				
"		40.000 Impervious length"				
"		2.000 Impervious slope"				
"		0.250 Pervious Manning 'n'"				
"		74.000 Pervious SCS Curve No."				
"		0.258 Pervious Runoff coefficient"				
"		0.100 Pervious Ia/S coefficient"				
"		8.924 Pervious Initial abstraction"				
"		0.015 Impervious Manning 'n'"				
"		98.000 Impervious SCS Curve No."				
"		0.000 Impervious Runoff coefficient"				
"		0.100 Impervious Ia/S coefficient"				
"		0.518 Impervious Initial abstraction"				
"		0.004	0.157	0.028	0.028 c.m/sec"	
"		Catchment 11	Pervious	Impervious	Total Area	"
"		Surface Area	0.130	0.000	0.130	hectare"
"		Time of concentration	24.883	2.563	24.883	minutes"
"		Time to Centroid	128.082	88.517	128.082	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"

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"		Rainfall volume	64.73	0.00	64.73	c.m"
"		Rainfall losses	36.970	6.066	36.969	mm"
"		Runoff depth	12.822	43.726	12.822	mm"
"		Runoff volume	16.67	0.00	16.67	c.m"
"		Runoff coefficient	0.258	0.000	0.258	"
"		Maximum flow	0.004	0.000	0.004	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"	"	4 Add Runoff "				
"	"		0.004	0.159	0.028	0.028"
"	33	CATCHMENT 40"				
"	"	1 Triangular SCS"				
"	"	1 Equal length"				
"	"	1 SCS method"				
"	"	40 Catchment 40"				
"	"	0.000 % Impervious"				
"	"	6.340 Total Area"				
"	"	60.000 Flow length"				
"	"	2.000 overland Slope"				
"	"	6.340 Pervious Area"				
"	"	60.000 Pervious length"				
"	"	2.000 Pervious slope"				
"	"	0.000 Impervious Area"				
"	"	60.000 Impervious length"				
"	"	2.000 Impervious slope"				
"	"	0.250 Pervious Manning 'n'"				
"	"	74.000 Pervious SCS Curve No."				
"	"	0.258 Pervious Runoff coefficient"				
"	"	0.100 Pervious Ia/S coefficient"				
"	"	8.924 Pervious Initial abstraction"				
"	"	0.015 Impervious Manning 'n'"				
"	"	98.000 Impervious SCS Curve No."				
"	"	0.000 Impervious Runoff coefficient"				
"	"	0.100 Impervious Ia/S coefficient"				
"	"	0.518 Impervious Initial abstraction"				
"	"		0.186	0.159	0.028	0.028 c.m/sec"
"	"	Catchment 40	Pervious	Impervious	Total Area	"
"	"	Surface Area	6.340	0.000	6.340	hectare"
"	"	Time of concentration	31.736	3.269	31.736	minutes"
"	"	Time to Centroid	136.024	89.581	136.024	minutes"
"	"	Rainfall depth	49.792	49.792	49.792	mm"
"	"	Rainfall volume	3156.79	0.00	3156.79	c.m"
"	"	Rainfall losses	36.968	6.236	36.968	mm"
"	"	Runoff depth	12.824	43.556	12.824	mm"
"	"	Runoff volume	813.01	0.00	813.02	c.m"
"	"	Runoff coefficient	0.258	0.000	0.258	"
"	"	Maximum flow	0.186	0.000	0.186	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"	"	4 Add Runoff "				
"	"		0.186	0.331	0.028	0.028"
"	54	POND DESIGN"				
"	"	0.331 Current peak flow	c.m/sec"			
"	"	0.050 Target outflow	c.m/sec"			
"	"	1825.6 Hydrograph volume	c.m"			
"	"	6. Number of stages"				
"	"	409.630 Minimum water level	metre"			
"	"	410.750 Maximum water level	metre"			
"	"	409.630 Starting water level	metre"			
"	"	0 Keep Design Data: 1 = True; 0 = False"				
"	"	Level Discharge	Volume"			
"	"	409.630	0.000	0.000"		
"	"	409.750	0.5400	232.250"		
"	"	410.000	1.632	2017.880"		
"	"	410.250	3.737	5148.940"		

```

"           410.500   1.345  9472.330"
"           410.750   1.885  15057.74"
"           Peak outflow           0.315   c.m/sec"
"           Maximum level           409.700  metre"
"           Maximum storage          135.453  c.m"
"           Centroidal lag           2.630   hours"
"           0.186   0.331   0.315   0.028 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.186   0.315   0.315   0.028"
" 52 CHANNEL DESIGN"
"     0.315 Current peak flow   c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     0.000 Basewidth   metre"
"     7.410 Left bank slope"
"     6.000 Right bank slope"
"     0.950 Channel depth   metre"
"     1.040 Gradient   %"
"           Depth of flow           0.254   metre"
"           Velocity                 0.730   m/sec"
"           Channel capacity         10.655  c.m/sec"
"           Critical depth           0.214   metre"
" 53 ROUTE Channel Route 72"
"     72.40 Channel Route 72 Reach length (metre)"
"     0.437 x-factor <= 0.5"
"     74.373 K-lag (seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 x-factor <= 0.5"
"     30.000 K-lag (seconds)"
"     0.500 Beta weighting factor"
"     75.000 Routing time step (seconds)"
"     1 No. of sub-reaches"
"           Peak outflow           0.313   c.m/sec"
"           0.186   0.315   0.313   0.028 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.186   0.313   0.313   0.028"
" 52 CHANNEL DESIGN"
"     0.313 Current peak flow   c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     2.000 Basewidth   metre"
"     2.950 Left bank slope"
"     3.000 Right bank slope"
"     0.950 Channel depth   metre"
"     1.040 Gradient   %"
"           Depth of flow           0.164   metre"
"           Velocity                 0.766   m/sec"
"           Channel capacity         9.246   c.m/sec"
"           Critical depth           0.127   metre"
" 53 ROUTE Channel Route 40"
"     39.80 Channel Route 40 Reach length (metre)"
"     0.388 x-factor <= 0.5"
"     38.965 K-lag (seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 x-factor <= 0.5"
"     30.000 K-lag (seconds)"
"     0.500 Beta weighting factor"
"     42.857 Routing time step (seconds)"
"     1 No. of sub-reaches"
"           Peak outflow           0.313   c.m/sec"
"           0.186   0.313   0.313   0.028 c.m/sec"

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" 40 HYDROGRAPH Next link "
" 5 Next link "
" 0.186 0.313 0.313 0.028"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.186 0.313 0.313 0.028"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.313 c.m/sec"
" Hydrograph volume 1825.586 c.m"
" 0.186 0.313 0.313 0.313"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.186 0.000 0.313 0.313"
" 33 CATCHMENT 20"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 20 Catchment 20"
" 0.000 % Impervious"
" 6.650 Total Area"
" 150.000 Flow length"
" 2.000 Overland slope"
" 6.650 Pervious Area"
" 150.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 150.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.258 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.135 0.000 0.313 0.313 c.m/sec"
" Catchment 20 Pervious Impervious Total Area "
" Surface Area 6.650 0.000 6.650 hectare"
" Time of concentration 54.995 5.665 54.994 minutes"
" Time to Centroid 162.956 92.780 162.955 minutes"
" Rainfall depth 49.792 49.792 49.792 mm"
" Rainfall volume 3311.14 0.00 3311.15 c.m"
" Rainfall losses 36.958 5.466 36.958 mm"
" Runoff depth 12.834 44.325 12.834 mm"
" Runoff volume 853.44 0.00 853.44 c.m"
" Runoff coefficient 0.258 0.000 0.258 "
" Maximum flow 0.135 0.000 0.135 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.135 0.135 0.313 0.313"
" 33 CATCHMENT 21"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 21 Catchment 20"
" 10.000 % Impervious"
" 0.820 Total Area"

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" 40.000 Flow length"
" 2.000 Overland Slope"
" 0.738 Pervious Area"
" 40.000 Pervious length"
" 2.000 Pervious slope"
" 0.082 Impervious Area"
" 40.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.258 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.878 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.030 0.135 0.313 0.313 c.m/sec"
" Catchment 21 Pervious Impervious Total Area "
" Surface Area 0.738 0.082 0.820 hectare"
" Time of concentration 24.883 2.563 18.750 minutes"
" Time to Centroid 128.082 88.517 117.210 minutes"
" Rainfall depth 49.792 49.792 49.792 mm"
" Rainfall volume 367.46 40.83 408.29 c.m"
" Rainfall losses 36.970 6.066 33.879 mm"
" Runoff depth 12.822 43.726 15.913 mm"
" Runoff volume 94.63 35.86 130.48 c.m"
" Runoff coefficient 0.258 0.878 0.320 "
" Maximum flow 0.025 0.021 0.030 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.030 0.152 0.313 0.313"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.030 0.152 0.152 0.313"
" 64 SHOW TABLE"
" 2 Flow hydrograph"
" 4 Inflow Hydrograph"
" Maximum flow 0.152 c.m/sec"
" Hydrograph volume 983.926 c.m"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.463 c.m/sec"
" Hydrograph volume 2809.509 c.m"
" 0.030 0.152 0.152 0.463"
" 38 START/RE-START TOTALS 21"
" 3 Runoff Totals on EXIT"
" Total Catchment area 22.700 hectare"
" Total Impervious area 0.082 hectare"
" Total % impervious 0.361"
" 19 EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          W:\Kitchener\411-2011\411009\Design Data\
"          Modelling Files\2017-01-02\Existing\2017-03-07"
"          Output filename:                    411009_10YR EX_MARCH 2017.out"
"          Licensee name:                      gmbp"
"          Company                             Hewlett-Packard Company"
"          Date & Time last used:              3/8/2017 at 8:25:44 AM"
31  TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
32  STORM Chicago storm"
"          1  Chicago storm"
"          2327.596 Coefficient A"
"          19.500  Constant B"
"          0.894  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    126.171  mm/hr"
"          Total depth                          61.359  mm"
"          6  010hyd Hydrograph extension used in this file"
33  CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.780  Total Area"
"          75.000  Flow length"
"          2.000  Overland Slope"
"          0.780  Pervious Area"
"          75.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          75.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.316  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.036  0.000  0.000  0.000 c.m/sec"
"          Catchment 30  Pervious  Impervious  Total Area  "
"          Surface Area  0.780  0.000  0.780  hectare"
"          Time of concentration  31.343  3.560  31.343  minutes"
"          Time to Centroid  133.488  89.347  133.488  minutes"
"          Rainfall depth  61.359  61.359  61.359  mm"
"          Rainfall volume  478.60  0.00  478.60  c.m"
"          Rainfall losses  41.966  6.521  41.966  mm"
"          Runoff depth  19.393  54.839  19.393  mm"
"          Runoff volume  151.27  0.00  151.27  c.m"
"          Runoff coefficient  0.316  0.000  0.316  "
"          Maximum flow  0.036  0.000  0.036  c.m/sec"
40  HYDROGRAPH Add Runoff "

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"      4  Add Runoff "
"      0.036      0.036      0.000      0.000"
" 33    CATCHMENT 31"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      31  Catchment 31"
"      0.000  % Impervious"
"      0.220  Total Area"
"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.220  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.316  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.013      0.036      0.000      0.000 c.m/sec"
"      Catchment 31      Pervious      Impervious      Total Area "
"      Surface Area      0.220      0.000      0.220      hectare"
"      Time of concentration      21.495      2.441      21.495      minutes"
"      Time to Centroid      122.241      87.742      122.241      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      134.99      0.00      134.99      c.m"
"      Rainfall losses      41.967      6.310      41.967      mm"
"      Runoff depth      19.392      55.050      19.393      mm"
"      Runoff volume      42.66      0.00      42.66      c.m"
"      Runoff coefficient      0.316      0.000      0.316      "
"      Maximum flow      0.013      0.000      0.013      c.m/sec"
" 40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.013      0.048      0.000      0.000"
" 40    HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.013      0.048      0.048      0.000"
" 40    HYDROGRAPH Combine 2"
"      6  Combine "
"      2  Node #"
"      To Walser Street"
"      Maximum flow      0.048      c.m/sec"
"      Hydrograph volume      193.932      c.m"
"      0.013      0.048      0.048      0.048"
" 40    HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.013      0.000      0.048      0.048"
" 33    CATCHMENT 10"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      10  Catchment 10"
"      0.000  % Impervious"
"      7.760  Total Area"
"      150.000  Flow length"

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"      2.000  Overland Slope"
"      7.760  Pervious Area"
"     150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.316  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.273      0.000      0.048      0.048 c.m/sec"
"      Catchment 10      Pervious      Impervious      Total Area  "
"      Surface Area      7.760      0.000      7.760      hectare"
"      Time of concentration  47.507      5.395      47.507      minutes"
"      Time to Centroid      151.963      91.698      151.963      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      4761.47      0.00      4761.48      c.m"
"      Rainfall losses      41.963      5.633      41.963      mm"
"      Runoff depth      19.396      55.726      19.396      mm"
"      Runoff volume      1505.11      0.00      1505.12      c.m"
"      Runoff coefficient      0.316      0.000      0.316      "
"      Maximum flow      0.273      0.000      0.273      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.273      0.273      0.048      0.048"
" 33  CATCHMENT 11"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"     11  Catchment 11"
"      0.000  % Impervious"
"      0.130  Total Area"
"     40.000  Flow length"
"      2.000  Overland Slope"
"      0.130  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.316  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.008      0.273      0.048      0.048 c.m/sec"
"      Catchment 11      Pervious      Impervious      Total Area  "
"      Surface Area      0.130      0.000      0.130      hectare"
"      Time of concentration  21.495      2.441      21.495      minutes"
"      Time to Centroid      122.241      87.742      122.240      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"

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"		Rainfall volume	79.77	0.00	79.77	c.m"
"		Rainfall losses	41.967	6.310	41.967	mm"
"		Runoff depth	19.392	55.050	19.393	mm"
"		Runoff volume	25.21	0.00	25.21	c.m"
"		Runoff coefficient	0.316	0.000	0.316	"
"		Maximum flow	0.008	0.000	0.008	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"		4 Add Runoff "				
"			0.008	0.277	0.048	0.048"
"	33	CATCHMENT 40"				
"		1 Triangular SCS"				
"		1 Equal length"				
"		1 SCS method"				
"		40 Catchment 40"				
"		0.000 % Impervious"				
"		6.340 Total Area"				
"		60.000 Flow length"				
"		2.000 Overland Slope"				
"		6.340 Pervious Area"				
"		60.000 Pervious length"				
"		2.000 Pervious slope"				
"		0.000 Impervious Area"				
"		60.000 Impervious length"				
"		2.000 Impervious slope"				
"		0.250 Pervious Manning 'n'"				
"		74.000 Pervious SCS Curve No."				
"		0.316 Pervious Runoff coefficient"				
"		0.100 Pervious Ia/S coefficient"				
"		8.924 Pervious Initial abstraction"				
"		0.015 Impervious Manning 'n'"				
"		98.000 Impervious SCS Curve No."				
"		0.000 Impervious Runoff coefficient"				
"		0.100 Impervious Ia/S coefficient"				
"		0.518 Impervious Initial abstraction"				
"			0.320	0.277	0.048	0.048 c.m/sec"
"		Catchment 40	Pervious	Impervious	Total Area	"
"		Surface Area	6.340	0.000	6.340	hectare"
"		Time of concentration	27.416	3.114	27.416	minutes"
"		Time to Centroid	128.990	88.727	128.990	minutes"
"		Rainfall depth	61.359	61.359	61.359	mm"
"		Rainfall volume	3890.17	0.00	3890.18	c.m"
"		Rainfall losses	41.968	6.469	41.968	mm"
"		Runoff depth	19.392	54.890	19.392	mm"
"		Runoff volume	1229.42	0.00	1229.42	c.m"
"		Runoff coefficient	0.316	0.000	0.316	"
"		Maximum flow	0.320	0.000	0.320	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"		4 Add Runoff "				
"			0.320	0.571	0.048	0.048"
"	54	POND DESIGN"				
"		0.571 Current peak flow	c.m/sec"			
"		0.050 Target outflow	c.m/sec"			
"		2759.8 Hydrograph volume	c.m"			
"		6. Number of stages"				
"		409.630 Minimum water level	metre"			
"		410.750 Maximum water level	metre"			
"		409.630 Starting water level	metre"			
"		0 Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge	Volume"			
"		409.630	0.000	0.000"		
"		409.750	0.5400	232.250"		
"		410.000	1.632	2017.880"		
"		410.250	3.737	5148.940"		

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"           410.500   1.345  9472.330"
"           410.750   1.885  15057.74"
"           Peak outflow           0.536   c.m/sec"
"           Maximum level           409.749   metre"
"           Maximum storage           231.241   c.m"
"           Centroidal lag           2.477   hours"
"           0.320   0.571   0.536   0.048 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.320   0.536   0.536   0.048"
" 52 CHANNEL DESIGN"
"     0.536 Current peak flow   c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     0.000 Basewidth   metre"
"     7.410 Left bank slope"
"     6.000 Right bank slope"
"     0.950 Channel depth   metre"
"     1.040 Gradient   %"
"           Depth of flow           0.310   metre"
"           Velocity           0.834   m/sec"
"           Channel capacity           10.655   c.m/sec"
"           Critical depth           0.265   metre"
" 53 ROUTE Channel Route 72"
"     72.40 Channel Route 72 Reach length ( metre)"
"     0.423 x-factor <= 0.5"
"     65.118 K-lag ( seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 x-factor <= 0.5"
"     30.000 K-lag ( seconds)"
"     0.500 Beta weighting factor"
"     75.000 Routing time step ( seconds)"
"     1 No. of sub-reaches"
"           Peak outflow           0.534   c.m/sec"
"           0.320   0.536   0.534   0.048 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.320   0.534   0.534   0.048"
" 52 CHANNEL DESIGN"
"     0.534 Current peak flow   c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     2.000 Basewidth   metre"
"     2.950 Left bank slope"
"     3.000 Right bank slope"
"     0.950 Channel depth   metre"
"     1.040 Gradient   %"
"           Depth of flow           0.221   metre"
"           Velocity           0.907   m/sec"
"           Channel capacity           9.246   c.m/sec"
"           Critical depth           0.177   metre"
" 53 ROUTE Channel Route 40"
"     39.80 Channel Route 40 Reach length ( metre)"
"     0.352 x-factor <= 0.5"
"     32.911 K-lag ( seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 x-factor <= 0.5"
"     30.000 K-lag ( seconds)"
"     0.500 Beta weighting factor"
"     37.500 Routing time step ( seconds)"
"     1 No. of sub-reaches"
"           Peak outflow           0.534   c.m/sec"
"           0.320   0.534   0.534   0.048 c.m/sec"

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" 40 HYDROGRAPH Next link "
" 5 Next link "
" 0.320 0.534 0.534 0.048"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.320 0.534 0.534 0.048"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.534 c.m/sec"
" Hydrograph volume 2759.753 c.m"
" 0.320 0.534 0.534 0.534"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.320 0.000 0.534 0.534"
" 33 CATCHMENT 20"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 20 Catchment 20"
" 0.000 % Impervious"
" 6.650 Total Area"
" 150.000 Flow length"
" 2.000 Overland slope"
" 6.650 Pervious Area"
" 150.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 150.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.316 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.234 0.000 0.534 0.534 c.m/sec"
" Catchment 20 Pervious Impervious Total Area "
" Surface Area 6.650 0.000 6.650 hectare"
" Time of concentration 47.507 5.395 47.507 minutes"
" Time to Centroid 151.963 91.698 151.963 minutes"
" Rainfall depth 61.359 61.359 61.359 mm"
" Rainfall volume 4080.39 0.00 4080.39 c.m"
" Rainfall losses 41.963 5.633 41.963 mm"
" Runoff depth 19.396 55.726 19.396 mm"
" Runoff volume 1289.82 0.00 1289.82 c.m"
" Runoff coefficient 0.316 0.000 0.316 "
" Maximum flow 0.234 0.000 0.234 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.234 0.234 0.534 0.534"
" 33 CATCHMENT 21"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 21 Catchment 20"
" 10.000 % Impervious"
" 0.820 Total Area"

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"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.738  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      0.082  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.316  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.897  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.049      0.234      0.534      0.534 c.m/sec"
"      Catchment 21      Pervious      Impervious      Total Area      "
"      Surface Area      0.738      0.082      0.820      hectare"
"      Time of concentration      21.495      2.441      16.926      minutes"
"      Time to Centroid      122.241      87.742      113.968      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      452.83      50.31      503.15      c.m"
"      Rainfall losses      41.967      6.310      38.401      mm"
"      Runoff depth      19.392      55.050      22.958      mm"
"      Runoff volume      143.12      45.14      188.26      c.m"
"      Runoff coefficient      0.316      0.897      0.374      "
"      Maximum flow      0.043      0.024      0.049      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.049      0.263      0.534      0.534"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.049      0.263      0.263      0.534"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow      0.263      c.m/sec"
"      Hydrograph volume      1478.082      c.m"
" 40      HYDROGRAPH Combine      1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow      0.794      c.m/sec"
"      Hydrograph volume      4237.834      c.m"
"          0.049      0.263      0.263      0.794"
" 38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      22.700      hectare"
"      Total Impervious area      0.082      hectare"
"      Total % impervious      0.361"
" 19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          w:\Kitchener\411-2011\411009\Design Data\
"          Modelling Files\2017-01-02\Existing\2017-03-07"
"          Output filename:                    411009_25YR EX_MARCH 2017.out"
"          Licensee name:                      gmbp"
"          Company                            Hewlett-Packard Company"
"          Date & Time last used:              3/8/2017 at 8:31:55 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1      Chicago storm"
"          3701.648 Coefficient A"
"          25.500  Constant B"
"          0.937  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    143.371  mm/hr"
"          Total depth                          75.581  mm"
"          6  025hyd Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          30     Catchment 30"
"          0.000  % Impervious"
"          0.780  Total Area"
"          75.000 Flow length"
"          2.000  Overland slope"
"          0.780  Pervious Area"
"          75.000 Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          75.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.377  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.059  0.000  0.000  0.000 c.m/sec"
"          Catchment 30      Pervious      Impervious      Total Area  "
"          Surface Area      0.780      0.000      0.780      hectare"
"          Time of concentration 27.338      3.366      27.338      minutes"
"          Time to Centroid    127.236      88.558      127.236      minutes"
"          Rainfall depth      75.581      75.581      75.581      mm"
"          Rainfall volume     589.53      0.00      589.53      c.m"
"          Rainfall losses     47.096      7.005      47.096      mm"
"          Runoff depth        28.485      68.576      28.485      mm"
"          Runoff volume       222.18      0.00      222.18      c.m"
"          Runoff coefficient   0.377      0.000      0.377      "
"          Maximum flow        0.059      0.000      0.059      c.m/sec"
" 40      HYDROGRAPH Add Runoff "

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"      4  Add Runoff "
"      0.059      0.059      0.000      0.000"
" 33  CATCHMENT 31"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      31  Catchment 31"
"      0.000  % Impervious"
"      0.220  Total Area"
"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.220  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.376  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.020      0.059      0.000      0.000 c.m/sec"
"      Catchment 31      Pervious      Impervious      Total Area "
"      Surface Area      0.220      0.000      0.220      hectare"
"      Time of concentration      18.749      2.308      18.749      minutes"
"      Time to Centroid      117.510      87.059      117.510      minutes"
"      Rainfall depth      75.581      75.581      75.581      mm"
"      Rainfall volume      166.28      0.00      166.28      c.m"
"      Rainfall losses      47.127      6.593      47.127      mm"
"      Runoff depth      28.453      68.988      28.453      mm"
"      Runoff volume      62.60      0.00      62.60      c.m"
"      Runoff coefficient      0.376      0.000      0.376      "
"      Maximum flow      0.020      0.000      0.020      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.020      0.078      0.000      0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.020      0.078      0.078      0.000"
" 40  HYDROGRAPH Combine 2"
"      6  Combine "
"      2  Node #"
"      To Walser Street"
"      Maximum flow      0.078      c.m/sec"
"      Hydrograph volume      284.778      c.m"
"      0.020      0.078      0.078      0.078"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.020      0.000      0.078      0.078"
" 33  CATCHMENT 10"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      10  Catchment 10"
"      0.000  % Impervious"
"      7.760  Total Area"
"      150.000  Flow length"

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"      2.000  Overland Slope"
"      7.760  Pervious Area"
"     150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.377  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.454  0.000  0.078  0.078 c.m/sec"
"      Catchment 10  Pervious  Impervious  Total Area  "
"      Surface Area  7.760  0.000  7.760  hectare"
"      Time of concentration  41.437  5.102  41.437  minutes"
"      Time to Centroid  143.191  90.751  143.190  minutes"
"      Rainfall depth  75.581  75.581  75.581  mm"
"      Rainfall volume  5865.07  0.01  5865.07  c.m"
"      Rainfall losses  47.093  5.908  47.093  mm"
"      Runoff depth  28.488  69.673  28.488  mm"
"      Runoff volume  2210.64  0.01  2210.65  c.m"
"      Runoff coefficient  0.377  0.000  0.377  "
"      Maximum flow  0.454  0.000  0.454  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.454  0.454  0.078  0.078"
" 33  CATCHMENT 11"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"     11  Catchment 11"
"      0.000  % Impervious"
"      0.130  Total Area"
"     40.000  Flow length"
"      2.000  Overland Slope"
"      0.130  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.376  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.012  0.454  0.078  0.078 c.m/sec"
"      Catchment 11  Pervious  Impervious  Total Area  "
"      Surface Area  0.130  0.000  0.130  hectare"
"      Time of concentration  18.749  2.308  18.749  minutes"
"      Time to Centroid  117.510  87.059  117.510  minutes"
"      Rainfall depth  75.581  75.581  75.581  mm"

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"		Rainfall volume	98.25	0.00	98.26	c.m"
"		Rainfall losses	47.127	6.593	47.127	mm"
"		Runoff depth	28.453	68.988	28.453	mm"
"		Runoff volume	36.99	0.00	36.99	c.m"
"		Runoff coefficient	0.376	0.000	0.376	"
"		Maximum flow	0.012	0.000	0.012	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"	"	4 Add Runoff "				
"	"		0.012	0.461	0.078	0.078"
"	33	CATCHMENT 40"				
"	"	1 Triangular SCS"				
"	"	1 Equal length"				
"	"	1 SCS method"				
"	"	40 Catchment 40"				
"	"	0.000 % Impervious"				
"	"	6.340 Total Area"				
"	"	60.000 Flow length"				
"	"	2.000 Overland Slope"				
"	"	6.340 Pervious Area"				
"	"	60.000 Pervious length"				
"	"	2.000 Pervious slope"				
"	"	0.000 Impervious Area"				
"	"	60.000 Impervious length"				
"	"	2.000 Impervious slope"				
"	"	0.250 Pervious Manning 'n'"				
"	"	74.000 Pervious SCS Curve No."				
"	"	0.377 Pervious Runoff coefficient"				
"	"	0.100 Pervious Ia/S coefficient"				
"	"	8.924 Pervious Initial abstraction"				
"	"	0.015 Impervious Manning 'n'"				
"	"	98.000 Impervious SCS Curve No."				
"	"	0.000 Impervious Runoff coefficient"				
"	"	0.100 Impervious Ia/S coefficient"				
"	"	0.518 Impervious Initial abstraction"				
"	"		0.520	0.461	0.078	0.078 c.m/sec"
"	"	Catchment 40	Pervious	Impervious	Total Area	"
"	"	Surface Area	6.340	0.000	6.340	hectare"
"	"	Time of concentration	23.913	2.944	23.913	minutes"
"	"	Time to Centroid	123.357	87.974	123.357	minutes"
"	"	Rainfall depth	75.581	75.581	75.581	mm"
"	"	Rainfall volume	4791.82	0.00	4791.83	c.m"
"	"	Rainfall losses	47.108	6.942	47.107	mm"
"	"	Runoff depth	28.473	68.639	28.473	mm"
"	"	Runoff volume	1805.21	0.00	1805.21	c.m"
"	"	Runoff coefficient	0.377	0.000	0.377	"
"	"	Maximum flow	0.520	0.000	0.520	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"	"	4 Add Runoff "				
"	"		0.520	0.940	0.078	0.078"
"	54	POND DESIGN"				
"	"	0.940 Current peak flow	c.m/sec"			
"	"	0.050 Target outflow	c.m/sec"			
"	"	4052.8 Hydrograph volume	c.m"			
"	"	6. Number of stages"				
"	"	409.630 Minimum water level	metre"			
"	"	410.750 Maximum water level	metre"			
"	"	409.630 Starting water level	metre"			
"	"	0 Keep Design Data: 1 = True; 0 = False"				
"	"	Level Discharge	Volume"			
"	"	409.630	0.000	0.000"		
"	"	409.750	0.5400	232.250"		
"	"	410.000	1.632	2017.880"		
"	"	410.250	3.737	5148.940"		

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"           410.500   1.345  9472.330"
"           410.750   1.885  15057.74"
"           Peak outflow           0.736   c.m/sec"
"           Maximum level           409.795  metre"
"           Maximum storage         553.883   c.m"
"           Centroidal lag           2.393   hours"
"           0.520   0.940   0.736   0.078 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.520   0.736   0.736   0.078"
" 52 CHANNEL DESIGN"
"     0.736 Current peak flow   c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     0.000 Basewidth   metre"
"     7.410 Left bank slope"
"     6.000 Right bank slope"
"     0.950 Channel depth   metre"
"     1.040 Gradient   %"
"           Depth of flow           0.349   metre"
"           Velocity                 0.903   m/sec"
"           Channel capacity         10.655  c.m/sec"
"           Critical depth           0.301   metre"
" 53 ROUTE Channel Route 72"
"     72.40 Channel Route 72 Reach length ( metre)"
"     0.413 x-factor <= 0.5"
"     60.155 K-lag ( seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 x-factor <= 0.5"
"     30.000 K-lag ( seconds)"
"     0.500 Beta weighting factor"
"     60.000 Routing time step ( seconds)"
"     1 No. of sub-reaches"
"           Peak outflow           0.735   c.m/sec"
"           0.520   0.736   0.735   0.078 c.m/sec"
" 40 HYDROGRAPH Next link "
"     5 Next link "
"           0.520   0.735   0.735   0.078"
" 52 CHANNEL DESIGN"
"     0.735 Current peak flow   c.m/sec"
"     0.035 Manning 'n'"
"     0. Cross-section type: 0=trapezoidal; 1=general"
"     2.000 Basewidth   metre"
"     2.950 Left bank slope"
"     3.000 Right bank slope"
"     0.950 Channel depth   metre"
"     1.040 Gradient   %"
"           Depth of flow           0.264   metre"
"           Velocity                 1.000   m/sec"
"           Channel capacity         9.246   c.m/sec"
"           Critical depth           0.214   metre"
" 53 ROUTE Channel Route 40"
"     39.80 Channel Route 40 Reach length ( metre)"
"     0.326 x-factor <= 0.5"
"     29.854 K-lag ( seconds)"
"     0.000 Default(0) or user spec.(1) values used"
"     0.500 x-factor <= 0.5"
"     30.000 K-lag ( seconds)"
"     0.500 Beta weighting factor"
"     37.500 Routing time step ( seconds)"
"     1 No. of sub-reaches"
"           Peak outflow           0.735   c.m/sec"
"           0.520   0.735   0.735   0.078 c.m/sec"

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" 40 HYDROGRAPH Next link "
" 5 Next link "
" 0.520 0.735 0.735 0.078"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.520 0.735 0.735 0.078"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.735 c.m/sec"
" Hydrograph volume 4053.139 c.m"
" 0.520 0.735 0.735 0.735"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.520 0.000 0.735 0.735"
" 33 CATCHMENT 20"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 20 Catchment 20"
" 0.000 % Impervious"
" 6.650 Total Area"
" 150.000 Flow length"
" 2.000 Overland Slope"
" 6.650 Pervious Area"
" 150.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 150.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.377 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.389 0.000 0.735 0.735 c.m/sec"
" Catchment 20 Pervious Impervious Total Area "
" Surface Area 6.650 0.000 6.650 hectare"
" Time of concentration 41.437 5.102 41.437 minutes"
" Time to Centroid 143.191 90.751 143.190 minutes"
" Rainfall depth 75.581 75.581 75.581 mm"
" Rainfall volume 5026.12 0.01 5026.13 c.m"
" Rainfall losses 47.093 5.908 47.093 mm"
" Runoff depth 28.488 69.673 28.488 mm"
" Runoff volume 1894.43 0.00 1894.44 c.m"
" Runoff coefficient 0.377 0.000 0.377 "
" Maximum flow 0.389 0.000 0.389 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.389 0.389 0.735 0.735"
" 33 CATCHMENT 21"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 21 Catchment 20"
" 10.000 % Impervious"
" 0.820 Total Area"

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"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.738  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      0.082  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.376  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.913  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.078    0.389    0.735    0.735 c.m/sec"
"      Catchment 21      Pervious      Impervious      Total Area      "
"      Surface Area      0.738      0.082      0.820      hectare"
"      Time of concentration 18.749      2.308      15.260      minutes"
"      Time to Centroid 117.510      87.059      111.048      minutes"
"      Rainfall depth 75.581      75.581      75.581      mm"
"      Rainfall volume 557.79      61.98      619.76      c.m"
"      Rainfall losses 47.127      6.593      43.074      mm"
"      Runoff depth 28.453      68.988      32.507      mm"
"      Runoff volume 209.99      56.57      266.56      c.m"
"      Runoff coefficient 0.376      0.913      0.430      "
"      Maximum flow 0.068      0.028      0.078      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.078    0.435    0.735    0.735"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"          0.078    0.435    0.435    0.735"
" 64 SHOW TABLE"
"      2 Flow hydrograph"
"      4 Inflow Hydrograph"
"          Maximum flow          0.435      c.m/sec"
"          Hydrograph volume 2160.991      c.m"
" 40 HYDROGRAPH Combine 1"
"      6 Combine "
"      1 Node #"
"          Total"
"          Maximum flow          1.148      c.m/sec"
"          Hydrograph volume 6214.133      c.m"
"          0.078    0.435    0.435    1.148"
" 38 START/RE-START TOTALS 21"
"      3 Runoff Totals on EXIT"
"          Total Catchment area          22.700      hectare"
"          Total Impervious area          0.082      hectare"
"          Total % impervious          0.361"
" 19 EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        w:\Kitchener\411-2011\411009\Design Data\
"          Modelling Files\2017-01-02\Existing\2017-03-07"
"          Output filename:                   411009_50YR_EX_MARCH 2017.out"
"          Licensee name:                     gmbp"
"          Company                           Hewlett-Packard Company"
"          Date & Time last used:            3/8/2017 at 8:34:38 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          5089.418 Coefficient A"
"          30.000  Constant B"
"          0.967  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                156.350  mm/hr"
"          Total depth                      86.737  mm"
"          6  050hyd Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.780  Total Area"
"          75.000  Flow length"
"          2.000  Overland Slope"
"          0.780  Pervious Area"
"          75.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          75.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.418  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.080  0.000  0.000  0.000 c.m/sec"
"          Catchment 30  Pervious  Impervious  Total Area  "
"          Surface Area  0.780  0.000  0.780  hectare"
"          Time of concentration  25.140  3.243  25.140  minutes"
"          Time to Centroid  123.790  88.110  123.790  minutes"
"          Rainfall depth  86.737  86.737  86.737  mm"
"          Rainfall volume  676.55  0.00  676.55  c.m"
"          Rainfall losses  50.521  7.225  50.521  mm"
"          Runoff depth  36.216  79.512  36.216  mm"
"          Runoff volume  282.49  0.00  282.49  c.m"
"          Runoff coefficient  0.418  0.000  0.418  "
"          Maximum flow  0.080  0.000  0.080  c.m/sec"
" 40      HYDROGRAPH Add Runoff "

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"      4  Add Runoff "
"      0.080      0.080      0.000      0.000"
" 33  CATCHMENT 31"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      31  Catchment 31"
"      0.000  % Impervious"
"      0.220  Total Area"
"     40.000  Flow length"
"      2.000  Overland Slope"
"      0.220  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.417  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.027      0.080      0.000      0.000 c.m/sec"
"      Catchment 31      Pervious      Impervious      Total Area "
"      Surface Area      0.220      0.000      0.220      hectare"
"      Time of concentration      17.241      2.224      17.241      minutes"
"      Time to Centroid      114.897      86.667      114.897      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      190.82      0.00      190.82      c.m"
"      Rainfall losses      50.540      6.773      50.540      mm"
"      Runoff depth      36.197      79.963      36.197      mm"
"      Runoff volume      79.63      0.00      79.63      c.m"
"      Runoff coefficient      0.417      0.000      0.417      "
"      Maximum flow      0.027      0.000      0.027      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.027      0.105      0.000      0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.027      0.105      0.105      0.000"
" 40  HYDROGRAPH Combine 2"
"      6  Combine "
"      2  Node #"
"      To Walser Street"
"      Maximum flow      0.105      c.m/sec"
"      Hydrograph volume      362.120      c.m"
"      0.027      0.105      0.105      0.105"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.027      0.000      0.105      0.105"
" 33  CATCHMENT 10"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      10  Catchment 10"
"      0.000  % Impervious"
"      7.760  Total Area"
"     150.000  Flow length"

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"      2.000  Overland slope"
"      7.760  Pervious Area"
"     150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.418  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.618  0.000  0.105  0.105 c.m/sec"
"      Catchment 10  Pervious  Impervious  Total Area  "
"      Surface Area  7.760  0.000  7.760  hectare"
"      Time of concentration  38.106  4.916  38.106  minutes"
"      Time to Centroid  138.366  90.175  138.366  minutes"
"      Rainfall depth  86.737  86.737  86.737  mm"
"      Rainfall volume  6730.77  0.01  6730.77  c.m"
"      Rainfall losses  50.510  5.941  50.510  mm"
"      Runoff depth  36.227  80.796  36.227  mm"
"      Runoff volume  2811.21  0.01  2811.22  c.m"
"      Runoff coefficient  0.418  0.000  0.418  "
"      Maximum flow  0.618  0.000  0.618  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.618  0.618  0.105  0.105"
" 33  CATCHMENT 11"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"     11  Catchment 11"
"      0.000  % Impervious"
"      0.130  Total Area"
"     40.000  Flow length"
"      2.000  Overland Slope"
"      0.130  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.417  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.016  0.618  0.105  0.105 c.m/sec"
"      Catchment 11  Pervious  Impervious  Total Area  "
"      Surface Area  0.130  0.000  0.130  hectare"
"      Time of concentration  17.241  2.224  17.241  minutes"
"      Time to Centroid  114.897  86.667  114.896  minutes"
"      Rainfall depth  86.737  86.737  86.737  mm"

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"		Rainfall volume	112.76	0.00	112.76	c.m"
"		Rainfall losses	50.540	6.773	50.540	mm"
"		Runoff depth	36.197	79.963	36.197	mm"
"		Runoff volume	47.06	0.00	47.06	c.m"
"		Runoff coefficient	0.417	0.000	0.417	"
"		Maximum flow	0.016	0.000	0.016	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"		4 Add Runoff "				
"			0.016	0.628	0.105	0.105"
"	33	CATCHMENT 40"				
"		1 Triangular SCS"				
"		1 Equal length"				
"		1 SCS method"				
"		40 Catchment 40"				
"		0.000 % Impervious"				
"		6.340 Total Area"				
"		60.000 Flow length"				
"		2.000 Overland Slope"				
"		6.340 Pervious Area"				
"		60.000 Pervious length"				
"		2.000 Pervious slope"				
"		0.000 Impervious Area"				
"		60.000 Impervious length"				
"		2.000 Impervious slope"				
"		0.250 Pervious Manning 'n'"				
"		74.000 Pervious SCS Curve No."				
"		0.417 Pervious Runoff coefficient"				
"		0.100 Pervious Ia/S coefficient"				
"		8.924 Pervious Initial abstraction"				
"		0.015 Impervious Manning 'n'"				
"		98.000 Impervious SCS Curve No."				
"		0.000 Impervious Runoff coefficient"				
"		0.100 Impervious Ia/S coefficient"				
"		0.518 Impervious Initial abstraction"				
"			0.703	0.628	0.105	0.105 c.m/sec"
"		Catchment 40 Pervious				
"		Surface Area	6.340	0.000	6.340	hectare"
"		Time of concentration	21.990	2.837	21.990	minutes"
"		Time to Centroid	120.254	87.552	120.254	minutes"
"		Rainfall depth	86.737	86.737	86.737	mm"
"		Rainfall volume	5499.11	0.01	5499.11	c.m"
"		Rainfall losses	50.538	7.307	50.538	mm"
"		Runoff depth	36.199	79.429	36.199	mm"
"		Runoff volume	2295.03	0.01	2295.03	c.m"
"		Runoff coefficient	0.417	0.000	0.417	"
"		Maximum flow	0.703	0.000	0.703	c.m/sec"
"	40	HYDROGRAPH Add Runoff "				
"		4 Add Runoff "				
"			0.703	1.261	0.105	0.105"
"	54	POND DESIGN"				
"		1.261 Current peak flow				c.m/sec"
"		0.050 Target outflow				c.m/sec"
"		5153.3 Hydrograph volume				c.m"
"		6. Number of stages"				
"		409.630 Minimum water level				metre"
"		410.750 Maximum water level				metre"
"		409.630 Starting water level				metre"
"		0 Keep Design Data: 1 = True; 0 = False"				
"		Level Discharge				Volume"
"		409.630	0.000	0.000		
"		409.750	0.5400	232.250		
"		410.000	1.632	2017.880		
"		410.250	3.737	5148.940		

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"           410.500   1.345  9472.330"
"           410.750   1.885  15057.74"
"           Peak outflow           0.935   c.m/sec"
"           Maximum level           409.841  metre"
"           Maximum storage         880.637   c.m"
"           Centroidal lag           2.362    hours"
"           0.703   1.261   0.935   0.105 c.m/sec"
" 40         HYDROGRAPH Next link "
"           5 Next link "
"           0.703   0.935   0.935   0.105"
" 52         CHANNEL DESIGN"
"           0.935 Current peak flow   c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           0.000 Basewidth   metre"
"           7.410 Left bank slope"
"           6.000 Right bank slope"
"           0.950 Channel depth  metre"
"           1.040 Gradient   %"
"           Depth of flow           0.381    metre"
"           Velocity                 0.958    m/sec"
"           Channel capacity         10.655   c.m/sec"
"           Critical depth            0.331    metre"
" 53         ROUTE Channel Route 72"
"           72.40 Channel Route 72 Reach length ( metre)"
"           0.405 X-factor <= 0.5"
"           56.662 K-lag ( seconds)"
"           0.000 Default(0) or user spec.(1) values used"
"           0.500 X-factor <= 0.5"
"           30.000 K-lag ( seconds)"
"           0.500 Beta weighting factor"
"           60.000 Routing time step ( seconds)"
"           1 No. of sub-reaches"
"           Peak outflow           0.934   c.m/sec"
"           0.703   0.935   0.934   0.105 c.m/sec"
" 40         HYDROGRAPH Next link "
"           5 Next link "
"           0.703   0.934   0.934   0.105"
" 52         CHANNEL DESIGN"
"           0.934 Current peak flow   c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           2.000 Basewidth   metre"
"           2.950 Left bank slope"
"           3.000 Right bank slope"
"           0.950 Channel depth  metre"
"           1.040 Gradient   %"
"           Depth of flow           0.301    metre"
"           Velocity                 1.074    m/sec"
"           Channel capacity         9.246    c.m/sec"
"           Critical depth            0.247    metre"
" 53         ROUTE Channel Route 40"
"           39.80 Channel Route 40 Reach length ( metre)"
"           0.305 X-factor <= 0.5"
"           27.794 K-lag ( seconds)"
"           0.000 Default(0) or user spec.(1) values used"
"           0.500 X-factor <= 0.5"
"           30.000 K-lag ( seconds)"
"           0.500 Beta weighting factor"
"           37.500 Routing time step ( seconds)"
"           1 No. of sub-reaches"
"           Peak outflow           0.934   c.m/sec"
"           0.703   0.934   0.934   0.105 c.m/sec"

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```

" 40 HYDROGRAPH Next link "
" 5 Next link "
" 0.703 0.934 0.934 0.105"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.703 0.934 0.934 0.105"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 0.934 c.m/sec"
" Hydrograph volume 5154.222 c.m"
" 0.703 0.934 0.934 0.934"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.703 0.000 0.934 0.934"
" 33 CATCHMENT 20"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 20 Catchment 20"
" 0.000 % Impervious"
" 6.650 Total Area"
" 150.000 Flow length"
" 2.000 Overland Slope"
" 6.650 Pervious Area"
" 150.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 150.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.418 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.530 0.000 0.934 0.934 c.m/sec"
" Catchment 20 Pervious Impervious Total Area "
" Surface Area 6.650 0.000 6.650 hectare"
" Time of concentration 38.106 4.916 38.106 minutes"
" Time to Centroid 138.366 90.175 138.366 minutes"
" Rainfall depth 86.737 86.737 86.737 mm"
" Rainfall volume 5767.99 0.01 5768.00 c.m"
" Rainfall losses 50.510 5.941 50.510 mm"
" Runoff depth 36.227 80.796 36.227 mm"
" Runoff volume 2409.09 0.01 2409.10 c.m"
" Runoff coefficient 0.418 0.000 0.418 "
" Maximum flow 0.530 0.000 0.530 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.530 0.530 0.934 0.934"
" 33 CATCHMENT 21"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 21 Catchment 20"
" 10.000 % Impervious"
" 0.820 Total Area"

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411009_50YR EX_MARCH 2017

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"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.738  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      0.082  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.417  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.922  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.103      0.530      0.934      0.934 c.m/sec"
"      Catchment 21      Pervious      Impervious      Total Area      "
"      Surface Area      0.738      0.082      0.820      hectare"
"      Time of concentration      17.241      2.224      14.282      minutes"
"      Time to Centroid      114.897      86.667      109.333      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      640.12      71.12      711.24      c.m"
"      Rainfall losses      50.540      6.773      46.163      mm"
"      Runoff depth      36.197      79.963      40.574      mm"
"      Runoff volume      267.13      65.57      332.70      c.m"
"      Runoff coefficient      0.417      0.922      0.468      "
"      Maximum flow      0.091      0.031      0.103      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.103      0.592      0.934      0.934"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.103      0.592      0.592      0.934"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"          Maximum flow      0.592      c.m/sec"
"          Hydrograph volume      2741.804      c.m"
" 40      HYDROGRAPH Combine 1"
"      6      Combine "
"      1      Node #"
"          Total"
"          Maximum flow      1.478      c.m/sec"
"          Hydrograph volume      7896.021      c.m"
"          0.103      0.592      0.592      1.478"
" 38      START/RE-START TOTALS 21"
"      3      Runoff Totals on EXIT"
"          Total Catchment area      22.700      hectare"
"          Total Impervious area      0.082      hectare"
"          Total % impervious      0.361"
" 19      EXIT"

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411009_100YR_EX_MARCH_2017

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        W:\Kitchener\411-2011\411009\Design Data\
"          Modelling Files\2017-01-02\Existing\2017-03-07"
"          Output filename:                   411009_100YR_EX_MARCH_2017.out"
"          Licensee name:                     gmbp"
"          Company                           Hewlett-Packard Company"
"          Date & Time last used:             3/8/2017 at 8:38:25 AM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          6933.019 Coefficient A"
"          34.699 Constant B"
"          0.998 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                   168.777 mm/hr"
"          Total depth                         97.921 mm"
"          6 100hyd Hydrograph extension used in this file"
" 33      CATCHMENT 30"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          30 Catchment 30"
"          0.000 % Impervious"
"          0.780 Total Area"
"          75.000 Flow length"
"          2.000 Overland Slope"
"          0.780 Pervious Area"
"          75.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          75.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          74.000 Pervious SCS Curve No."
"          0.453 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          8.924 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.102 0.000 0.000 0.000 c.m/sec"
"          Catchment 30 Pervious Impervious Total Area "
"          Surface Area 0.780 0.000 0.780 hectare"
"          Time of concentration 23.441 3.140 23.441 minutes"
"          Time to Centroid 121.062 87.744 121.061 minutes"
"          Rainfall depth 97.921 97.921 97.921 mm"
"          Rainfall volume 763.79 0.00 763.79 c.m"
"          Rainfall losses 53.525 7.562 53.525 mm"
"          Runoff depth 44.396 90.359 44.396 mm"
"          Runoff volume 346.29 0.00 346.29 c.m"
"          Runoff coefficient 0.453 0.000 0.453 "
"          Maximum flow 0.102 0.000 0.102 c.m/sec"
" 40      HYDROGRAPH Add Runoff "

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"      4  Add Runoff "
"      0.102      0.102      0.000      0.000"
" 33  CATCHMENT 31"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      31  Catchment 31"
"      0.000  % Impervious"
"      0.220  Total Area"
"     40.000  Flow length"
"      2.000  Overland Slope"
"      0.220  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.453  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"     8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"     0.518  Impervious Initial abstraction"
"      0.034      0.102      0.000      0.000 c.m/sec"
"      Catchment 31      Pervious      Impervious      Total Area "
"      Surface Area      0.220      0.000      0.220      hectare"
"      Time of concentration      16.076      2.153      16.076      minutes"
"      Time to Centroid      112.853      86.345      112.853      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      215.43      0.00      215.43      c.m"
"      Rainfall losses      53.605      6.948      53.605      mm"
"      Runoff depth      44.316      90.973      44.316      mm"
"      Runoff volume      97.50      0.00      97.50      c.m"
"      Runoff coefficient      0.453      0.000      0.453      "
"      Maximum flow      0.034      0.000      0.034      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.034      0.134      0.000      0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.034      0.134      0.134      0.000"
" 40  HYDROGRAPH Combine 2"
"      6  Combine "
"      2  Node #"
"      To walser Street"
"      Maximum flow      0.134      c.m/sec"
"      Hydrograph volume      443.787      c.m"
"      0.034      0.134      0.134      0.134"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.034      0.000      0.134      0.134"
" 33  CATCHMENT 10"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      10  Catchment 10"
"      0.000  % Impervious"
"      7.760  Total Area"
"     150.000  Flow length"

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"      2.000  Overland Slope"
"      7.760  Pervious Area"
"     150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.454  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.801  0.000  0.134  0.134 c.m/sec"
"      Catchment 10      Pervious      Impervious      Total Area      "
"      Surface Area      7.760      0.000      7.760      hectare"
"      Time of concentration  35.531  4.759  35.530  minutes"
"      Time to Centroid      134.554  89.737  134.554  minutes"
"      Rainfall depth      97.921  97.921  97.921  mm"
"      Rainfall volume      7598.69  0.01  7598.69  c.m"
"      Rainfall losses      53.501  6.084  53.501  mm"
"      Runoff depth      44.420  91.837  44.420  mm"
"      Runoff volume      3447.00  0.01  3447.01  c.m"
"      Runoff coefficient      0.454  0.000  0.454  "
"      Maximum flow      0.801  0.000  0.801  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.801  0.801  0.134  0.134"
" 33  CATCHMENT 11"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"     11  Catchment 11"
"      0.000  % Impervious"
"      0.130  Total Area"
"     40.000  Flow length"
"      2.000  Overland Slope"
"      0.130  Pervious Area"
"     40.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     74.000  Pervious SCS Curve No."
"      0.453  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.020  0.801  0.134  0.134 c.m/sec"
"      Catchment 11      Pervious      Impervious      Total Area      "
"      Surface Area      0.130      0.000      0.130      hectare"
"      Time of concentration  16.076  2.153  16.076  minutes"
"      Time to Centroid      112.853  86.345  112.853  minutes"
"      Rainfall depth      97.921  97.921  97.921  mm"

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411009_100YR_EX_MARCH_2017

"	Rainfall volume	127.30	0.00	127.30	c.m"
"	Rainfall losses	53.605	6.948	53.605	mm"
"	Runoff depth	44.316	90.973	44.316	mm"
"	Runoff volume	57.61	0.00	57.61	c.m"
"	Runoff coefficient	0.453	0.000	0.453	"
"	Maximum flow	0.020	0.000	0.020	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.020	0.813	0.134	0.134"
" 33	CATCHMENT 40"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	40 Catchment 40"				
"	0.000 % Impervious"				
"	6.340 Total Area"				
"	60.000 Flow length"				
"	2.000 Overland Slope"				
"	6.340 Pervious Area"				
"	60.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	60.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.453 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"		0.877	0.813	0.134	0.134 c.m/sec"
"	Catchment 40	Pervious	Impervious	Total Area	"
"	Surface Area	6.340	0.000	6.340	hectare"
"	Time of concentration	20.504	2.747	20.504	minutes"
"	Time to Centroid	117.777	87.189	117.777	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	6208.20	0.01	6208.21	c.m"
"	Rainfall losses	53.532	7.496	53.532	mm"
"	Runoff depth	44.389	90.426	44.389	mm"
"	Runoff volume	2814.26	0.01	2814.27	c.m"
"	Runoff coefficient	0.453	0.000	0.453	"
"	Maximum flow	0.877	0.000	0.877	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.877	1.614	0.134	0.134"
" 54	POND DESIGN"				
"	1.614 Current peak flow	c.m/sec"			
"	0.050 Target outflow	c.m/sec"			
"	6318.9 Hydrograph volume	c.m"			
"	6. Number of stages"				
"	409.630 Minimum water level	metre"			
"	410.750 Maximum water level	metre"			
"	409.630 Starting water level	metre"			
"	0 Keep Design Data: 1 = True; 0 = False"				
"	Level Discharge	Volume"			
"	409.630	0.000	0.000"		
"	409.750	0.5400	232.250"		
"	410.000	1.632	2017.880"		
"	410.250	3.737	5148.940"		


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"           410.500      1.345  9472.330"
"           410.750      1.885  15057.74"
"           Peak outflow                1.157      c.m/sec"
"           Maximum level                409.891     metre"
"           Maximum storage              1241.886     c.m"
"           Centroidal lag                2.341     hours"
"           0.877      1.614      1.157      0.134 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5      Next link "
"           0.877      1.157      1.157      0.134"
" 52      CHANNEL DESIGN"
"           1.157      Current peak flow      c.m/sec"
"           0.035      Manning 'n'"
"           0.      Cross-section type: 0=trapezoidal; 1=general"
"           0.000      Basewidth      metre"
"           7.410      Left bank slope"
"           6.000      Right bank slope"
"           0.950      Channel depth      metre"
"           1.040      Gradient      %"
"           Depth of flow                0.413     metre"
"           Velocity                    1.011     m/sec"
"           Channel capacity              10.655     c.m/sec"
"           Critical depth                0.360     metre"
" 53      ROUTE      Channel Route 72"
"           72.40      Channel Route 72 Reach length (metre)"
"           0.397      X-factor <= 0.5"
"           53.723      K-lag (seconds)"
"           0.000      Default(0) or user spec.(1) values used"
"           0.500      X-factor <= 0.5"
"           30.000      K-lag (seconds)"
"           0.500      Beta weighting factor"
"           60.000      Routing time step (seconds)"
"           1      No. of sub-reaches"
"           Peak outflow                1.153     c.m/sec"
"           0.877      1.157      1.153     0.134 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5      Next link "
"           0.877      1.153      1.153     0.134"
" 52      CHANNEL DESIGN"
"           1.153      Current peak flow      c.m/sec"
"           0.035      Manning 'n'"
"           0.      Cross-section type: 0=trapezoidal; 1=general"
"           2.000      Basewidth      metre"
"           2.950      Left bank slope"
"           3.000      Right bank slope"
"           0.950      Channel depth      metre"
"           1.040      Gradient      %"
"           Depth of flow                0.336     metre"
"           Velocity                    1.142     m/sec"
"           Channel capacity              9.246     c.m/sec"
"           Critical depth                0.280     metre"
" 53      ROUTE      Channel Route 40"
"           39.80      Channel Route 40 Reach length (metre)"
"           0.284      X-factor <= 0.5"
"           26.130      K-lag (seconds)"
"           0.000      Default(0) or user spec.(1) values used"
"           0.500      X-factor <= 0.5"
"           30.000      K-lag (seconds)"
"           0.500      Beta weighting factor"
"           33.333      Routing time step (seconds)"
"           1      No. of sub-reaches"
"           Peak outflow                1.153     c.m/sec"
"           0.877      1.153      1.153     0.134 c.m/sec"

```

```

" 40 HYDROGRAPH Next link "
" 5 Next link "
" 0.877 1.153 1.153 0.134"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.877 1.153 1.153 0.134"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 1.153 c.m/sec"
" Hydrograph volume 6312.710 c.m"
" 0.877 1.153 1.153 1.153"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.877 0.000 1.153 1.153"
" 33 CATCHMENT 20"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 20 Catchment 20"
" 0.000 % Impervious"
" 6.650 Total Area"
" 150.000 Flow length"
" 2.000 overland Slope"
" 6.650 Pervious Area"
" 150.000 Pervious length"
" 2.000 Pervious slope"
" 0.000 Impervious Area"
" 150.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.454 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.000 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.687 0.000 1.153 1.153 c.m/sec"
" Catchment 20 Pervious Impervious Total Area "
" Surface Area 6.650 0.000 6.650 hectare"
" Time of concentration 35.531 4.759 35.530 minutes"
" Time to Centroid 134.554 89.737 134.554 minutes"
" Rainfall depth 97.921 97.921 97.921 mm"
" Rainfall volume 6511.76 0.01 6511.77 c.m"
" Rainfall losses 53.501 6.084 53.501 mm"
" Runoff depth 44.420 91.837 44.420 mm"
" Runoff volume 2953.94 0.01 2953.94 c.m"
" Runoff coefficient 0.454 0.000 0.454 "
" Maximum flow 0.687 0.000 0.687 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.687 0.687 1.153 1.153"
" 33 CATCHMENT 21"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 21 Catchment 20"
" 10.000 % Impervious"
" 0.820 Total Area"

```

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```

" 40.000 Flow Length"
" 2.000 Overland Slope"
" 0.738 Pervious Area"
" 40.000 Pervious length"
" 2.000 Pervious slope"
" 0.082 Impervious Area"
" 40.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 74.000 Pervious SCS Curve No."
" 0.453 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 8.924 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.929 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.134 0.687 1.153 1.153 c.m/sec"
" Catchment 21 Pervious Impervious Total Area "
" Surface Area 0.738 0.082 0.820 hectare"
" Time of concentration 16.076 2.153 13.490 minutes"
" Time to Centroid 112.853 86.345 107.930 minutes"
" Rainfall depth 97.921 97.921 97.921 mm"
" Rainfall volume 722.66 80.30 802.95 c.m"
" Rainfall losses 53.605 6.948 48.939 mm"
" Runoff depth 44.316 90.973 48.982 mm"
" Runoff volume 327.05 74.60 401.65 c.m"
" Runoff coefficient 0.453 0.929 0.500 "
" Maximum flow 0.115 0.034 0.134 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.134 0.763 1.153 1.153"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
" 0.134 0.763 0.763 1.153"
" 64 SHOW TABLE"
" 2 Flow hydrograph"
" 4 Inflow Hydrograph"
" Maximum flow 0.763 c.m/sec"
" Hydrograph volume 3355.596 c.m"
" 40 HYDROGRAPH Combine 1"
" 6 Combine "
" 1 Node #"
" Total"
" Maximum flow 1.850 c.m/sec"
" Hydrograph volume 9668.306 c.m"
" 0.134 0.763 0.763 1.850"
" 38 START/RE-START TOTALS 21"
" 3 Runoff Totals on EXIT"
" Total Catchment area 22.700 hectare"
" Total Impervious area 0.082 hectare"
" Total % impervious 0.361"
" 19 EXIT"

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411009_REG EX_MARCH 2017

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          W:\Kitchener\411-2011\411009\Design Data\
"          Modelling Files\2017-01-02\Existing\2017-03-07"
"          Output filename:                    411009_REG EX_MARCH 2017.out"
"          Licensee name:                      gmbp"
"          Company                            Hewlett-Packard Company"
"          Date & Time last used:              3/8/2017 at 8:41:16 AM"
" 31  TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000  Max. Storm length"
"          12000.000  Max. Hydrograph"
" 32  STORM Historic"
"          5  Historic"
"          2880.000  Duration"
"          48.000  Rainfall intensity values"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.026    2.026    2.026    2.028"
"                   2.026    6.000    4.000    6.000    13.000"
"                   17.000    13.000    23.000    13.000    13.000"
"                   53.000    38.000    13.000"
"          Maximum intensity                    53.000  mm/hr"
"          Total depth                          285.000  mm"
" 33  6 200hyd Hydrograph extension used in this file"
"      CATCHMENT 30"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          30  Catchment 30"
"          0.000  % Impervious"
"          0.780  Total Area"
"          75.000  Flow length"
"          2.000  Overland Slope"
"          0.780  Pervious Area"
"          75.000  Pervious length"
"          2.000  Pervious slope"
"          0.000  Impervious Area"
"          75.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          74.000  Pervious SCS Curve No."
"          0.719  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          8.924  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.000  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"                   0.087    0.000    0.000    0.000  c.m/sec"
"          Catchment 30  Pervious  Impervious  Total Area  "
"          Surface Area    0.780    0.000    0.780  hectare"
"          Time of concentration  27.920    4.957    27.920  minutes"
"          Time to Centroid    2555.648    2258.090    2555.648  minutes"
"          Rainfall depth    285.000    285.000    285.000  mm"
"          Rainfall volume    2223.00    0.00    2223.00  c.m"

```

411009_REG_EX_MARCH 2017

"	Rainfall losses	80.023	36.749	80.023	mm"
"	Runoff depth	204.977	248.251	204.977	mm"
"	Runoff volume	1598.82	0.00	1598.82	c.m"
"	Runoff coefficient	0.719	0.000	0.719	"
"	Maximum flow	0.087	0.000	0.087	c.m/sec"
40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.087	0.087	0.000	0.000"	
33	CATCHMENT 31"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	31 Catchment 31"				
"	0.000 % Impervious"				
"	0.220 Total Area"				
"	40.000 Flow length"				
"	2.000 Overland Slope"				
"	0.220 Pervious Area"				
"	40.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.000 Impervious Area"				
"	40.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	74.000 Pervious SCS Curve No."				
"	0.723 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	8.924 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.000 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.023	0.087	0.000	0.000 c.m/sec"	
"	Catchment 31	Pervious	Impervious	Total Area	"
"	Surface Area	0.220	0.000	0.220	hectare"
"	Time of concentration	19.148	3.399	19.148	minutes"
"	Time to Centroid	2545.193	2266.333	2545.193	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	627.00	0.00	627.00	c.m"
"	Rainfall losses	78.940	42.646	78.940	mm"
"	Runoff depth	206.060	242.354	206.060	mm"
"	Runoff volume	453.33	0.00	453.33	c.m"
"	Runoff coefficient	0.723	0.000	0.723	"
"	Maximum flow	0.023	0.000	0.023	c.m/sec"
40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.023	0.110	0.000	0.000"	
40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.023	0.110	0.110	0.000"	
40	HYDROGRAPH Combine 2"				
"	6 Combine "				
"	2 Node #"				
"	To Walser Street"				
"	Maximum flow		0.110	c.m/sec"	
"	Hydrograph volume		2052.151	c.m"	
"	0.023	0.110	0.110	0.110"	
40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"	0.023	0.000	0.110	0.110"	
33	CATCHMENT 10"				
"	1 Triangular SCS"				

```

"      1 Equal length"
"      1 SCS method"
"     10 Catchment 10"
"     0.000 % Impervious"
"     7.760 Total Area"
"    150.000 Flow length"
"     2.000 Overland Slope"
"     7.760 Pervious Area"
"    150.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    150.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.714 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.881      0.000      0.110      0.110 c.m/sec"
"      Catchment 10      Pervious      Impervious      Total Area      "
"      Surface Area      7.760      0.000      7.760      hectare"
"      Time of concentration      42.319      7.513      42.319      minutes"
"      Time to Centroid      2572.242      2276.224      2572.241      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      2.2116      0.0000      2.2116      ha-m"
"      Rainfall losses      81.644      25.621      81.644      mm"
"      Runoff depth      203.356      259.379      203.356      mm"
"      Runoff volume      1.5780      0.0000      1.5780      ha-m"
"      Runoff coefficient      0.714      0.000      0.714      "
"      Maximum flow      0.881      0.000      0.881      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.881      0.881      0.110      0.110"
" 33 CATCHMENT 11"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     11 Catchment 11"
"     0.000 % Impervious"
"     0.130 Total Area"
"    40.000 Flow length"
"     2.000 Overland Slope"
"     0.130 Pervious Area"
"    40.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    40.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.723 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"

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	0.014	0.881	0.110	0.110	c.m/sec"
Catchment 11			Pervious	Impervious	Total Area "
Surface Area	0.130		0.000	0.130	hectare"
Time of concentration	19.148		3.399	19.148	minutes"
Time to Centroid	2545.193		2266.333	2545.193	minutes"
Rainfall depth	285.000		285.000	285.000	mm"
Rainfall volume	370.50		0.00	370.50	c.m"
Rainfall losses	78.940		42.646	78.940	mm"
Runoff depth	206.060		242.354	206.060	mm"
Runoff volume	267.88		0.00	267.88	c.m"
Runoff coefficient	0.723		0.000	0.723	"
Maximum flow	0.014		0.000	0.014	c.m/sec"

40 HYDROGRAPH Add Runoff "

	0.014	0.894	0.110	0.110"
--	-------	-------	-------	--------

33 CATCHMENT 40"

1	Triangular SCS"
1	Equal length"
1	SCS method"
40	Catchment 40"
0.000	% Impervious"
6.340	Total Area"
60.000	Flow length"
2.000	Overland Slope"
6.340	Pervious Area"
60.000	Pervious length"
2.000	Pervious slope"
0.000	Impervious Area"
60.000	Impervious length"
2.000	Impervious slope"
0.250	Pervious Manning 'n'"
74.000	Pervious SCS Curve No."
0.716	Pervious Runoff coefficient"
0.100	Pervious Ia/S coefficient"
8.924	Pervious Initial abstraction"
0.015	Impervious Manning 'n'"
98.000	Impervious SCS Curve No."
0.000	Impervious Runoff coefficient"
0.100	Impervious Ia/S coefficient"
0.518	Impervious Initial abstraction"

	0.688	0.894	0.110	0.110	c.m/sec"
Catchment 40			Pervious	Impervious	Total Area "
Surface Area	6.340		0.000	6.340	hectare"
Time of concentration	24.421		4.336	24.421	minutes"
Time to Centroid	2549.942		2258.969	2549.942	minutes"
Rainfall depth	285.000		285.000	285.000	mm"
Rainfall volume	1.8069		0.0000	1.8069	ha-m"
Rainfall losses	80.848		39.404	80.848	mm"
Runoff depth	204.152		245.596	204.152	mm"
Runoff volume	1.2943		0.0000	1.2943	ha-m"
Runoff coefficient	0.716		0.000	0.716	"
Maximum flow	0.688		0.000	0.688	c.m/sec"

40 HYDROGRAPH Add Runoff "

	0.688	1.582	0.110	0.110"
--	-------	-------	-------	--------

54 POND DESIGN"

1.582	Current peak flow	c.m/sec"
0.050	Target outflow	c.m/sec"
28991.5	Hydrograph volume	c.m"
6.	Number of stages"	
409.630	Minimum water level	metre"
410.750	Maximum water level	metre"
409.630	Starting water level	metre"

```

"      0  Keep Design Data: 1 = True; 0 = False"
"      Level Discharge      Volume"
"      409.630      0.000      0.000"
"      409.750      0.5400     232.250"
"      410.000      1.632     2017.880"
"      410.250      3.737     5148.940"
"      410.500      1.345     9472.330"
"      410.750      1.885     15057.74"
"      Peak outflow                1.422      c.m/sec"
"      Maximum level                409.952     metre"
"      Maximum storage              1675.131     c.m"
"      Centroidal lag                42.968     hours"
"      0.688      1.582      1.422      0.110 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"      0.688      1.422      1.422      0.110"
" 52  CHANNEL DESIGN"
"      1.422 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"      Depth of flow                0.446     metre"
"      Velocity                      1.064     m/sec"
"      Channel capacity              10.655     c.m/sec"
"      Critical depth                0.391     metre"
" 53  ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length (metre)"
"      0.389 X-factor <= 0.5"
"      51.023 K-lag (seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag (seconds)"
"      0.500 Beta weighting factor"
"      62.069 Routing time step (seconds)"
"      1 No. of sub-reaches"
"      Peak outflow                1.415     c.m/sec"
"      0.688      1.422      1.415     0.110 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"      0.688      1.415      1.415     0.110"
" 52  CHANNEL DESIGN"
"      1.415 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      2.000 Basewidth      metre"
"      2.950 Left bank slope"
"      3.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"      Depth of flow                0.375     metre"
"      Velocity                      1.212     m/sec"
"      Channel capacity              9.246     c.m/sec"
"      Critical depth                0.315     metre"
" 53  ROUTE Channel Route 40"
"      39.80 Channel Route 40 Reach length (metre)"
"      0.262 X-factor <= 0.5"
"      24.631 K-lag (seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"

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"      30.000  K-lag ( seconds)"
"      0.500  Beta weighting factor"
"      36.000  Routing time step ( seconds)"
"      1      No. of sub-reaches"
"      Peak outflow                1.412  c.m/sec"
"      0.688      1.415      1.412  0.110 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"      0.688      1.412      1.412  0.110"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"      0.688      1.412      1.412  0.110"
" 40      HYDROGRAPH Combine 1"
"      6      Combine "
"      1      Node #"
"      Total"
"      Maximum flow                1.412  c.m/sec"
"      Hydrograph volume          29214.979 c.m"
"      0.688      1.412      1.412  1.412"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"      0.688      0.000      1.412  1.412"
" 33      CATCHMENT 20"
"      1      Triangular scs"
"      1      Equal length"
"      1      SCS method"
"      20     Catchment 20"
"      0.000  % Impervious"
"      6.650  Total Area"
" 150.000  Flow length"
"      2.000  Overland Slope"
"      6.650  Pervious Area"
" 150.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
" 150.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000 Pervious SCS Curve No."
"      0.714  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
" 98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.755      0.000      1.412  1.412 c.m/sec"
"      Catchment 20      Pervious      Impervious      Total Area "
"      Surface Area      6.650      0.000      6.650      hectare"
"      Time of concentration 42.319      7.513      42.319      minutes"
"      Time to Centroid      2572.242      2276.224      2572.241      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1.8952      0.0000      1.8952      ha-m"
"      Rainfall losses      81.644      25.621      81.644      mm"
"      Runoff depth      203.356      259.379      203.356      mm"
"      Runoff volume      1.3523      0.0000      1.3523      ha-m"
"      Runoff coefficient      0.714      0.000      0.714      "
"      Maximum flow      0.755      0.000      0.755      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      0.755      0.755      1.412  1.412"
" 33      CATCHMENT 21"

```

```

"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     21 Catchment 20"
"    10.000 % Impervious"
"     0.820 Total Area"
"    40.000 Flow length"
"     2.000 Overland Slope"
"     0.738 Pervious Area"
"    40.000 Pervious length"
"     2.000 Pervious slope"
"     0.082 Impervious Area"
"    40.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.723 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.850 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.087      0.755      1.412      1.412 c.m/sec"
"      Catchment 21      Pervious      Impervious      Total Area      "
"      Surface Area      0.738      0.082      0.820      hectare"
"      Time of concentration      19.148      3.399      17.327      minutes"
"      Time to Centroid      2545.193      2266.333      2512.963      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      2103.30      233.70      2337.00      c.m"
"      Rainfall losses      78.940      42.646      75.310      mm"
"      Runoff depth      206.060      242.354      209.690      mm"
"      Runoff volume      1520.72      198.73      1719.45      c.m"
"      Runoff coefficient      0.723      0.850      0.736      "
"      Maximum flow      0.078      0.010      0.087      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.087      0.840      1.412      1.412"
" 40      HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"           0.087      0.840      0.840      1.412"
" 64      SHOW TABLE"
"      2 Flow hydrograph"
"      4 Inflow Hydrograph"
"      Maximum flow      0.840      c.m/sec"
"      Hydrograph volume      15242.604      c.m"
" 40      HYDROGRAPH Combine 1"
"      6 Combine "
"      1 Node #"
"      Total"
"      Maximum flow      2.252      c.m/sec"
"      Hydrograph volume      44457.578      c.m"
"           0.087      0.840      0.840      2.252"
" 38      START/RE-START TOTALS 21"
"      3 Runoff Totals on EXIT"
"      Total Catchment area      22.700      hectare"
"      Total Impervious area      0.082      hectare"
"      Total % impervious      0.361"
" 19      EXIT"

```

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017

Catchment 1000 & 1100 : Stormwater Management Facility No. 1

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m²)	(m³)	(m³)	
411.00	0.00	6,075	0	0.0	Knockout
411.10	0.10	6,163	612	611.9	
411.20	0.20	6,252	621	1,232.7	
411.30	0.30	6,340	630	1,862.3	
411.40	0.40	6,429	638	2,500.7	
411.50	0.50	6,519	647	3,148.1	
411.60	0.60	6,609	656	3,804.5	CB.1 Lip
411.70	0.70	6,699	665	4,469.9	
411.75	0.75	6,744	336	4,806.0	CB.2 Lip
411.80	0.80	6,790	338	5,144.3	
411.85	0.85	6,835	341	5,485.0	Weir
412.00	1.00	3,972	811	6,295.5	Top of bank

Outlet #1

130 mm Diameter Knockout

Q = 0.026 m³/s
 Cd = 0.600
 H = 0.535 m
 2g = 19.620

A = 0.013 m²
 D = 0.130 m
 D/2 = 0.065 m

Outlet #2

450 mm diameter pipe
 300 mm orifice

Q = 0.188 m³/s
 Cd = 0.600
 H = 1.000 m
 2g = 19.620

A = 0.071 m²
 D = 0.300 m
 D/2 = 0.150 m
 Invert = 410.85

Outlet #3

450 mm diameter pipe
 200 mm orifice

Q = 0.086 m³/s
 Cd = 0.600
 H = 1.050 m
 2g = 19.620

A = 0.031 m²
 D = 0.200 m
 D/2 = 0.100 m
 Invert = 410.85

Overflow Weir

Elevation = 411.90

d1 = 1.00 m
 h = 0.85 m
 H = 0.15 m
 2g = 19.620
 L = 20.00 m

Q = 1.638 m³/s

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Outlet #1	Outlet #2	Outlet #3	Overflow	Actual	
(m)	(m)	(m³)	130 mm	450 mm	450mm	Weir	Discharge	
			(m³/s)	(m³/s)	(m³/s)	(m³/s)	(m³/s)	
411.00	0.00	0.0	0.000	0.000	0.000	0.000	0.000	Knockout
411.10	0.10	611.9	0.006	0.000	0.000	0.000	0.006	
411.20	0.20	1,232.7	0.013	0.000	0.000	0.000	0.013	
411.30	0.30	1,862.3	0.017	0.000	0.000	0.000	0.017	
411.40	0.40	2,500.7	0.020	0.000	0.000	0.000	0.020	
411.50	0.50	3,148.1	0.023	0.000	0.000	0.000	0.023	
411.60	0.60	3,804.5	0.026	0.000	0.000	0.000	0.026	CB.1 Lip
411.70	0.70	4,469.9	0.000	0.157	0.000	0.000	0.157	
411.75	0.75	4,806.0	0.000	0.163	0.000	0.000	0.163	CB.2 Lip
411.80	0.80	5,144.3	0.000	0.168	0.077	0.000	0.245	
411.85	0.85	5,485.0	0.000	0.173	0.079	0.000	0.252	Weir
412.00	1.00	6,295.5	0.000	0.188	0.086	1.638	1.912	Top of bank

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017

Catchment 2100 : Stormwater Management Facility No. 2

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m²)	(m³)	(m³)	
410.65	0.00	488.80	0	0.0	Bottom of Pond/Knockout
410.70	0.05	511.10	25	25.0	
410.80	0.15	572.00	54	79.2	
410.90	0.25	634.70	60	139.5	
411.00	0.35	699.30	67	206.2	
411.10	0.45	765.60	73	279.4	
411.20	0.55	833.70	80	359.4	CB Lip
411.30	0.65	903.60	87	446.3	
411.40	0.75	975.23	94	540.2	
411.50	0.85	1048.80	101	641.4	
411.60	0.95	1124.10	109	750.1	
411.65	1.00	1150.00	57	806.9	Weir
411.95	1.30	1440.00	389	1,195.4	Top of bank

Outlet #1

150 mm Diameter Knockout

Q = 0.032 m³/s
 Cd = 0.600
 H = 0.475 m
 2g = 19.620

A = 0.018 m²
 D = 0.150 m
 D/2 = 0.075 m

Outlet #2

450 mm diameter pipe

Q = 0.448 m³/s
 Cd = 0.600
 H = 0.725 m
 2g = 19.620

A = 0.159 m²
 D = 0.450 m
 D/2 = 0.225 m
 Invert = 410.60

Overflow Weir

Elevation = 411.65

d1 = 1.30 m
 h = 1.00 m
 H = 0.30 m
 2g = 19.62
 L = 10.00 m

Q = 2.369 m³/s

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Outlet #1	Outlet #2	Overflow	Actual	
(m)	(m)	(m³)	130 mm	425 mm	Weir	Discharge	
			(m³/s)	(m³/s)	(m³/s)	(m³/s)	
410.65	0.00	0.0	0.000	0.000	0.000	0.000	Bottom of Pond/Knockout
410.70	0.05	25.0	0.006	0.000	0.000	0.006	
410.80	0.15	79.2	0.013	0.000	0.000	0.013	
410.90	0.25	139.5	0.020	0.000	0.000	0.020	
411.00	0.35	206.2	0.025	0.000	0.000	0.025	
411.10	0.45	279.4	0.029	0.000	0.000	0.029	
411.20	0.55	359.4	0.032	0.000	0.000	0.032	CB Lip
411.30	0.65	446.3	0.000	0.291	0.000	0.291	
411.40	0.75	540.2	0.000	0.321	0.000	0.321	
411.50	0.85	641.4	0.000	0.347	0.000	0.347	
411.60	0.95	750.1	0.000	0.372	0.000	0.372	
411.65	1.00	806.9	0.000	0.384	0.000	0.384	Weir
411.95	1.30	1,195.4	0.000	0.448	2.369	2.818	Top of bank

**Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017**

Catchment 2200 : Private Stormwater Management Facility Multi-Family Block

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m²)	(m³)	(m³)	
413.70	0.00	847	0	0.0	Bottom of Pond/Knockout
413.80	0.10	924	89	88.6	
413.90	0.20	1,049	99	187.2	
414.00	0.30	1,175	111	298.4	
414.10	0.40	1,302	124	422.2	
414.20	0.50	1,431	137	558.9	
414.30	0.60	1,561	150	708.5	
414.40	0.70	1,692	163	871.1	CB Lip
414.50	0.80	1,825	176	1,046.9	
414.60	0.90	1,959	189	1,236.1	
414.70	1.00	2,094	203	1,438.7	Weir
415.00	1.30	2231	649	2,087.4	Top of bank

Outlet #1

110 mm Diameter Knockout

Q = 0.020 m³/s
Cd = 0.600
H = 0.645 m
2g = 19.620

A = 0.010 m²
D = 0.110 m
D/2 = 0.055 m

Outlet #2

450 mm diameter pipe

Q = 0.372 m³/s
Cd = 0.600
H = 0.775 m
2g = 19.620

A = 0.159 m²
D = 0.450 m
D/2 = 0.225 m
Invert = 413.7

Overflow Weir

Elevation = 414.70

d1 = 1.30 m
h = 1.00 m
H = 0.30 m
2g = 19.62
L = 10.00 m

Q = 2.369 m³/s

**Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017**

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Outlet #1	Outlet #2	Overflow	Actual	
(m)	(m)	(m³)	110 mm	450 mm	Weir	Discharge	
			(m³/s)	(m³/s)	(m³/s)	(m³/s)	
413.70	0.00	0.0	0.000	0.000	0.000	0.000	Bottom of Pond/Knockout
413.80	0.10	88.6	0.005	0.000	0.000	0.005	
413.90	0.20	187.2	0.010	0.000	0.000	0.010	
414.00	0.30	298.4	0.013	0.000	0.000	0.013	
414.10	0.40	422.2	0.015	0.000	0.000	0.015	
414.20	0.50	558.9	0.017	0.000	0.000	0.017	
414.30	0.60	708.5	0.019	0.000	0.000	0.019	
414.40	0.70	871.1	0.020	0.000	0.000	0.020	CB Lip
414.50	0.80	1,046.9	0.000	0.321	0.000	0.321	
414.60	0.90	1,236.1	0.000	0.347	0.000	0.347	
414.70	1.00	1,438.7	0.000	0.372	0.000	0.372	Weir
415.00	1.30	2,087.4	0.000	0.438	2.369	2.808	Top of bank

Ainley Farm Subdivision
Township of Centre Wellington (Elora)
G&M File: 411-009
April 17, 2017

Catchment 4000: Wetland

Stage Storage Volume Calculations

Elevation	Stage	Surface Area	Increm. Storage	Accum. Storage	
(m)	(m)	(m²)	(m³)	(m³)	
409.63	0.00	0	0	0.0	Wetland Bottom
409.75	0.12	3871	232	232.2	
410.00	0.37	10414	1786	2017.9	
410.25	0.62	14634	3131	5148.9	
410.50	0.87	19953	4323	9472.3	Weir
410.75	1.12	24730	5585	15057.7	Overflow

WEIR CALCULATIONS

d1 = 1.12 m
h = 0.87 m
H = 0.25 m
2g = 19.62
L = 3 m

Q = 0.540 m³/s

Stage/Storage/Discharge Table

Elevation	Stage	Storage	Actual Discharge	
(m)	(m)	(m³)	(m³/s)	
409.63	0.00	0.0	0.000	Wetland Bottom
409.75	0.12	232.2	0.540	
410.00	0.37	2017.9	1.632	
410.25	0.62	5148.9	3.737	
410.50	0.87	9472.3	1.345	Weir
410.75	1.12	15057.7	1.885	Overflow

411009_2YR POST

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        C:\Users\akroetsch\Documents\
"                                         Work in Progress\411009 Ainley Farm SWM Junk"
"          Output filename:                   411009_2YR POST.out"
"          Licensee name:                     gmbp"
"          Company                           Hewlett-Packard Company"
"          Date & Time last used:            4/17/2017 at 3:42:34 PM"

```

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" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          695.047 Coefficient A"
"          6.387 Constant B"
"          0.793 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                93.292 mm/hr"
"          Total depth                      33.014 mm"
"          6 002hyd Hydrograph extension used in this file"

```

```

" 33      CATCHMENT 3100"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          3100 Catchment 3100"
"          60.000 % Impervious"
"          0.400 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.160 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.240 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.207 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.838 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"

```

	0.043	0.000	0.000	0.000 c.m/sec"	
Catchment 3100	Pervious	Impervious	Total Area		"
Surface Area	0.160	0.240	0.400		hectare"
Time of concentration	20.437	1.868	4.499		minutes"
Time to Centroid	125.085	88.659	93.819		minutes"
Rainfall depth	33.014	33.014	33.014		mm"
Rainfall volume	52.82	79.23	132.06		c.m"
Rainfall losses	26.169	5.363	13.685		mm"
Runoff depth	6.845	27.651	19.329		mm"
Runoff volume	10.95	66.36	77.31		c.m"
Runoff coefficient	0.207	0.838	0.585		"
Maximum flow	0.003	0.043	0.043		c.m/sec"

```

" 40      HYDROGRAPH Add Runoff "

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411009_2YR POST

```

"      4  Add Runoff "
"      0.043  0.043  0.000  0.000"
" 56  DIVERSION"
"      3100  Node number"
"      0.062  Overflow threshold"
"      1.000  Required diverted fraction"
"      0  Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow 0.000 c.m/sec"
"      Volume of diverted flow 0.000 c.m"
"      DIV03100.002hyd"
"      Major flow at 3100"
"      0.043  0.043  0.043  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"      0.043  0.043  0.043  0.000"
" 33  CATCHMENT 1000"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      1000  Catchment 1000"
"      50.000  % Impervious"
"      11.280  Total Area"
"      20.000  Flow length"
"      2.000  Overland Slope"
"      5.640  Pervious Area"
"      20.000  Pervious length"
"      2.000  Pervious slope"
"      5.640  Impervious Area"
"      20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.838  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      1.014  0.043  0.043  0.000 c.m/sec"
"      Catchment 1000  Pervious  Impervious Total Area "
"      Surface Area  5.640  5.640  11.280  hectare"
"      Time of concentration  20.437  1.868  5.553  minutes"
"      Time to Centroid  125.085  88.659  95.887  minutes"
"      Rainfall depth  33.014  33.014  33.014  mm"
"      Rainfall volume  1861.98  1861.98  3723.96  c.m"
"      Rainfall losses  26.169  5.363  15.766  mm"
"      Runoff depth  6.845  27.651  17.248  mm"
"      Runoff volume  386.07  1559.50  1945.58  c.m"
"      Runoff coefficient  0.207  0.838  0.522  "
"      Maximum flow  0.110  1.008  1.014  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      1.014  1.057  0.043  0.000"
" 33  CATCHMENT 1100"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      1100  Catchment 1100"
"      0.000  % Impervious"
"      0.470  Total Area"
"      20.000  Flow length"

```

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```

"      2.000  Overland Slope"
"      0.470  Pervious Area"
"     20.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.009  1.057  0.043  0.000 c.m/sec"
"      Catchment 1100  Pervious  Impervious  Total Area  "
"      Surface Area      0.470  0.000  0.470  hectare"
"      Time of concentration  20.437  1.868  20.437  minutes"
"      Time to Centroid      125.085  88.659  125.085  minutes"
"      Rainfall depth      33.014  33.014  33.014  mm"
"      Rainfall volume      155.16  0.00  155.16  c.m"
"      Rainfall losses      26.169  5.363  26.169  mm"
"      Runoff depth         6.845  27.651  6.845  mm"
"      Runoff volume        32.17  0.00  32.17  c.m"
"      Runoff coefficient    0.207  0.000  0.207  "
"      Maximum flow         0.009  0.000  0.009  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.009  1.059  0.043  0.000"
" 54  POND DESIGN"
"      1.059  Current peak flow  c.m/sec"
"      0.756  Target outflow  c.m/sec"
"     2055.1  Hydrograph volume  c.m"
"      12.    Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge  Volume"
"         411.000  0.000  0.000"
"         411.100  0.00600  612.000"
"         411.200  0.01300  1233.000"
"         411.300  0.01700  1862.000"
"         411.400  0.02000  2501.000"
"         411.500  0.02300  3148.000"
"         411.600  0.02600  3805.000"
"         411.700  0.1570  4470.000"
"         411.750  0.1630  4806.000"
"         411.800  0.2450  5144.000"
"         411.850  0.2520  5485.000"
"         412.000  1.912  6295.000"
"      Peak outflow      0.017  c.m/sec"
"      Maximum level     411.307  metre"
"      Maximum storage   1908.194  c.m"
"      Centroidal lag    29.598  hours"
"          0.009  1.059  0.017  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"          0.009  0.017  0.017  0.000"
" 33  CATCHMENT 4000"

```

```

"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     4000 Catchment 4000"
"     0.000 % Impervious"
"     6.340 Total Area"
"    60.000 Flow length"
"     2.000 Overland Slope"
"     6.340 Pervious Area"
"    60.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    60.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.155 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.054 0.017 0.017 0.000 c.m/sec"
" Catchment 4000 Pervious Impervious Total Area "
" Surface Area 6.340 0.000 6.340 hectare"
" Time of concentration 45.052 3.611 45.052 minutes"
" Time to Centroid 156.495 91.497 156.495 minutes"
" Rainfall depth 33.014 33.014 33.014 mm"
" Rainfall volume 2093.07 0.00 2093.08 c.m"
" Rainfall losses 27.895 5.642 27.895 mm"
" Runoff depth 5.119 27.372 5.119 mm"
" Runoff volume 324.52 0.00 324.52 c.m"
" Runoff coefficient 0.155 0.000 0.155 "
" Maximum flow 0.054 0.000 0.054 c.m/sec"
40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.054 0.069 0.017 0.000"
54 POND DESIGN"
"     0.069 Current peak flow c.m/sec"
"     0.756 Target outflow c.m/sec"
"    2377.8 Hydrograph volume c.m"
"     6. Number of stages"
"   409.630 Minimum water level metre"
"   410.750 Maximum water level metre"
"   409.630 Starting water level metre"
"     0 Keep Design Data: 1 = True; 0 = False"
"       Level Discharge Volume"
"   409.630 0.000 0.000"
"   409.750 0.5400 232.250"
"   410.000 1.632 2017.880"
"   410.250 3.737 5148.940"
"   410.500 1.345 9472.330"
"   410.750 1.885 15057.74"
" Peak outflow 0.067 c.m/sec"
" Maximum level 409.645 metre"
" Maximum storage 28.760 c.m"
" Centroidal lag 26.034 hours"
"           0.054 0.069 0.067 0.000 c.m/sec"
40 HYDROGRAPH Next link "
"      5 Next link "
"           0.054 0.067 0.067 0.000"

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411009_2YR POST

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" 52      CHANNEL DESIGN"
"          0.067  Current peak flow    c.m/sec"
"          0.035  Manning 'n'"
"          0.      Cross-section type: 0=trapezoidal; 1=general"
"          0.000  Basewidth    metre"
"          7.410  Left bank slope"
"          6.000  Right bank slope"
"          0.950  Channel depth    metre"
"          1.040  Gradient    %"
"          Depth of flow                0.142    metre"
"          Velocity                      0.496    m/sec"
"          Channel capacity              10.655    c.m/sec"
"          Critical depth                0.115    metre"
" 53      ROUTE    Channel Route 72"
"          72.40  Channel Route 72 Reach length (metre)"
"          0.465  X-factor <= 0.5"
"          109.515 K-lag (seconds)"
"          0.000  Default(0) or user spec.(1) values used"
"          0.500  X-factor <= 0.5"
"          30.000  K-lag (seconds)"
"          0.500  Beta weighting factor"
"          100.000 Routing time step (seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                0.067    c.m/sec"
"          0.054    0.067    0.067    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5      Next link "
"          0.054    0.067    0.067    0.000"
" 52      CHANNEL DESIGN"
"          0.067  Current peak flow    c.m/sec"
"          0.035  Manning 'n'"
"          0.      Cross-section type: 0=trapezoidal; 1=general"
"          2.000  Basewidth    metre"
"          2.950  Left bank slope"
"          3.000  Right bank slope"
"          0.950  Channel depth    metre"
"          1.040  Gradient    %"
"          Depth of flow                0.067    metre"
"          Velocity                      0.452    m/sec"
"          Channel capacity              9.246    c.m/sec"
"          Critical depth                0.047    metre"
" 53      ROUTE    Channel Route 40"
"          39.80  Channel Route 40 Reach length (metre)"
"          0.452  X-factor <= 0.5"
"          66.003 K-lag (seconds)"
"          0.000  Default(0) or user spec.(1) values used"
"          0.500  X-factor <= 0.5"
"          30.000  K-lag (seconds)"
"          0.500  Beta weighting factor"
"          60.000 Routing time step (seconds)"
"          1      No. of sub-reaches"
"          Peak outflow                0.067    c.m/sec"
"          0.054    0.067    0.067    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5      Next link "
"          0.054    0.067    0.067    0.000"
" 40      HYDROGRAPH Copy to Outflow"
"          8      Copy to Outflow"
"          0.054    0.067    0.067    0.000"
" 64      SHOW TABLE"
"          2      Flow hydrograph"
"          4      Inflow Hydrograph"
"          Maximum flow                0.067    c.m/sec"

```

```

411009_2YR POST
" Hydrograph volume 2377.747 c.m"
" 40 HYDROGRAPH Combine 999"
" 6 Combine "
" 999 Node #"
" To wetland"
" Maximum flow 0.067 c.m/sec"
" Hydrograph volume 2377.747 c.m"
" 0.054 0.067 0.067 0.067"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.054 0.000 0.067 0.067"
" 47 FILEI_0 Read/Open DIV03100.002hyd"
" 1 1=read/open; 2=write/save"
" 2 1=rainfall; 2=hydrograph"
" 1 1=runoff; 2=inflow; 3=outflow; 4=junction"
" DIV03100.002hyd"
" Major flow at 3100"
" Total volume 0.000 c.m"
" Maximum flow 0.000 c.m/sec"
" 0.000 0.000 0.067 0.067 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.000 0.000 0.067 0.067"
" 33 CATCHMENT 3200"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 3200 Catchment 3200"
" 60.000 % Impervious"
" 0.350 Total Area"
" 20.000 Flow length"
" 2.000 Overland slope"
" 0.140 Pervious Area"
" 20.000 Pervious length"
" 2.000 Pervious slope"
" 0.210 Impervious Area"
" 20.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 78.000 Pervious SCS Curve No."
" 0.207 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 7.164 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.838 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.038 0.000 0.067 0.067 c.m/sec"
" Catchment 3200 Pervious Impervious Total Area "
" Surface Area 0.140 0.210 0.350 hectare"
" Time of concentration 20.437 1.868 4.499 minutes"
" Time to Centroid 125.085 88.659 93.819 minutes"
" Rainfall depth 33.014 33.014 33.014 mm"
" Rainfall volume 46.22 69.33 115.55 c.m"
" Rainfall losses 26.169 5.363 13.685 mm"
" Runoff depth 6.845 27.651 19.329 mm"
" Runoff volume 9.58 58.07 67.65 c.m"
" Runoff coefficient 0.207 0.838 0.585 "
" Maximum flow 0.003 0.038 0.038 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.038 0.038 0.067 0.067"

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" 33      CATCHMENT 3300"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          3300 Catchment 3300"
"          60.000 % Impervious"
"          0.220 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.088 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.132 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.207 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.838 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"              0.024      0.038      0.067      0.067 c.m/sec"
"          Catchment 3300      Pervious      Impervious      Total Area      "
"          Surface Area      0.088      0.132      0.220      hectare"
"          Time of concentration      20.437      1.868      4.499      minutes"
"          Time to Centroid      125.085      88.659      93.819      minutes"
"          Rainfall depth      33.014      33.014      33.014      mm"
"          Rainfall volume      29.05      43.58      72.63      c.m"
"          Rainfall losses      26.169      5.363      13.685      mm"
"          Runoff depth      6.845      27.651      19.329      mm"
"          Runoff volume      6.02      36.50      42.52      c.m"
"          Runoff coefficient      0.207      0.838      0.585      "
"          Maximum flow      0.002      0.024      0.024      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.024      0.061      0.067      0.067"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.024      0.061      0.061      0.067"
" 64      SHOW TABLE"
"          2  Flow hydrograph"
"          4  Inflow Hydrograph"
"          Maximum flow      0.061      c.m/sec"
"          Hydrograph volume      110.173      c.m"
" 40      HYDROGRAPH Combine 999"
"          6  Combine "
"          999 Node #"
"          To Wetland"
"          Maximum flow      0.073      c.m/sec"
"          Hydrograph volume      2487.915      c.m"
"              0.024      0.061      0.061      0.073"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.024      0.000      0.061      0.073"
" 33      CATCHMENT 2100"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2100 Catchment 2100"

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411009_2YR POST

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"      60.000  % Impervious"
"      2.180  Total Area"
"      40.000  Flow length"
"      2.000  overland slope"
"      0.872  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      1.308  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.207  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.834  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"              0.248  0.000  0.061  0.073 c.m/sec"
"      Catchment 2100  Pervious  Impervious  Total Area  "
"      Surface Area  0.872  1.308  2.180  hectare"
"      Time of concentration  30.977  2.832  6.834  minutes"
"      Time to Centroid  137.612  90.217  96.956  minutes"
"      Rainfall depth  33.014  33.014  33.014  mm"
"      Rainfall volume  287.88  431.82  719.70  c.m"
"      Rainfall losses  26.164  5.467  13.746  mm"
"      Runoff depth  6.850  27.547  19.268  mm"
"      Runoff volume  59.73  360.32  420.05  c.m"
"      Runoff coefficient  0.207  0.834  0.584  "
"      Maximum flow  0.013  0.245  0.248  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"              0.248  0.248  0.061  0.073"
" 54  POND DESIGN"
"      0.248  Current peak flow  c.m/sec"
"      0.396  Target outflow  c.m/sec"
"      420.0  Hydrograph volume  c.m"
"      13.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"      Level Discharge  Volume"
"      410.650  0.000  0.000"
"      410.700  0.00600  25.000"
"      410.800  0.01300  79.000"
"      410.900  0.02000  139.000"
"      411.000  0.02500  206.000"
"      411.100  0.02900  279.000"
"      411.200  0.03200  359.000"
"      411.300  0.2910  446.000"
"      411.400  0.3210  540.000"
"      411.500  0.3470  641.000"
"      411.600  0.3720  750.000"
"      411.650  0.3840  807.000"
"      411.950  2.818  1195.000"
"      Peak outflow  0.028  c.m/sec"
"      Maximum level  411.064  metre"
"      Maximum storage  252.820  c.m"
"      Centroidal lag  3.690  hours"
"              0.248  0.248  0.028  0.073 c.m/sec"

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" 40      HYDROGRAPH Next link "
"          5 Next link "
"              0.248      0.028      0.028      0.073"
" 33      CATCHMENT 2200"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2200 Catchment 2200"
"          0.000 % Impervious"
"          0.910 Total Area"
"          40.000 Flow length"
"          2.000 Overland Slope"
"          0.910 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.207 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"              0.014      0.028      0.028      0.073 c.m/sec"
"          Catchment 2200      Pervious      Impervious      Total Area "
"          Surface Area      0.910      0.000      0.910      hectare"
"          Time of concentration      30.977      2.832      30.977      minutes"
"          Time to Centroid      137.612      90.217      137.612      minutes"
"          Rainfall depth      33.014      33.014      33.014      mm"
"          Rainfall volume      300.43      0.00      300.43      c.m"
"          Rainfall losses      26.164      5.467      26.164      mm"
"          Runoff depth      6.850      27.547      6.850      mm"
"          Runoff volume      62.33      0.00      62.33      c.m"
"          Runoff coefficient      0.207      0.000      0.207      "
"          Maximum flow      0.014      0.000      0.014      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"              0.014      0.041      0.028      0.073"
" 33      CATCHMENT 2300"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2300 Catchment 2300"
"          10.000 % Impervious"
"          0.470 Total Area"
"          40.000 Flow length"
"          2.000 Overland Slope"
"          0.423 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.047 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.207 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"

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"      0.015  Impervious Manning 'n'"
" 98.000  Impervious SCS Curve No."
"      0.834  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"              0.010      0.041      0.028      0.073 c.m/sec"
"      Catchment 2300      Pervious      Impervious      Total Area      "
"      Surface Area      0.423      0.047      0.470      hectare"
"      Time of concentration      30.977      2.832      22.285      minutes"
"      Time to Centroid      137.612      90.217      122.974      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      139.65      15.52      155.16      c.m"
"      Rainfall losses      26.164      5.467      24.094      mm"
"      Runoff depth      6.850      27.547      8.919      mm"
"      Runoff volume      28.97      12.95      41.92      c.m"
"      Runoff coefficient      0.207      0.834      0.270      "
"      Maximum flow      0.006      0.009      0.010      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"              0.010      0.048      0.028      0.073"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"              0.010      0.048      0.048      0.073"
" 64  SHOW TABLE"
"      2  Flow hydrograph"
"      4  Inflow Hydrograph"
"      Maximum flow              0.048      c.m/sec"
"      Hydrograph volume      524.342      c.m"
" 40  HYDROGRAPH Combine 999"
"      6  Combine "
" 999  Node #"
"      To wetland"
"      Maximum flow              0.116      c.m/sec"
"      Hydrograph volume      3012.260      c.m"
"              0.010      0.048      0.048      0.116"
" 40  HYDROGRAPH Confluence 999"
"      7  Confluence "
" 999  Node #"
"      To wetland"
"      Maximum flow              0.116      c.m/sec"
"      Hydrograph volume      3012.260      c.m"
"              0.010      0.116      0.048      0.000"
" 38  START/RE-START TOTALS 999"
"      3  Runoff Totals on EXIT"
"      Total Catchment area              22.620      hectare"
"      Total Impervious area              7.577      hectare"
"      Total % impervious              33.497"
" 19  EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\akroetsch\Documents\
"          Work in Progress\411009 Ainley Farm SWM Junk"
"          Output filename:                    411009_5YR POST.out"
"          Licensee name:                      gmbp"
"          Company                            Hewlett-Packard Company"
"          Date & Time last used:              4/17/2017 at 3:43:15 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1      Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850   Exponent C"
"          0.380   Fraction R"
"          180.000 Duration"
"          1.000   Time step multiplier"
"          Maximum intensity          113.586   mm/hr"
"          Total depth                49.792   mm"
"          6 005hyd Hydrograph extension used in this file"
" 33      CATCHMENT 3100"
"          1      Triangular SCS"
"          1      Equal length"
"          1      SCS method"
"          3100   Catchment 3100"
"          60.000 % Impervious"
"          0.400  Total Area"
"          20.000 Flow length"
"          2.000  Overland Slope"
"          0.160  Pervious Area"
"          20.000 Pervious length"
"          2.000  Pervious slope"
"          0.240  Impervious Area"
"          20.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.319  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.883  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.062  0.000  0.000  0.000 c.m/sec"
"          Catchment 3100 Pervious Impervious Total Area "
"          Surface Area 0.160 0.240 0.400 hectare"
"          Time of concentration 14.957 1.691 4.264 minutes"
"          Time to Centroid 114.999 87.210 92.599 minutes"
"          Rainfall depth 49.792 49.792 49.792 mm"
"          Rainfall volume 79.67 119.50 199.17 c.m"
"          Rainfall losses 33.921 5.811 17.055 mm"
"          Runoff depth 15.871 43.981 32.737 mm"
"          Runoff volume 25.39 105.55 130.95 c.m"
"          Runoff coefficient 0.319 0.883 0.657 "
"          Maximum flow 0.009 0.059 0.062 c.m/sec"
" 40      HYDROGRAPH Add Runoff "

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"      4  Add Runoff "
"      0.062      0.062      0.000      0.000"
" 56  DIVERSION"
"      3100  Node number"
"      0.062  overflow threshold"
"      1.000  Required diverted fraction"
"      0      Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow      0.000      c.m/sec"
"      Volume of diverted flow      0.000      c.m"
"      DIV03100.005hyd"
"      Major flow at 3100"
"      0.062      0.062      0.062      0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5      Next link "
"      0.062      0.062      0.062      0.000"
" 33  CATCHMENT 1000"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      1000   Catchment 1000"
"      50.000 % Impervious"
"      11.280 Total Area"
"      20.000 Flow length"
"      2.000  overland Slope"
"      5.640  Pervious Area"
"      20.000 Pervious length"
"      2.000  Pervious slope"
"      5.640  Impervious Area"
"      20.000 Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.883  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      1.493      0.062      0.062      0.000 c.m/sec"
"      Catchment 1000      Pervious      Impervious      Total Area "
"      Surface Area      5.640      5.640      11.280      hectare"
"      Time of concentration      14.957      1.691      5.209      minutes"
"      Time to Centroid      115.000      87.210      94.579      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      2808.25      2808.25      5616.50      c.m"
"      Rainfall losses      33.921      5.811      19.866      mm"
"      Runoff depth      15.871      43.981      29.926      mm"
"      Runoff volume      895.12      2480.51      3375.63      c.m"
"      Runoff coefficient      0.319      0.883      0.601      "
"      Maximum flow      0.329      1.386      1.493      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"      1.493      1.555      0.062      0.000"
" 33  CATCHMENT 1100"
"      1      Triangular SCS"
"      1      Equal length"
"      1      SCS method"
"      1100   Catchment 1100"
"      0.000  % Impervious"
"      0.470  Total Area"
"      20.000  Flow length"

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"      2.000  Overland Slope"
"      0.470  Pervious Area"
"     20.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.027  1.555  0.062  0.000 c.m/sec"
"      Catchment 1100  Pervious  Impervious  Total Area  "
"      Surface Area  0.470  0.000  0.470  hectare"
"      Time of concentration  14.957  1.691  14.957  minutes"
"      Time to Centroid  115.000  87.210  114.999  minutes"
"      Rainfall depth  49.792  49.792  49.792  mm"
"      Rainfall volume  234.02  0.00  234.02  c.m"
"      Rainfall losses  33.921  5.811  33.921  mm"
"      Runoff depth  15.871  43.981  15.871  mm"
"      Runoff volume  74.59  0.00  74.59  c.m"
"      Runoff coefficient  0.319  0.000  0.319  "
"      Maximum flow  0.027  0.000  0.027  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.027  1.565  0.062  0.000"
" 54  POND DESIGN"
"      1.565  Current peak flow  c.m/sec"
"      0.756  Target outflow  c.m/sec"
"     3581.2  Hydrograph volume  c.m"
"      12.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"          Level  Discharge  Volume"
"         411.000  0.000  0.000"
"         411.100  0.00600  612.000"
"         411.200  0.01300  1233.000"
"         411.300  0.01700  1862.000"
"         411.400  0.02000  2501.000"
"         411.500  0.02300  3148.000"
"         411.600  0.02600  3805.000"
"         411.700  0.1570  4470.000"
"         411.750  0.1630  4806.000"
"         411.800  0.2450  5144.000"
"         411.850  0.2520  5485.000"
"         412.000  1.912  6295.000"
"      Peak outflow  0.024  c.m/sec"
"      Maximum level  411.535  metre"
"      Maximum storage  3379.855  c.m"
"      Centroidal lag  32.923  hours"
"          0.027  1.565  0.024  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"          0.027  0.024  0.024  0.000"
" 33  CATCHMENT 4000"

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```

"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     4000 Catchment 4000"
"     0.000 % Impervious"
"     6.340 Total Area"
"    60.000 Flow length"
"     2.000 Overland Slope"
"     6.340 Pervious Area"
"    60.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    60.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.258 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.186 0.024 0.024 0.000 c.m/sec"
"      Catchment 4000 Pervious Impervious Total Area "
"      Surface Area 6.340 0.000 6.340 hectare"
"      Time of concentration 31.736 3.269 31.736 minutes"
"      Time to Centroid 136.024 89.581 136.024 minutes"
"      Rainfall depth 49.792 49.792 49.792 mm"
"      Rainfall volume 3156.79 0.00 3156.79 c.m"
"      Rainfall losses 36.968 6.236 36.968 mm"
"      Runoff depth 12.824 43.556 12.824 mm"
"      Runoff volume 813.01 0.00 813.02 c.m"
"      Runoff coefficient 0.258 0.000 0.258 "
"      Maximum flow 0.186 0.000 0.186 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.186 0.206 0.024 0.000"
" 54 POND DESIGN"
"     0.206 Current peak flow c.m/sec"
"     0.756 Target outflow c.m/sec"
"    4390.5 Hydrograph volume c.m"
"     6. Number of stages"
"    409.630 Minimum water level metre"
"    410.750 Maximum water level metre"
"    409.630 Starting water level metre"
"     0 Keep Design Data: 1 = True; 0 = False"
"       Level Discharge Volume"
"     409.630 0.000 0.000"
"     409.750 0.5400 232.250"
"     410.000 1.632 2017.880"
"     410.250 3.737 5148.940"
"     410.500 1.345 9472.330"
"     410.750 1.885 15057.74"
"       Peak outflow 0.195 c.m/sec"
"       Maximum level 409.673 metre"
"       Maximum storage 83.982 c.m"
"       Centroidal lag 27.365 hours"
"           0.186 0.206 0.195 0.000 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"           0.186 0.195 0.195 0.000"

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```

" 52      CHANNEL DESIGN"
"         0.195 Current peak flow    c.m/sec"
"         0.035 Manning 'n'"
"         0.    Cross-section type: 0=trapezoidal; 1=general"
"         0.000 Basewidth    metre"
"         7.410 Left bank slope"
"         6.000 Right bank slope"
"         0.950 Channel depth    metre"
"         1.040 Gradient    %"
"         Depth of flow                0.212    metre"
"         Velocity                      0.648    m/sec"
"         Channel capacity              10.655    c.m/sec"
"         Critical depth                0.177    metre"
" 53      ROUTE Channel Route 72"
"         72.40 Channel Route 72 Reach length (metre)"
"         0.447 X-factor <= 0.5"
"         83.846 K-lag (seconds)"
"         0.000 Default(0) or user spec.(1) values used"
"         0.500 X-factor <= 0.5"
"         30.000 K-lag (seconds)"
"         0.500 Beta weighting factor"
"         75.000 Routing time step (seconds)"
"         1 No. of sub-reaches"
"         Peak outflow                0.194    c.m/sec"
"         0.186 0.195 0.194 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"         5 Next link "
"         0.186 0.194 0.194 0.000"
" 52      CHANNEL DESIGN"
"         0.194 Current peak flow    c.m/sec"
"         0.035 Manning 'n'"
"         0.    Cross-section type: 0=trapezoidal; 1=general"
"         2.000 Basewidth    metre"
"         2.950 Left bank slope"
"         3.000 Right bank slope"
"         0.950 Channel depth    metre"
"         1.040 Gradient    %"
"         Depth of flow                0.125    metre"
"         Velocity                      0.654    m/sec"
"         Channel capacity              9.246    c.m/sec"
"         Critical depth                0.094    metre"
" 53      ROUTE Channel Route 40"
"         39.80 Channel Route 40 Reach length (metre)"
"         0.413 X-factor <= 0.5"
"         45.609 K-lag (seconds)"
"         0.000 Default(0) or user spec.(1) values used"
"         0.500 X-factor <= 0.5"
"         30.000 K-lag (seconds)"
"         0.500 Beta weighting factor"
"         50.000 Routing time step (seconds)"
"         1 No. of sub-reaches"
"         Peak outflow                0.193    c.m/sec"
"         0.186 0.194 0.193 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"         5 Next link "
"         0.186 0.193 0.193 0.000"
" 40      HYDROGRAPH Copy to Outflow"
"         8 Copy to Outflow"
"         0.186 0.193 0.193 0.000"
" 64      SHOW TABLE"
"         2 Flow hydrograph"
"         4 Inflow Hydrograph"
"         Maximum flow                0.193    c.m/sec"

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33      CATCHMENT 3300"
      1  Triangular SCS"
      1  Equal length"
      1  SCS method"
      3300 Catchment 3300"
60.000 % Impervious"
      0.220 Total Area"
20.000 Flow length"
      2.000 Overland Slope"
      0.088 Pervious Area"
20.000 Pervious length"
      2.000 Pervious slope"
      0.132 Impervious Area"
20.000 Impervious length"
      2.000 Impervious slope"
      0.250 Pervious Manning 'n'"
78.000 Pervious SCS Curve No."
      0.319 Pervious Runoff coefficient"
      0.100 Pervious Ia/S coefficient"
      7.164 Pervious Initial abstraction"
      0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
      0.883 Impervious Runoff coefficient"
      0.100 Impervious Ia/S coefficient"
      0.518 Impervious Initial abstraction"
      0.034 0.054 0.193 0.193 c.m/sec"
      Catchment 3300 Pervious Impervious Total Area "
      Surface Area 0.088 0.132 0.220 hectare"
      Time of concentration 14.957 1.691 4.264 minutes"
      Time to Centroid 115.000 87.210 92.599 minutes"
      Rainfall depth 49.792 49.792 49.792 mm"
      Rainfall volume 43.82 65.73 109.54 c.m"
      Rainfall losses 33.921 5.811 17.055 mm"
      Runoff depth 15.871 43.981 32.737 mm"
      Runoff volume 13.97 58.05 72.02 c.m"
      Runoff coefficient 0.319 0.883 0.657 "
      Maximum flow 0.005 0.032 0.034 c.m/sec"
40      HYDROGRAPH Add Runoff "
      4  Add Runoff "
      0.034 0.088 0.193 0.193"
40      HYDROGRAPH Copy to Outflow"
      8  Copy to Outflow"
      0.034 0.088 0.088 0.193"
64      SHOW TABLE"
      2  Flow hydrograph"
      4  Inflow Hydrograph"
      Maximum flow 0.088 c.m/sec"
      Hydrograph volume 186.600 c.m"
40      HYDROGRAPH Combine 999"
      6  Combine "
999  Node #"
      To Wetland"
      Maximum flow 0.208 c.m/sec"
      Hydrograph volume 4577.066 c.m"
      0.034 0.088 0.088 0.208"
40      HYDROGRAPH Start - New Tributary"
      2  Start - New Tributary"
      0.034 0.000 0.088 0.208"
33      CATCHMENT 2100"
      1  Triangular SCS"
      1  Equal length"
      1  SCS method"
2100  Catchment 2100"

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"      60.000  % Impervious"
"      2.180  Total Area"
"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.872  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      1.308  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.319  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.878  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.338  0.000  0.088  0.208 c.m/sec"
"      Catchment 2100  Pervious  Impervious  Total Area  "
"      Surface Area  0.872  1.308  2.180  hectare"
"      Time of concentration  22.670  2.563  6.484  minutes"
"      Time to Centroid  124.006  88.517  95.437  minutes"
"      Rainfall depth  49.792  49.792  49.792  mm"
"      Rainfall volume  434.18  651.28  1085.46  c.m"
"      Rainfall losses  33.904  6.066  17.201  mm"
"      Runoff depth  15.888  43.726  32.591  mm"
"      Runoff volume  138.54  571.94  710.48  c.m"
"      Runoff coefficient  0.319  0.878  0.655  "
"      Maximum flow  0.040  0.328  0.338  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.338  0.338  0.088  0.208"
" 54  POND DESIGN"
"      0.338  Current peak flow  c.m/sec"
"      0.396  Target outflow  c.m/sec"
"      710.5  Hydrograph volume  c.m"
"      13.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge  Volume"
"          410.650  0.000  0.000"
"          410.700  0.00600  25.000"
"          410.800  0.01300  79.000"
"          410.900  0.02000  139.000"
"          411.000  0.02500  206.000"
"          411.100  0.02900  279.000"
"          411.200  0.03200  359.000"
"          411.300  0.2910  446.000"
"          411.400  0.3210  540.000"
"          411.500  0.3470  641.000"
"          411.600  0.3720  750.000"
"          411.650  0.3840  807.000"
"          411.950  2.818  1195.000"
"      Peak outflow  0.103  c.m/sec"
"      Maximum level  411.227  metre"
"      Maximum storage  382.700  c.m"
"      Centroidal lag  3.642  hours"
"          0.338  0.338  0.103  0.208 c.m/sec"

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" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.338    0.103    0.103    0.208"
" 33      CATCHMENT 2200"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2200 Catchment 2200"
"          0.000 % Impervious"
"          0.910 Total Area"
"          40.000 Flow length"
"          2.000 Overland Slope"
"          0.910 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.319 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.042    0.103    0.103    0.208 c.m/sec"
"          Catchment 2200 Pervious Impervious Total Area "
"          Surface Area 0.910 0.000 0.910 hectare"
"          Time of concentration 22.670 2.563 22.670 minutes"
"          Time to Centroid 124.006 88.517 124.005 minutes"
"          Rainfall depth 49.792 49.792 49.792 mm"
"          Rainfall volume 453.10 0.00 453.10 c.m"
"          Rainfall losses 33.904 6.066 33.904 mm"
"          Runoff depth 15.888 43.726 15.888 mm"
"          Runoff volume 144.58 0.00 144.58 c.m"
"          Runoff coefficient 0.319 0.000 0.319 "
"          Maximum flow 0.042 0.000 0.042 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"          0.042    0.141    0.103    0.208"
" 33      CATCHMENT 2300"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2300 Catchment 2300"
"          10.000 % Impervious"
"          0.470 Total Area"
"          40.000 Flow length"
"          2.000 Overland Slope"
"          0.423 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.047 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.319 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"

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"	0.015	Impervious Manning 'n'"				
"	98.000	Impervious SCS Curve No."				
"	0.878	Impervious Runoff coefficient"				
"	0.100	Impervious Ia/S coefficient"				
"	0.518	Impervious Initial abstraction"				
"		0.023	0.141	0.103	0.208	c.m/sec"
"		Catchment 2300	Pervious	Impervious	Total Area	"
"		Surface Area	0.423	0.047	0.470	hectare"
"		Time of concentration	22.670	2.563	17.961	minutes"
"		Time to Centroid	124.006	88.517	115.695	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	210.62	23.40	234.02	c.m"
"		Rainfall losses	33.904	6.066	31.120	mm"
"		Runoff depth	15.888	43.726	18.672	mm"
"		Runoff volume	67.21	20.55	87.76	c.m"
"		Runoff coefficient	0.319	0.878	0.375	"
"		Maximum flow	0.019	0.012	0.023	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.023	0.162	0.103	0.208"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.023	0.162	0.162	0.208"	
" 64		SHOW TABLE"				
"	2	Flow hydrograph"				
"	4	Inflow Hydrograph"				
"		Maximum flow		0.162	c.m/sec"	
"		Hydrograph volume		944.229	c.m"	
" 40		HYDROGRAPH Combine	999"			
"	6	Combine "				
"	999	Node #"				
"		To Wetland"				
"		Maximum flow		0.349	c.m/sec"	
"		Hydrograph volume		5521.301	c.m"	
"		0.023	0.162	0.162	0.349"	
" 40		HYDROGRAPH Confluence	999"			
"	7	Confluence "				
"	999	Node #"				
"		To Wetland"				
"		Maximum flow		0.349	c.m/sec"	
"		Hydrograph volume		5521.301	c.m"	
"		0.023	0.349	0.162	0.000"	
" 38		START/RE-START TOTALS 999"				
"	3	Runoff Totals on EXIT"				
"		Total Catchment area			22.620	hectare"
"		Total Impervious area			7.577	hectare"
"		Total % impervious			33.497"	
" 19		EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        C:\Users\akroetsch\Documents\
"          Work in Progress\411009 Ainley Farm SWM Junk"
"          Output filename:                   411009_10YR POST.out"
"          Licensee name:                     gmbp"
"          Company                            Hewlett-Packard Company"
"          Date & Time last used:            4/17/2017 at 3:43:59 PM"
" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          2327.596 Coefficient A"
"          19.500 Constant B"
"          0.894 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                  126.171 mm/hr"
"          Total depth                       61.359 mm"
"          6 010hyd Hydrograph extension used in this file"
" 33      CATCHMENT 3100"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          3100 Catchment 3100"
"          60.000 % Impervious"
"          0.400 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.160 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.240 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.379 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.901 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.075 0.000 0.000 0.000 c.m/sec"
"          Catchment 3100 Pervious Impervious Total Area "
"          Surface Area 0.160 0.240 0.400 hectare"
"          Time of concentration 13.094 1.611 4.125 minutes"
"          Time to Centroid 111.234 86.563 91.965 minutes"
"          Rainfall depth 61.359 61.359 61.359 mm"
"          Rainfall volume 98.17 147.26 245.44 c.m"
"          Rainfall losses 38.098 6.044 18.866 mm"
"          Runoff depth 23.262 55.315 42.494 mm"
"          Runoff volume 37.22 132.76 169.97 c.m"
"          Runoff coefficient 0.379 0.901 0.693 "
"          Maximum flow 0.014 0.069 0.075 c.m/sec"
" 40      HYDROGRAPH Add Runoff "

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"      4  Add Runoff "
"      0.075      0.075      0.000      0.000"
" 56  DIVERSION"
"      3100 Node number"
"      0.062 Overflow threshold"
"      1.000 Required diverted fraction"
"      0 Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow      0.013      c.m/sec"
"      Volume of diverted flow      6.527      c.m"
"      DIV03100.010hyd"
"      Major flow at 3100"
"      0.075      0.075      0.062      0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5 Next link "
"      0.075      0.062      0.062      0.000"
" 33  CATCHMENT 1000"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      1000 Catchment 1000"
"      50.000 % Impervious"
"      11.280 Total Area"
"      20.000 Flow length"
"      2.000 overland Slope"
"      5.640 Pervious Area"
"      20.000 Pervious length"
"      2.000 Pervious slope"
"      5.640 Impervious Area"
"      20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.379 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.901 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      1.840      0.062      0.062      0.000 c.m/sec"
"      Catchment 1000      Pervious      Impervious      Total Area "
"      Surface Area      5.640      5.640      11.280      hectare"
"      Time of concentration      13.094      1.611      5.010      minutes"
"      Time to Centroid      111.234      86.563      93.866      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      3460.66      3460.66      6921.32      c.m"
"      Rainfall losses      38.098      6.044      22.071      mm"
"      Runoff depth      23.262      55.315      39.288      mm"
"      Runoff volume      1311.96      3119.77      4431.73      c.m"
"      Runoff coefficient      0.379      0.901      0.640      "
"      Maximum flow      0.509      1.619      1.840      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      1.840      1.902      0.062      0.000"
" 33  CATCHMENT 1100"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      1100 Catchment 1100"
"      0.000 % Impervious"
"      0.470 Total Area"
"      20.000 Flow length"

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"      2.000  Overland Slope"
"      0.470  Pervious Area"
"     20.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.379  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.042  1.902  0.062  0.000 c.m/sec"
"      Catchment 1100  Pervious  Impervious  Total Area  "
"      Surface Area      0.470  0.000  0.470  hectare"
"      Time of concentration  13.094  1.611  13.094  minutes"
"      Time to Centroid      111.234  86.563  111.234  minutes"
"      Rainfall depth        61.359  61.359  61.359  mm"
"      Rainfall volume       288.39  0.00  288.39  c.m"
"      Rainfall losses       38.098  6.044  38.097  mm"
"      Runoff depth          23.262  55.315  23.262  mm"
"      Runoff volume         109.33  0.00  109.33  c.m"
"      Runoff coefficient     0.379  0.000  0.379  "
"      Maximum flow          0.042  0.000  0.042  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.042  1.922  0.062  0.000"
" 54  POND DESIGN"
"      1.922  Current peak flow  c.m/sec"
"      0.756  Target outflow  c.m/sec"
"     4704.5  Hydrograph volume  c.m"
"      12.    Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge  Volume"
"         411.000  0.000  0.000"
"         411.100  0.00600  612.000"
"         411.200  0.01300  1233.000"
"         411.300  0.01700  1862.000"
"         411.400  0.02000  2501.000"
"         411.500  0.02300  3148.000"
"         411.600  0.02600  3805.000"
"         411.700  0.1570  4470.000"
"         411.750  0.1630  4806.000"
"         411.800  0.2450  5144.000"
"         411.850  0.2520  5485.000"
"         412.000  1.912  6295.000"
"      Peak outflow      0.108  c.m/sec"
"      Maximum level     411.663  metre"
"      Maximum storage   4221.662  c.m"
"      Centroidal lag    30.799  hours"
"          0.042  1.922  0.108  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"          0.042  0.108  0.108  0.000"
" 33  CATCHMENT 4000"

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"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     4000 Catchment 4000"
"     0.000 % Impervious"
"     6.340 Total Area"
"    60.000 Flow length"
"     2.000 Overland Slope"
"     6.340 Pervious Area"
"    60.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    60.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.316 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.320 0.108 0.108 0.000 c.m/sec"
"      Catchment 4000 Pervious Impervious Total Area "
"      Surface Area 6.340 0.000 6.340 hectare"
"      Time of concentration 27.416 3.114 27.416 minutes"
"      Time to Centroid 128.990 88.727 128.990 minutes"
"      Rainfall depth 61.359 61.359 61.359 mm"
"      Rainfall volume 3890.17 0.00 3890.18 c.m"
"      Rainfall losses 41.968 6.469 41.968 mm"
"      Runoff depth 19.392 54.890 19.392 mm"
"      Runoff volume 1229.42 0.00 1229.42 c.m"
"      Runoff coefficient 0.316 0.000 0.316 "
"      Maximum flow 0.320 0.000 0.320 c.m/sec"
40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.320 0.343 0.108 0.000"
54 POND DESIGN"
"      0.343 Current peak flow c.m/sec"
"      0.756 Target outflow c.m/sec"
"     5929.3 Hydrograph volume c.m"
"         6. Number of stages"
"    409.630 Minimum water level metre"
"    410.750 Maximum water level metre"
"    409.630 Starting water level metre"
"         0 Keep Design Data: 1 = True; 0 = False"
"           Level Discharge Volume"
"    409.630 0.000 0.000"
"    409.750 0.5400 232.250"
"    410.000 1.632 2017.880"
"    410.250 3.737 5148.940"
"    410.500 1.345 9472.330"
"    410.750 1.885 15057.74"
"           Peak outflow 0.319 c.m/sec"
"           Maximum level 409.701 metre"
"           Maximum storage 137.358 c.m"
"           Centroidal lag 24.978 hours"
"           0.320 0.343 0.319 0.000 c.m/sec"
40 HYDROGRAPH Next link "
"      5 Next link "
"           0.320 0.319 0.319 0.000"

```

411009_10YR POST

```

" 52      CHANNEL DESIGN"
"          0.319 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          0.000 Basewidth    metre"
"          7.410 Left bank slope"
"          6.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.255    metre"
"          Velocity                      0.732    m/sec"
"          Channel capacity              10.655    c.m/sec"
"          Critical depth                0.215    metre"
" 53      ROUTE Channel Route 72"
"          72.40 Channel Route 72 Reach length (metre)"
"          0.437 X-factor <= 0.5"
"          74.139 K-lag (seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag (seconds)"
"          0.500 Beta weighting factor"
"          75.000 Routing time step (seconds)"
"          1    No. of sub-reaches"
"          Peak outflow                0.316    c.m/sec"
"          0.320    0.319    0.316    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5    Next link "
"          0.320    0.316    0.316    0.000"
" 52      CHANNEL DESIGN"
"          0.316 Current peak flow    c.m/sec"
"          0.035 Manning 'n'"
"          0.    Cross-section type: 0=trapezoidal; 1=general"
"          2.000 Basewidth    metre"
"          2.950 Left bank slope"
"          3.000 Right bank slope"
"          0.950 Channel depth    metre"
"          1.040 Gradient    %"
"          Depth of flow                0.165    metre"
"          Velocity                      0.768    m/sec"
"          Channel capacity              9.246    c.m/sec"
"          Critical depth                0.128    metre"
" 53      ROUTE Channel Route 40"
"          39.80 Channel Route 40 Reach length (metre)"
"          0.387 X-factor <= 0.5"
"          38.845 K-lag (seconds)"
"          0.000 Default(0) or user spec.(1) values used"
"          0.500 X-factor <= 0.5"
"          30.000 K-lag (seconds)"
"          0.500 Beta weighting factor"
"          42.857 Routing time step (seconds)"
"          1    No. of sub-reaches"
"          Peak outflow                0.316    c.m/sec"
"          0.320    0.316    0.316    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5    Next link "
"          0.320    0.316    0.316    0.000"
" 40      HYDROGRAPH Copy to Outflow"
"          8    Copy to Outflow"
"          0.320    0.316    0.316    0.000"
" 64      SHOW TABLE"
"          2    Flow hydrograph"
"          4    Inflow Hydrograph"
"          Maximum flow                0.316    c.m/sec"

```

```

411009_10YR POST
" 40 Hydrograph volume 5929.319 c.m"
HYDROGRAPH Combine 999"
" 6 Combine "
" 999 Node #"
" To Wetland"
" Maximum flow 0.316 c.m/sec"
" Hydrograph volume 5929.319 c.m"
" 0.320 0.316 0.316 0.316"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.320 0.000 0.316 0.316"
" 47 FILEI_0 Read/Open DIV03100.010hyd"
" 1 1=read/open; 2=write/save"
" 2 1=rainfall; 2=hydrograph"
" 1 1=runoff; 2=inflow; 3=outflow; 4=junction"
" DIV03100.010hyd"
" Major flow at 3100"
" Total volume 6.527 c.m"
" Maximum flow 0.013 c.m/sec"
" 0.013 0.000 0.316 0.316 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.013 0.013 0.316 0.316"
" 33 CATCHMENT 3200"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 3200 Catchment 3200"
" 60.000 % Impervious"
" 0.350 Total Area"
" 20.000 Flow length"
" 2.000 Overland Slope"
" 0.140 Pervious Area"
" 20.000 Pervious length"
" 2.000 Pervious slope"
" 0.210 Impervious Area"
" 20.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 78.000 Pervious SCS Curve No."
" 0.379 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 7.164 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.901 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.066 0.013 0.316 0.316 c.m/sec"
" Catchment 3200 Pervious Impervious Total Area "
" Surface Area 0.140 0.210 0.350 hectare"
" Time of concentration 13.094 1.611 4.125 minutes"
" Time to Centroid 111.234 86.563 91.965 minutes"
" Rainfall depth 61.359 61.359 61.359 mm"
" Rainfall volume 85.90 128.85 214.76 c.m"
" Rainfall losses 38.098 6.044 18.866 mm"
" Runoff depth 23.262 55.315 42.494 mm"
" Runoff volume 32.57 116.16 148.73 c.m"
" Runoff coefficient 0.379 0.901 0.693 "
" Maximum flow 0.013 0.060 0.066 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.066 0.078 0.316 0.316"

```

411009_10YR POST

```

" 33      CATCHMENT 3300"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          3300 Catchment 3300"
"          60.000 % Impervious"
"          0.220 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.088 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.132 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.379 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.901 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.041 0.078 0.316 0.316 c.m/sec"
"          Catchment 3300 Pervious Impervious Total Area "
"          Surface Area 0.088 0.132 0.220 hectare"
"          Time of concentration 13.094 1.611 4.125 minutes"
"          Time to Centroid 111.234 86.563 91.965 minutes"
"          Rainfall depth 61.359 61.359 61.359 mm"
"          Rainfall volume 54.00 80.99 134.99 c.m"
"          Rainfall losses 38.098 6.044 18.866 mm"
"          Runoff depth 23.262 55.315 42.494 mm"
"          Runoff volume 20.47 73.02 93.49 c.m"
"          Runoff coefficient 0.379 0.901 0.693 "
"          Maximum flow 0.008 0.038 0.041 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.041 0.120 0.316 0.316"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"          0.041 0.120 0.120 0.316"
" 64      SHOW TABLE"
"          2  Flow hydrograph"
"          4  Inflow Hydrograph"
"          Maximum flow 0.120 c.m/sec"
"          Hydrograph volume 248.741 c.m"
" 40      HYDROGRAPH Combine 999"
"          6  Combine "
"          999 Node #"
"          To Wetland"
"          Maximum flow 0.338 c.m/sec"
"          Hydrograph volume 6178.059 c.m"
"          0.041 0.120 0.120 0.338"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.041 0.000 0.120 0.338"
" 33      CATCHMENT 2100"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2100 Catchment 2100"

```

411009_10YR POST

```

"      60.000  % Impervious"
"      2.180  Total Area"
"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.872  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      1.308  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.380  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.897  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      0.400  0.000  0.120  0.338 c.m/sec"
"      Catchment 2100  Pervious  Impervious  Total Area  "
"      Surface Area  0.872  1.308  2.180  hectare"
"      Time of concentration  19.847  2.441  6.272  minutes"
"      Time to Centroid  118.992  87.742  94.620  minutes"
"      Rainfall depth  61.359  61.359  61.359  mm"
"      Rainfall volume  535.05  802.58  1337.63  c.m"
"      Rainfall losses  38.054  6.310  19.008  mm"
"      Runoff depth  23.305  55.050  42.352  mm"
"      Runoff volume  203.22  720.05  923.27  c.m"
"      Runoff coefficient  0.380  0.897  0.690  "
"      Maximum flow  0.065  0.381  0.400  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.400  0.400  0.120  0.338"
" 54  POND DESIGN"
"      0.400  Current peak flow  c.m/sec"
"      0.396  Target outflow  c.m/sec"
"      923.3  Hydrograph volume  c.m"
"      13.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"      Level Discharge  Volume"
"      410.650  0.000  0.000"
"      410.700  0.00600  25.000"
"      410.800  0.01300  79.000"
"      410.900  0.02000  139.000"
"      411.000  0.02500  206.000"
"      411.100  0.02900  279.000"
"      411.200  0.03200  359.000"
"      411.300  0.2910  446.000"
"      411.400  0.3210  540.000"
"      411.500  0.3470  641.000"
"      411.600  0.3720  750.000"
"      411.650  0.3840  807.000"
"      411.950  2.818  1195.000"
"      Peak outflow  0.195  c.m/sec"
"      Maximum level  411.263  metre"
"      Maximum storage  413.673  c.m"
"      Centroidal lag  3.255  hours"
"      0.400  0.400  0.195  0.338 c.m/sec"

```

"	40	HYDROGRAPH Next link "						
"	"	5 Next link "						
"	"		0.400	0.195	0.195	0.338"		
"	33	CATCHMENT 2200"						
"	"	1 Triangular SCS"						
"	"	1 Equal length"						
"	"	1 SCS method"						
"	"	2200 Catchment 2200"						
"	"	0.000 % Impervious"						
"	"	0.910 Total Area"						
"	"	40.000 Flow length"						
"	"	2.000 Overland Slope"						
"	"	0.910 Pervious Area"						
"	"	40.000 Pervious length"						
"	"	2.000 Pervious slope"						
"	"	0.000 Impervious Area"						
"	"	40.000 Impervious length"						
"	"	2.000 Impervious slope"						
"	"	0.250 Pervious Manning 'n'"						
"	"	78.000 Pervious SCS Curve No."						
"	"	0.380 Pervious Runoff coefficient"						
"	"	0.100 Pervious Ia/S coefficient"						
"	"	7.164 Pervious Initial abstraction"						
"	"	0.015 Impervious Manning 'n'"						
"	"	98.000 Impervious SCS Curve No."						
"	"	0.000 Impervious Runoff coefficient"						
"	"	0.100 Impervious Ia/S coefficient"						
"	"	0.518 Impervious Initial abstraction"						
"	"		0.067	0.195	0.195	0.338 c.m/sec"		
"	"	Catchment 2200	Pervious	Impervious	Total Area	"		
"	"	Surface Area	0.910	0.000	0.910	hectare"		
"	"	Time of concentration	19.847	2.441	19.847	minutes"		
"	"	Time to Centroid	118.992	87.742	118.992	minutes"		
"	"	Rainfall depth	61.359	61.359	61.359	mm"		
"	"	Rainfall volume	558.37	0.00	558.37	c.m"		
"	"	Rainfall losses	38.054	6.310	38.054	mm"		
"	"	Runoff depth	23.305	55.050	23.305	mm"		
"	"	Runoff volume	212.07	0.00	212.07	c.m"		
"	"	Runoff coefficient	0.380	0.000	0.380	"		
"	"	Maximum flow	0.067	0.000	0.067	c.m/sec"		
"	40	HYDROGRAPH Add Runoff "						
"	"	4 Add Runoff "						
"	"		0.067	0.262	0.195	0.338"		
"	33	CATCHMENT 2300"						
"	"	1 Triangular SCS"						
"	"	1 Equal length"						
"	"	1 SCS method"						
"	"	2300 Catchment 2300"						
"	"	10.000 % Impervious"						
"	"	0.470 Total Area"						
"	"	40.000 Flow length"						
"	"	2.000 Overland Slope"						
"	"	0.423 Pervious Area"						
"	"	40.000 Pervious length"						
"	"	2.000 Pervious slope"						
"	"	0.047 Impervious Area"						
"	"	40.000 Impervious length"						
"	"	2.000 Impervious slope"						
"	"	0.250 Pervious Manning 'n'"						
"	"	78.000 Pervious SCS Curve No."						
"	"	0.380 Pervious Runoff coefficient"						
"	"	0.100 Pervious Ia/S coefficient"						
"	"	7.164 Pervious Initial abstraction"						

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```

"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.897  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"              0.036      0.262      0.195      0.338 c.m/sec"
"      Catchment 2300      Pervious      Impervious      Total Area      "
"      Surface Area      0.423      0.047      0.470      hectare"
"      Time of concentration      19.847      2.441      16.228      minutes"
"      Time to Centroid      118.992      87.742      112.496      minutes"
"      Rainfall depth      61.359      61.359      61.359      mm"
"      Rainfall volume      259.55      28.84      288.39      c.m"
"      Rainfall losses      38.054      6.310      34.880      mm"
"      Runoff depth      23.305      55.050      26.479      mm"
"      Runoff volume      98.58      25.87      124.45      c.m"
"      Runoff coefficient      0.380      0.897      0.432      "
"      Maximum flow      0.031      0.014      0.036      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.036      0.298      0.195      0.338"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"              0.036      0.298      0.298      0.338"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow              0.298      c.m/sec"
"      Hydrograph volume      1256.451      c.m"
" 40      HYDROGRAPH Combine      999"
"      6      Combine "
"      999      Node #"
"      To Wetland"
"      Maximum flow              0.566      c.m/sec"
"      Hydrograph volume      7434.511      c.m"
"              0.036      0.298      0.298      0.566"
" 40      HYDROGRAPH Confluence      999"
"      7      Confluence "
"      999      Node #"
"      To Wetland"
"      Maximum flow              0.566      c.m/sec"
"      Hydrograph volume      7434.511      c.m"
"              0.036      0.566      0.298      0.000"
" 38      START/RE-START TOTALS 999"
"      3      Runoff Totals on EXIT"
"      Total Catchment area              22.620      hectare"
"      Total Impervious area              7.577      hectare"
"      Total % impervious              33.497"
" 19      EXIT"

```

411009_25YR POST

```

" MIDUSS Output ----->"
" MIDUSS version Version 2.25 rev. 473"
" MIDUSS created Sunday, February 07, 2010"
" 10 Units used: ie METRIC"
" Job folder: C:\Users\akroetsch\Documents\
" Work in Progress\411009 Ainley Farm SWM Junk"
" Output filename: 411009_25YR POST.out"
" Licensee name: gmbp"
" Company Hewlett-Packard Company"
" Date & Time last used: 4/17/2017 at 3:44:40 PM"

```

```

" 31 TIME PARAMETERS"
" 5.000 Time Step"
" 180.000 Max. Storm length"
" 12000.000 Max. Hydrograph"
" 32 STORM Chicago storm"
" 1 Chicago storm"
" 3701.648 Coefficient A"
" 25.500 Constant B"
" 0.937 Exponent C"
" 0.380 Fraction R"
" 180.000 Duration"
" 1.000 Time step multiplier"
" Maximum intensity 143.371 mm/hr"
" Total depth 75.581 mm"
" 6 025hyd Hydrograph extension used in this file"

```

```

" 33 CATCHMENT 3100"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 3100 Catchment 3100"
" 60.000 % Impervious"
" 0.400 Total Area"
" 20.000 Flow length"
" 2.000 Overland Slope"
" 0.160 Pervious Area"
" 20.000 Pervious length"
" 2.000 Pervious slope"
" 0.240 Impervious Area"
" 20.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 78.000 Pervious SCS Curve No."
" 0.441 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 7.164 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.916 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"

```

	0.092	0.000	0.000	0.000 c.m/sec"	
Catchment 3100	Pervious	Impervious	Total Area		"
Surface Area	0.160	0.240	0.400		hectare"
Time of concentration	11.553	1.523	3.959		minutes"
Time to Centroid	108.042	85.984	91.342		minutes"
Rainfall depth	75.581	75.581	75.581		mm"
Rainfall volume	120.93	181.39	302.32		c.m"
Rainfall losses	42.253	6.330	20.699		mm"
Runoff depth	33.328	69.250	54.882		mm"
Runoff volume	53.32	166.20	219.53		c.m"
Runoff coefficient	0.441	0.916	0.726		"
Maximum flow	0.022	0.081	0.092		c.m/sec"

```

" 40 HYDROGRAPH Add Runoff "

```


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```

"      4  Add Runoff "
"      0.092  0.092  0.000  0.000"
" 56  DIVERSION"
"      3100 Node number"
"      0.062 Overflow threshold"
"      1.000 Required diverted fraction"
"      0 Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow 0.030 c.m/sec"
"      Volume of diverted flow 20.839 c.m"
"      DIV03100.025hyd"
"      Major flow at 3100"
"      0.092  0.092  0.062  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5 Next link "
"      0.092  0.062  0.062  0.000"
" 33  CATCHMENT 1000"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      1000 Catchment 1000"
"      50.000 % Impervious"
"      11.280 Total Area"
"      20.000 Flow length"
"      2.000 Overland Slope"
"      5.640 Pervious Area"
"      20.000 Pervious length"
"      2.000 Pervious slope"
"      5.640 Impervious Area"
"      20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.441 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.916 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      2.321  0.062  0.062  0.000 c.m/sec"
"      Catchment 1000 Pervious Impervious Total Area "
"      Surface Area 5.640 5.640 11.280 hectare"
"      Time of concentration 11.553 1.523 4.782 minutes"
"      Time to Centroid 108.042 85.984 93.151 minutes"
"      Rainfall depth 75.581 75.581 75.581 mm"
"      Rainfall volume 4262.76 4262.76 8525.52 c.m"
"      Rainfall losses 42.253 6.330 24.292 mm"
"      Runoff depth 33.328 69.250 51.289 mm"
"      Runoff volume 1879.71 3905.72 5785.43 c.m"
"      Runoff coefficient 0.441 0.916 0.679 "
"      Maximum flow 0.765 1.907 2.321 c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      2.321  2.383  0.062  0.000"
" 33  CATCHMENT 1100"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      1100 Catchment 1100"
"      0.000 % Impervious"
"      0.470 Total Area"
"      20.000 Flow length"

```

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```

"      2.000  Overland Slope"
"      0.470  Pervious Area"
"     20.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.441  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.064  2.383  0.062  0.000 c.m/sec"
"      Catchment 1100  Pervious  Impervious  Total Area  "
"      Surface Area  0.470  0.000  0.470  hectare"
"      Time of concentration  11.553  1.523  11.553  minutes"
"      Time to Centroid  108.042  85.984  108.042  minutes"
"      Rainfall depth  75.581  75.581  75.581  mm"
"      Rainfall volume  355.23  0.00  355.23  c.m"
"      Rainfall losses  42.253  6.330  42.253  mm"
"      Runoff depth  33.328  69.250  33.328  mm"
"      Runoff volume  156.64  0.00  156.64  c.m"
"      Runoff coefficient  0.441  0.000  0.441  "
"      Maximum flow  0.064  0.000  0.064  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.064  2.420  0.062  0.000"
" 54  POND DESIGN"
"      2.420  Current peak flow  c.m/sec"
"      0.756  Target outflow  c.m/sec"
"     6140.8  Hydrograph volume  c.m"
"      12.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"          Level  Discharge  Volume"
"          411.000  0.000  0.000"
"          411.100  0.00600  612.000"
"          411.200  0.01300  1233.000"
"          411.300  0.01700  1862.000"
"          411.400  0.02000  2501.000"
"          411.500  0.02300  3148.000"
"          411.600  0.02600  3805.000"
"          411.700  0.1570  4470.000"
"          411.750  0.1630  4806.000"
"          411.800  0.2450  5144.000"
"          411.850  0.2520  5485.000"
"          412.000  1.912  6295.000"
"      Peak outflow  0.220  c.m/sec"
"      Maximum level  411.785  metre"
"      Maximum storage  5041.114  c.m"
"      Centroidal lag  25.177  hours"
"          0.064  2.420  0.220  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"          0.064  0.220  0.220  0.000"
" 33  CATCHMENT 4000"

```

411009_25YR POST

```

"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     4000 Catchment 4000"
"     0.000 % Impervious"
"     6.340 Total Area"
"    60.000 Flow length"
"     2.000 Overland Slope"
"     6.340 Pervious Area"
"    60.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    60.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.377 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.520 0.220 0.220 0.000 c.m/sec"
" Catchment 4000 Pervious Impervious Total Area "
" Surface Area 6.340 0.000 6.340 hectare"
" Time of concentration 23.913 2.944 23.913 minutes"
" Time to Centroid 123.357 87.974 123.357 minutes"
" Rainfall depth 75.581 75.581 75.581 mm"
" Rainfall volume 4791.82 0.00 4791.83 c.m"
" Rainfall losses 47.108 6.942 47.107 mm"
" Runoff depth 28.473 68.639 28.473 mm"
" Runoff volume 1805.21 0.00 1805.21 c.m"
" Runoff coefficient 0.377 0.000 0.377 "
" Maximum flow 0.520 0.000 0.520 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"           0.520 0.615 0.220 0.000"
" 54 POND DESIGN"
" 0.615 Current peak flow c.m/sec"
" 0.756 Target outflow c.m/sec"
" 7942.9 Hydrograph volume c.m"
" 6. Number of stages"
" 409.630 Minimum water level metre"
" 410.750 Maximum water level metre"
" 409.630 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 409.630 0.000 0.000"
" 409.750 0.5400 232.250"
" 410.000 1.632 2017.880"
" 410.250 3.737 5148.940"
" 410.500 1.345 9472.330"
" 410.750 1.885 15057.74"
" Peak outflow 0.555 c.m/sec"
" Maximum level 409.753 metre"
" Maximum storage 256.902 c.m"
" Centroidal lag 20.043 hours"
"           0.520 0.615 0.555 0.000 c.m/sec"
" 40 HYDROGRAPH Next link "
" 5 Next link "
"           0.520 0.555 0.555 0.000"

```

411009_25YR POST

```

" 52      CHANNEL DESIGN"
"      0.555 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"      Depth of flow              0.314      metre"
"      Velocity                    0.841      m/sec"
"      Channel capacity            10.655     c.m/sec"
"      Critical depth              0.269     metre"
" 53      ROUTE Channel Route 72"
"      72.40 Channel Route 72 Reach length (metre)"
"      0.422 X-factor <= 0.5"
"      64.553 K-lag (seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag (seconds)"
"      0.500 Beta weighting factor"
"      60.000 Routing time step (seconds)"
"      1 No. of sub-reaches"
"      Peak outflow              0.554      c.m/sec"
"      0.520 0.555 0.554 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5 Next link "
"      0.520 0.554 0.554 0.000"
" 52      CHANNEL DESIGN"
"      0.554 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0. Cross-section type: 0=trapezoidal; 1=general"
"      2.000 Basewidth      metre"
"      2.950 Left bank slope"
"      3.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"      Depth of flow              0.226     metre"
"      Velocity                    0.917     m/sec"
"      Channel capacity            9.246     c.m/sec"
"      Critical depth              0.181     metre"
" 53      ROUTE channel Route 40"
"      39.80 Channel Route 40 Reach length (metre)"
"      0.349 X-factor <= 0.5"
"      32.539 K-lag (seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag (seconds)"
"      0.500 Beta weighting factor"
"      37.500 Routing time step (seconds)"
"      1 No. of sub-reaches"
"      Peak outflow              0.553      c.m/sec"
"      0.520 0.554 0.553 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5 Next link "
"      0.520 0.553 0.553 0.000"
" 40      HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.520 0.553 0.553 0.000"
" 64      SHOW TABLE"
"      2 Flow hydrograph"
"      4 Inflow Hydrograph"
"      Maximum flow              0.553      c.m/sec"

```

```

"          411009_25YR POST
"          Hydrograph volume 7942.502 c.m"
" 40 HYDROGRAPH Combine 999"
"      6 Combine "
"      999 Node #"
"          To wetland"
"          Maximum flow 0.553 c.m/sec"
"          Hydrograph volume 7942.502 c.m"
"          0.520 0.553 0.553 0.553"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"          0.520 0.000 0.553 0.553"
" 47 FILEI_0 Read/Open DIV03100.025hyd"
"      1 1=read/open; 2=write/save"
"      2 1=rainfall; 2=hydrograph"
"      1 1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV03100.025hyd"
"          Major flow at 3100"
"          Total volume 20.839 c.m"
"          Maximum flow 0.030 0.000 0.553 0.553 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.030 0.030 0.553 0.553"
" 33 CATCHMENT 3200"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      3200 Catchment 3200"
"      60.000 % Impervious"
"      0.350 Total Area"
"      20.000 Flow length"
"      2.000 Overland Slope"
"      0.140 Pervious Area"
"      20.000 Pervious length"
"      2.000 Pervious slope"
"      0.210 Impervious Area"
"      20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.441 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.916 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.081 0.030 0.553 0.553 c.m/sec"
"          Catchment 3200 Pervious Impervious Total Area "
"          Surface Area 0.140 0.210 0.350 hectare"
"          Time of concentration 11.553 1.523 3.959 minutes"
"          Time to Centroid 108.042 85.984 91.342 minutes"
"          Rainfall depth 75.581 75.581 75.581 mm"
"          Rainfall volume 105.81 158.72 264.53 c.m"
"          Rainfall losses 42.253 6.330 20.699 mm"
"          Runoff depth 33.328 69.250 54.881 mm"
"          Runoff volume 46.66 145.43 192.09 c.m"
"          Runoff coefficient 0.441 0.916 0.726 "
"          Maximum flow 0.019 0.071 0.081 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.081 0.111 0.553 0.553"

```

```

" 33      CATCHMENT 3300"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          3300 Catchment 3300"
"          60.000 % Impervious"
"          0.220 Total Area"
"          20.000 Flow length"
"          2.000 overland Slope"
"          0.088 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.132 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.441 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.916 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"              0.051      0.111      0.553      0.553 c.m/sec"
"          Catchment 3300      Pervious      Impervious      Total Area      "
"          Surface Area      0.088      0.132      0.220      hectare"
"          Time of concentration      11.553      1.523      3.959      minutes"
"          Time to Centroid      108.042      85.984      91.342      minutes"
"          Rainfall depth      75.581      75.581      75.581      mm"
"          Rainfall volume      66.51      99.77      166.28      c.m"
"          Rainfall losses      42.253      6.330      20.699      mm"
"          Runoff depth      33.328      69.250      54.881      mm"
"          Runoff volume      29.33      91.41      120.74      c.m"
"          Runoff coefficient      0.441      0.916      0.726      "
"          Maximum flow      0.012      0.045      0.051      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.051      0.162      0.553      0.553"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.051      0.162      0.162      0.553"
" 64      SHOW TABLE"
"          2  Flow hydrograph"
"          4  Inflow Hydrograph"
"              Maximum flow      0.162      c.m/sec"
"              Hydrograph volume      333.663      c.m"
" 40      HYDROGRAPH Combine      999"
"          6  Combine "
"          999 Node #"
"              To Wetland"
"              Maximum flow      0.582      c.m/sec"
"              Hydrograph volume      8276.165      c.m"
"              0.051      0.162      0.162      0.582"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.051      0.000      0.162      0.582"
" 33      CATCHMENT 2100"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2100 Catchment 2100"

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411009_25YR POST

```

" 60.000 % Impervious"
" 2.180 Total Area"
" 40.000 Flow length"
" 2.000 Overland Slope"
" 0.872 Pervious Area"
" 40.000 Pervious length"
" 2.000 Pervious slope"
" 1.308 Impervious Area"
" 40.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 78.000 Pervious SCS Curve No."
" 0.442 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 7.164 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.913 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.483 0.000 0.162 0.582 c.m/sec"
" Catchment 2100 Pervious Impervious Total Area "
" Surface Area 0.872 1.308 2.180 hectare"
" Time of concentration 17.510 2.308 6.016 minutes"
" Time to Centroid 114.842 87.059 93.835 minutes"
" Rainfall depth 75.581 75.581 75.581 mm"
" Rainfall volume 659.06 988.60 1647.66 c.m"
" Rainfall losses 42.200 6.593 20.835 mm"
" Runoff depth 33.381 68.988 54.745 mm"
" Runoff volume 291.08 902.37 1193.45 c.m"
" Runoff coefficient 0.442 0.913 0.724 "
" Maximum flow 0.100 0.447 0.483 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.483 0.483 0.162 0.582"
" 54 POND DESIGN"
" 0.483 Current peak flow c.m/sec"
" 0.396 Target outflow c.m/sec"
" 1193.5 Hydrograph volume c.m"
" 13. Number of stages"
" 0.000 Minimum water level metre"
" 3.000 Maximum water level metre"
" 0.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 410.650 0.000 0.000"
" 410.700 0.00600 25.000"
" 410.800 0.01300 79.000"
" 410.900 0.02000 139.000"
" 411.000 0.02500 206.000"
" 411.100 0.02900 279.000"
" 411.200 0.03200 359.000"
" 411.300 0.2910 446.000"
" 411.400 0.3210 540.000"
" 411.500 0.3470 641.000"
" 411.600 0.3720 750.000"
" 411.650 0.3840 807.000"
" 411.950 2.818 1195.000"
" Peak outflow 0.294 c.m/sec"
" Maximum level 411.311 metre"
" Maximum storage 456.530 c.m"
" Centroidal lag 2.925 hours"
" 0.483 0.483 0.294 0.582 c.m/sec"

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```

" 40      HYDROGRAPH Next link "
"          5  Next link "
"              0.483      0.294      0.294      0.582"
" 33      CATCHMENT 2200"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2200 Catchment 2200"
"          0.000 % Impervious"
"          0.910 Total Area"
"          40.000 Flow length"
"          2.000 Overland slope"
"          0.910 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.442 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"              0.105      0.294      0.294      0.582 c.m/sec"
"          Catchment 2200      Pervious      Impervious      Total Area "
"          Surface Area      0.910      0.000      0.910      hectare"
"          Time of concentration      17.510      2.308      17.510      minutes"
"          Time to Centroid      114.842      87.059      114.842      minutes"
"          Rainfall depth      75.581      75.581      75.581      mm"
"          Rainfall volume      687.78      0.00      687.79      c.m"
"          Rainfall losses      42.200      6.593      42.199      mm"
"          Runoff depth      33.381      68.988      33.381      mm"
"          Runoff volume      303.77      0.00      303.77      c.m"
"          Runoff coefficient      0.442      0.000      0.442      "
"          Maximum flow      0.105      0.000      0.105      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.105      0.398      0.294      0.582"
" 33      CATCHMENT 2300"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2300 Catchment 2300"
"          10.000 % Impervious"
"          0.470 Total Area"
"          40.000 Flow length"
"          2.000 Overland slope"
"          0.423 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.047 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.442 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"

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411009_25YR POST

```

"      0.015  Impervious Manning 'n'"
" 98.000  Impervious SCS Curve No."
"      0.913  Impervious Runoff coefficient"
"      0.100  Impervious Ia/s coefficient"
"      0.518  Impervious Initial abstraction"
"              0.055      0.398      0.294      0.582 c.m/sec"
"      Catchment 2300      Pervious      Impervious      Total Area  "
"      Surface Area      0.423      0.047      0.470      hectare"
"      Time of concentration 17.510      2.308      14.671      minutes"
"      Time to Centroid 114.842      87.059      109.653      minutes"
"      Rainfall depth 75.581      75.581      75.581      mm"
"      Rainfall volume 319.71      35.52      355.23      c.m"
"      Rainfall losses 42.200      6.593      38.639      mm"
"      Runoff depth 33.381      68.988      36.942      mm"
"      Runoff volume 141.20      32.42      173.63      c.m"
"      Runoff coefficient 0.442      0.913      0.489      "
"      Maximum flow 0.049      0.016      0.055      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"              0.055      0.452      0.294      0.582"
" 40 HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"              0.055      0.452      0.452      0.582"
" 64 SHOW TABLE"
"      2  Flow hydrograph"
"      4  Inflow Hydrograph"
"      Maximum flow              0.452      c.m/sec"
"      Hydrograph volume 1665.676      c.m"
" 40 HYDROGRAPH Combine 999"
"      6  Combine "
"      999 Node #"
"      To Wetland"
"      Maximum flow              0.897      c.m/sec"
"      Hydrograph volume 9941.869      c.m"
"              0.055      0.452      0.452      0.897"
" 40 HYDROGRAPH Confluence 999"
"      7  Confluence "
"      999 Node #"
"      To Wetland"
"      Maximum flow              0.897      c.m/sec"
"      Hydrograph volume 9941.869      c.m"
"              0.055      0.897      0.452      0.000"
" 38 START/RE-START TOTALS 999"
"      3  Runoff Totals on EXIT"
"      Total Catchment area              22.620      hectare"
"      Total Impervious area              7.577      hectare"
"      Total % impervious              33.497"
" 19 EXIT"

```

411009_50YR POST

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10 Units used:                      ie METRIC"
"          Job folder:                        C:\Users\akroetsch\Documents\
"                                         work in Progress\411009 Ainley Farm SWM Junk"
"          Output filename:                   411009_50YR POST.out"
"          Licensee name:                     gmbp"
"          Company                            Hewlett-Packard Company"
"          Date & Time last used:             4/17/2017 at 3:45:25 PM"

```

```

" 31      TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1 Chicago storm"
"          5089.418 Coefficient A"
"          30.000 Constant B"
"          0.967 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity                156.350 mm/hr"
"          Total depth                      86.737 mm"
"          6 050hyd Hydrograph extension used in this file"

```

```

" 33      CATCHMENT 3100"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          3100 Catchment 3100"
"          60.000 % Impervious"
"          0.400 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.160 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.240 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.481 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.924 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.107 0.000 0.000 0.000 c.m/sec"

```

```

"          Catchment 3100 Pervious Impervious Total Area "
"          Surface Area 0.160 0.240 0.400 hectare"
"          Time of concentration 10.695 1.467 3.845 minutes"
"          Time to Centroid 106.283 85.675 90.985 minutes"
"          Rainfall depth 86.737 86.737 86.737 mm"
"          Rainfall volume 138.78 208.17 346.95 c.m"
"          Rainfall losses 44.994 6.561 21.934 mm"
"          Runoff depth 41.743 80.176 64.803 mm"
"          Runoff volume 66.79 192.42 259.21 c.m"
"          Runoff coefficient 0.481 0.924 0.747 "
"          Maximum flow 0.027 0.090 0.107 c.m/sec"
" 40      HYDROGRAPH Add Runoff "

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411009_50YR POST

```

"      4  Add Runoff "
"      0.107      0.107      0.000      0.000"
" 56  DIVERSION"
"      3100 Node number"
"      0.062 Overflow threshold"
"      1.000 Required diverted fraction"
"      0 Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow      0.045      c.m/sec"
"      Volume of diverted flow      36.365      c.m"
"      DIV03100.050hyd"
"      Major flow at 3100"
"      0.107      0.107      0.062      0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5 Next link "
"      0.107      0.062      0.062      0.000"
" 33  CATCHMENT 1000"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      1000 Catchment 1000"
"      50.000 % Impervious"
"      11.280 Total Area"
"      20.000 Flow length"
"      2.000 Overland Slope"
"      5.640 Pervious Area"
"      20.000 Pervious length"
"      2.000 Pervious slope"
"      5.640 Impervious Area"
"      20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.481 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.924 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      2.726      0.062      0.062      0.000 c.m/sec"
"      Catchment 1000      Pervious      Impervious      Total Area "
"      Surface Area      5.640      5.640      11.280      hectare"
"      Time of concentration      10.695      1.467      4.627      minutes"
"      Time to Centroid      106.283      85.675      92.731      minutes"
"      Rainfall depth      86.737      86.737      86.737      mm"
"      Rainfall volume      4891.95      4891.95      9783.91      c.m"
"      Rainfall losses      44.994      6.561      25.777      mm"
"      Runoff depth      41.743      80.176      60.959      mm"
"      Runoff volume      2354.31      4521.90      6876.22      c.m"
"      Runoff coefficient      0.481      0.924      0.703      "
"      Maximum flow      0.969      2.118      2.726      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      2.726      2.788      0.062      0.000"
" 33  CATCHMENT 1100"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      1100 Catchment 1100"
"      0.000 % Impervious"
"      0.470 Total Area"
"      20.000 Flow length"

```

411009_50YR POST

```

"      2.000  Overland Slope"
"      0.470  Pervious Area"
"     20.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.481  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.081  2.788  0.062  0.000 c.m/sec"
"      Catchment 1100  Pervious  Impervious  Total Area  "
"      Surface Area  0.470  0.000  0.470  hectare"
"      Time of concentration  10.695  1.467  10.695  minutes"
"      Time to Centroid  106.283  85.675  106.283  minutes"
"      Rainfall depth  86.737  86.737  86.737  mm"
"      Rainfall volume  407.66  0.00  407.66  c.m"
"      Rainfall losses  44.994  6.561  44.994  mm"
"      Runoff depth  41.743  80.176  41.743  mm"
"      Runoff volume  196.19  0.00  196.19  c.m"
"      Runoff coefficient  0.481  0.000  0.481  "
"      Maximum flow  0.081  0.000  0.081  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.081  2.843  0.062  0.000"
" 54  POND DESIGN"
"      2.843  Current peak flow  c.m/sec"
"      0.756  Target outflow  c.m/sec"
"     7295.3  Hydrograph volume  c.m"
"      12.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge  Volume"
"      411.000  0.000  0.000"
"      411.100  0.00600  612.000"
"      411.200  0.01300  1233.000"
"      411.300  0.01700  1862.000"
"      411.400  0.02000  2501.000"
"      411.500  0.02300  3148.000"
"      411.600  0.02600  3805.000"
"      411.700  0.1570  4470.000"
"      411.750  0.1630  4806.000"
"      411.800  0.2450  5144.000"
"      411.850  0.2520  5485.000"
"      412.000  1.912  6295.000"
"      Peak outflow  0.464  c.m/sec"
"      Maximum level  411.869  metre"
"      Maximum storage  5588.438  c.m"
"      Centroidal lag  21.941  hours"
"          0.081  2.843  0.464  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"          0.081  0.464  0.464  0.000"
" 33  CATCHMENT 4000"

```

411009_50YR POST

```

"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     4000 Catchment 4000"
"     0.000 % Impervious"
"     6.340 Total Area"
"    60.000 Flow length"
"     2.000 Overland Slope"
"     6.340 Pervious Area"
"    60.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    60.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.417 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.703 0.464 0.464 0.000 c.m/sec"
"      Catchment 4000 Pervious Impervious Total Area "
"      Surface Area 6.340 0.000 6.340 hectare"
"      Time of concentration 21.990 2.837 21.990 minutes"
"      Time to Centroid 120.254 87.552 120.254 minutes"
"      Rainfall depth 86.737 86.737 86.737 mm"
"      Rainfall volume 5499.11 0.01 5499.11 c.m"
"      Rainfall losses 50.538 7.307 50.538 mm"
"      Runoff depth 36.199 79.429 36.199 mm"
"      Runoff volume 2295.03 0.01 2295.03 c.m"
"      Runoff coefficient 0.417 0.000 0.417 "
"      Maximum flow 0.703 0.000 0.703 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.703 0.864 0.464 0.000"
" 54 POND DESIGN"
"      0.864 Current peak flow c.m/sec"
"      0.756 Target outflow c.m/sec"
"     9583.6 Hydrograph volume c.m"
"           6. Number of stages"
"    409.630 Minimum water level metre"
"    410.750 Maximum water level metre"
"    409.630 Starting water level metre"
"           0 Keep Design Data: 1 = True; 0 = False"
"           Level Discharge Volume"
"    409.630 0.000 0.000"
"    409.750 0.5400 232.250"
"    410.000 1.632 2017.880"
"    410.250 3.737 5148.940"
"    410.500 1.345 9472.330"
"    410.750 1.885 15057.74"
"      Peak outflow 0.706 c.m/sec"
"      Maximum level 409.788 metre"
"      Maximum storage 503.390 c.m"
"      Centroidal lag 17.298 hours"
"           0.703 0.864 0.706 0.000 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"           0.703 0.706 0.706 0.000"

```

411009_50YR POST

```

52 CHANNEL DESIGN"
0.706 Current peak flow c.m/sec"
0.035 Manning 'n'"
0. Cross-section type: 0=trapezoidal; 1=general"
0.000 Basewidth metre"
7.410 Left bank slope"
6.000 Right bank slope"
0.950 Channel depth metre"
1.040 Gradient %"
Depth of flow 0.343 metre"
Velocity 0.893 m/sec"
Channel capacity 10.655 c.m/sec"
Critical depth 0.296 metre"
53 ROUTE Channel Route 72"
72.40 Channel Route 72 Reach length (metre)"
0.415 X-factor <= 0.5"
60.784 K-lag (seconds)"
0.000 Default(0) or user spec.(1) values used"
0.500 X-factor <= 0.5"
30.000 K-lag (seconds)"
0.500 Beta weighting factor"
60.000 Routing time step (seconds)"
1 No. of sub-reaches"
Peak outflow 0.705 c.m/sec"
0.703 0.706 0.705 0.000 c.m/sec"
40 HYDROGRAPH Next link "
5 Next link "
0.703 0.705 0.705 0.000"
52 CHANNEL DESIGN"
0.705 Current peak flow c.m/sec"
0.035 Manning 'n'"
0. Cross-section type: 0=trapezoidal; 1=general"
2.000 Basewidth metre"
2.950 Left bank slope"
3.000 Right bank slope"
0.950 Channel depth metre"
1.040 Gradient %"
Depth of flow 0.258 metre"
Velocity 0.987 m/sec"
Channel capacity 9.246 c.m/sec"
Critical depth 0.209 metre"
53 ROUTE Channel Route 40"
39.80 Channel Route 40 Reach length (metre)"
0.330 X-factor <= 0.5"
30.231 K-lag (seconds)"
0.000 Default(0) or user spec.(1) values used"
0.500 X-factor <= 0.5"
30.000 K-lag (seconds)"
0.500 Beta weighting factor"
37.500 Routing time step (seconds)"
1 No. of sub-reaches"
Peak outflow 0.705 c.m/sec"
0.703 0.705 0.705 0.000 c.m/sec"
40 HYDROGRAPH Next link "
5 Next link "
0.703 0.705 0.705 0.000"
40 HYDROGRAPH Copy to Outflow"
8 Copy to Outflow"
0.703 0.705 0.705 0.000"
64 SHOW TABLE"
2 Flow hydrograph"
4 Inflow Hydrograph"
Maximum flow 0.705 c.m/sec"

```

```

411009_50YR POST
Hydrograph volume 9586.549 c.m"
40 HYDROGRAPH Combine 999"
6 Combine "
999 Node #"
To Wetland"
Maximum flow 0.705 c.m/sec"
Hydrograph volume 9586.549 c.m"
0.703 0.705 0.705 0.705"
40 HYDROGRAPH Start - New Tributary"
2 Start - New Tributary"
0.703 0.000 0.705 0.705"
47 FILEI_0 Read/Open DIV03100.050hyd"
1 1=read/open; 2=write/save"
2 1=rainfall; 2=hydrograph"
1 1=runoff; 2=inflow; 3=outflow; 4=junction"
DIV03100.050hyd"
Major flow at 3100"
Total volume 36.365 c.m"
Maximum flow 0.045 0.000 0.705 0.705 c.m/sec"
40 HYDROGRAPH Add Runoff "
4 Add Runoff "
0.045 0.045 0.705 0.705"
33 CATCHMENT 3200"
1 Triangular SCS"
1 Equal length"
1 SCS method"
3200 Catchment 3200"
60.000 % Impervious"
0.350 Total Area"
20.000 Flow length"
2.000 Overland Slope"
0.140 Pervious Area"
20.000 Pervious length"
2.000 Pervious slope"
0.210 Impervious Area"
20.000 Impervious length"
2.000 Impervious slope"
0.250 Pervious Manning 'n'"
78.000 Pervious SCS Curve No."
0.481 Pervious Runoff coefficient"
0.100 Pervious Ia/S coefficient"
7.164 Pervious Initial abstraction"
0.015 Impervious Manning 'n'"
98.000 Impervious SCS Curve No."
0.924 Impervious Runoff coefficient"
0.100 Impervious Ia/S coefficient"
0.518 Impervious Initial abstraction"
0.093 0.045 0.705 0.705 c.m/sec"
Catchment 3200 Pervious Impervious Total Area "
Surface Area 0.140 0.210 0.350 hectare"
Time of concentration 10.695 1.467 3.845 minutes"
Time to Centroid 106.283 85.675 90.985 minutes"
Rainfall depth 86.737 86.737 86.737 mm"
Rainfall volume 121.43 182.15 303.58 c.m"
Rainfall losses 44.994 6.561 21.934 mm"
Runoff depth 41.743 80.176 64.803 mm"
Runoff volume 58.44 168.37 226.81 c.m"
Runoff coefficient 0.481 0.924 0.747 "
Maximum flow 0.024 0.079 0.093 c.m/sec"
40 HYDROGRAPH Add Runoff "
4 Add Runoff "
0.093 0.138 0.705 0.705"

```

```

33      CATCHMENT 3300"
      1  Triangular SCS"
      1  Equal length"
      1  SCS method"
      3300 Catchment 3300"
      60.000 % Impervious"
      0.220 Total Area"
      20.000 Flow length"
      2.000 overland Slope"
      0.088 Pervious Area"
      20.000 Pervious length"
      2.000 Pervious slope"
      0.132 Impervious Area"
      20.000 Impervious length"
      2.000 Impervious slope"
      0.250 Pervious Manning 'n'"
      78.000 Pervious SCS Curve No."
      0.481 Pervious Runoff coefficient"
      0.100 Pervious Ia/S coefficient"
      7.164 Pervious Initial abstraction"
      0.015 Impervious Manning 'n'"
      98.000 Impervious SCS Curve No."
      0.924 Impervious Runoff coefficient"
      0.100 Impervious Ia/S coefficient"
      0.518 Impervious Initial abstraction"
      0.059 0.138 0.705 0.705 c.m/sec"
      Catchment 3300 Pervious Impervious Total Area "
      Surface Area 0.088 0.132 0.220 hectare"
      Time of concentration 10.695 1.467 3.845 minutes"
      Time to Centroid 106.283 85.675 90.985 minutes"
      Rainfall depth 86.737 86.737 86.737 mm"
      Rainfall volume 76.33 114.49 190.82 c.m"
      Rainfall losses 44.994 6.561 21.934 mm"
      Runoff depth 41.743 80.176 64.803 mm"
      Runoff volume 36.73 105.83 142.57 c.m"
      Runoff coefficient 0.481 0.924 0.747 "
      Maximum flow 0.015 0.050 0.059 c.m/sec"
40      HYDROGRAPH Add Runoff "
      4  Add Runoff "
      0.059 0.197 0.705 0.705"
40      HYDROGRAPH Copy to Outflow"
      8  Copy to Outflow"
      0.059 0.197 0.197 0.705"
64      SHOW TABLE"
      2  Flow hydrograph"
      4  Inflow Hydrograph"
      Maximum flow 0.197 c.m/sec"
      Hydrograph volume 405.739 c.m"
40      HYDROGRAPH Combine 999"
      6  Combine "
      999 Node #"
      To wetland"
      Maximum flow 0.724 c.m/sec"
      Hydrograph volume 9992.290 c.m"
      0.059 0.197 0.197 0.724"
40      HYDROGRAPH Start - New Tributary"
      2  Start - New Tributary"
      0.059 0.000 0.197 0.724"
33      CATCHMENT 2100"
      1  Triangular SCS"
      1  Equal length"
      1  SCS method"
      2100 Catchment 2100"

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"      60.000  % Impervious"
"      2.180  Total Area"
"      40.000  Flow length"
"      2.000  Overland Slope"
"      0.872  Pervious Area"
"      40.000  Pervious length"
"      2.000  Pervious slope"
"      1.308  Impervious Area"
"      40.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.481  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.922  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.546  0.000  0.197  0.724 c.m/sec"
"      Catchment 2100  Pervious  Impervious  Total Area  "
"      Surface Area      0.872      1.308      2.180  hectare"
"      Time of concentration  16.211  2.224  5.836  minutes"
"      Time to Centroid      112.570  86.667  93.357  minutes"
"      Rainfall depth      86.737  86.737  86.737  mm"
"      Rainfall volume      756.34  1134.52  1890.86  c.m"
"      Rainfall losses      44.974  6.774  22.054  mm"
"      Runoff depth      41.763  79.963  64.683  mm"
"      Runoff volume      364.17  1045.92  1410.09  c.m"
"      Runoff coefficient    0.481  0.922  0.746  "
"      Maximum flow      0.132  0.496  0.546  c.m/sec"
"  40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.546  0.546  0.197  0.724"
"  54  POND DESIGN"
"      0.546  Current peak flow  c.m/sec"
"      0.396  Target outflow  c.m/sec"
"     1410.1  Hydrograph volume  c.m"
"      13.  Number of stages"
"      0.000  Minimum water level  metre"
"      3.000  Maximum water level  metre"
"      0.000  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge  Volume"
"      410.650  0.000  0.000"
"      410.700  0.00600  25.000"
"      410.800  0.01300  79.000"
"      410.900  0.02000  139.000"
"      411.000  0.02500  206.000"
"      411.100  0.02900  279.000"
"      411.200  0.03200  359.000"
"      411.300  0.2910  446.000"
"      411.400  0.3210  540.000"
"      411.500  0.3470  641.000"
"      411.600  0.3720  750.000"
"      411.650  0.3840  807.000"
"      411.950  2.818  1195.000"
"      Peak outflow      0.316  c.m/sec"
"      Maximum level      411.383  metre"
"      Maximum storage      523.927  c.m"
"      Centroidal lag      2.750  hours"
"          0.546  0.546  0.316  0.724 c.m/sec"

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```

" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.546      0.316      0.316      0.724"
" 33      CATCHMENT 2200"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2200 Catchment 2200"
"          0.000 % Impervious"
"          0.910 Total Area"
"          40.000 Flow length"
"          2.000 Overland Slope"
"          0.910 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.481 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.138      0.316      0.316      0.724 c.m/sec"
"          Catchment 2200      Pervious      Impervious      Total Area "
"          Surface Area      0.910      0.000      0.910      hectare"
"          Time of concentration      16.211      2.224      16.211      minutes"
"          Time to Centroid      112.570      86.667      112.570      minutes"
"          Rainfall depth      86.737      86.737      86.737      mm"
"          Rainfall volume      789.30      0.00      789.30      c.m"
"          Rainfall losses      44.974      6.773      44.974      mm"
"          Runoff depth      41.763      79.963      41.763      mm"
"          Runoff volume      380.04      0.00      380.04      c.m"
"          Runoff coefficient      0.481      0.000      0.481      "
"          Maximum flow      0.138      0.000      0.138      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"          0.138      0.451      0.316      0.724"
" 33      CATCHMENT 2300"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2300 Catchment 2300"
"          10.000 % Impervious"
"          0.470 Total Area"
"          40.000 Flow length"
"          2.000 Overland Slope"
"          0.423 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.047 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.481 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"

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                                411009_50YR POST
"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.922  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"              0.073    0.451    0.316    0.724 c.m/sec"
"      Catchment 2300      Pervious      Impervious      Total Area  "
"      Surface Area      0.423      0.047      0.470      hectare"
"      Time of concentration 16.211      2.224      13.757      minutes"
"      Time to Centroid    112.570     86.667     108.026     minutes"
"      Rainfall depth      86.737     86.737     86.737     mm"
"      Rainfall volume     366.90     40.77     407.66     c.m"
"      Rainfall losses     44.974     6.774     41.154     mm"
"      Runoff depth        41.763     79.963     45.583     mm"
"      Runoff volume       176.66     37.58     214.24     c.m"
"      Runoff coefficient   0.481     0.922     0.526     "
"      Maximum flow        0.064     0.018     0.073     c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4  Add Runoff "
"              0.073    0.523    0.316    0.724"
" 40 HYDROGRAPH Copy to Outflow"
" 8  Copy to Outflow"
"              0.073    0.523    0.523    0.724"
" 64 SHOW TABLE"
" 2  Flow hydrograph"
" 4  Inflow Hydrograph"
"      Maximum flow              0.523    c.m/sec"
"      Hydrograph volume        2002.184  c.m"
" 40 HYDROGRAPH Combine 999"
" 6  Combine "
" 999 Node #"
"      To Wetland"
"      Maximum flow              1.128    c.m/sec"
"      Hydrograph volume        11994.473  c.m"
"              0.073    0.523    0.523    1.128"
" 40 HYDROGRAPH Confluence 999"
" 7  Confluence "
" 999 Node #"
"      To Wetland"
"      Maximum flow              1.128    c.m/sec"
"      Hydrograph volume        11994.472  c.m"
"              0.073    1.128    0.523    0.000"
" 38 START/RE-START TOTALS 999"
" 3  Runoff Totals on EXIT"
"      Total Catchment area              22.620  hectare"
"      Total Impervious area              7.577  hectare"
"      Total % impervious              33.497"
" 19 EXIT"

```

411009_100YR POST

```

"          MIDUSS Output ----->"
"          MIDUSS version              Version 2.25 rev. 473"
"          MIDUSS created              Sunday, February 07, 2010"
"          10 Units used:              ie METRIC"
"          Job folder:                  C:\Users\akroetsch\Documents\
"                                     Work in Progress\411009 Ainley Farm SWM Junk"
"          Output filename:            411009_100YR POST.out"
"          Licensee name:              gmbp"
"          Company                     Hewlett-Packard Company"
"          Date & Time last used:      4/17/2017 at 3:46:06 PM"
" 31    TIME PARAMETERS"
"          5.000 Time Step"
"          180.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32    STORM Chicago storm"
"          1 Chicago storm"
"          6933.019 Coefficient A"
"          34.699 Constant B"
"          0.998 Exponent C"
"          0.380 Fraction R"
"          180.000 Duration"
"          1.000 Time step multiplier"
"          Maximum intensity            168.777 mm/hr"
"          Total depth                  97.921 mm"
"          6 100hyd Hydrograph extension used in this file"
" 33    CATCHMENT 3100"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          3100 Catchment 3100"
"          60.000 % Impervious"
"          0.400 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.160 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.240 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.515 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.931 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.121 0.000 0.000 0.000 c.m/sec"
"          Catchment 3100 Pervious Impervious Total Area "
"          Surface Area 0.160 0.240 0.400 hectare"
"          Time of concentration 10.027 1.421 3.740 minutes"
"          Time to Centroid 104.871 85.423 90.664 minutes"
"          Rainfall depth 97.921 97.921 97.921 mm"
"          Rainfall volume 156.67 235.01 391.69 c.m"
"          Rainfall losses 47.483 6.787 23.065 mm"
"          Runoff depth 50.438 91.134 74.856 mm"
"          Runoff volume 80.70 218.72 299.42 c.m"
"          Runoff coefficient 0.515 0.931 0.764 "
"          Maximum flow 0.034 0.099 0.121 c.m/sec"
" 40    HYDROGRAPH Add Runoff "

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411009_100YR POST

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"      4  Add Runoff "
"      0.121  0.121  0.000  0.000"
" 56  DIVERSION"
"      3100  Node number"
"      0.062  Overflow threshold"
"      1.000  Required diverted fraction"
"      0  Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow  0.059  c.m/sec"
"      Volume of diverted flow  53.317  c.m"
"      DIV03100.100hyd"
"      Major flow at 3100"
"      0.121  0.121  0.062  0.000 c.m/sec"
" 40  HYDROGRAPH Next link "
"      5  Next link "
"      0.121  0.062  0.062  0.000"
" 33  CATCHMENT 1000"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      1000  Catchment 1000"
"      50.000  % Impervious"
"      11.280  Total Area"
"      20.000  Flow length"
"      2.000  overland slope"
"      5.640  Pervious Area"
"      20.000  Pervious length"
"      2.000  Pervious slope"
"      5.640  Impervious Area"
"      20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.515  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.931  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"      3.124  0.062  0.062  0.000 c.m/sec"
"      Catchment 1000  Pervious  Impervious  Total Area  "
"      Surface Area  5.640  5.640  11.280  hectare"
"      Time of concentration  10.027  1.421  4.487  minutes"
"      Time to Centroid  104.871  85.423  92.351  minutes"
"      Rainfall depth  97.921  97.921  97.921  mm"
"      Rainfall volume  0.5523  0.5523  1.1046  ha-m"
"      Rainfall losses  47.483  6.787  27.135  mm"
"      Runoff depth  50.438  91.134  70.786  mm"
"      Runoff volume  2844.72  5139.96  7984.69  c.m"
"      Runoff coefficient  0.515  0.931  0.723  "
"      Maximum flow  1.188  2.318  3.124  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      3.124  3.186  0.062  0.000"
" 33  CATCHMENT 1100"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      1100  Catchment 1100"
"      0.000  % Impervious"
"      0.470  Total Area"
"      20.000  Flow length"

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"      2.000  Overland Slope"
"      0.470  Pervious Area"
"     20.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"     20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.515  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.099      3.186      0.062      0.000 c.m/sec"
"      Catchment 1100      Pervious      Impervious      Total Area      "
"      Surface Area      0.470      0.000      0.470      hectare"
"      Time of concentration      10.027      1.421      10.027      minutes"
"      Time to Centroid      104.871      85.423      104.871      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      460.23      0.00      460.23      c.m"
"      Rainfall losses      47.483      6.787      47.483      mm"
"      Runoff depth      50.438      91.134      50.438      mm"
"      Runoff volume      237.06      0.00      237.06      c.m"
"      Runoff coefficient      0.515      0.000      0.515      "
"      Maximum flow      0.099      0.000      0.099      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.099      3.258      0.062      0.000"
" 54      POND DESIGN"
"      3.258  Current peak flow      c.m/sec"
"      0.756  Target outflow      c.m/sec"
"     8467.9  Hydrograph volume      c.m"
"      12.    Number of stages"
"      0.000  Minimum water level      metre"
"      3.000  Maximum water level      metre"
"      0.000  Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"         411.000      0.000      0.000"
"         411.100      0.00600      612.000"
"         411.200      0.01300      1233.000"
"         411.300      0.01700      1862.000"
"         411.400      0.02000      2501.000"
"         411.500      0.02300      3148.000"
"         411.600      0.02600      3805.000"
"         411.700      0.1570      4470.000"
"         411.750      0.1630      4806.000"
"         411.800      0.2450      5144.000"
"         411.850      0.2520      5485.000"
"         412.000      1.912      6295.000"
"      Peak outflow      0.917      c.m/sec"
"      Maximum level      411.910      metre"
"      Maximum storage      5809.319      c.m"
"      Centroidal lag      19.195      hours"
"          0.099      3.258      0.917      0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5      Next link "
"          0.099      0.917      0.917      0.000"
" 33      CATCHMENT 4000"

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411009_100YR POST

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"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"     4000 Catchment 4000"
"     0.000 % Impervious"
"     6.340 Total Area"
"    60.000 Flow length"
"     2.000 Overland slope"
"     6.340 Pervious Area"
"    60.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    60.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    74.000 Pervious SCS Curve No."
"     0.453 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     8.924 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.877 0.917 0.917 0.000 c.m/sec"
"      Catchment 4000 Pervious Impervious Total Area "
"      Surface Area 6.340 0.000 6.340 hectare"
"      Time of concentration 20.504 2.747 20.504 minutes"
"      Time to Centroid 117.777 87.189 117.777 minutes"
"      Rainfall depth 97.921 97.921 97.921 mm"
"      Rainfall volume 6208.20 0.01 6208.21 c.m"
"      Rainfall losses 53.532 7.496 53.532 mm"
"      Runoff depth 44.389 90.426 44.389 mm"
"      Runoff volume 2814.26 0.01 2814.27 c.m"
"      Runoff coefficient 0.453 0.000 0.453 "
"      Maximum flow 0.877 0.000 0.877 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.877 1.561 0.917 0.000"
" 54 POND DESIGN"
"     1.561 Current peak flow c.m/sec"
"     0.756 Target outflow c.m/sec"
"    11277.6 Hydrograph volume c.m"
"     6. Number of stages"
"    409.630 Minimum water level metre"
"    410.750 Maximum water level metre"
"    409.630 Starting water level metre"
"     0 Keep Design Data: 1 = True; 0 = False"
"           Level Discharge Volume"
"    409.630 0.000 0.000"
"    409.750 0.5400 232.250"
"    410.000 1.632 2017.880"
"    410.250 3.737 5148.940"
"    410.500 1.345 9472.330"
"    410.750 1.885 15057.74"
"           Peak outflow 1.104 c.m/sec"
"           Maximum level 409.880 metre"
"           Maximum storage 1158.570 c.m"
"           Centroidal lag 15.065 hours"
"           0.877 1.561 1.104 0.000 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"           0.877 1.104 1.104 0.000"

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" 52      CHANNEL DESIGN"
"      1.104 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      0.000 Basewidth      metre"
"      7.410 Left bank slope"
"      6.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"      Depth of flow              0.406      metre"
"      Velocity                    0.999      m/sec"
"      Channel capacity            10.655     c.m/sec"
"      Critical depth              0.354      metre"
" 53      ROUTE      Channel Route 72"
"      72.40 Channel Route 72 Reach length (metre)"
"      0.399 X-factor <= 0.5"
"      54.356 K-lag (seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag (seconds)"
"      0.500 Beta weighting factor"
"      60.000 Routing time step (seconds)"
"      1 No. of sub-reaches"
"      Peak outflow              1.103      c.m/sec"
"      0.877 1.104 1.103 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5 Next link "
"      0.877 1.103 1.103 0.000"
" 52      CHANNEL DESIGN"
"      1.103 Current peak flow      c.m/sec"
"      0.035 Manning 'n'"
"      0.      Cross-section type: 0=trapezoidal; 1=general"
"      2.000 Basewidth      metre"
"      2.950 Left bank slope"
"      3.000 Right bank slope"
"      0.950 Channel depth      metre"
"      1.040 Gradient      %"
"      Depth of flow              0.329      metre"
"      Velocity                    1.128      m/sec"
"      Channel capacity            9.246      c.m/sec"
"      Critical depth              0.272      metre"
" 53      ROUTE      Channel Route 40"
"      39.80 Channel Route 40 Reach length (metre)"
"      0.288 X-factor <= 0.5"
"      26.470 K-lag (seconds)"
"      0.000 Default(0) or user spec.(1) values used"
"      0.500 X-factor <= 0.5"
"      30.000 K-lag (seconds)"
"      0.500 Beta weighting factor"
"      37.500 Routing time step (seconds)"
"      1 No. of sub-reaches"
"      Peak outflow              1.103      c.m/sec"
"      0.877 1.103 1.103 0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"      5 Next link "
"      0.877 1.103 1.103 0.000"
" 40      HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"      0.877 1.103 1.103 0.000"
" 64      SHOW TABLE"
"      2 Flow hydrograph"
"      4 Inflow Hydrograph"
"      Maximum flow              1.103      c.m/sec"

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411009_100YR POST
" 40 Hydrograph volume 11280.867 c.m"
" HYDROGRAPH Combine 999"
" 6 Combine "
" 999 Node #"
" To Wetland"
" Maximum flow 1.103 c.m/sec"
" Hydrograph volume 11280.867 c.m"
" 0.877 1.103 1.103 1.103"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
" 0.877 0.000 1.103 1.103"
" 47 FILEI_O Read/Open DIV03100.100hyd"
" 1 1=read/open; 2=write/save"
" 2 1=rainfall; 2=hydrograph"
" 1 1=runoff; 2=inflow; 3=outflow; 4=junction"
" DIV03100.100hyd"
" Major flow at 3100"
" Total volume 53.317 c.m"
" Maximum flow 0.059 c.m/sec"
" 0.059 0.000 1.103 1.103 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.059 0.059 1.103 1.103"
" 33 CATCHMENT 3200"
" 1 Triangular SCS"
" 1 Equal length"
" 1 SCS method"
" 3200 Catchment 3200"
" 60.000 % Impervious"
" 0.350 Total Area"
" 20.000 Flow length"
" 2.000 Overland Slope"
" 0.140 Pervious Area"
" 20.000 Pervious length"
" 2.000 Pervious slope"
" 0.210 Impervious Area"
" 20.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 78.000 Pervious SCS Curve No."
" 0.515 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 7.164 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.931 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.106 0.059 1.103 1.103 c.m/sec"
" Catchment 3200 Pervious Impervious Total Area "
" Surface Area 0.140 0.210 0.350 hectare"
" Time of concentration 10.027 1.421 3.740 minutes"
" Time to Centroid 104.871 85.423 90.664 minutes"
" Rainfall depth 97.921 97.921 97.921 mm"
" Rainfall volume 137.09 205.63 342.72 c.m"
" Rainfall losses 47.483 6.787 23.065 mm"
" Runoff depth 50.438 91.134 74.856 mm"
" Runoff volume 70.61 191.38 262.00 c.m"
" Runoff coefficient 0.515 0.931 0.764 "
" Maximum flow 0.029 0.086 0.106 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.106 0.164 1.103 1.103"

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411009_100YR POST

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" 33      CATCHMENT 3300"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          3300 Catchment 3300"
"          60.000 % Impervious"
"          0.220 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.088 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.132 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.515 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.931 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.066      0.164      1.103      1.103 c.m/sec"
"          Catchment 3300      Pervious      Impervious      Total Area      "
"          Surface Area      0.088      0.132      0.220      hectare"
"          Time of concentration      10.027      1.421      3.740      minutes"
"          Time to Centroid      104.871      85.423      90.664      minutes"
"          Rainfall depth      97.921      97.921      97.921      mm"
"          Rainfall volume      86.17      129.26      215.43      c.m"
"          Rainfall losses      47.483      6.787      23.065      mm"
"          Runoff depth      50.438      91.134      74.856      mm"
"          Runoff volume      44.39      120.30      164.68      c.m"
"          Runoff coefficient      0.515      0.931      0.764      "
"          Maximum flow      0.019      0.054      0.066      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.066      0.230      1.103      1.103"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"          0.066      0.230      0.230      1.103"
" 64      SHOW TABLE"
"          2  Flow hydrograph"
"          4  Inflow Hydrograph"
"          Maximum flow      0.230      c.m/sec"
"          Hydrograph volume      479.995      c.m"
" 40      HYDROGRAPH Combine 999"
"          6  Combine "
"          999 Node #"
"          To Wetland"
"          Maximum flow      1.129      c.m/sec"
"          Hydrograph volume      11760.858      c.m"
"          0.066      0.230      0.230      1.129"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.066      0.000      0.230      1.129"
" 33      CATCHMENT 2100"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2100 Catchment 2100"

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411009_100YR POST

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" 60.000 % Impervious"
" 2.180 Total Area"
" 40.000 Flow length"
" 2.000 Overland Slope"
" 0.872 Pervious Area"
" 40.000 Pervious length"
" 2.000 Pervious slope"
" 1.308 Impervious Area"
" 40.000 Impervious length"
" 2.000 Impervious slope"
" 0.250 Pervious Manning 'n'"
" 78.000 Pervious SCS Curve No."
" 0.517 Pervious Runoff coefficient"
" 0.100 Pervious Ia/S coefficient"
" 7.164 Pervious Initial abstraction"
" 0.015 Impervious Manning 'n'"
" 98.000 Impervious SCS Curve No."
" 0.929 Impervious Runoff coefficient"
" 0.100 Impervious Ia/S coefficient"
" 0.518 Impervious Initial abstraction"
" 0.616 0.000 0.230 1.129 c.m/sec"
" Catchment 2100 Pervious Impervious Total Area "
" Surface Area 0.872 1.308 2.180 hectare"
" Time of concentration 15.199 2.153 5.683 minutes"
" Time to Centroid 110.688 86.345 92.932 minutes"
" Rainfall depth 97.921 97.921 97.921 mm"
" Rainfall volume 853.87 1280.81 2134.68 c.m"
" Rainfall losses 47.301 6.948 23.089 mm"
" Runoff depth 50.621 90.973 74.832 mm"
" Runoff volume 441.41 1189.93 1631.34 c.m"
" Runoff coefficient 0.517 0.929 0.764 "
" Maximum flow 0.164 0.542 0.616 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
" 0.616 0.616 0.230 1.129"
" 54 POND DESIGN"
" 0.616 Current peak flow c.m/sec"
" 0.396 Target outflow c.m/sec"
" 1631.3 Hydrograph volume c.m"
" 13. Number of stages"
" 0.000 Minimum water level metre"
" 3.000 Maximum water level metre"
" 0.000 Starting water level metre"
" 0 Keep Design Data: 1 = True; 0 = False"
" Level Discharge Volume"
" 410.650 0.000 0.000"
" 410.700 0.00600 25.000"
" 410.800 0.01300 79.000"
" 410.900 0.02000 139.000"
" 411.000 0.02500 206.000"
" 411.100 0.02900 279.000"
" 411.200 0.03200 359.000"
" 411.300 0.2910 446.000"
" 411.400 0.3210 540.000"
" 411.500 0.3470 641.000"
" 411.600 0.3720 750.000"
" 411.650 0.3840 807.000"
" 411.950 2.818 1195.000"
" Peak outflow 0.338 c.m/sec"
" Maximum level 411.469 metre"
" Maximum storage 609.580 c.m"
" Centroidal lag 2.626 hours"
" 0.616 0.616 0.338 1.129 c.m/sec"

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411009_100YR POST

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" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.616      0.338      0.338      1.129"
" 33      CATCHMENT 2200"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2200 Catchment 2200"
"          0.000 % Impervious"
"          0.910 Total Area"
"          40.000 Flow length"
"          2.000 overland slope"
"          0.910 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.000 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.517 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.000 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"          0.172      0.338      0.338      1.129 c.m/sec"
"          Catchment 2200      Pervious      Impervious      Total Area      "
"          Surface Area      0.910      0.000      0.910      hectare"
"          Time of concentration      15.199      2.153      15.198      minutes"
"          Time to Centroid      110.688      86.345      110.688      minutes"
"          Rainfall depth      97.921      97.921      97.921      mm"
"          Rainfall volume      891.08      0.00      891.08      c.m"
"          Rainfall losses      47.301      6.948      47.301      mm"
"          Runoff depth      50.621      90.973      50.621      mm"
"          Runoff volume      460.65      0.00      460.65      c.m"
"          Runoff coefficient      0.517      0.000      0.517      "
"          Maximum flow      0.172      0.000      0.172      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"          0.172      0.504      0.338      1.129"
" 33      CATCHMENT 2300"
"          1 Triangular SCS"
"          1 Equal length"
"          1 SCS method"
"          2300 Catchment 2300"
"          10.000 % Impervious"
"          0.470 Total Area"
"          40.000 Flow length"
"          2.000 Overland slope"
"          0.423 Pervious Area"
"          40.000 Pervious length"
"          2.000 Pervious slope"
"          0.047 Impervious Area"
"          40.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.517 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"

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                                411009_100YR POST
"      0.015  Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.929  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"              0.090    0.504    0.338    1.129 c.m/sec"
"      Catchment 2300      Pervious      Impervious      Total Area  "
"      Surface Area      0.423      0.047      0.470      hectare"
"      Time of concentration 15.199      2.153      13.027      minutes"
"      Time to Centroid    110.688      86.345      106.636      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      414.21      46.02      460.23      c.m"
"      Rainfall losses      47.301      6.948      43.265      mm"
"      Runoff depth        50.621      90.973      54.656      mm"
"      Runoff volume        214.13      42.76      256.88      c.m"
"      Runoff coefficient    0.517      0.929      0.558      "
"      Maximum flow        0.080      0.019      0.090      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4  Add Runoff "
"              0.090    0.593    0.338    1.129"
" 40 HYDROGRAPH Copy to Outflow"
" 8  Copy to Outflow"
"              0.090    0.593    0.593    1.129"
" 64 SHOW TABLE"
" 2  Flow hydrograph"
" 4  Inflow Hydrograph"
"      Maximum flow              0.593    c.m/sec"
"      Hydrograph volume      2354.448    c.m"
" 40 HYDROGRAPH Combine 999"
" 6  Combine "
" 999 Node #"
"      To Wetland"
"      Maximum flow              1.488    c.m/sec"
"      Hydrograph volume      14115.316    c.m"
"              0.090    0.593    0.593    1.488"
" 40 HYDROGRAPH Confluence 999"
" 7  Confluence "
" 999 Node #"
"      To Wetland"
"      Maximum flow              1.488    c.m/sec"
"      Hydrograph volume      14115.318    c.m"
"              0.090    1.488    0.593    0.000"
" 38 START/RE-START TOTALS 999"
" 3  Runoff Totals on EXIT"
"      Total Catchment area              22.620    hectare"
"      Total Impervious area              7.577    hectare"
"      Total % impervious              33.497"
" 19 EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25 rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        C:\Users\akroetsch\Documents\
"          Work in Progress\411009 Ainley Farm SWM Junk"
"          Output filename:                   411009_REG POST.out"
"          Licensee name:                     gmbp"
"          Company                           Hewlett-Packard Company"
"          Date & Time last used:            4/17/2017 at 3:46:47 PM"
" 31      TIME PARAMETERS"
"          60.000 Time Step"
"          2880.000 Max. Storm length"
"          12000.000 Max. Hydrograph"
" 32      STORM Historic"
"          5 Historic"
"          2880.000 Duration"
"          48.000 Rainfall intensity values"
"              2.028      2.028      2.028      2.028      2.028"
"              2.028      2.028      2.028      2.028      2.028"
"              2.028      2.028      2.028      2.028      2.028"
"              2.028      2.028      2.028      2.028      2.028"
"              2.028      2.028      2.028      2.028      2.028"
"              2.028      2.026      2.026      2.026      2.028"
"              2.026      6.000      4.000      6.000      13.000"
"              17.000     13.000     23.000     13.000     13.000"
"              53.000     38.000     13.000"
"          Maximum intensity                    53.000 mm/hr"
"          Total depth                          285.000 mm"
" 33      6 200hyd Hydrograph extension used in this file"
"          CATCHMENT 3100"
"          1 Triangular scs"
"          1 Equal length"
"          1 SCS method"
"          3100 Catchment 3100"
"          60.000 % Impervious"
"          0.400 Total Area"
"          20.000 Flow length"
"          2.000 Overland Slope"
"          0.160 Pervious Area"
"          20.000 Pervious length"
"          2.000 Pervious slope"
"          0.240 Impervious Area"
"          20.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          78.000 Pervious SCS Curve No."
"          0.754 Pervious Runoff coefficient"
"          0.100 Pervious Ia/S coefficient"
"          7.164 Pervious Initial abstraction"
"          0.015 Impervious Manning 'n'"
"          98.000 Impervious SCS Curve No."
"          0.846 Impervious Runoff coefficient"
"          0.100 Impervious Ia/S coefficient"
"          0.518 Impervious Initial abstraction"
"              0.049      0.000      0.000      0.000 c.m/sec"
"          Catchment 3100 Pervious Impervious Total Area "
"          Surface Area      0.160      0.240      0.400      hectare"
"          Time of concentration 12.485      2.243      6.061      minutes"
"          Time to Centroid      2505.276      2290.972      2370.872      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      456.00      684.00      1140.00      c.m"

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"      Rainfall losses      70.073      43.972      54.413      mm"
"      Runoff depth         214.927      241.028      230.587      mm"
"      Runoff volume        343.88       578.47       922.35       c.m"
"      Runoff coefficient    0.754        0.846        0.809        "
"      Maximum flow         0.018        0.031        0.049        c.m/sec"
40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.049      0.049      0.000      0.000"
56    DIVERSION"
"      3100  Node number"
"      0.062  Overflow threshold"
"      1.000  Required diverted fraction"
"      0      Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow      0.000      c.m/sec"
"      Volume of diverted flow    0.000      c.m"
"      DIV03100.200hyd"
"      Major flow at 3100"
"          0.049      0.049      0.049      0.000 c.m/sec"
40    HYDROGRAPH Next link "
"      5  Next link "
"          0.049      0.049      0.049      0.000"
33    CATCHMENT 1000"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      1000  Catchment 1000"
"      50.000  % Impervious"
"      11.280  Total Area"
"      20.000  Flow length"
"      2.000  Overland Slope"
"      5.640  Pervious Area"
"      20.000  Pervious length"
"      2.000  Pervious slope"
"      5.640  Impervious Area"
"      20.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      78.000  Pervious SCS Curve No."
"      0.754  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.846  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          1.367      0.049      0.049      0.000 c.m/sec"
"      Catchment 1000      Pervious      Impervious      Total Area      "
"      Surface Area      5.640      5.640      11.280      hectare"
"      Time of concentration      12.485      2.243      7.071      minutes"
"      Time to Centroid      2505.277      2290.972      2391.990      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1.6074      1.6074      3.2148      ha-m"
"      Rainfall losses      70.073      43.972      57.023      mm"
"      Runoff depth      214.927      241.028      227.977      mm"
"      Runoff volume      1.2122      1.3594      2.5716      ha-m"
"      Runoff coefficient    0.754      0.846      0.800      "
"      Maximum flow      0.642      0.724      1.367      c.m/sec"
40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          1.367      1.416      0.049      0.000"
33    CATCHMENT 1100"
"      1  Triangular SCS"

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"      1 Equal length"
"      1 SCS method"
"     1100 Catchment 1100"
"     0.000 % Impervious"
"     0.470 Total Area"
"    20.000 Flow length"
"     2.000 overland slope"
"     0.470 Pervious Area"
"    20.000 Pervious length"
"     2.000 Pervious slope"
"     0.000 Impervious Area"
"    20.000 Impervious length"
"     2.000 Impervious slope"
"     0.250 Pervious Manning 'n'"
"    78.000 Pervious SCS Curve No."
"     0.754 Pervious Runoff coefficient"
"     0.100 Pervious Ia/S coefficient"
"     7.164 Pervious Initial abstraction"
"     0.015 Impervious Manning 'n'"
"    98.000 Impervious SCS Curve No."
"     0.000 Impervious Runoff coefficient"
"     0.100 Impervious Ia/S coefficient"
"     0.518 Impervious Initial abstraction"
"           0.054 1.416 0.049 0.000 c.m/sec"
"    Catchment 1100 Pervious Impervious Total Area "
"    Surface Area 0.470 0.000 0.470 hectare"
"    Time of concentration 12.485 2.243 12.485 minutes"
"    Time to Centroid 2505.277 2290.972 2505.276 minutes"
"    Rainfall depth 285.000 285.000 285.000 mm"
"    Rainfall volume 1339.50 0.00 1339.50 c.m"
"    Rainfall losses 70.073 43.972 70.073 mm"
"    Runoff depth 214.927 241.028 214.927 mm"
"    Runoff volume 1010.15 0.00 1010.16 c.m"
"    Runoff coefficient 0.754 0.000 0.754 "
"    Maximum flow 0.054 0.000 0.054 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"           0.054 1.469 0.049 0.000"
" 54 POND DESIGN"
"     1.469 Current peak flow c.m/sec"
"     0.756 Target outflow c.m/sec"
"    27648.3 Hydrograph volume c.m"
"     12. Number of stages"
"     0.000 Minimum water level metre"
"     3.000 Maximum water level metre"
"     0.000 Starting water level metre"
"     0 Keep Design Data: 1 = True; 0 = False"
"           Level Discharge Volume"
"     411.000 0.000 0.000"
"     411.100 0.00600 612.000"
"     411.200 0.01300 1233.000"
"     411.300 0.01700 1862.000"
"     411.400 0.02000 2501.000"
"     411.500 0.02300 3148.000"
"     411.600 0.02600 3805.000"
"     411.700 0.1570 4470.000"
"     411.750 0.1630 4806.000"
"     411.800 0.2450 5144.000"
"     411.850 0.2520 5485.000"
"     412.000 1.912 6295.000"
"     Peak outflow 1.322 c.m/sec"
"     Maximum level 411.956 metre"
"     Maximum storage 6057.783 c.m"

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"          Centroidal lag          49.341  hours"
"          0.054  1.469  1.322  0.000  c.m/sec"
40  HYDROGRAPH Next link "
"      5  Next link "
"          0.054  1.322  1.322  0.000"
33  CATCHMENT 4000"
"      1  Triangular SCS"
"      1  Equal length"
"      1  SCS method"
"      4000  Catchment 4000"
"      0.000  % Impervious"
"      6.340  Total Area"
"      60.000  Flow length"
"      2.000  Overland slope"
"      6.340  Pervious Area"
"      60.000  Pervious length"
"      2.000  Pervious slope"
"      0.000  Impervious Area"
"      60.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      74.000  Pervious SCS Curve No."
"      0.716  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      8.924  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"      98.000  Impervious SCS Curve No."
"      0.000  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"          0.688  1.322  1.322  0.000  c.m/sec"
"      Catchment 4000  Pervious  Impervious  Total Area  "
"      Surface Area  6.340  0.000  6.340  hectare"
"      Time of concentration  24.421  4.336  24.421  minutes"
"      Time to Centroid  2549.942  2258.969  2549.942  minutes"
"      Rainfall depth  285.000  285.000  285.000  mm"
"      Rainfall volume  1.8069  0.0000  1.8069  ha-m"
"      Rainfall losses  80.848  39.404  80.848  mm"
"      Runoff depth  204.152  245.596  204.152  mm"
"      Runoff volume  1.2943  0.0000  1.2943  ha-m"
"      Runoff coefficient  0.716  0.000  0.716  "
"      Maximum flow  0.688  0.000  0.688  c.m/sec"
40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.688  1.936  1.322  0.000"
54  POND DESIGN"
"      1.936  Current peak flow  c.m/sec"
"      0.756  Target outflow  c.m/sec"
"      40500.4  Hydrograph volume  c.m"
"      6.  Number of stages"
"      409.630  Minimum water level  metre"
"      410.750  Maximum water level  metre"
"      409.630  Starting water level  metre"
"      0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge  Volume"
"      409.630  0.000  0.000"
"      409.750  0.5400  232.250"
"      410.000  1.632  2017.880"
"      410.250  3.737  5148.940"
"      410.500  1.345  9472.330"
"      410.750  1.885  15057.74"
"      Peak outflow  1.869  c.m/sec"
"      Maximum level  410.028  metre"

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"           Maximum storage           2369.749      c.m"
"           Centroidal lag             47.388      hours"
"           0.688      1.936      1.869      0.000 c.m/sec"
40 HYDROGRAPH Next link "
"           5 Next link "
"           0.688      1.869      1.869      0.000"
52 CHANNEL DESIGN"
"           1.869 Current peak flow      c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           0.000 Basewidth      metre"
"           7.410 Left bank slope"
"           6.000 Right bank slope"
"           0.950 Channel depth      metre"
"           1.040 Gradient      %"
"           Depth of flow              0.495      metre"
"           Velocity                   1.139      m/sec"
"           Channel capacity            10.655     c.m/sec"
"           Critical depth              0.436      metre"
53 ROUTE Channel Route 72"
"           72.40 Channel Route 72 Reach length (metre)"
"           0.377 x-factor <= 0.5"
"           47.653 K-lag (seconds)"
"           0.000 Default(0) or user spec.(1) values used"
"           0.500 x-factor <= 0.5"
"           30.000 K-lag (seconds)"
"           0.500 Beta weighting factor"
"           59.016 Routing time step (seconds)"
"           1 No. of sub-reaches"
"           Peak outflow              1.862      c.m/sec"
"           0.688      1.869      1.862      0.000 c.m/sec"
40 HYDROGRAPH Next link "
"           5 Next link "
"           0.688      1.862      1.862      0.000"
52 CHANNEL DESIGN"
"           1.862 Current peak flow      c.m/sec"
"           0.035 Manning 'n'"
"           0. Cross-section type: 0=trapezoidal; 1=general"
"           2.000 Basewidth      metre"
"           2.950 Left bank slope"
"           3.000 Right bank slope"
"           0.950 Channel depth      metre"
"           1.040 Gradient      %"
"           Depth of flow              0.432      metre"
"           Velocity                   1.310      m/sec"
"           Channel capacity            9.246      c.m/sec"
"           Critical depth              0.368      metre"
53 ROUTE Channel Route 40"
"           39.80 Channel Route 40 Reach length (metre)"
"           0.230 x-factor <= 0.5"
"           22.786 K-lag (seconds)"
"           0.000 Default(0) or user spec.(1) values used"
"           0.500 x-factor <= 0.5"
"           30.000 K-lag (seconds)"
"           0.500 Beta weighting factor"
"           34.951 Routing time step (seconds)"
"           1 No. of sub-reaches"
"           Peak outflow              1.859      c.m/sec"
"           0.688      1.862      1.859      0.000 c.m/sec"
40 HYDROGRAPH Next link "
"           5 Next link "
"           0.688      1.859      1.859      0.000"
40 HYDROGRAPH Copy to Outflow"

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"      8 Copy to Outflow"
"      0.688      1.859      1.859      0.000"
" 64  SHOW TABLE"
"      2 Flow hydrograph"
"      4 Inflow Hydrograph"
"      Maximum flow      1.859      c.m/sec"
"      Hydrograph volume 40659.859      c.m"
" 40  HYDROGRAPH Combine 999"
"      6 Combine "
" 999 Node #"
"      To Wetland"
"      Maximum flow      1.859      c.m/sec"
"      Hydrograph volume 40659.859      c.m"
"      0.688      1.859      1.859      1.859"
" 40  HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"      0.688      0.000      1.859      1.859"
" 47  FILEI_O Read/Open DIV03100.200hyd"
"      1 1=read/open; 2=write/save"
"      2 1=rainfall; 2=hydrograph"
"      1 1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV03100.200hyd"
"      Major flow at 3100"
"      Total volume      0.000      c.m"
"      Maximum flow      0.000      c.m/sec"
"      0.000      0.000      1.859      1.859 c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"      0.000      0.000      1.859      1.859"
" 33  CATCHMENT 3200"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      3200 Catchment 3200"
"      60.000 % Impervious"
"      0.350 Total Area"
"      20.000 Flow length"
"      2.000 Overland slope"
"      0.140 Pervious Area"
"      20.000 Pervious length"
"      2.000 Pervious slope"
"      0.210 Impervious Area"
"      20.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.754 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.846 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"      0.043      0.000      1.859      1.859 c.m/sec"
"      Catchment 3200 Pervious Impervious Total Area "
"      Surface Area      0.140      0.210      0.350      hectare"
"      Time of concentration 12.485      2.243      6.061      minutes"
"      Time to Centroid 2505.277      2290.972      2370.872      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      399.00      598.50      997.50      c.m"
"      Rainfall losses      70.073      43.972      54.413      mm"
"      Runoff depth      214.927      241.028      230.587      mm"

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"	Runoff volume	300.90	506.16	807.06	c.m"
"	Runoff coefficient	0.754	0.846	0.809	"
"	Maximum flow	0.016	0.027	0.043	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.043	0.043	1.859	1.859"	
" 33	CATCHMENT 3300"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	1 SCS method"				
"	3300 Catchment 3300"				
"	60.000 % Impervious"				
"	0.220 Total Area"				
"	20.000 Flow length"				
"	2.000 Overland Slope"				
"	0.088 Pervious Area"				
"	20.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.132 Impervious Area"				
"	20.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	78.000 Pervious SCS Curve No."				
"	0.754 Pervious Runoff coefficient"				
"	0.100 Pervious Ia/S coefficient"				
"	7.164 Pervious Initial abstraction"				
"	0.015 Impervious Manning 'n'"				
"	98.000 Impervious SCS Curve No."				
"	0.846 Impervious Runoff coefficient"				
"	0.100 Impervious Ia/S coefficient"				
"	0.518 Impervious Initial abstraction"				
"	0.027	0.043	1.859	1.859 c.m/sec"	
"	Catchment 3300	Pervious	Impervious	Total Area	"
"	Surface Area	0.088	0.132	0.220	hectare"
"	Time of concentration	12.485	2.243	6.061	minutes"
"	Time to Centroid	2505.277	2290.972	2370.872	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	250.80	376.20	627.00	c.m"
"	Rainfall losses	70.073	43.972	54.413	mm"
"	Runoff depth	214.927	241.028	230.587	mm"
"	Runoff volume	189.14	318.16	507.29	c.m"
"	Runoff coefficient	0.754	0.846	0.809	"
"	Maximum flow	0.010	0.017	0.027	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.027	0.070	1.859	1.859"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.027	0.070	0.070	1.859"	
" 64	SHOW TABLE"				
"	2 Flow hydrograph"				
"	4 Inflow Hydrograph"				
"	Maximum flow		0.070	c.m/sec"	
"	Hydrograph volume		1314.348	c.m"	
" 40	HYDROGRAPH Combine	999"			
"	6 Combine "				
"	999 Node #"				
"	To Wetland"				
"	Maximum flow		1.913	c.m/sec"	
"	Hydrograph volume		41974.203	c.m"	
"	0.027	0.070	0.070	1.913"	
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				

```

"          0.027      0.000      0.070      1.913"
" 33      CATCHMENT 2100"
"          1  Triangular SCS"
"          1  Equal length"
"          1  SCS method"
"          2100  Catchment 2100"
"          60.000  % Impervious"
"          2.180  Total Area"
"          40.000  Flow length"
"          2.000  Overland Slope"
"          0.872  Pervious Area"
"          40.000  Pervious length"
"          2.000  Pervious slope"
"          1.308  Impervious Area"
"          40.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          78.000  Pervious SCS Curve No."
"          0.765  Pervious Runoff coefficient"
"          0.100  Pervious Ia/S coefficient"
"          7.164  Pervious Initial abstraction"
"          0.015  Impervious Manning 'n'"
"          98.000  Impervious SCS Curve No."
"          0.850  Impervious Runoff coefficient"
"          0.100  Impervious Ia/S coefficient"
"          0.518  Impervious Initial abstraction"
"          0.258      0.000      0.070      1.913 c.m/sec"
"          Catchment 2100      Pervious      Impervious      Total Area      "
"          Surface Area      0.872      1.308      2.180      hectare"
"          Time of concentration      18.924      3.399      9.220      minutes"
"          Time to Centroid      2520.774      2266.333      2361.738      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      2485.20      3727.80      6213.00      c.m"
"          Rainfall losses      66.918      42.646      52.354      mm"
"          Runoff depth      218.082      242.354      232.646      mm"
"          Runoff volume      1901.68      3170.00      5071.67      c.m"
"          Runoff coefficient      0.765      0.850      0.816      "
"          Maximum flow      0.094      0.165      0.258      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.258      0.258      0.070      1.913"
" 54      POND DESIGN"
"          0.258  Current peak flow      c.m/sec"
"          0.396  Target outflow      c.m/sec"
"          5071.7  Hydrograph volume      c.m"
"          13.  Number of stages"
"          0.000  Minimum water level      metre"
"          3.000  Maximum water level      metre"
"          0.000  Starting water level      metre"
"          0  Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"          410.650      0.000      0.000"
"          410.700      0.00600      25.000"
"          410.800      0.01300      79.000"
"          410.900      0.02000      139.000"
"          411.000      0.02500      206.000"
"          411.100      0.02900      279.000"
"          411.200      0.03200      359.000"
"          411.300      0.2910      446.000"
"          411.400      0.3210      540.000"
"          411.500      0.3470      641.000"
"          411.600      0.3720      750.000"
"          411.650      0.3840      807.000"

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"          411.950      2.818  411009_REG POST
"          Peak outflow      0.240      c.m/sec"
"          Maximum level      411.285      metre"
"          Maximum storage      432.765      c.m"
"          Centroidal lag      40.573      hours"
"          0.258      0.258      0.240      1.913 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.258      0.240      0.240      1.913"
" 33 CATCHMENT 2200"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2200 Catchment 2200"
"      0.000 % Impervious"
"      0.910 Total Area"
"      40.000 Flow length"
"      2.000 Overland Slope"
"      0.910 Pervious Area"
"      40.000 Pervious length"
"      2.000 Pervious slope"
"      0.000 Impervious Area"
"      40.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      78.000 Pervious SCS Curve No."
"      0.765 Pervious Runoff coefficient"
"      0.100 Pervious Ia/S coefficient"
"      7.164 Pervious Initial abstraction"
"      0.015 Impervious Manning 'n'"
"      98.000 Impervious SCS Curve No."
"      0.000 Impervious Runoff coefficient"
"      0.100 Impervious Ia/S coefficient"
"      0.518 Impervious Initial abstraction"
"          0.098      0.240      0.240      1.913 c.m/sec"
"      Catchment 2200      Pervious      Impervious      Total Area "
"      Surface Area      0.910      0.000      0.910      hectare"
"      Time of concentration      18.924      3.399      18.924      minutes"
"      Time to Centroid      2520.773      2266.333      2520.773      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      2593.50      0.00      2593.50      c.m"
"      Rainfall losses      66.918      42.646      66.918      mm"
"      Runoff depth      218.082      242.354      218.082      mm"
"      Runoff volume      1984.55      0.00      1984.55      c.m"
"      Runoff coefficient      0.765      0.000      0.765      "
"      Maximum flow      0.098      0.000      0.098      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.098      0.338      0.240      1.913"
" 33 CATCHMENT 2300"
"      1 Triangular SCS"
"      1 Equal length"
"      1 SCS method"
"      2300 Catchment 2300"
"      10.000 % Impervious"
"      0.470 Total Area"
"      40.000 Flow length"
"      2.000 Overland Slope"
"      0.423 Pervious Area"
"      40.000 Pervious length"
"      2.000 Pervious slope"
"      0.047 Impervious Area"
"      40.000 Impervious length"

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411009_REG POST

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"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     78.000  Pervious SCS Curve No."
"      0.765  Pervious Runoff coefficient"
"      0.100  Pervious Ia/S coefficient"
"      7.164  Pervious Initial abstraction"
"      0.015  Impervious Manning 'n'"
"     98.000  Impervious SCS Curve No."
"      0.850  Impervious Runoff coefficient"
"      0.100  Impervious Ia/S coefficient"
"      0.518  Impervious Initial abstraction"
"              0.051      0.338      0.240      1.913 c.m/sec"
"      Catchment 2300      Pervious      Impervious      Total Area      "
"      Surface Area      0.423      0.047      0.470      hectare"
"      Time of concentration      18.924      3.399      17.218      minutes"
"      Time to Centroid      2520.774      2266.333      2492.809      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1205.55      133.95      1339.50      c.m"
"      Rainfall losses      66.918      42.646      64.490      mm"
"      Runoff depth      218.082      242.354      220.510      mm"
"      Runoff volume      922.49      113.91      1036.39      c.m"
"      Runoff coefficient      0.765      0.850      0.774      "
"      Maximum flow      0.046      0.006      0.051      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.051      0.389      0.240      1.913"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"              0.051      0.389      0.389      1.913"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow      0.389      c.m/sec"
"      Hydrograph volume      8072.756      c.m"
" 40      HYDROGRAPH Combine      999"
"      6      Combine "
"      999      Node #"
"      To Wetland"
"      Maximum flow      2.277      c.m/sec"
"      Hydrograph volume      50046.957      c.m"
"              0.051      0.389      0.389      2.277"
" 40      HYDROGRAPH Confluence      999"
"      7      Confluence "
"      999      Node #"
"      To Wetland"
"      Maximum flow      2.277      c.m/sec"
"      Hydrograph volume      50046.953      c.m"
"              0.051      2.277      0.389      0.000"
" 38      START/RE-START TOTALS 999"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      22.620      hectare"
"      Total Impervious area      7.577      hectare"
"      Total % impervious      33.497"
" 19      EXIT"

```



Stormceptor Design Summary

PCSWMM for Stormceptor

Project Information

Date	03/08/2017
Project Name	Ainley Farm - STC 1 (Storm Outlet #2)
Project Number	411009
Location	Elora

Designer Information

Company	N/A
Contact	N/A

Notes

N/A

Drainage Area

Total Area (ha)	4.2
Imperviousness (%)	50

The Stormceptor System model STC 9000 achieves the water quality objective removing 83% TSS for a Fine (organics, silts and sand) particle size distribution and 95% runoff volume.

Rainfall

Name	TORONTO CENTRAL
State	ON
ID	100
Years of Records	1982 to 1999
Latitude	45°30'N
Longitude	90°30'W

Water Quality Objective

TSS Removal (%)	80
Runoff Volume (%)	90

Upstream Storage

Storage (ha-m)	Discharge (L/s)
0	0

Stormceptor Sizing Summary

Stormceptor Model	TSS Removal	Runoff Volume
	%	%
STC 300	50	49
STC 750	62	71
STC 1000	62	71
STC 1500	63	71
STC 2000	69	82
STC 3000	70	82
STC 4000	75	89
STC 5000	76	89
STC 6000	79	92
STC 9000	83	95
STC 10000	83	95
STC 14000	86	97



Particle Size Distribution

Removing silt particles from runoff ensures that the majority of the pollutants, such as hydrocarbons and heavy metals that adhere to fine particles, are not discharged into our natural water courses. The table below lists the particle size distribution used to define the annual TSS removal.

Fine (organics, silts and sand)							
Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s	Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s
20	20	1.3	0.0004				
60	20	1.8	0.0016				
150	20	2.2	0.0108				
400	20	2.65	0.0647				
2000	20	2.65	0.2870				

Stormceptor Design Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor version 1.0
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal.
- Only the STC 300 is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 750 to STC 6000 may accommodate multiple inlet pipes.
- Inlet and outlet invert elevation differences are as follows:

Inlet and Outlet Pipe Invert Elevations Differences			
Inlet Pipe Configuration	STC 300	STC 750 to STC 6000	STC 9000 to STC 14000
Single inlet pipe	75 mm	25 mm	75 mm
Multiple inlet pipes	75 mm	75 mm	Only one inlet pipe.

- Design estimates are based on stable site conditions only, after construction is completed.
- Design estimates assume that the storm drain is not submerged during zero flows. For submerged applications, please contact your local Stormceptor representative.
- Design estimates may be modified for specific spills controls. Please contact your local Stormceptor representative for further assistance.
- For pricing inquiries or assistance, please contact Imbrium Systems Inc., 1-800-565-4801.



Stormceptor Design Summary

PCSWMM for Stormceptor

Project Information

Date	02/09/2017
Project Name	Ainley Farm - STC 2 (Storm Outlet #3)
Project Number	411009
Location	Elora

Designer Information

Company	N/A
Contact	N/A

Notes

N/A

Drainage Area

Total Area (ha)	3.01
Imperviousness (%)	50

The Stormceptor System model STC 6000 achieves the water quality objective removing 82% TSS for a Fine (organics, silts and sand) particle size distribution and 95% runoff volume.

Rainfall

Name	TORONTO CENTRAL
State	ON
ID	100
Years of Records	1982 to 1999
Latitude	45°30'N
Longitude	90°30'W

Water Quality Objective

TSS Removal (%)	80
Runoff Volume (%)	90

Upstream Storage

Storage (ha-m)	Discharge (L/s)
0	0

Stormceptor Sizing Summary

Stormceptor Model	TSS Removal	Runoff Volume
	%	%
STC 300	55	58
STC 750	66	78
STC 1000	67	78
STC 1500	67	78
STC 2000	73	86
STC 3000	74	86
STC 4000	79	92
STC 5000	79	92
STC 6000	82	95
STC 9000	86	97
STC 10000	86	97
STC 14000	89	98



Particle Size Distribution

Removing silt particles from runoff ensures that the majority of the pollutants, such as hydrocarbons and heavy metals that adhere to fine particles, are not discharged into our natural water courses. The table below lists the particle size distribution used to define the annual TSS removal.

Fine (organics, silts and sand)							
Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s	Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s
20	20	1.3	0.0004				
60	20	1.8	0.0016				
150	20	2.2	0.0108				
400	20	2.65	0.0647				
2000	20	2.65	0.2870				

Stormceptor Design Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor version 1.0
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal.
- Only the STC 300 is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 750 to STC 6000 may accommodate multiple inlet pipes.
- Inlet and outlet invert elevation differences are as follows:

Inlet and Outlet Pipe Invert Elevations Differences

Inlet Pipe Configuration	STC 300	STC 750 to STC 6000	STC 9000 to STC 14000
Single inlet pipe	75 mm	25 mm	75 mm
Multiple inlet pipes	75 mm	75 mm	Only one inlet pipe.

- Design estimates are based on stable site conditions only, after construction is completed.
- Design estimates assume that the storm drain is not submerged during zero flows. For submerged applications, please contact your local Stormceptor representative.
- Design estimates may be modified for specific spills controls. Please contact your local Stormceptor representative for further assistance.
- For pricing inquiries or assistance, please contact Imbrium Systems Inc., 1-800-565-4801.



Stormceptor Design Summary

PCSWMM for Stormceptor

Project Information

Date	02/09/2017
Project Name	Ainley Farm - STC 1 (Storm Outlet #4)
Project Number	411009
Location	Elora

Designer Information

Company	N/A
Contact	N/A

Notes

N/A

Drainage Area

Total Area (ha)	2.65
Imperviousness (%)	50

The Stormceptor System model STC 4000 achieves the water quality objective removing 80% TSS for a Fine (organics, silts and sand) particle size distribution and 93% runoff volume.

Rainfall

Name	TORONTO CENTRAL
State	ON
ID	100
Years of Records	1982 to 1999
Latitude	45°30'N
Longitude	90°30'W

Water Quality Objective

TSS Removal (%)	80
Runoff Volume (%)	90

Upstream Storage

Storage (ha-m)	Discharge (L/s)
0	0

Stormceptor Sizing Summary

Stormceptor Model	TSS Removal	Runoff Volume
	%	%
STC 300	57	61
STC 750	68	80
STC 1000	68	80
STC 1500	69	80
STC 2000	75	88
STC 3000	76	88
STC 4000	80	93
STC 5000	81	93
STC 6000	84	95
STC 9000	87	97
STC 10000	87	97
STC 14000	90	98



Particle Size Distribution

Removing silt particles from runoff ensures that the majority of the pollutants, such as hydrocarbons and heavy metals that adhere to fine particles, are not discharged into our natural water courses. The table below lists the particle size distribution used to define the annual TSS removal.

Fine (organics, silts and sand)							
Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s	Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s
20	20	1.3	0.0004				
60	20	1.8	0.0016				
150	20	2.2	0.0108				
400	20	2.65	0.0647				
2000	20	2.65	0.2870				

Stormceptor Design Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor version 1.0
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal.
- Only the STC 300 is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 750 to STC 6000 may accommodate multiple inlet pipes.
- Inlet and outlet invert elevation differences are as follows:

Inlet and Outlet Pipe Invert Elevations Differences

Inlet Pipe Configuration	STC 300	STC 750 to STC 6000	STC 9000 to STC 14000
Single inlet pipe	75 mm	25 mm	75 mm
Multiple inlet pipes	75 mm	75 mm	Only one inlet pipe.

- Design estimates are based on stable site conditions only, after construction is completed.
- Design estimates assume that the storm drain is not submerged during zero flows. For submerged applications, please contact your local Stormceptor representative.
- Design estimates may be modified for specific spills controls. Please contact your local Stormceptor representative for further assistance.
- For pricing inquiries or assistance, please contact Imbrium Systems Inc., 1-800-565-4801.



Stormceptor Design Summary

PCSWMM for Stormceptor

Project Information

Date	02/09/2017
Project Name	Ainley Farm - STC 4 (Storm Outlet #5)
Project Number	411009
Location	Elora

Designer Information

Company	N/A
Contact	N/A

Notes

N/A

Drainage Area

Total Area (ha)	2.43
Imperviousness (%)	60

The Stormceptor System model STC 5000 achieves the water quality objective removing 80% TSS for a Fine (organics, silts and sand) particle size distribution and 93% runoff volume.

Rainfall

Name	TORONTO CENTRAL
State	ON
ID	100
Years of Records	1982 to 1999
Latitude	45°30'N
Longitude	90°30'W

Water Quality Objective

TSS Removal (%)	80
Runoff Volume (%)	90

Upstream Storage

Storage (ha-m)	Discharge (L/s)
0	0

Stormceptor Sizing Summary

Stormceptor Model	TSS Removal	Runoff Volume
	%	%
STC 300	56	59
STC 750	67	79
STC 1000	67	79
STC 1500	68	79
STC 2000	74	87
STC 3000	75	87
STC 4000	79	93
STC 5000	80	93
STC 6000	83	95
STC 9000	86	97
STC 10000	86	97
STC 14000	89	98



Particle Size Distribution

Removing silt particles from runoff ensures that the majority of the pollutants, such as hydrocarbons and heavy metals that adhere to fine particles, are not discharged into our natural water courses. The table below lists the particle size distribution used to define the annual TSS removal.

Fine (organics, silts and sand)							
Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s	Particle Size µm	Distribution %	Specific Gravity	Settling Velocity m/s
20	20	1.3	0.0004				
60	20	1.8	0.0016				
150	20	2.2	0.0108				
400	20	2.65	0.0647				
2000	20	2.65	0.2870				

Stormceptor Design Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor version 1.0
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal.
- Only the STC 300 is adaptable to function with a catch basin inlet and/or inline pipes.
- Only the Stormceptor models STC 750 to STC 6000 may accommodate multiple inlet pipes.
- Inlet and outlet invert elevation differences are as follows:

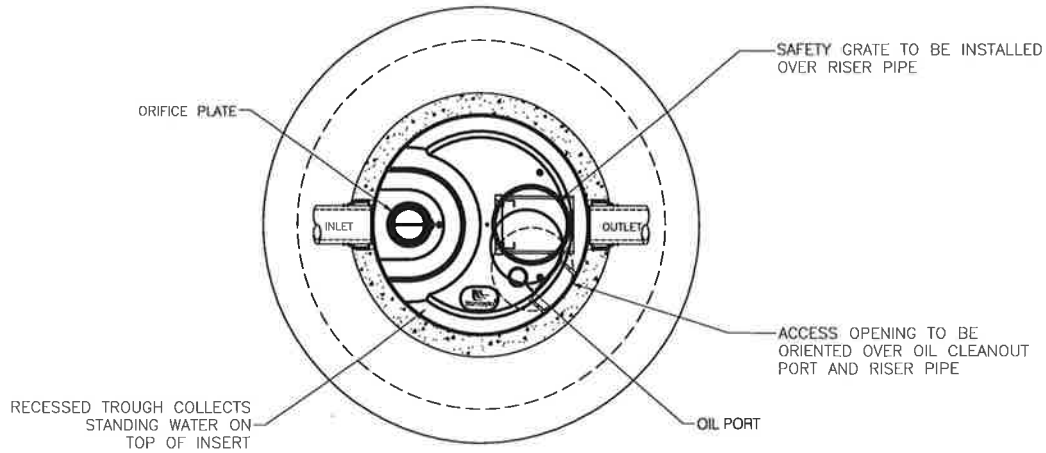
Inlet and Outlet Pipe Invert Elevations Differences			
Inlet Pipe Configuration	STC 300	STC 750 to STC 6000	STC 9000 to STC 14000
Single inlet pipe	75 mm	25 mm	75 mm
Multiple inlet pipes	75 mm	75 mm	Only one inlet pipe.

- Design estimates are based on stable site conditions only, after construction is completed.
- Design estimates assume that the storm drain is not submerged during zero flows. For submerged applications, please contact your local Stormceptor representative.
- Design estimates may be modified for specific spills controls. Please contact your local Stormceptor representative for further assistance.
- For pricing inquiries or assistance, please contact Imbrium Systems Inc., 1-800-565-4801.

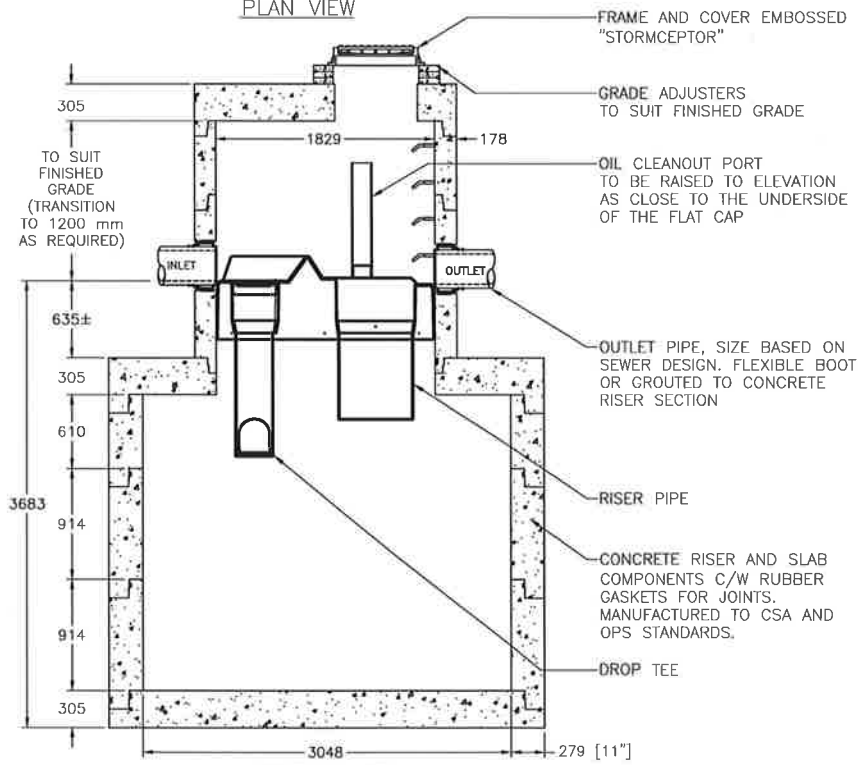
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THE STORMCEPTOR SYSTEM IS PROTECTED BY ONE OR MORE OF THE FOLLOWING PATENTS:

United States Patent No. 5,753,115 • 5,849,181 • 6,068,765 • 6,371,690 • 7,582,216 • 7,666,303 | Australia Patent No. 729,096 • 779,401 • 2008,279,378 • 2008,288,900 |
 Canadian Patent No. 2,206,338 • 2,327,768 • 2,694,159 • 2,697,287 | European Patent No. EP 2,176,171 | Indonesian Patent No. 0,007,058 | Japan Patent No. 3,581,233 • 9-11476 • 5,555,160 |
 Korea Patent No. 10-1451593 • 0519,212 | Malaysia Patent No. 116,987 | New Zealand Patent No. 314,646 • 583,583 • 583,008 | South African Patent No. 2010,00683 • 2010,01796 |




PLAN VIEW



SECTION VIEW

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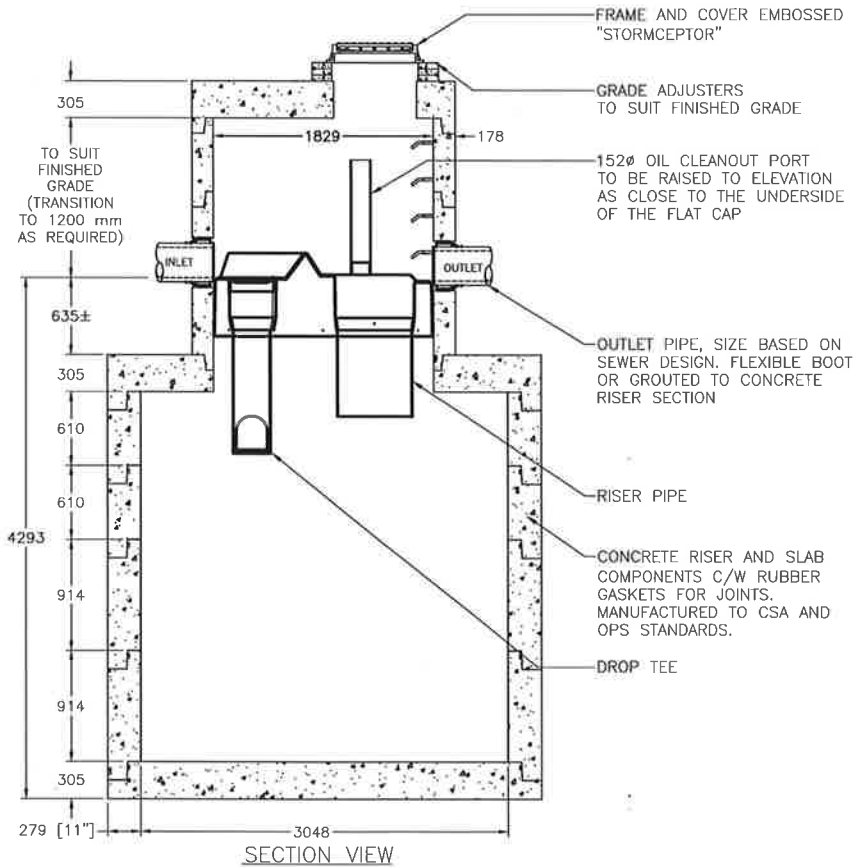
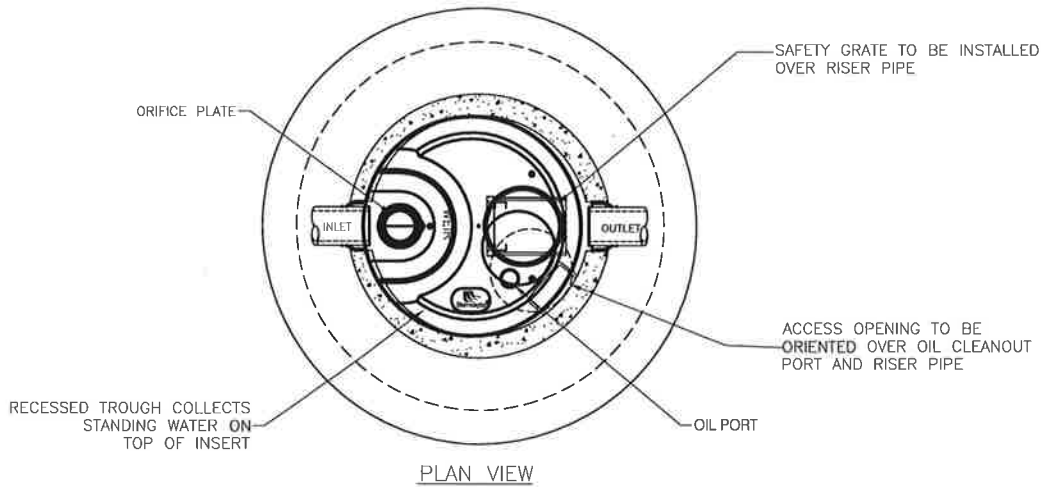
 605 GLOBAL WAY, SUITE 113, LINTHICUM, MD 21231 USA 888-279-8826 CA 800-585-4801 INTL +1-416-960-9900	STC 4000 STANDARD MODEL #####	REV # DATE REVISION DESCRIPTION BY	SHEET NUMBER 1 OF 1	
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	DATE: #####	SCALE: 50		PROJECT No.: #####
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USA 888-279-8826 CA 800-585-4801 INTL +1-416-960-9900	DATE: ##### SCALE: 50	PROJECT No.: ##### DRAWN: ### CHECKED: ###		

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United States Patent No. 5,753,115 • 5,849,181 • 6,068,765 • 6,371,690 • 7,582,216 • 7,666,303 | Australia Patent No. 729,096 • 779,401 • 2008,279,378 • 2008,288,900 |
 Canadian Patent No. 2,206,336 • 2,327,768 • 2,694,159 • 2,697,287 | European Patent No. EP 2,176,171 | Indonesian Patent No. 0,007,058 | Japan Patent No. 3,581,233 • 9-11476 • 5,555,160 |
 Korea Patent No. 10-1451593 • 0519,212 | Malaysia Patent No. 118,987 | New Zealand Patent No. 314,646 • 583,583 • 583,008 | South African Patent No. 2010,00683 • 2010,01796 |



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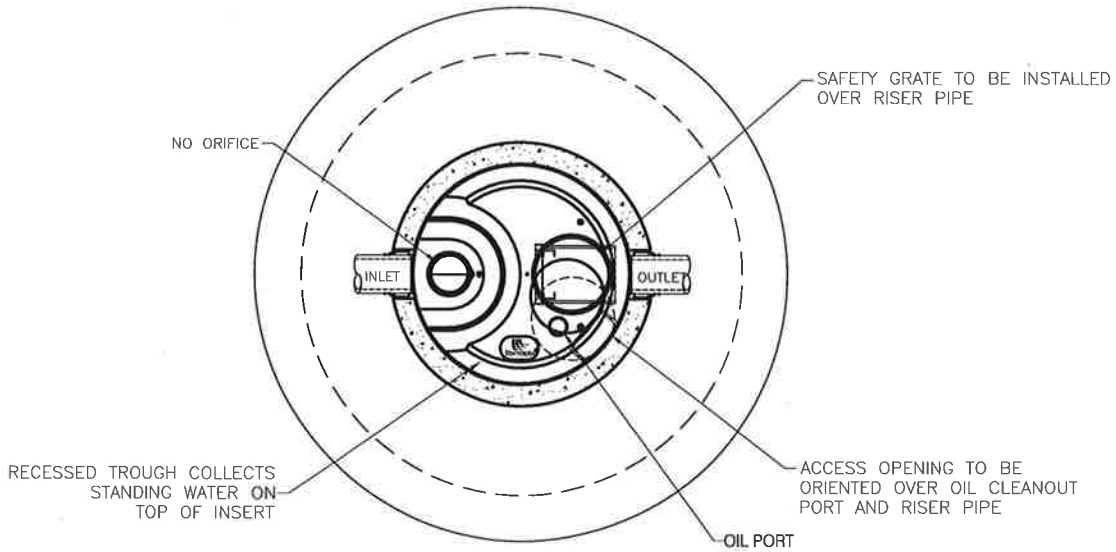
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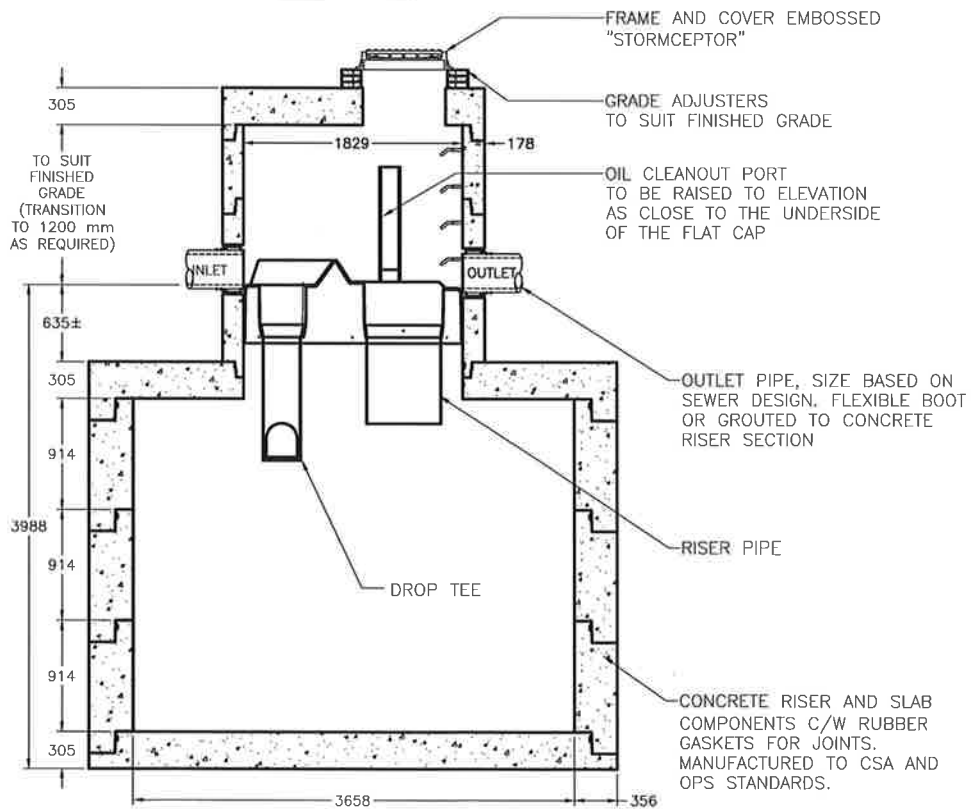
United States Patent No. 5,753,115 • 5,849,181 • 6,068,785 • 6,371,690 • 7,582,216 • 7,666,303 | Australia Patent No. 729,096 • 779,401 • 2008,279,378 • 2008,288,900 |

Canadian Patent No. 2,206,338 • 2,327,768 • 2,694,159 • 2,697,287 | Indonesian Patent No. 007058 | Japan Patent No. 9-11476 • 3,581,233 • 5,555,160 |

Korea Patent No. 10-1451593 • 0519212 | Malaysia Patent No. 118987 | New Zealand Patent No. 314,646 • 583,583 • 583,008 | South African Patent No. 2010,00683 • 2010,01796 |




PLAN VIEW



SECTION VIEW

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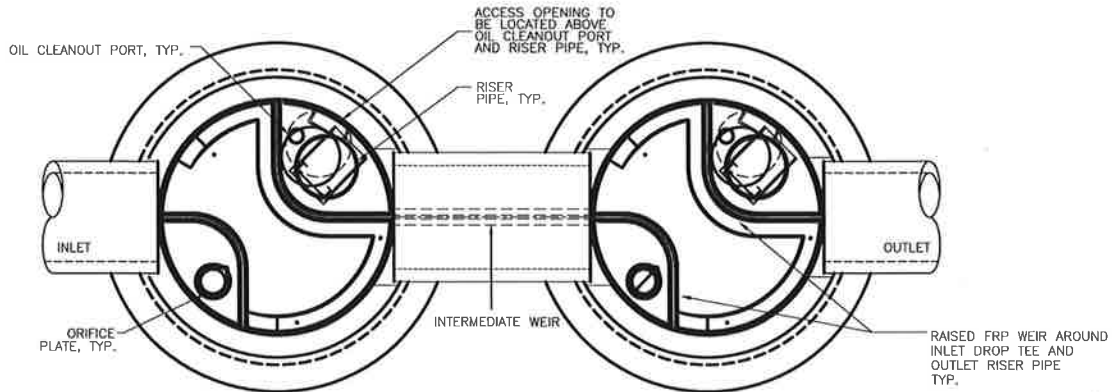
 <p>7037 RIDGE ROAD, SUITE 305, HANOVER, MD 21076</p> <p>866-740-3318 410-798-5505 866-376-8511 FAX</p>	<p>STC 6000 STANDARD MODEL</p> <p>####</p>		REV #	DATE	REVISION DESCRIPTION	BY	SHEET NUMBER
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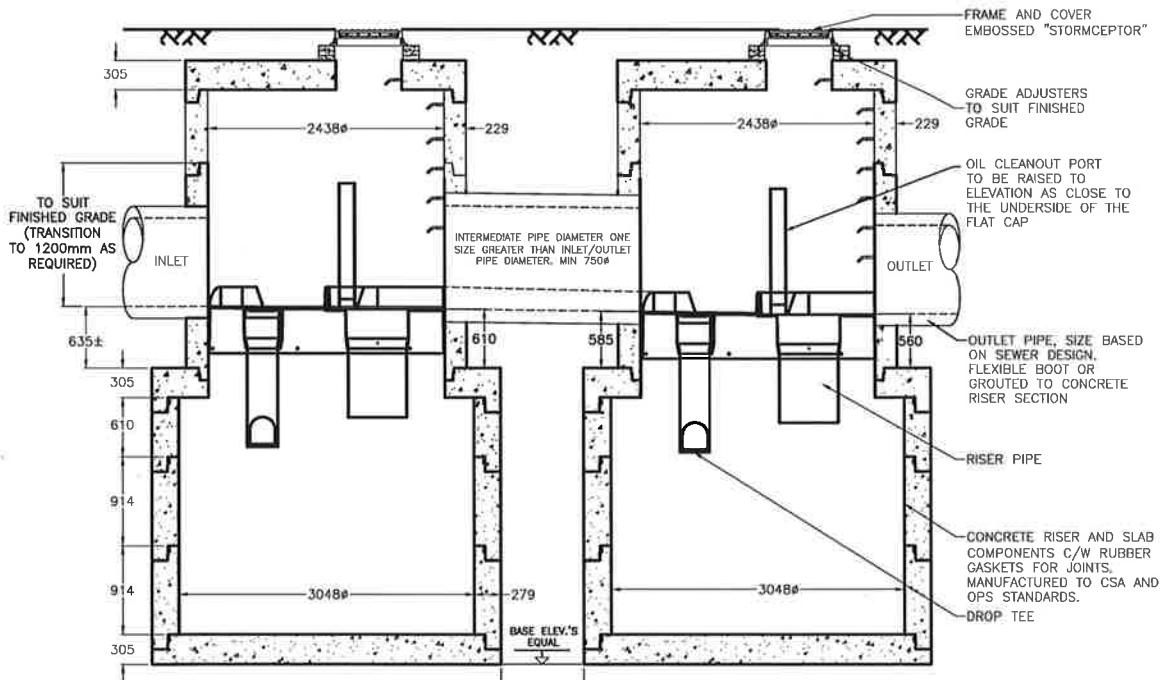
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THE STORMCEPTOR SYSTEM IS PROTECTED BY ONE OR MORE OF THE FOLLOWING PATENTS:

United States Patent No. 5,753,115 • 5,849,181 • 6,068,765 • 6,371,690 • 7,582,216 • 7,666,303 | Australia Patent No. 729,096 • 779,401 • 2008,279,378 • 2008,288,900 |
 Canadian Patent No. 2,206,398 • 2,327,768 • 2,694,159 • 2,697,287 | Indonesian Patent No. 007058 | Japan Patent No. 9-11476 • 3,581,233 • 5,555,160 |
 Korea Patent No. 10-1451593 • 0519212 | Malaysia Patent No. 118987 | New Zealand Patent No. 314,846 • 583,583 • 583,008 | South African Patent No. 2010,00683 • 2010,01796 |



PLAN VIEW




SECTION VIEW

Stormceptor®

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