



**Residential Development,
Woolwich Street & Irvine Street,
Elora, ON
Transportation Impact Study**

Paradigm Transportation Solutions Limited

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Client

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Executive Summary

Content

Paradigm Transportation Solutions Limited (Paradigm) was retained to conduct this Transportation Impact Study for a residential development located in the southwest corner of Woolwich Street/Nichole Road 15 and Irvine Street in the community of Elora, Township of Centre Wellington, Ontario.

This Transportation Impact Study (TIS) includes an analysis of existing traffic conditions, a description of the proposed development, traffic forecasts for the assumed full build-out (2026) and five-year horizon (2031) from the assumed build-out, and any recommendations required to manage future traffic conditions.

Development Concept

The property owner is proposing to develop the approximately 12.4-hectare block into 286 residential units, in a mix of townhouses (131 units) and single detached homes (155 units). Vehicle access is proposed via new municipal street connection to Irvine Street and the extension of Marr Drive from Bricker Avenue.

Conclusions

Based on the investigations carried out, it is concluded that:

- ▶ **Existing Traffic Conditions:** The study area intersections are currently operating within acceptable levels of service with no specific problem movements during the AM and PM peak hours.
- ▶ **Development Trip Generation:** The residential development is forecast to generate approximately 174 and 227 trips during the AM and PM peak hours upon full build-out.
- ▶ **2026 Background Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements during the AM and PM peak hours.
- ▶ **2026 Total Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service during the AM and PM peak hours with no specific problem movements.
- ▶ The proposed municipal street connection to Irvine Street is forecast to operate within acceptable levels of service during the AM and PM peak hours.



- ▶ The addition of the site generated traffic increases the overall delay at the study area intersections by two second or less during the AM and PM peak hours.
- ▶ **2031 Background Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements during the AM and PM peak hours.
- ▶ **2031 Total Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service during the AM and PM peak hours with no specific problem movements.
- ▶ The proposed municipal street connection to Irvine Street is forecast to operate within acceptable levels of service during the AM and PM peak hours.
- ▶ The addition of the site generated traffic increases the overall delay at the study area intersections by three second or less during the AM and PM peak hours.
- ▶ **Remedial Measures:** Left-turn lanes are not warranted at the following intersections:
 - Westbound on Nichol Road 15 at Irvine;
 - Northbound on Irvine Street at Bricker Avenue;
 - Eastbound on East Mill Street (WR 18) at Irvine Street; and
 - Northbound on Irvine Street at Street A.
- ▶ **Alternative 2031 Scenario** with future Elora Sands and Keating developments identified critical movements at the following intersections under 2031 total traffic conditions:
 - Geddes Street (Wellington Road 18) at James Street); and
 - East Mill Street (Wellington Road 18) at Irvine Street.

Recommendations

Based on the findings of this study, it is recommended that the development application be approved with no provision for off-site transportation network improvements.

It is recommended that the County of Wellington and Township of Centre Wellington include the intersections of Geddes Street at James Street and East Mill Street at Irvine Street for the traffic studies of both Elora Sands and Keating future developments to ensure the proper form of traffic control is identified for future traffic conditions.



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1 Introduction

1.1 Overview

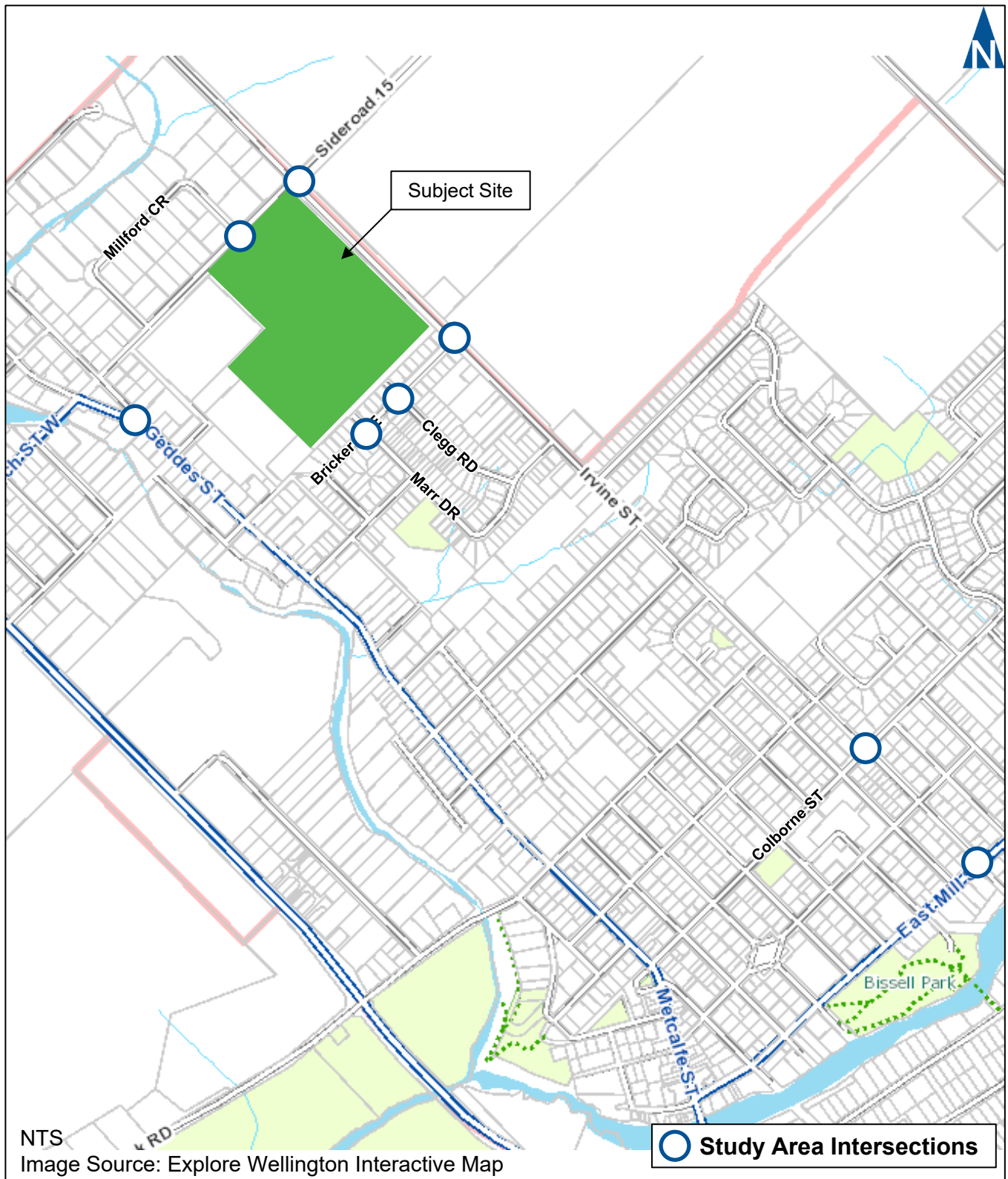
Cachet Developments retained Paradigm Transportation Solutions Limited (Paradigm) to conduct this Transportation Impact Study (TIS) for a residential development located in the southwest corner of Woolwich Street/Nichol Road 15 and Irvine Street in the community of Elora, Township of Centre Wellington, Ontario. **Figure 1.1** illustrates the location of the subject site.

This study determines the impacts of the additional traffic on the surrounding road network, and the remedial measures necessary (if any) to accommodate future traffic in a satisfactory manner. The scope of the study includes:

- ▶ Assessment of the current traffic and site conditions within the study area;
- ▶ Estimates of background traffic growth;
- ▶ Estimates of additional traffic generated by the subject site;
- ▶ Analysis of the impact of the future traffic on the surrounding road network for assumed full build-out (year 2026) and five-years after full build-out (year 2031) horizon years; and
- ▶ Recommendations necessary to mitigate this future traffic in a satisfactory manner.

The study scope was developed in consultation with the Township of Centre Wellington and County of Wellington via email in December 2021. **Appendix A** contains the pre-study consultation material and response from the Township and County.





1.2 Study Area

The intersections assessed in this study include:

- ▶ Woolwich Street/Nichol Road 15 and Irvine Street (unsignalized);
- ▶ Woolwich Street and Milford Crescent (unsignalized);
- ▶ Irvine Street and Bricker Avenue (unsignalized);
- ▶ Irvine Street and Colborne Street (unsignalized);
- ▶ Irvine Street and East Mill Street (unsignalized);
- ▶ Geddes Street and James Street (unsignalized);
- ▶ Bricker Avenue and Clegg Road (unsignalized);
- ▶ Bricker Avenue and Marr Drive (unsignalized); and
- ▶ One new municipal connection to Irvine Street.



2 Existing Conditions

2.1 Road Characteristics

The roadways are under the jurisdiction of the County of Wellington¹ and Township of Centre Wellington² and are generally described as follows:

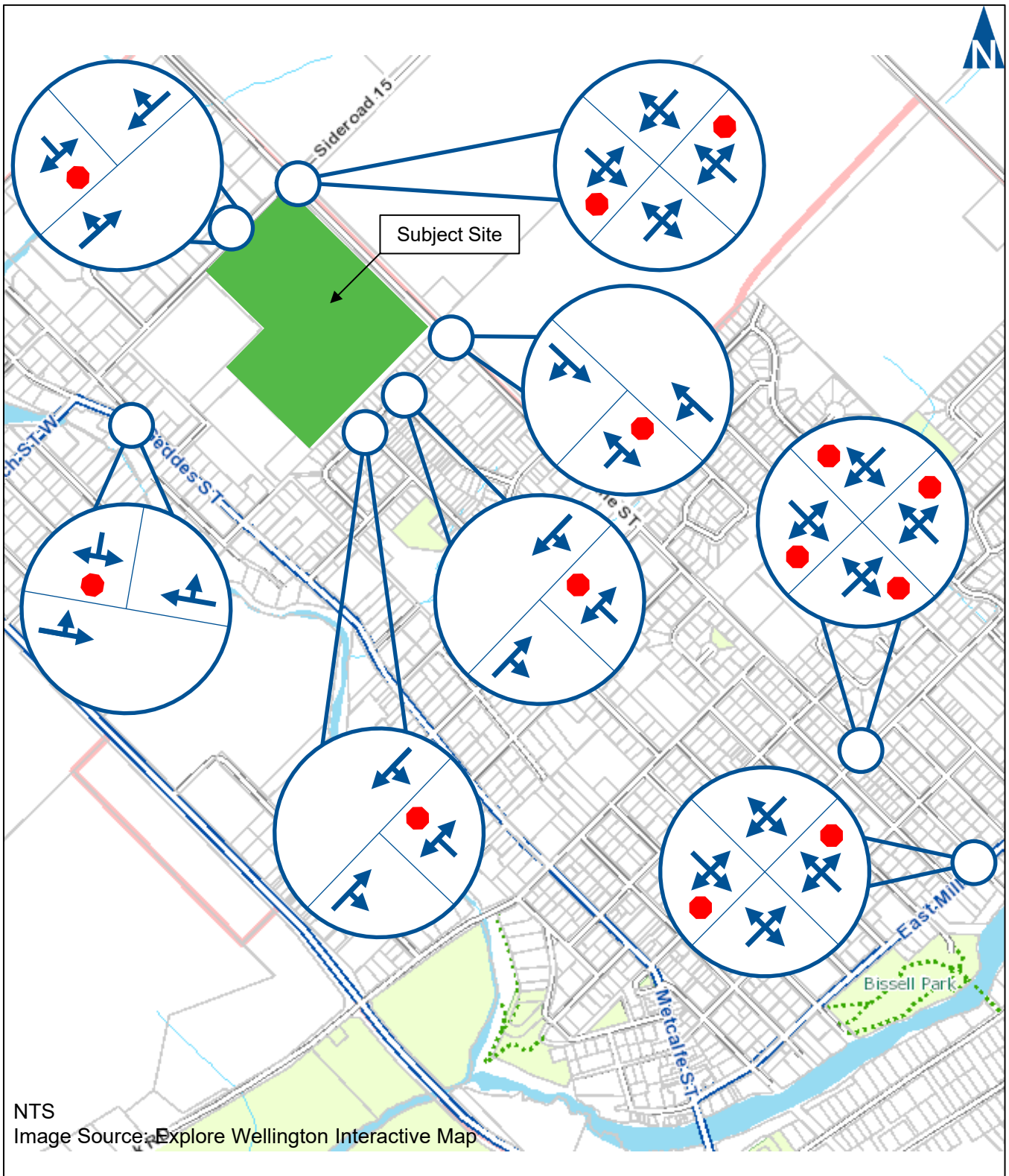
- ▶ **Woolwich Street/Nichol Road 15** is an east-west Township collector roadway with a two-lane cross-section. It has a posted speed limit of 40 km/h from James Street to east of Irvine Street where it transitions to a 80 km/h speed limit. A sidewalk is provided on the southside of the roadway from James Street to the east driveway of the public school.
- ▶ **Irvine Street** is a north-south Township collector roadway with a two-lane cross-section. Between Woolwich Street and Bricker Avenue, Irvine Street is gravel with a speed limit of 50 km/h. South of Marr Drive is has a posted speed limit of 40 km/h. A sidewalk is provided on the east side of the roadway from East Mill Street to Marr Drive, then on the west side of the roadway between Marr Drive and Bricker Avenue.
- ▶ **Colborne Street** is an east-west Township collector roadway with a posted speed limit of 40 km/h. A sidewalk is provided on the north side of the roadway in the study area.
- ▶ **East Mill Street (Wellington Road 18)** is an east-west County arterial roadway with a two-lane cross-section and a posted speed limit of 40 km/h. A sidewalk is provided on the north side of the roadway within the study area.
- ▶ **Geddes Street (Wellington Road 18)** is a north-south County arterial roadway with a two-lane cross-section and a posted speed limit of 50 km/h. A sidewalk is provided on the east side of the roadway within the study area.
- ▶ **Bricker Avenue, Clegg Drive, and Marr Drive** are local Township residential roads with two-lane cross-sections. Sidewalks are provided on both sides of the roadways of Bricker Avenue and Marr Drive. A sidewalk is provided on the west side only of Clegg Drive.

Figure 2.1 details the existing traffic control and lane configurations at the study area intersections.

¹ County of Wellington Official Plan, Schedule A1 Centre Wellington

² Township of Centre Wellington Transportation Master Plan, January 2019, Figure 12 Principal Roadway Classification Elora and Fergus





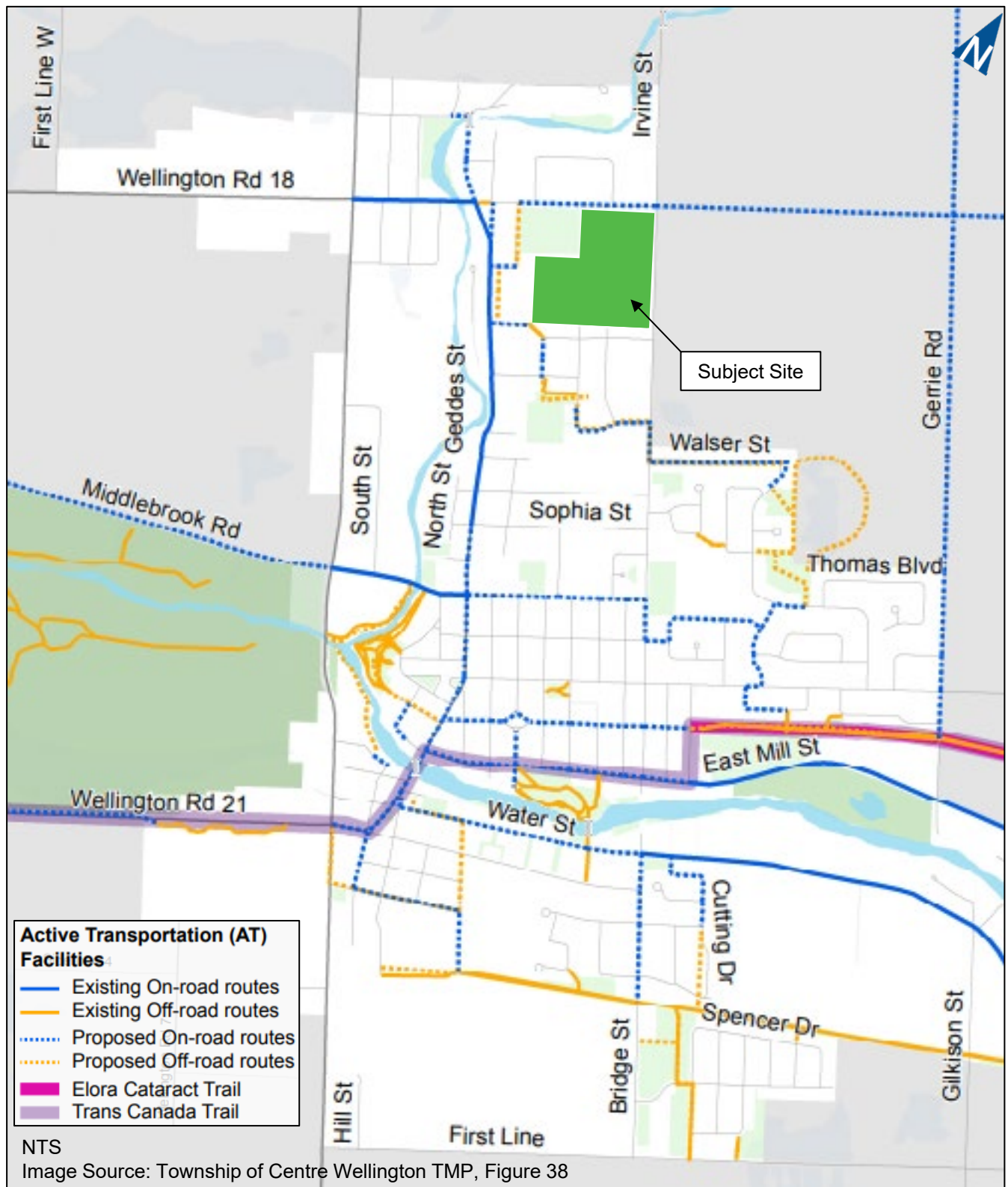
Existing Lane Configurations & Traffic Control

2.2 Active Transportation

Near the subject site, there are sidewalks on Irvine Street from north of Bricker Avenue southwards, on Woolwich Street from Salem Public School westwards, on both sides of Bricker Avenue, and on Marr Drive and Clegg Road.

Figure 2.2 illustrates the existing and proposed active transportation network in the community of Elora. It shows an existing on-road route along Geddes Street. Proposed routes include Woolwich Street/Nichol Road 15.





Cycle and Pedestrian Network

2.3 Traffic Volumes

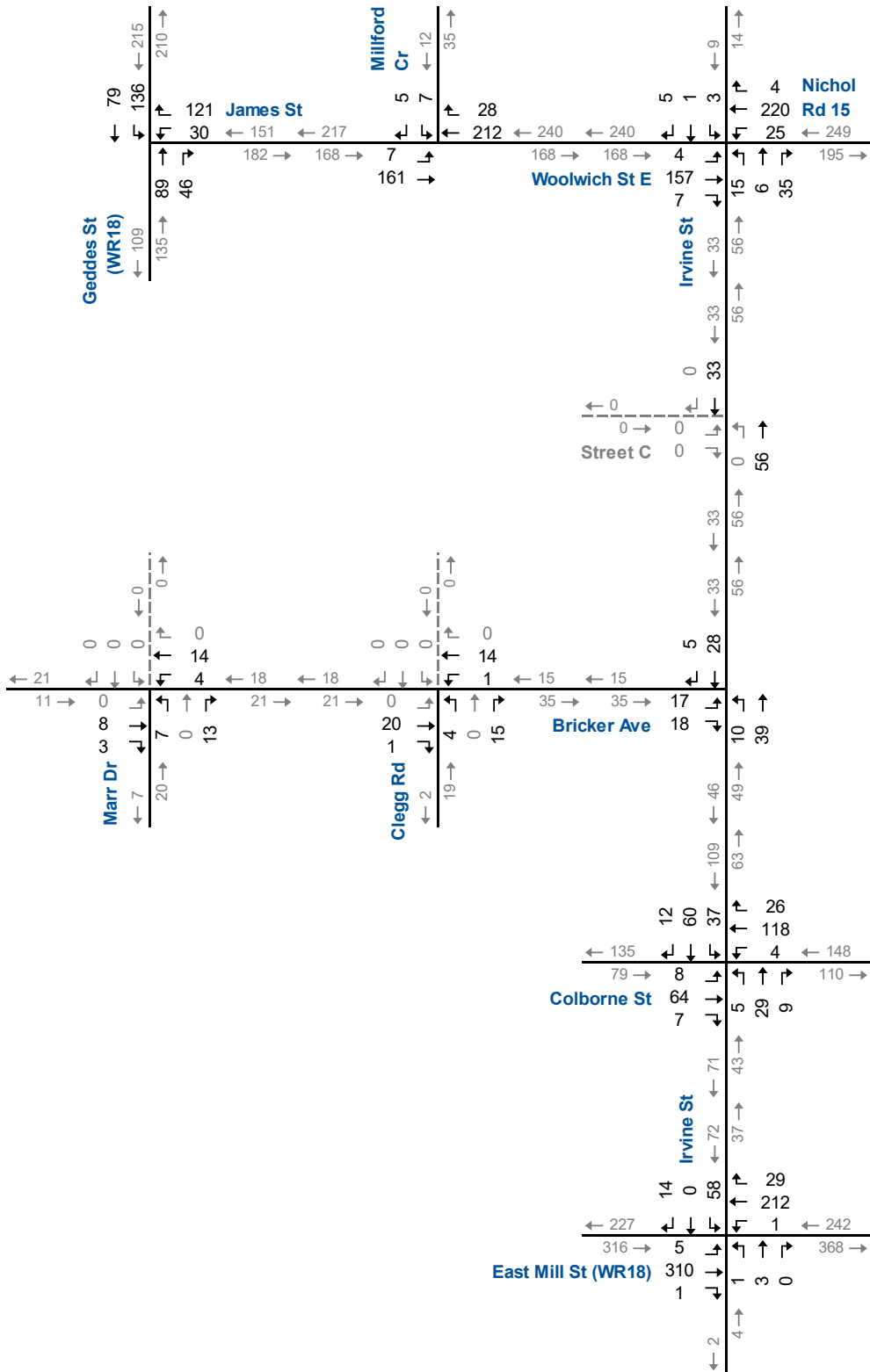
Traffic volumes were counted in November 2021 and February 2022 at the study area intersections. A factor³ was applied to the turning movement counts to account for any discrepancies due to lockdown and traveling restrictions.

Figure 2.3A-B display the factored base year weekday AM and PM peak hour traffic volumes respectively.

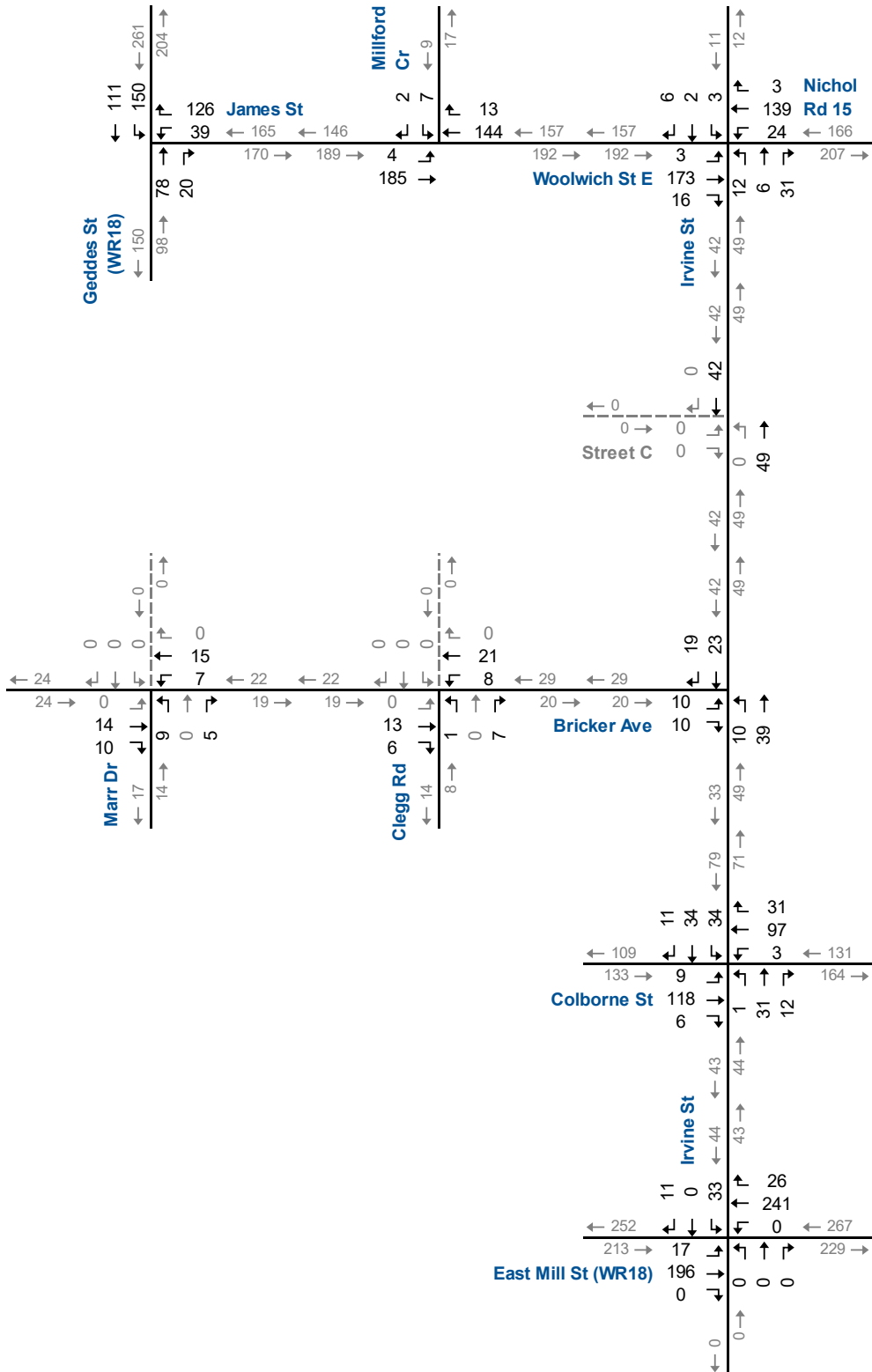
Appendix B contains the detailed historic traffic counts for the study area intersections.

³ A factor of 1.3 was applied to the AM peak hour traffic volumes and 1.1 was applied to the PM peak hour traffic volumes. Factors are from other intersections in Centre Wellington where current pandemic and historic turning movements counts were available.





Base Year Traffic Volumes AM Peak Hour



Base Year Traffic Volumes PM Peak Hour

2.4 Traffic Operations

Intersection level of service (LOS) is a recognized method of quantifying the average delay experienced by drivers at intersections. It is based on the delay experienced by individual vehicles executing the various movements. The delay is related to the number of vehicles intending to make a particular movement, compared to the estimated capacity for that movement. The capacity is based on a number of criteria related to the opposing traffic flows and intersection geometry.

The highest possible rating is LOS A, under which the average total delay is equal or less than 10.0 seconds per vehicle. When the average delay exceeds 80 seconds for signalized intersections, 50 seconds for unsignalized intersections or when the volume to capacity ratio is greater than 1.0, the movement is classed as LOS F and remedial measures are usually implemented if they are feasible. LOS E is usually used as a guideline for the determination of road improvement needs on through lanes, while LOS F may be acceptable for left-turn movements at peak times, depending on delays.

The operations of the study intersections were evaluated using the existing lane configurations, traffic controls, and the base year traffic peak volumes. The level of service conditions on the existing road network have been assessed using Synchro 10.

Table 2.1A-B summarizes the existing intersection operations with the entries in the table indicating level of service (LOS), volume to capacity ratios (V/C), and 95th percentile queues experienced for the weekday AM and PM peak hours, respectively.

The study area intersections are currently operating with acceptable levels of service with no specific problem movements.

Appendix C contains the detailed Synchro 10 reports.



TABLE 2.1A: BASE YEAR OPERATIONS (AM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0 > >	< < < <	A 0 0.15 0	> > > >	A 0 > >	< < < <	B 11 0.02 1	> > > >	B 11 > >	< < < <	B 11 > >	A 1 > >		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 1 0.02 1	> > > >	A 1 > >	< < < <	B 11 0.09 2	> > > >	B 11 > >	< < < <	B 11 > >	A 2 > >		
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 9 0.04 1	> > > >	A 9 > >	< < < <	A 9 0.01 0	> > > >	A 2 > >	A 2 > >	< < < <	A 2 0.01 0	> > > >	A 2 > >	< < < <	A 0 0.02 0	> > > >	A 3 > >	
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 9 0.02 1	> > > >	A 9 > >	< < < <	A 0 0.00 0	> > > >	A 3 > >	
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0.0	> > > >	A 0 > >	< < < <	A 2 0.00 0.1	> > > >	A 2 > >	< < < <	A 10 0.03 0.6	> > > >	A 10 > >	< < < <	A 0 0.00 0.0	> > > >	A 5 > >	
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q					B 11 0.22 6	> > > >	B 11 > >			A 0 0.09 0	> > > >	A 0 > >	< < < <	A 5 0.11 3		A 5 > >	A 6 > >
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	A 8 0.11 >	> > > >	A 8 > >	< < < <	A 8 0.20 >	> > > >	A 8 > >	< < < <	A 8 0.07 >	> > > >	A 8 > >	< < < <	A 9 0.16 >	> > > >	A 9 > >	
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	B 15 0.01 0	> > > >	B 15 > >	< < < <	C 16 0.19 5	> > > >	C 16 > >	A 2 > >

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



TABLE 2.1B: BASE YEAR OPERATIONS (PM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< 0 < 0	A 0 0.00 0	> > > >	A 0 > >	< 0 > >	A 0 > >	< 0 > >	A 0 > >	< 0 > >	< 0 > >	B 10 0.02 0	> > > >	B 10 > >	A 0 > >	A 0 > >		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< 0 < 0	A 0 > >	> > > >	A 1 > >	< 0.02 > >	A 1 > >	< 0 > >	B 11 > >	< 0.08 > >	B 11 > >	< 0 > >	B 10 > >	> > > >	B 10 > >	A 2 > >		
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 9 0.02 1	> > > >	A 9 > >	> > > >	A 9 > >	> > > >	A 2 > >	> 0.01 > >	A 2 > >	> 0 > >	A 0 > >	A 0 > >	A 0 > >	A 0 > >	A 2 > >		
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< 0 < 0	A 0 > >	> > > >	A 2 > >	< 0.01 > >	A 2 > >	< 0 > >	A 9 > >	< 0.01 > >	A 9 > >	< 0 > >	A 0 > >	> > > >	A 0 > >	A 2 > >		
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< 0 < 0	A 0 > >	> > > >	A 3 > >	< 0.01 > >	A 3 > >	< 0 > >	A 10 > >	< 0.02 > >	A 10 > >	< 0 > >	A 0 > >	> > > >	A 0 > >	A 3 > >		
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q				B 11 0.24 7	> > > >	B 11 > >	> > > >	A 0 > >	> 0.06 > >	A 0 > >	> 0 > >	A 5 > >	> 0.11 > >	A 5 > >	A 6 > >		
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< 8 < 0.18	A 0.18 > >	> > > >	A 8 > >	< 0.17 > >	A 8 > >	< 0 > >	A 8 > >	< 0.06 > >	A 8 > >	< 0 > >	A 8 > >	> > > >	A 8 > >	A 8 > >		
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< 1 < 0.01	A 1 > >	> > > >	A 1 > >	< 0 < 0.00	A 0 > >	< 0 > >	A 0 > >	< 0 > >	A 0 > >	< 0 > >	B 14 0.11 3	> > > >	B 14 > >	A 2 > >		

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



3 Development Concept

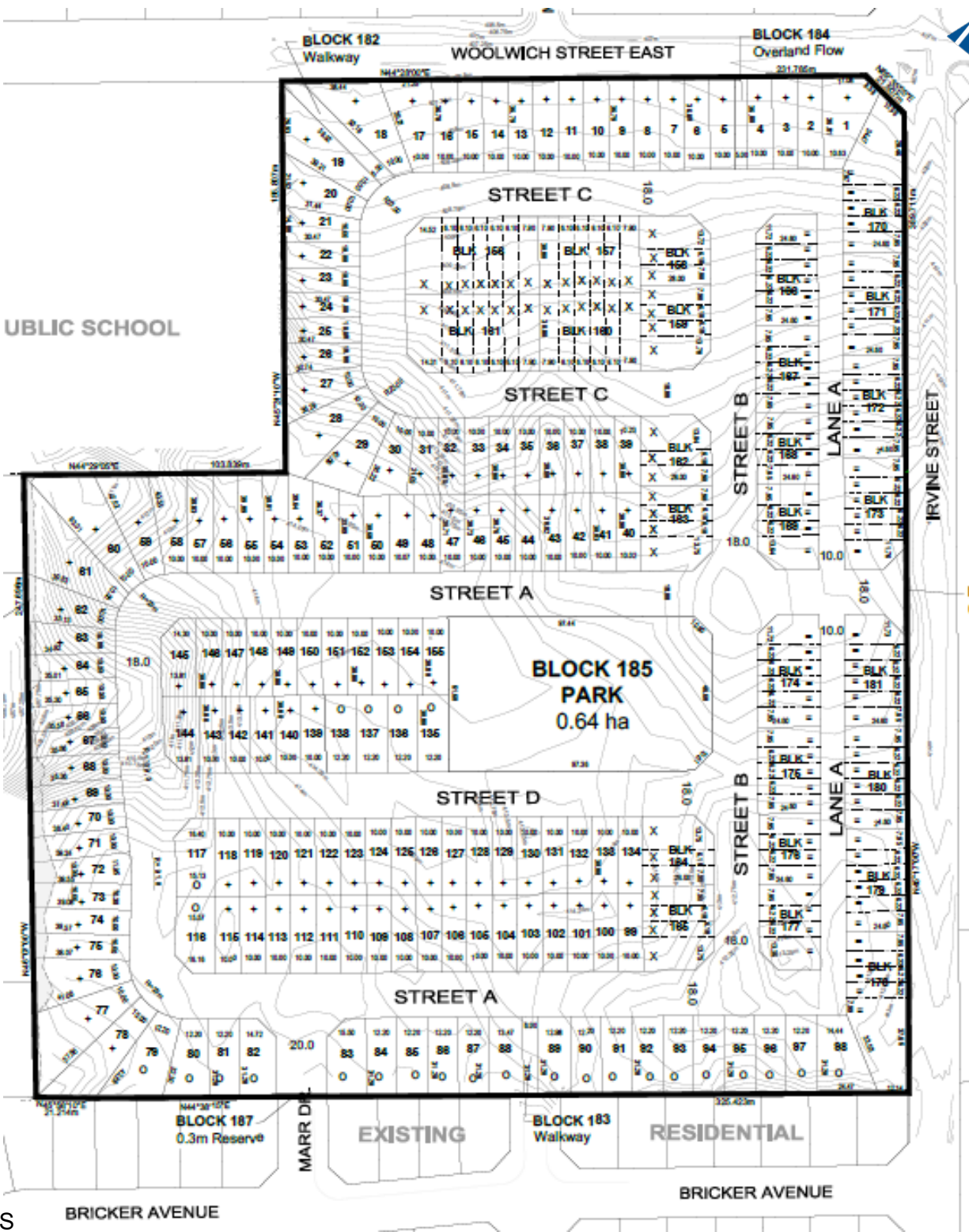
3.1 Development Description

The subject site is in the southwest corner of Woolwich Street/Nichol Road 15 and Irvine Street in the community of Elora, Township of Centre Wellington. The property owner is proposing to develop the approximately 12.4-hectare block with 286 residential units comprised of 155 single detached and 131 townhomes.

Vehicle access is proposed via a new street connection to Irvine Street (Street A), and the continuation of Marr Drive from Bricker Avenue. An active transportation corridor between Bricker Avenue and Street A is proposed to extend from Clegg Road. Irvine Street will be upgraded to include paved travel lanes and urban cross-section between Bricker Avenue Woolwich Street East/Nichol Road 15.

Figure 3.1 shows the proposed development concept.





Concept Plan

3.2 Site Trip Generation

The Institute of Transportation Engineers (ITE) Trip Generation⁴ methods are used to estimate the site trip generation. The following Land Use Code (LUC) was used to estimate the site trip generation:

- ▶ 210 – Single Family, Detached Housing (dwelling units); and
- ▶ 220 – Multifamily Housing, Low-Rise (dwelling units).

The fitted curve equations were used to calculate the trips generated by the development. **Table 3.1** summarizes the estimated trip generation and is estimated to be approximately 174 AM peak hour trips and 227 PM peak hour trips. No reductions for alternative modes of transportation were used in the calculation.

TABLE 3.1: TRIP GENERATION

ITE Land Use	Units	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
210 - Single-Family, Detached Housing (Dwelling Units)	155	29	82	111	94	56	150
220 - Multifamily Housing, Low-Rise (Dwelling Units)	131	16	47	63	48	29	77
Total Trip Generation	286	45	129	174	142	85	227

210: AM $\ln(T) = 0.91 \ln(X) + 0.12$ | PM $\ln(T) = 0.94 \ln(X) + 0.27$

220: AM: $T = 0.31(X) + 22.85$ | PM $T = 0.43(X) + 20.55$

The trip distribution used for this study was based on the existing traffic patterns at the boundary study area intersections. These intersections provide access to the local arterial/collector network and provide access to the neighbouring communities as well as typical commuting patterns in the Township. The trip distribution is shown in **Table 3.2**.

⁴ *Trip Generation Tenth Edition*, Institute of Transportation Engineers, Washington D.C., 2017

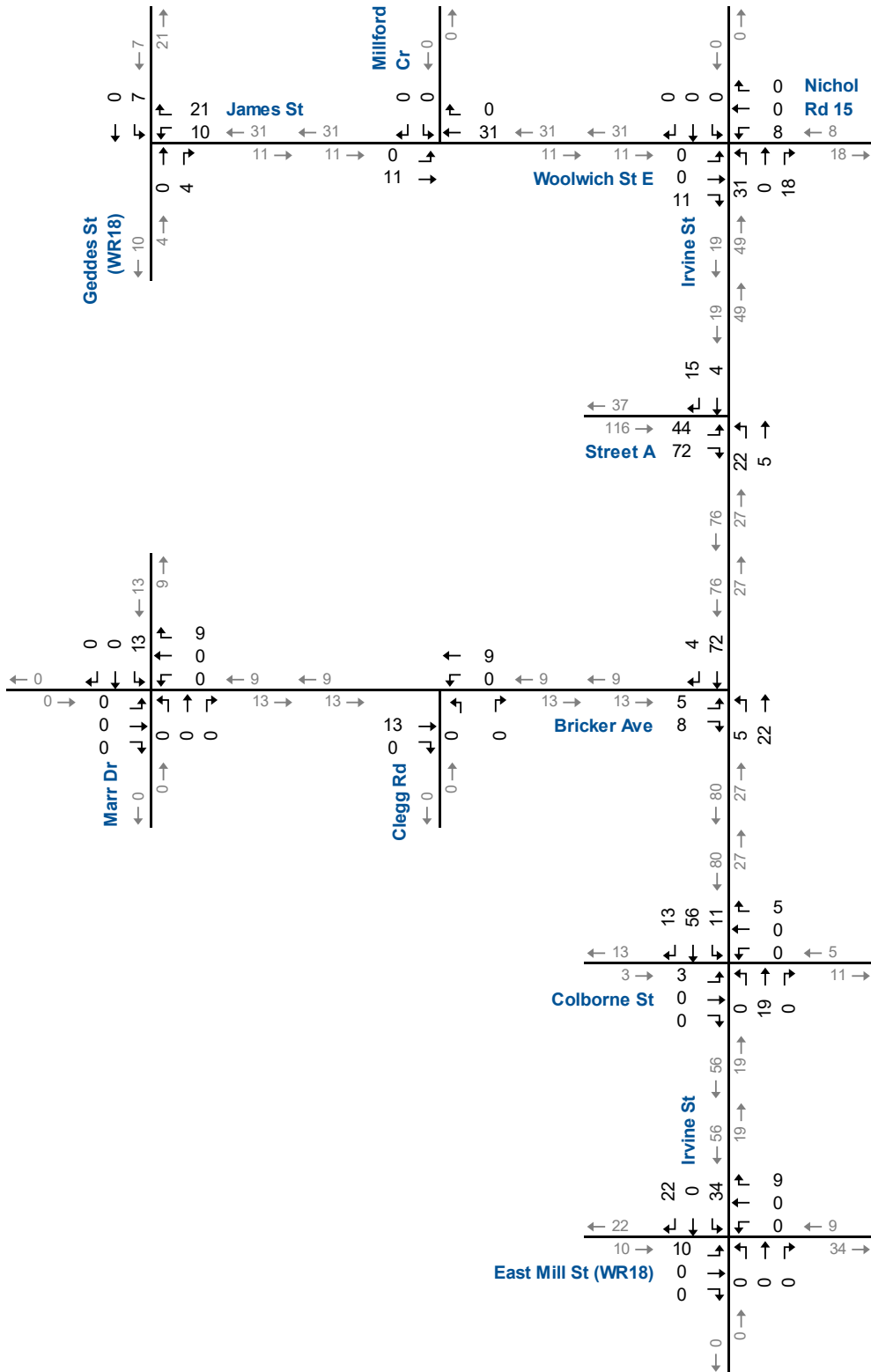


TABLE 3.2: TRIP DISTRIBUTION

Origin / Destination		AM Peak Hour		PM Peak Hour	
		Inbound	Outbound	Inbound	Outbound
North	Geddes Street (WR 18)	16%	16%	21%	16%
South	Geddes Street (WR 18)	10%	8%	8%	11%
East	Nichol Road 15	18%	14%	13%	16%
	Colborne Street	11%	8%	10%	12%
	East Mill Street (WR 18)	18%	27%	21%	17%
West	Colborne Street	6%	10%	10%	8%
	East Mill Street (WR 18)	23%	17%	17%	19%
Total		100%	100%	100%	100%

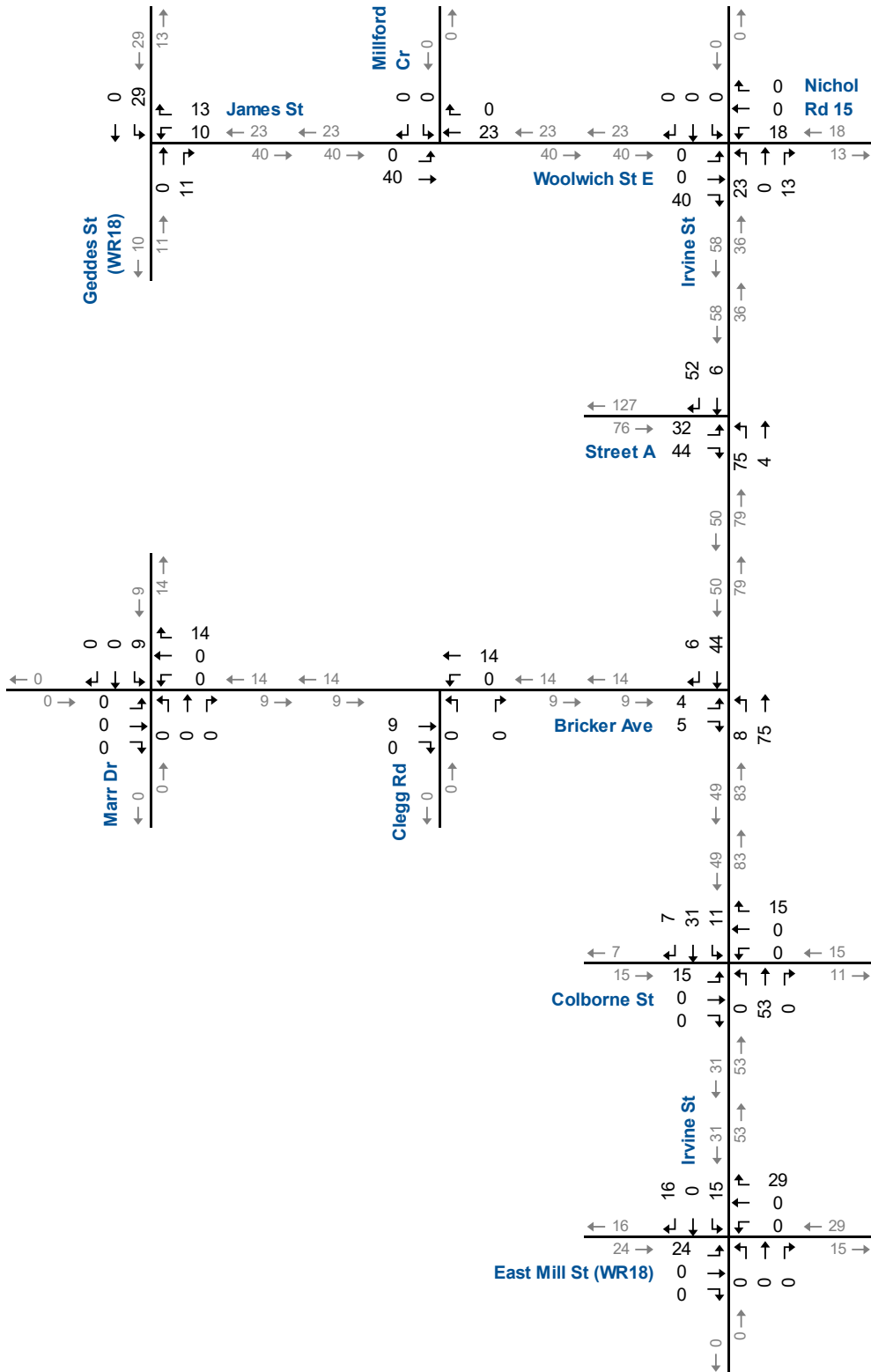
Figure 3.2A-B contains the AM and PM peak hour trip assignment, respectively





Site Generated Traffic Volumes AM Peak Hour

Figure 3.2A



Site Generated Traffic Volumes PM Peak Hour

Figure 3.2B

4 Evaluation of Future Traffic Conditions

The assessment of the future traffic conditions contained in this section includes the future traffic forecasts as well as the level of service analysis. An assumed full-build-out horizon (2026) and five-year horizon (2031) following from the assumed build-out date has been assessed to determine the impact of the subject site.

No changes to the road network traffic control and lane geometry has been assumed.

4.1 Forecast Traffic Volumes

The likely future traffic volumes are estimated to consist of:

- ▶ Increased non-site traffic (generalized background traffic growth) estimated to be 2.0 percent per annum as noted in the pre-study consultation.
- ▶ Traffic generated from the following developments:
 - Ainley Subdivision, Elora⁵ - 251 residential units comprised of 126 single detached, 63 apartments, and 62 townhouse units; and
 - North West Fergus Secondary Plan⁶ - a mixed-use site situated in the Colborne Street and Beatty Line area of the community of Fergus.
- ▶ Traffic generated by the subject site.

The traffic volumes from the background developments were obtained from their respective studies.

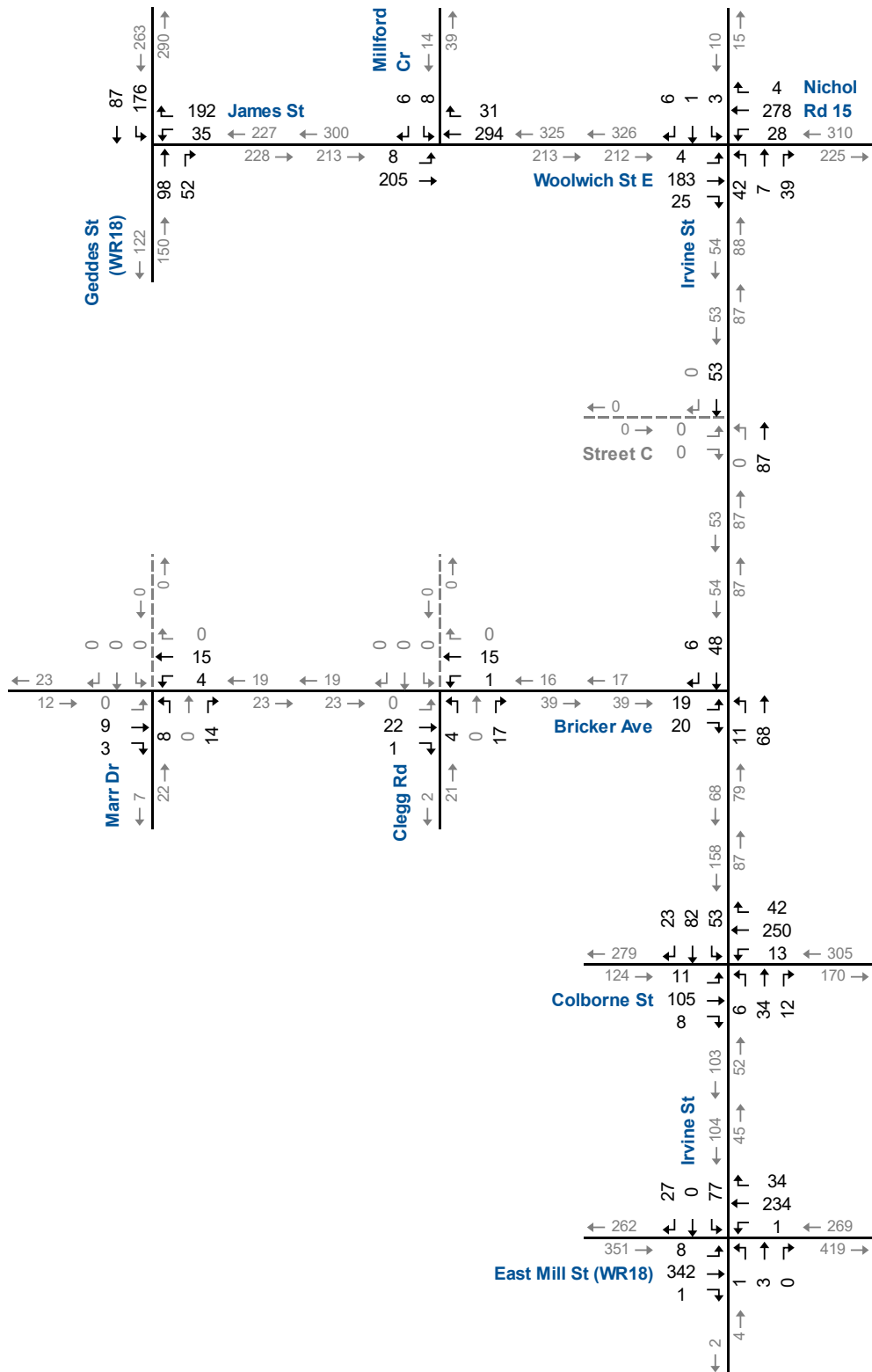
Appendix D contains the background development trip assignments.

Figure 4.1A-B details the forecast 2026 background traffic volumes for the weekday AM and PM peak hours, respectively. **Figure 4.2A-B** details the forecast 2031 background traffic volumes for the weekday AM and PM peak hours, respectively. **Figure 4.3A-B** details the forecast 2026 total traffic volumes for the weekday AM and PM peak hours, respectively. **Figure 4.4A-B** details the forecast 2031 total traffic volumes for the weekday AM and PM peak hours, respectively.

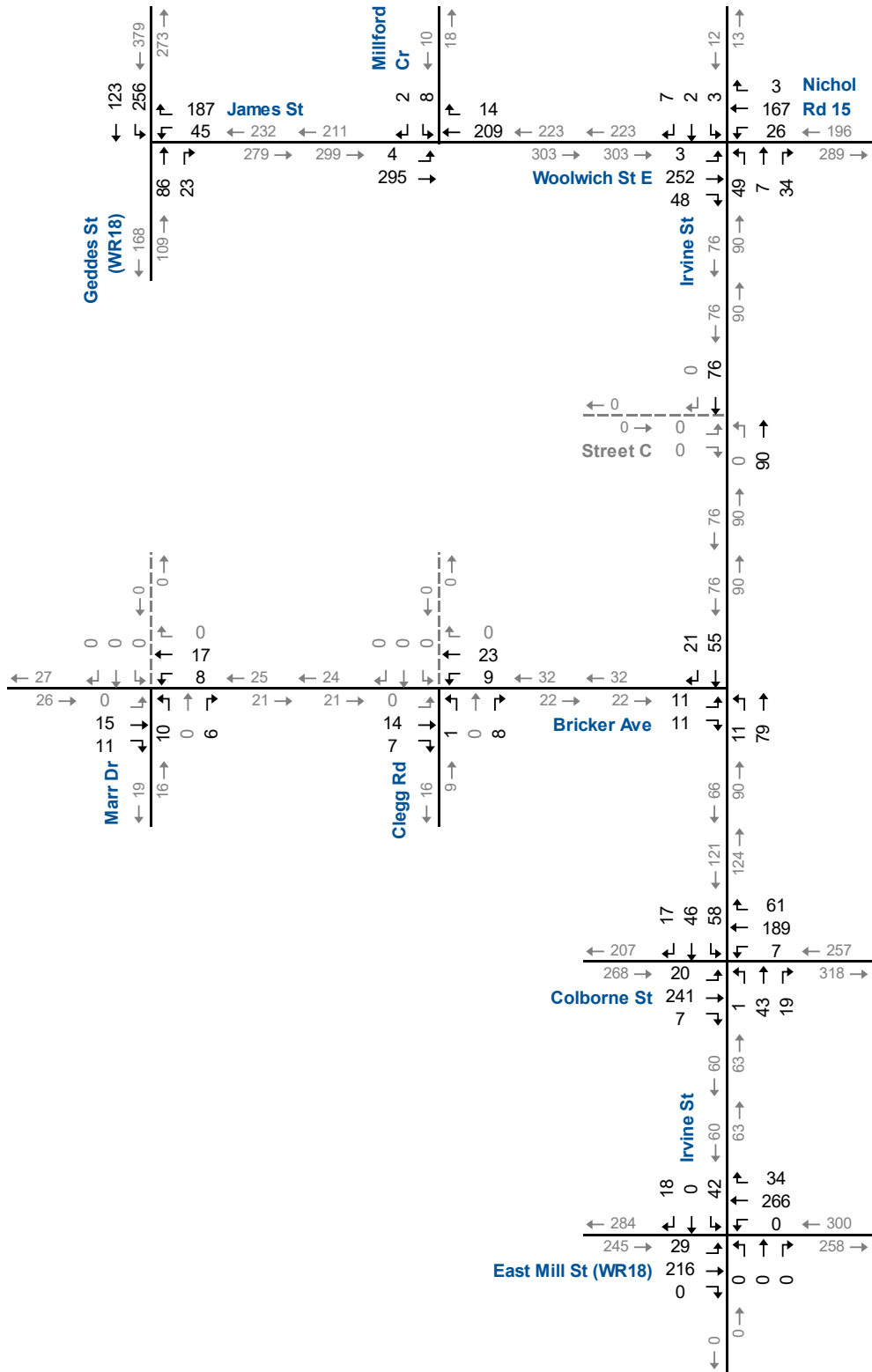
⁵ Ainley Subdivision, Elora Transportation Impact Study, Paradigm Transportation Solution Limited, October 2017 (170136)

⁶ Traffic Impact Study in support of Draft Plan Approval (Phases 2 & 3), Township of Centre Wellington North West Fergus Secondary Plan, RJ Burnside & Associates Limited, December 2016



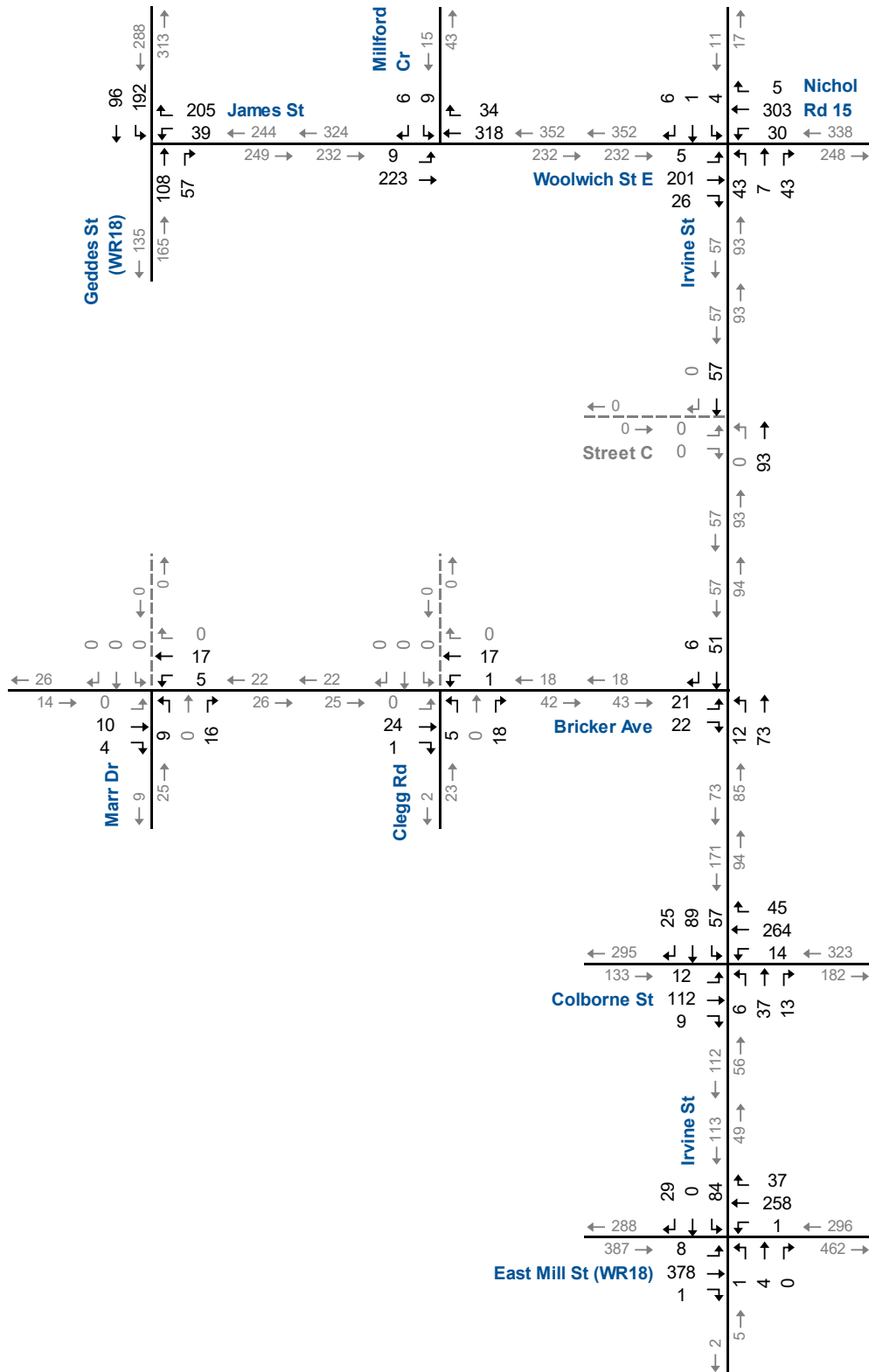


2026 Background Traffic Volumes AM Peak Hour

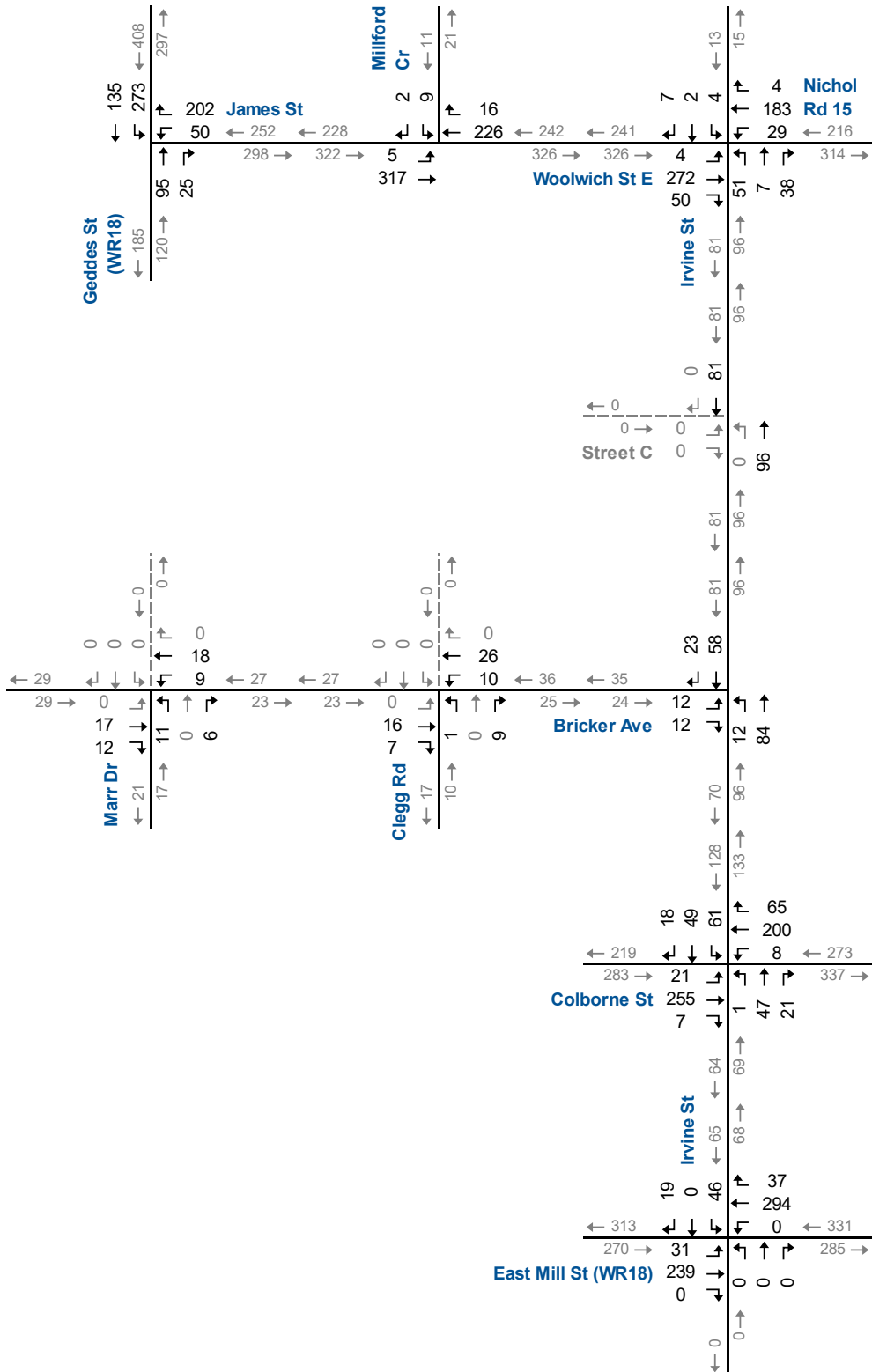


2026 Background Traffic Volumes PM Peak Hour

Figure 4.1B



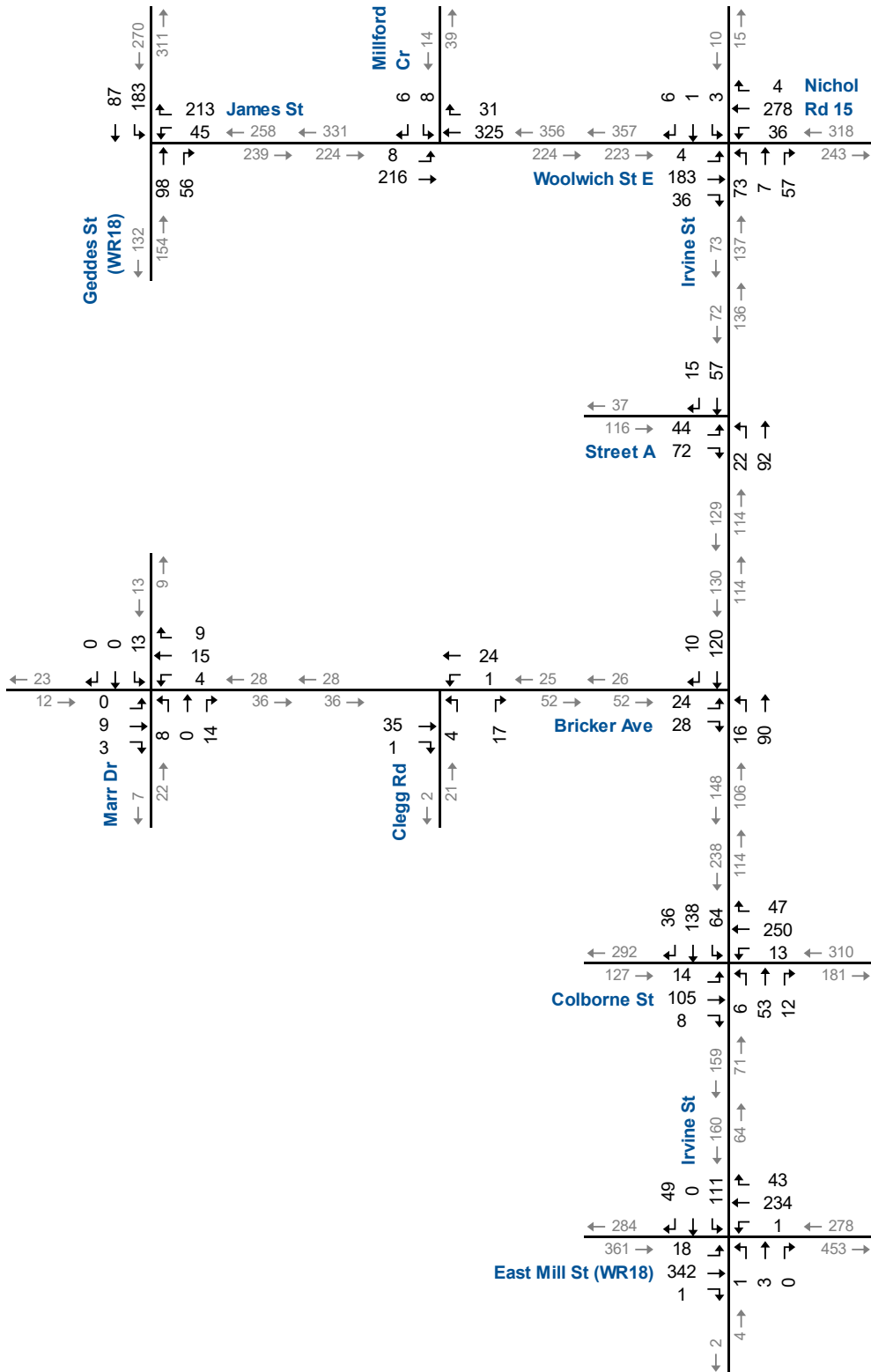
2031 Background Traffic Volumes AM Peak Hour



2031 Background Traffic Volumes PM Peak Hour

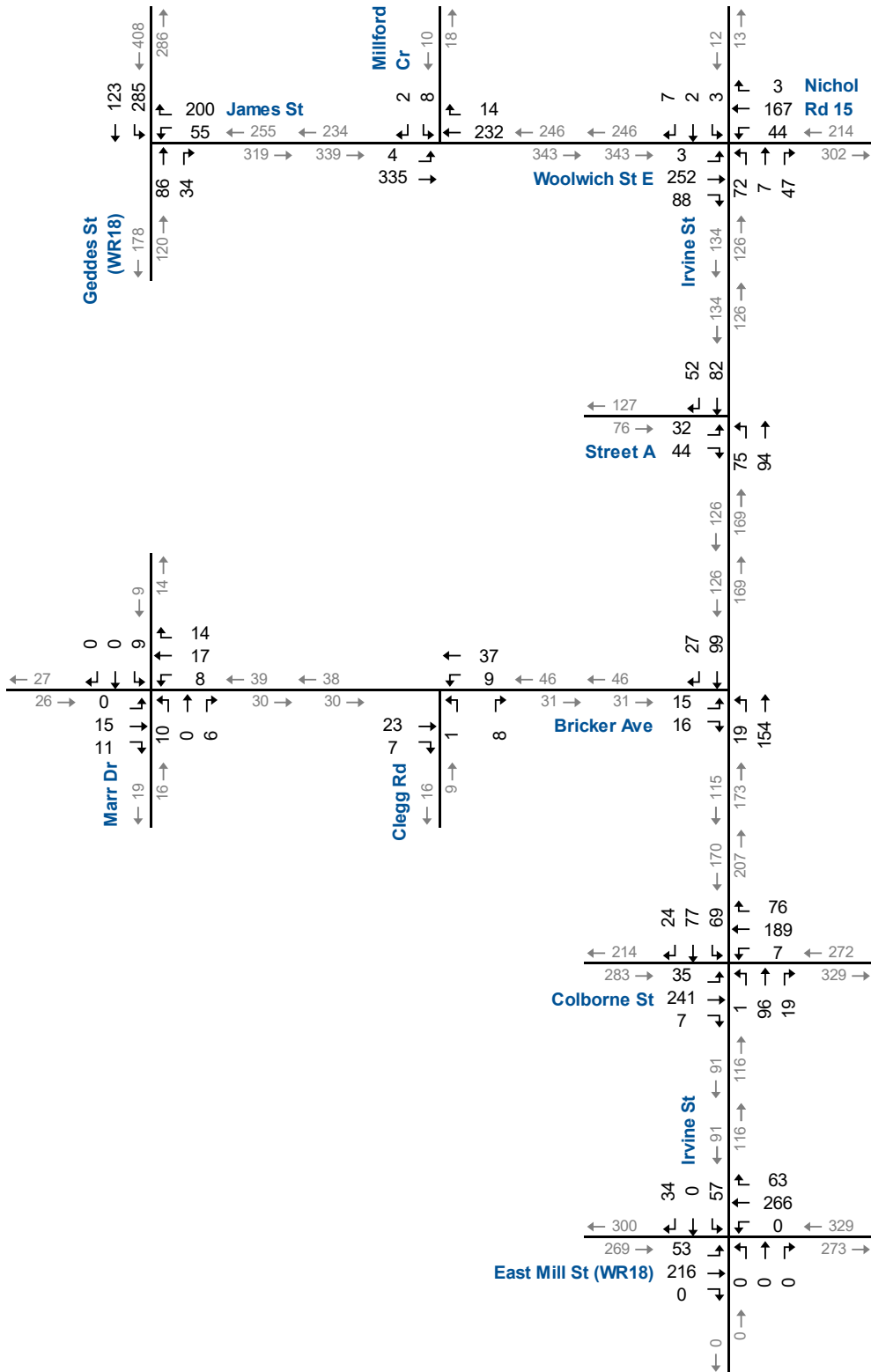


Figure 4.2B



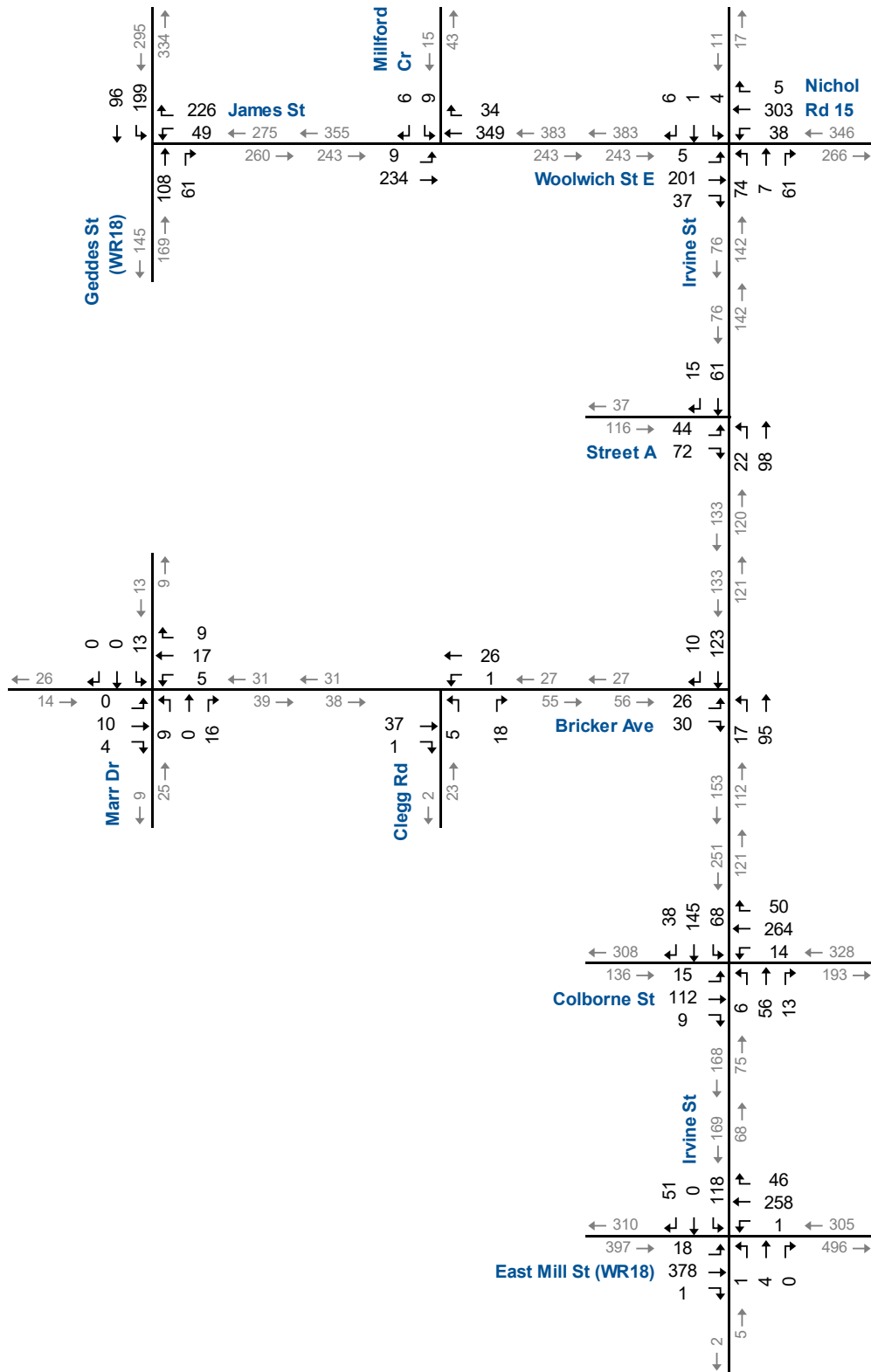
2026 Total Traffic Volumes AM Peak Hour

Figure 4.3A

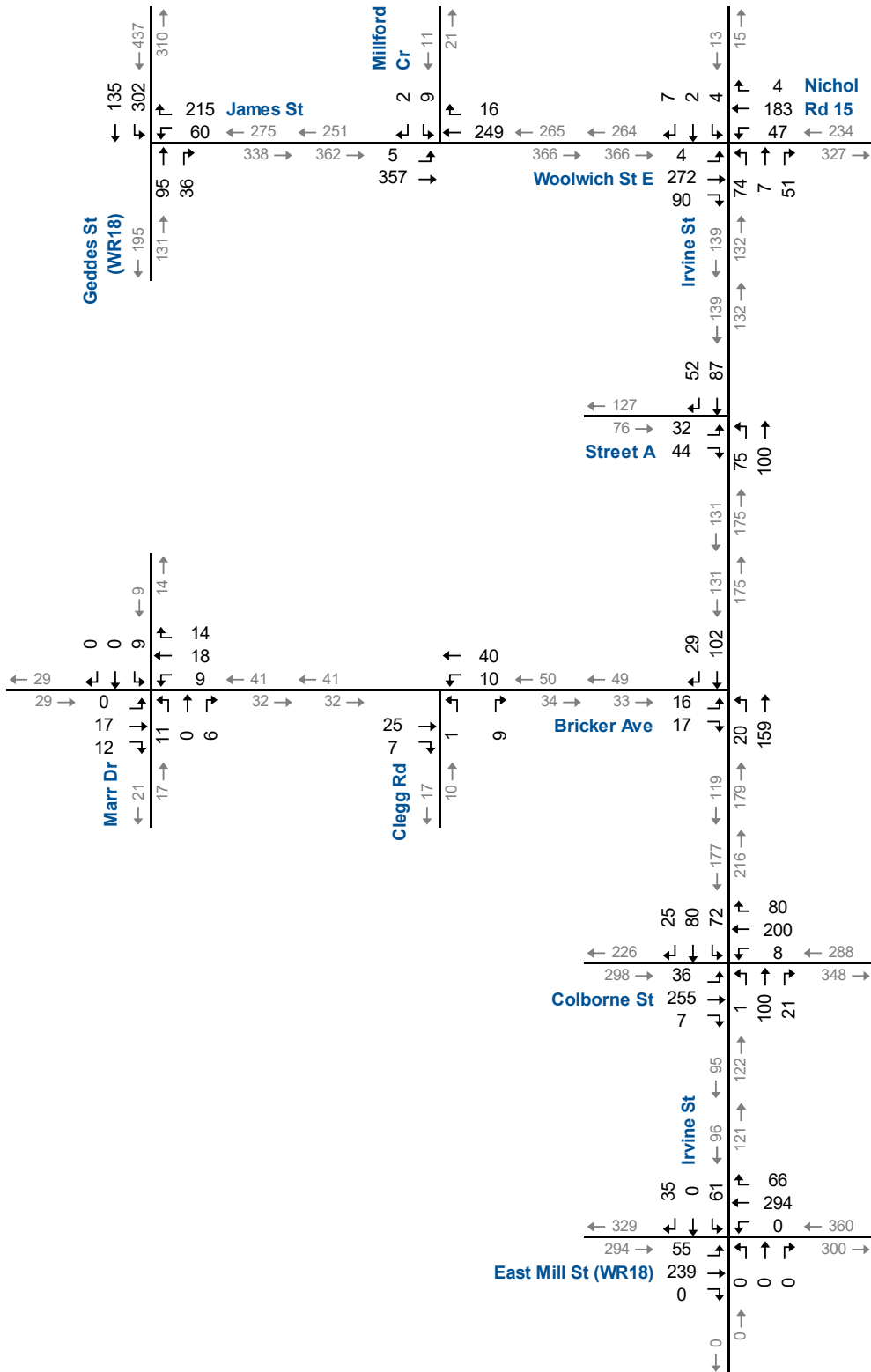


2026 Total Traffic Volumes PM Peak Hour

Figure 4.3B



2031 Total Traffic Volumes AM Peak Hour



2031 Total Traffic Volumes PM Peak Hour



Figure 4.4B

4.2 Forecast Traffic Operations

4.2.1 2026 Background Traffic Operations

The study area intersection operations analysis for the 2026 background traffic scenario followed the same methodology used for the existing traffic conditions with the existing lane configurations.

Table 4.1A-B details the level of service conditions for the weekday AM and PM peak hours, respectively.

The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements for the weekday AM and PM peak hours.

Appendix E1 contains the detailed Synchro 10 reports.



TABLE 4.1A: 2026 BACKGROUND OPERATIONS (AM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0 > >	< < < <	A 0 0.21 0	> > > >	A 0 > >	< < < <	< < < <	B 12 0.03 1	> > > >	> > > >	B 12 > >	A 1		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 1 0.02 1	> > > >	A 1 > >	< < < <	B 13 0.18 5	> > > >	B 13 0.02 1	> > > >	B 12 > >	A 3		
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 9 0.05 1	> > > >	> > > >	A 9 > >	< < < <	< < < <	A 1 0.01 0	> > > >	A 1 > >	< < < <	A 0 0.03 0	> > > >	A 0 > >	A 0 > >	A 3		
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 9 0.02 1	> > > >	A 9 0.00 0	> > > >	A 0 > >	A 0 > >	A 3	
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 2 0.00 0	> > > >	A 2 > >	< < < <	A 10 0.03 1	> > > >	A 10 0.00 0	> > > >	A 0 > >	A 5		
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q					B 12 0.34 11	> > > >	B 12 > >	< < < <	A 0 0.10 0	> > > >	A 0 0.14 4	> > > >	A 6 0.26 >		A 6 > >	A 7	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	A 9 0.19 >	> > > >	A 9 > >	< < < <	B 11 0.44 >	> > > >	B 11 > >	< < < <	A 9 0.09 >	> > > >	A 9 0.26 >	> > > >	B 10 > >	B 10 > >	B 11	
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	C 16 0.01 0	> > > >	C 16 0.30 9	> > > >	C 18 > >	C 18 > >	A 3	

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



TABLE 4.1B: 2026 BACKGROUND OPERATIONS (PM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0 0	> > > >	A 0 0.14 0	> > > >	A 0 0 0	> > > >	> > > >	> > > >	> > > >	B 12 0.02 1	> > > >	> > > >	B 12 0 0	A 0 0 0		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 1 0.02 1	> > > >	A 1 0 0	> > > >	< < < <	B 14 0.19 5	> > > >	B 14 0 1	> > > >	> > > >	B 11 0.02 1	A 3 0 0		
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 9 0.03 1	> > > >	> > > >	A 9 0 0	> > > >	> > > >	> > > >	< < < <	A 1 0.01 0	> > > >	A 1 0 0	> > > >	A 0 0.05 0	> > > >	A 0 0 0	A 2 0 0	
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 2 0.01 0	> > > >	A 2 0 0	> > > >	< < < <	A 9 0.01 0	> > > >	A 9 0 0	> > > >	> > > >	A 0 0.00 0	A 2 0 0		
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 3 0.01 0	> > > >	A 3 0 0	> > > >	< < < <	A 10 0.02 1	> > > >	A 10 0 0	> > > >	> > > >	A 0 0.00 0	A 3 0 0		
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q				B 14 0.39 14	> > > >	B 14 0 0	> > > >	> > > >	A 0 0.07 0	> > > >	A 0 0 0	> > > >	A 6 0.19 5	> > > >	A 6 0 0	A 8 0 0	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	B 11 0.40 >	> > > >	B 11 0.37 >	> > > >	B 10 0.11 >	> > > >	< < < <	A 9 0.11 >	> > > >	A 9 0 >	> > > >	> > > >	B 10 0.21 >	B 10 0 >	B 11 0 >	
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 1 0.03 1	> > > >	A 1 0.00 0	> > > >	A 0 0.00 0	> > > >	< < < <	A 0 0.00 0	> > > >	A 0 0.00 0	> > > >	C 16 0.16 4	> > > >	C 16 0 0	A 2 0 0	

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



4.2.2 2031 Background Traffic Operations

The study area intersection operations analysis for the 2031 background traffic scenario followed the same methodology used for the existing traffic conditions with the existing lane configurations.

Table 4.2A-B details the level of service conditions for the weekday AM and PM peak hours, respectively.

The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements for the weekday AM and PM peak hours.

Appendix E2 contains the detailed Synchro 10 reports.



TABLE 4.2A: 2031 BACKGROUND OPERATIONS (AM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0	< < < <	A 0 0.23 0	> > > >	A 0	< < < <	B 12 0.03 1	> > > >	B 12	A 1				
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0	< < < <	A 1 0.03 1	> > > >	A 1	< < < <	B 14 0.21 6	> > > >	B 14	A 3				
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 9 0.05 1	> > > >	A 9	< < < <	A 0 0.01 0	> > > >	A 1	< < < <	A 1	> > > >	A 0 0.04 0	A 0	A 3				
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0	< < < <	A 0 0.00 0	> > > >	A 0	< < < <	A 9 0.02 1	> > > >	A 9	A 3				
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0	< < < <	A 2 0.00 0	> > > >	A 2	< < < <	A 10 0.03 1	> > > >	A 10	A 5				
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q				B 13 0.38 13	> > > >	B 13	< < < <	A 0 0.11 0	> > > >	A 0	< < < <	A 6 0.15 4	A 6	A 7			
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	A 10 0.21 >	> > > >	A 10	< < < <	B 12 0.47 >	> > > >	B 12	< < < <	A 10 0.10 >	> > > >	A 10	B 11	B 11			
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0	< < < <	A 0 0.00 0	> > > >	A 0	< < < <	C 17 0.02 0	> > > >	C 17	C 21	A 3			

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



TABLE 4.2B: 2031 BACKGROUND OPERATIONS (PM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > 0	< < < <	A 0 0.15 0	> > > >	A 0 > >	< < < <	B 12 0.02 1	> > > >	B 12 > >	> > > >	B 12 > >	A 0		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 1 > 1	< < < <	A 1 0.03 1	> > > >	A 1 > >	< < < <	B 14 0.22 6	> > > >	B 14 > >	> > > >	B 12 > >	A 3		
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 9 0.03 1	> > > >	A 9 > >	< < < <	A 9 0.01 0	> > > >	A 1 > >	< < < <	A 1 0.05 0	> > > >	A 0 > >	A 0 > >	A 0 > >	A 0 > >	A 2		
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 2 0.01 0	> > > >	A 2 > >	< < < <	A 9 0.01 0	> > > >	A 9 > >	> > > >	A 0 > >	A 0 > >	A 2	
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 3 0.01 0	> > > >	A 3 > >	< < < <	A 10 0.02 1	> > > >	A 10 > >	> > > >	A 0 > >	A 0 > >	A 3	
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q	< < < <	B 12 0.43 >	> > > >	B 12 > >	< < < <	B 11 0.40 >	> > > >	B 11 > >	< < < <	A 10 0.12 >	> > > >	A 10 > >	> > > >	B 10 > >	B 10 > >	B 11	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	A 1 0.03 1	> > > >	A 1 > >	< < < <	A 0 0.00 >	> > > >	A 0 > >	< < < <	A 0 0.00 >	> > > >	A 0 > >	> > > >	C 17 0.19 >	C 17 > >	A 2	
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 1 0.03 1	> > > >	A 1 > >	< < < <	A 0 0.00 >	> > > >	A 0 > >	< < < <	A 0 0.00 >	> > > >	A 0 > >	> > > >	C 17 0.19 >	C 17 > >	A 2	

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



4.2.3 2026 Total Traffic Operations

The study area intersection operations analysis for the future total traffic scenario followed the same methodology used for the 2026 background traffic conditions. **Table 4.3A-B** details the level of service conditions for the weekday AM and PM peak hours, respectively.

The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements for the weekday AM and PM peak hours.

Appendix F1 contains the detailed Synchro 10 reports.

The new municipal roadway approach to Irvine Street is forecast to operate at LOS B or better and v/c ratio of 0.15 or lower during the AM and PM peak hours.

Site traffic impacts are minimal with minor changes in delay at the study area intersections.



TABLE 4.3A: 2026 TOTAL OPERATIONS (AM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
AM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0 > 0	< < < <	A 0 0.23 0	> > > >	A 0 > >	< < < <	C > > >	C 16 > >	< < < <	B 12 0.03 1	> > > >	B 12 > >	A 0 > >	
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 1 0.03 1	> > > >	A 1 > >	< < < <	C > > >	C 16 > >	< < < <	B 12 0.02 1	> > > >	B 12 > >	A 4 > >	
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	A 10 0.07 2	> > > >	A 10 > >	< < < <	A 1 0.01 0	> > > >	A 1 > >	< < < <	A 1 0.09 0	> > > >	A 0 > >	A 0 > >	> > > >	A 0 > >	A 2 > >		
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	> > > >	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 9 0.02 1	> > > >	A 9 > >	> > > >	A 9 > >	A 2 > >		
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 1 0.00 0	> > > >	A 1 > >	< < < <	A 10 0.03 1	> > > >	A 10 > >	< < < <	A 9 0.02 0	> > > >	A 9 > >	A 5 > >
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q	> > > >	A 14 0.40 15	> > > >	A 14 > >	< < < <	A 14 0.10 0	> > > >	A 14 > >	< < < <	A 0 0.15 4	> > > >	A 0 > >	< < < <	A 6 0.15 4	> > > >	A 6 > >	A 7 > >
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	B 10 0.21 >	> > > >	B 10 > >	< < < <	B 13 0.48 >	> > > >	B 13 > >	< < < <	B 10 0.13 >	> > > >	B 10 > >	< < < <	B 12 0.41 >	> > > >	B 12 > >	B 12 > >
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 1 0.02 0	> > > >	A 1 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	C 17 0.01 0	> > > >	C 17 > >	< < < <	C 23 0.48 19	> > > >	C 23 > >	A 5 > >
	Irvine St & Street A	TWSC	LOS Delay V/C Q	A 10 0.15 4	> > > >	A 10 > >	< < < <	A 2 0.02 0	> > > >	A 2 > >	< < < <	A 2 0.05 0	> > > >	A 0 > >	A 0 > >	> > > >	A 0 > >	A 5 > >		

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



TABLE 4.3B: 2026 TOTAL OPERATIONS (PM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > 0	< < < <	A 0 0.16 0	> > > >	A 0 > >	< < < <	C 16 0.30 10	> > > >	C 16 > >	< < < <	B 13 0.02 1	> > > >	B 13 > >	A 0 > >
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 2 0.05 1	> > > >	A 2 > >	< < < <	C 16 0.30 10	> > > >	C 16 > >	< < < <	B 12 0.02 1	> > > >	B 12 > >	A 4 > >
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	B 10 0.04 1	> > > >	B 10 > >	< < < <	A 1 0.02 0	> > > >	A 1 > >	< < < <	A 1 0.02 0	> > > >	A 1 0.08 0	> > > >	A 0 > >	A 0 > >	A 0 > >	A 1 > >	
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	> > > >	A 0 0.02 >	> > > >	A 0 > >	< < < <	A 2 0.01 0	> > > >	A 1 > >	< < < <	A 9 0.01 0	> > > >	A 9 > >	> > > >	> > > >	> > > >	A 2 > >	
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 2 0.01 0	> > > >	A 2 > >	< < < <	A 10 0.02 1	> > > >	A 10 > >	< < < <	A 9 0.01 0	> > > >	A 9 > >	A 3 > >
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q	> > > >	> > > >	> > > >	C 17 0.48 19	> > > >	C 17 > >	< < < <	A 0 0.08 0	> > > >	A 0 > >	< < < <	A 6 0.22 6	> > > >	A 6 > >	A 6 > >	A 9 > >	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	B 13 0.47 >	> > > >	B 13 > >	< < < <	B 12 0.44 >	> > > >	B 12 > >	< < < <	B 11 0.22 >	> > > >	B 11 > >	< < < <	B 12 0.32 >	> > > >	B 12 > >	B 12 > >
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 2 0.05 1	> > > >	A 2 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	C 18 0.27 8	> > > >	C 18 > >	A 3 > >
	Irvine St & Street A	TWSC	LOS Delay V/C Q	B 11 0.12 3	> > > >	A 11 > >	< < < <	A 4 0.06 2	> > > >	A 4 > >	< < < <	A 4 0.09 0	> > > >	A 0 > >	A 0 > >	A 0 > >	A 4 > >			

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



4.2.4 2031 Total Traffic Operations

The study area intersection operations analysis for the future total traffic scenario followed the same methodology used for the 2030 background traffic conditions. **Table 4.4A-B** details the level of service conditions for the weekday AM and PM peak hours, respectively.

The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements for the weekday AM and PM peak hours.

Appendix F1 contains the detailed Synchro reports.

The new municipal roadway approach to Irvine Street is forecast to operate at LOS B or better and v/c ratio of 0.16 or lower during the AM and PM peak hours.

Site traffic impacts are minimal with minor changes in delay at the study area intersections.



TABLE 4.4A: 2031 TOTAL OPERATIONS (AM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																		
				Eastbound				Westbound				Northbound				Southbound				Overall		
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach			
AM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0 0.01 0	< < < <	A 0 0.25 0	> > > >	A 0 0.25 0	< < < <	A 0 0.25 0	> > > >	B 13 0.03 1	> > > >	B 13 0.03 1	> > > >	A 1 13 1			
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.00 0	< < < <	A 1 0.03 1	> > > >	A 1 0.03 1	< < < <	C 17 0.34 11	> > > >	C 17 0.34 11	< < < <	B 13 0.03 1	> > > >	B 13 0.03 1	> > > >	A 4 13 1	
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	B 10 0.08 2	> > > >	B 10 0.08 2	> > > >	B 10 0.08 2	> > > >	B 10 0.08 2	> > > >	A 1 0.01 0	> > > >	A 1 0.01 0	> > > >	A 1 0.01 0	> > > >	A 0 0.09 0	> > > >	A 0 0.09 0	> > > >	A 2 0 0
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.02 0	> > > >	A 0 0.02 0	< < < <	A 0 0.02 0	> > > >	A 0 0.02 0	< < < <	A 9 0.03 1	> > > >	A 9 0.03 1	> > > >	A 9 0.03 1	> > > >	A 9 0.03 1	> > > >	A 2 9 1	
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.00 0	< < < <	A 1 0.00 0	> > > >	A 1 0.00 0	< < < <	A 10 0.03 1	> > > >	A 10 0.03 1	< < < <	A 9 0.02 0	> > > >	A 9 0.02 0	> > > >	A 5 9 0	
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q	< < < <	A 15 0.45 18	> > > >	A 15 0.45 18	< < < <	B 15 0.45 18	> > > >	B 15 0.45 18	< < < <	A 0 0.11 0	> > > >	A 0 0.11 0	< < < <	A 6 0.16 4	> > > >	A 6 0.16 4	> > > >	A 8 6 4	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	B 10 0.23 >	> > > >	B 10 0.23 >	< < < <	B 14 0.52 >	> > > >	B 14 0.52 >	< < < <	B 10 0.14 >	> > > >	B 10 0.14 >	< < < <	B 13 0.44 >	> > > >	B 13 0.44 >	> > > >	B 13 0.44 >	B 13 0.44 >
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 1 0.02 0	> > > >	A 1 0.02 0	< < < <	A 0 0.00 0	> > > >	A 0 0.00 0	< < < <	C 18 0.02 0	> > > >	C 18 0.02 0	< < < <	D 28 0.56 25	> > > >	D 28 0.56 25	> > > >	D 28 0.56 25	A 6 28 25
	Irvine St & Street A	TWSC	LOS Delay V/C Q	A 10 0.16 4	> > > >	A 10 0.16 4	> > > >	A 10 0.16 4	> > > >	A 10 0.16 4	> > > >	A 2 0.02 0	> > > >	A 2 0.02 0	> > > >	A 0 0.05 0	> > > >	A 0 0.05 0	> > > >	A 0 0.05 0	> > > >	A 4 0 0

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



TABLE 4.4B: 2031 TOTAL OPERATIONS (PM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 0 0.17 0	> > > >	> > > >	> > > >	> > > >	B 13 0.03 1	> > > >	> > > >	B 13 > >	A 0 > >		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 2 0.05 1	> > > >	> > > >	> > > >	> > > >	C 17 > >	< < < <	B 13 0.03 1	> > > >	B 13 > >	A 4 > >	
	Irvine St & Bricker St	TWSC	LOS Delay V/C Q	B 10 0.05 1	> > > >	> > > >	B 10 > >	> > > >	> > > >	> > > >	> > > >	> > > >	> > > >	A 1 0.02 0	> > > >	A 0 0.09 0	> > > >	A 0 > >	A 1 > >	
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q	> > > >	A 0 0.02 0	> > > >	A 0 > >	< < < <	A 2 0.01 0	> > > >	A 9 0.01 0	> > > >	> > > >	A 9 0.02 1	> > > >	> > > >	> > > >	A 9 0.01 0	A 2 > >	
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 > >	< < < <	A 2 0.01 0	> > > >	A 2 0.02 1	> > > >	> > > >	A 10 0.02 0	> > > >	A 9 0.01 0	> > > >	A 9 0.01 0	A 3 > >	
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q	> > > >	> > > >	> > > >	C 19 0.55 25	> > > >	C 19 > >	> > > >	A 0 0.08 0	> > > >	> > > >	A 0 0.23 7	> > > >	A 6 0.23 7	> > > >	A 6 > >	A 10 > >	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	B 14 0.50 >	> > > >	B 14 > >	< < < <	B 13 0.47 >	> > > >	B 13 > >	< < < <	B 11 0.24 >	> > > >	B 11 0.34 >	< < < <	B 12 0.34 >	> > > >	B 12 > >	B 13 > >
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 2 0.05 1	> > > >	A 2 > >	< < < <	A 0 0.00 0	> > > >	A 0 0.00 0	> > > >	> > > >	A 0 0.00 10	> > > >	C 20 0.31 10	> > > >	C 20 > >	A 3 > >	
	Irvine St & Street A	TWSC	LOS Delay V/C Q	B 11 0.12 3	> > > >	> > > >	B 11 > >	> > > >	> > > >	> > > >	A 4 0.06 2	> > > >	A 4 0.06 2	> > > >	A 0 0.09 0	> > > >	A 0 > >	A 0 > >	A 4 > >	

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



4.3 Future Daily Traffic Volumes

Table 4.5 shows the forecast future daily traffic volumes on the local roadways in the study area. The daily volumes were derived from the PM peak hour traffic volume shown in **Figure 2.3B**, **Figure 3.2B**, **Figure 4.1B**, **Figure 4.2B**, **Figure 4.3B** and **Figure 4.4B**. The PM peak hour volumes were assumed to be 10 per cent of total daily traffic.

The Transportation Association of Canada (TAC)⁷ identifies 1,000 or less vehicles per day for local residential roads, 8,000 or less for residential collector roads, and between 5,000 and 20,000 for arterial roadways. The roads in the study area are designated⁸ as follows:

- ▶ Irvine Street – Collector between Woolwich Street and David Street, Local between David Street and East Mill Street;
- ▶ Woolwich Street/Nichol Road 15 – Collector;
- ▶ Colborne Street – Collector;
- ▶ East Mill Street (WR 18) – Arterial;
- ▶ Geddes Street (WR 18) – Arterial;
- ▶ Bricker Avenue – Local
- ▶ Marr Drive – Local; and
- ▶ Clegg Drive – Local.

Based on the base year traffic volumes, the study road network are currently experiencing daily traffic volumes that are within their designations.

With the forecast background traffic growth and the introduction of the site generated traffic, majority of the study area roadways maintain their assigned designations. Bricker Avenue between Irvine Street and Marr Drive is forecast to exceed its TAC daily volume threshold for a local road designation with a forecast of approximately 1,860 vehicles per day.

The proposed extension of Marr Drive and Clegg Road are forecast to be under 1,000 vehicles per day which assigns them to local roads. The proposed Street C is forecast to have a daily traffic volume just over the local road threshold of 1,000 vehicles per day.

⁷ Transportation Association of Canada, Geometric Design Guide for Canadian Roads, Table 2.6.5: Characteristics of Urban Roads, June 2017

⁸ Township of Centre Wellington Transportation Master Plan Final Report, January 2019, Figure 12: Principal Roadway Classification in Elora and Fergus



TABLE 4.5: FUTURE DAILY TRAFFIC VOLUMES

Road Section	Two-Way Daily Traffic*				Transportation Association of Canada [^]	
	Base Year	Background 2031	Site Generated	Total 2031	Threshold	Designation
Irvine Street (Local/Collector)						
North of Woolwich Street	230	280	-	280	<1,000	Local
Between Woolwich Street & Street C	910	1,770	950	2,720	<8,000	Collector
Between Street C & Bricker Avenue	910	1,770	1,120	2,890	<8,000	Collector
Between Bricker Avenue & Colborne Street	1,500	2,610	1,330	3,940	<8,000	Collector
Between Colborne Street & East Mill Street	870	1,330	850	2,180	<8,000	Collector
Woolwich Street (James Street)/Nichol Road 15 (Collector)						
From Geddes Street to Milford Crescent	3,350	5,500	640	6,140	<8,000	Collector
Between Milford Crescent & Irvine Street	3,490	5,680	640	6,320	<8,000	Collector
East of Irvine Street	3,730	5,300	310	5,610	<8,000	Collector
Colborne Street (Collector)						
West of Irvine Street	2,420	5,020	220	5,240	<8,000	Collector
East of Irvine Street	2,950	6,100	260	6,360	<8,000	Collector
East Mill Street (Arterial)						
West of Irvine Street	4,650	5,830	400	6,230	5,000 - 20,000	Arterial
East Mill Street	4,960	6,160	450	6,610	5,000 - 20,000	Arterial
Geddes Street (Arterial)						
North of James Street	4,650	7,050	430	7,480	5,000 - 20,000	Arterial
South of James Street	2,480	3,050	210	3,260	5,000 - 20,000	Arterial
Bricker Avenue (Local)						
West of Marr Drive	480	580	-	580		Local
Between Marr Drive & Clegg Road	410	500	560	1,060	<8,000	Collector
Between Clegg Road & Irvine Street	490	610	1,250	1,860	<8,000	Collector
Clegg Road (Local)						
North of Bricker Avenue	-	-	690	690	<1,000	Local
South of Bricker Avenue	220	270	-	270	<1,000	Local
Marr Drive (Local)						
North of Bricker Avenue	-	-	560	560	<1,000	Local
South of Bricker Avenue	310	380	-	380	<1,000	Local
Street C						
West of Irvine Street	-	-	1,030	1,030	<8,000	Collector

* PM peak hour x 10

[^] TAC Table 2.6.5: Classification of Urban Roads

5 Remedial Measures

5.1 Left-Turn Lanes

The warrants for left-turn lanes follow the requirements in the Ministry of Transportation's (MTO) Geometric Design Standards⁹. The percentages of left-turning vehicles in the approaching volume were rounded to the nearest 5%, as nomographs are only provided for 5% increments. This apparent requirement is due to the nature of the warrant procedure that assumes a minimum of 5% of left turning vehicles in the advancing volume.

Appendix G contains the left-turn lane warrant nomographs.

5.1.1 Woolwich Street/Nichol Road 15 at Irvine Street

A design speed of 50 km/h (10 km/h over the posted speed limit was used for Nichol Road.

Table 5.1 summarizes the left-turn lane warrants for a westbound left-turn lane at Irvine Street with future traffic volumes. The warrant analysis suggests that a westbound left-turn lane on Nichol Road 15 at Irvine Street is not warranted.

5.1.2 Irvine Street at Bricker Avenue

A design speed of 60 km/h (10 km/h over the posted speed limit was used for Irvine Street.

Table 5.2 summarizes the left-turn lane warrants for a northbound left-turn lane on Irvine Street at Bricker Avenue with future traffic volumes. The warrant analysis suggests that a northbound left-turn lane on Irvine Street is not warranted.

5.1.3 East Mill Street (WR 18) at Irvine Street

A design speed of 50 km/h (10 km/h over the posted speed limit was used for Irvine Street.

Table 5.3 summarizes the left-turn lane warrants for an eastbound left-turn lane on East Mill Street (WR 18) at Irvine Street with future traffic volumes. The warrant analysis suggests that an eastbound left-turn lane on East Mill Street (WR 18) is not warranted.

⁹ Design Supplement for TAC Geometric Design Guide for Canadian Roads, Ministry of Transportation Ontario, June 2017



TABLE 5.1: LEFT-TURN LANE WARRANT SUMMARY – NICHOL ROAD 15

Intersection	Woolwich Street/Nichol Road 15 & Irvine Street							
Approach Direction	Westbound							
Design Speed	50 km/h							
Horizon	Background (2026)		Total (2026)		Background (2031)		Total (2031)	
Peak Hour	AM	PM	AM	PM	AM	PM	AM	PM
Advancing Volumes	310	196	318	216	338	216	346	236
Opposing Volumes	212	303	224	346	232	326	244	369
Left-Turning Traffic	28	26	36	46	30	29	38	49
% of Left-Turning Traffic	9%	13%	11%	21%	9%	13%	11%	21%
Figure Used*	9A-2	9A-3	9A-2	9A-3	9A-2	9A-3	9A-2	9A-3
Warranted	No	No	No	No	No	No	No	No
Storage Length Required	-	-	-	-	-	-	-	-

*Based on MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads - June 2017

TABLE 5.2: LEFT-TURN LANE WARRANT SUMMARY – IRVINE STREET

Intersection	Irvine Street & Bricker Avenue							
Approach Direction	Northbound							
Design Speed	60 km/h							
Horizon	Background (2026)		Total (2026)		Background (2031)		Total (2031)	
Peak Hour	AM	PM	AM	PM	AM	PM	AM	PM
Advancing Volumes	79	90	107	179	85	96	113	185
Opposing Volumes	54	76	135	130	57	81	138	135
Left-Turning Traffic	11	11	17	20	12	12	18	21
% of Left-Turning Traffic	14%	12%	16%	11%	14%	13%	16%	11%
Figure Used*	9A-7	9A-6	9A-7	9A-6	9A-7	9A-7	9A-7	9A-6
Warranted	No	No	No	No	No	No	No	No
Storage Length Required	-	-	-	-	-	-	-	-

*Based on MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads - June 2017

TABLE 5.3: LEFT-TURN LANE WARRANT SUMMARY – EAST MILL STREET (WR 18)

Intersection	East Mill Street (WR 18) & Irvine Street							
Approach Direction	Eastbound							
Design Speed	50 km/h							
Horizon	Background (2026)		Total (2026)		Background (2031)		Total (2031)	
Peak Hour	AM	PM	AM	PM	AM	PM	AM	PM
Advancing Volumes	351	245	362	271	387	270	398	296
Opposing Volumes	269	300	278	331	296	331	305	362
Left-Turning Traffic	8	29	19	55	8	31	19	57
% of Left-Turning Traffic	2%	12%	5%	20%	2%	11%	5%	19%
Figure Used*	9A-2	9A-2	9A-2	9A-3	9A-2	9A-2	9A-2	9A-3
Warranted	No	No	No	No	No	No	No	No
Storage Length Required	-	-	-	-	-	-	-	-

*Based on MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads - June 2017



5.1.4 Irvine Street at Street A

A design speed of 60 km/h (10 km/h over the posted speed limit was used for Irvine Street.

Table 5.4 summarizes the left-turn lane warrants for a northbound left-turn lane on Irvine Street at Street C with future total traffic volumes. The warrant analysis suggests that a northbound left-turn lane on Irvine Street is not warranted.

TABLE 5.4: LEFT-TURN LANE WARRANT SUMMARY – IRVINE STREET AT STREET A

Intersection	Irvine Street & Street A			
Approach Direction	Northbound			
Design Speed	60 km/h			
Horizon	Total (2026)		Total (2031)	
Peak Hour	AM	PM	AM	PM
Advancing Volumes	114	174	120	180
Opposing Volumes	73	139	77	144
Left-Turning Traffic	22	80	22	80
% of Left-Turning Traffic	19%	46%	18%	44%
Figure Used*	9A-7	9A-9	9A-7	9A-9
Warranted	No	No	No	No
Storage Length Required	-	-	-	-

*Based on MTO Design Supplement for TAC Geometric Design Guide for Canadian Roads - June 2017

As the future intersection operations at the study area intersections show no significant impacts, the need for auxiliary turn lanes are not warranted. No changes to the existing lane geometrics are recommended at this time.

5.2 Future Alternative Scenario

A future alternative scenario was requested to include two future developments in the area. The Elora Sands and Keating developments on the east side of Irvine Street from the subject site are currently in the early planning stages. The following provides the preliminary unit counts used to estimate the future traffic volumes:

- ▶ Elora Sands:
 - 304 single family units; and
 - 319 townhome units.



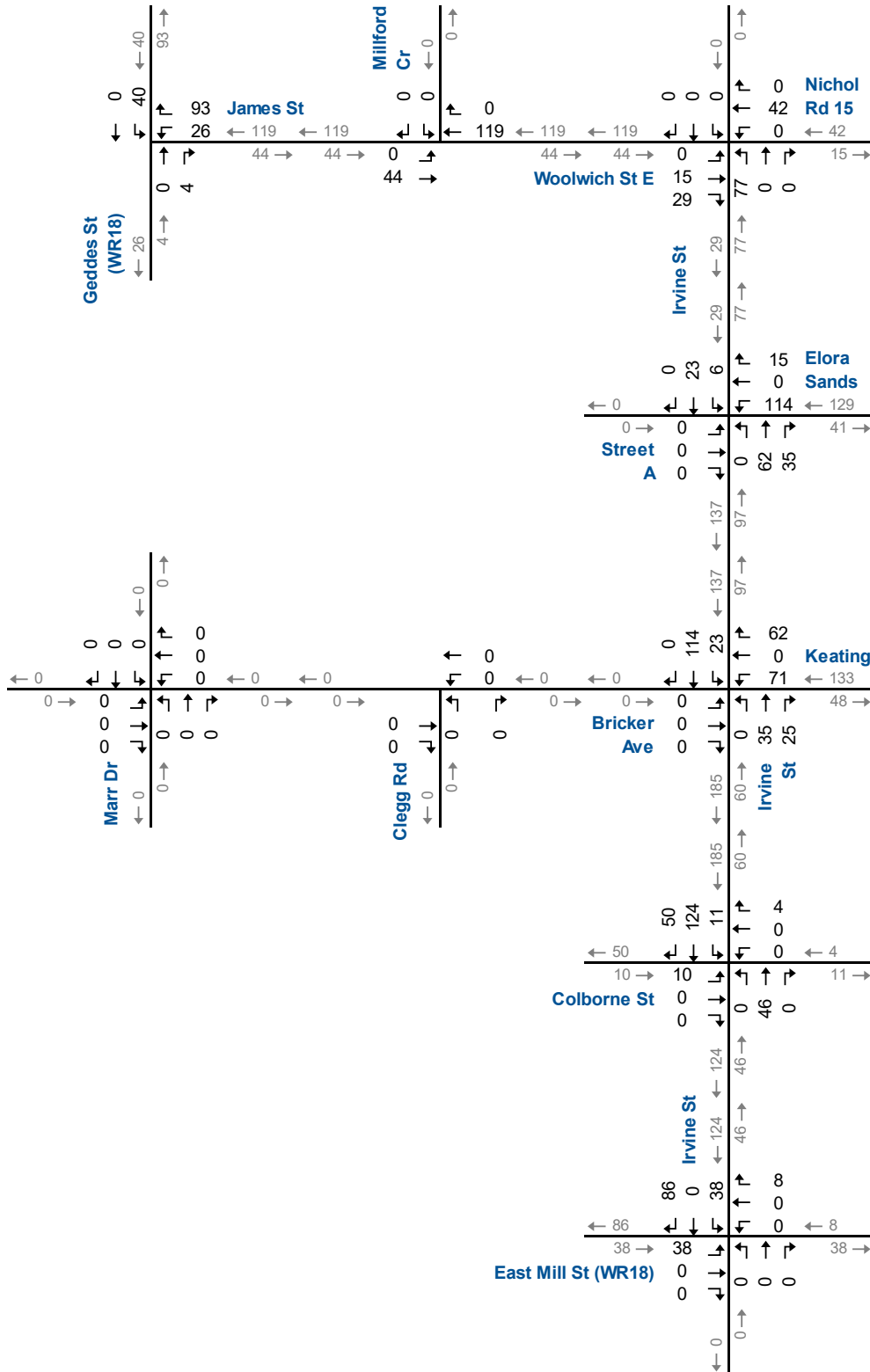
- ▶ Keating:
 - 329 single family units; and
 - 356 townhome units.

The traffic volumes from the future background developments were estimated using the ITE trip generation rates from Section 3.2. The future traffic volumes were then assigned to the local road network using the same distribution noted in Section 3.2.

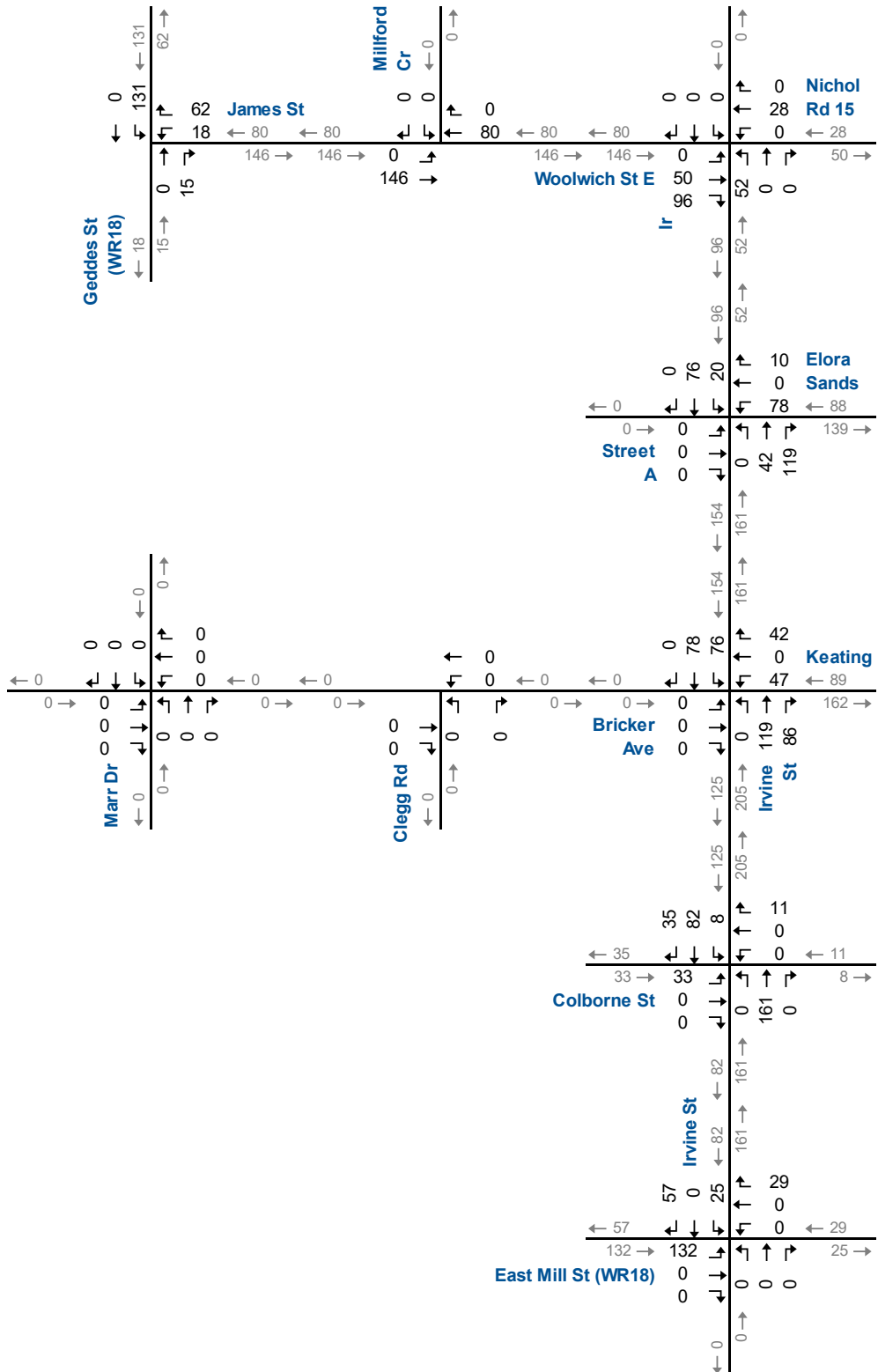
Figure 5.1A-B illustrates the future site generated traffic volumes from the Elora Sands and Keating developments for the weekday AM and PM peak hour.

The Elora Sands and Keating future traffic volumes were then added to the 2031 total traffic volumes. **Figure 5.2A-B** illustrates the 2031 future total traffic volumes.



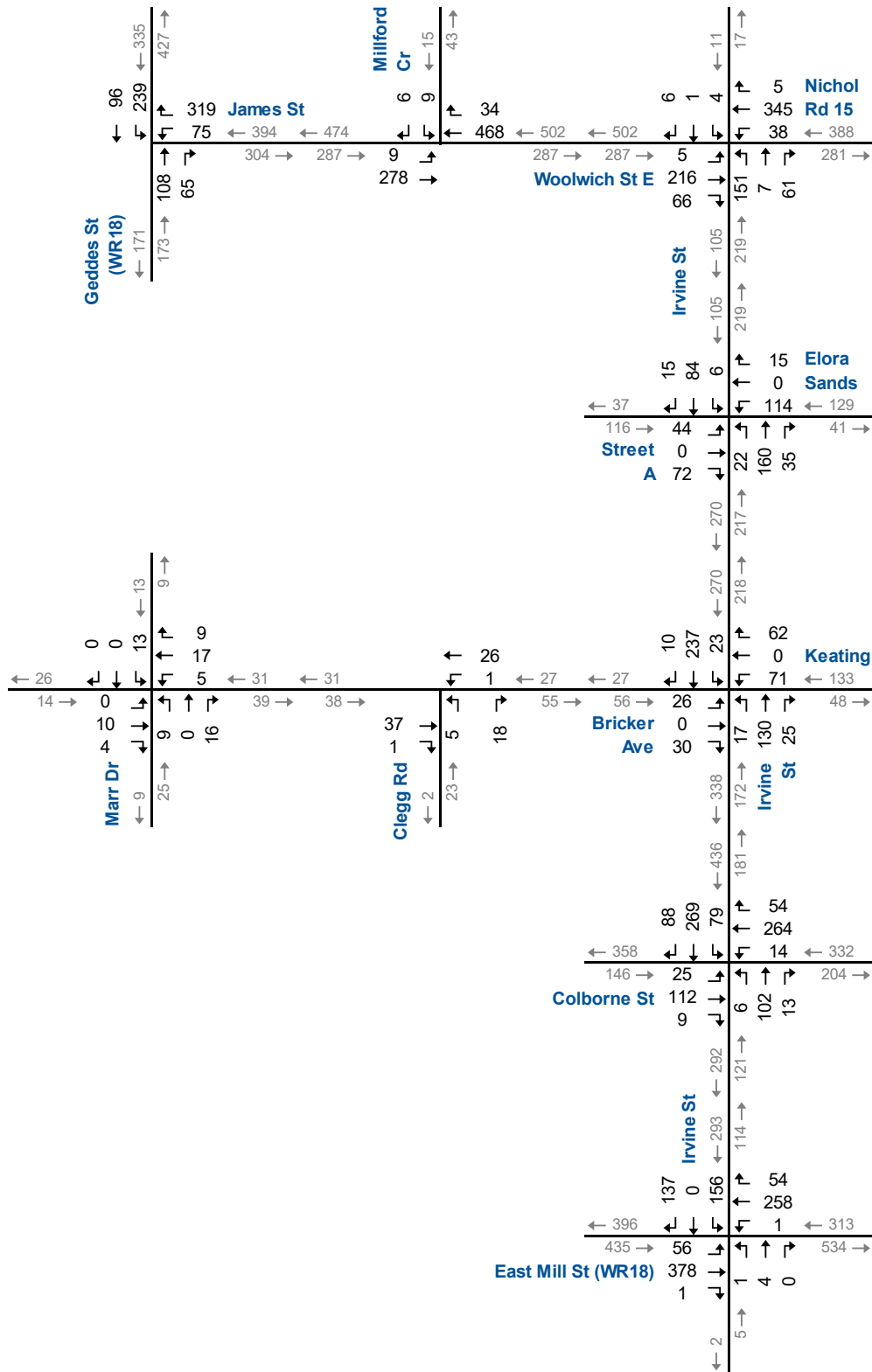


Future Development Site Generated Traffic Volumes AM Peak Hour

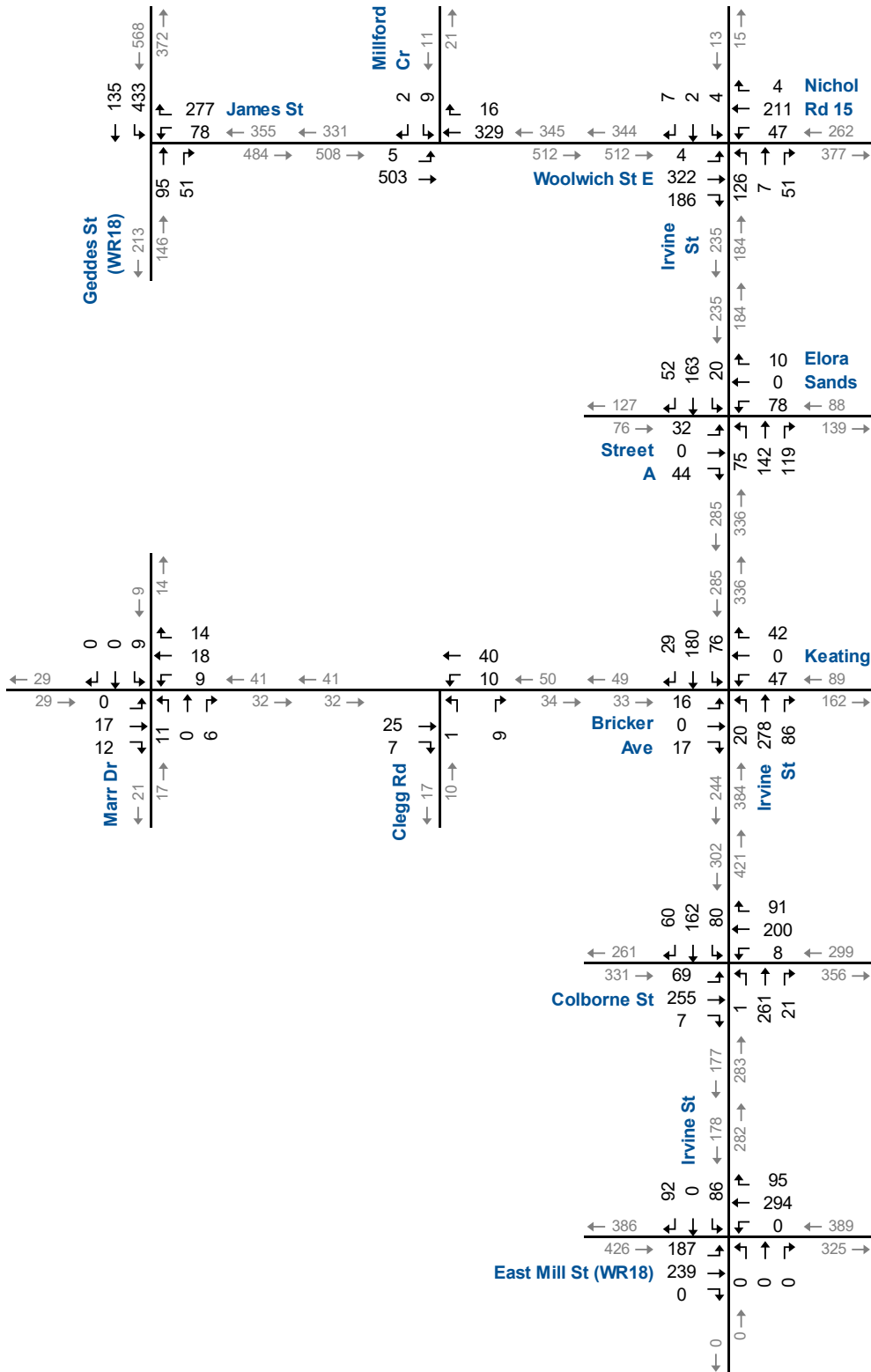


Future Development Site Generated Traffic Volumes PM Peak Hour

Figure 5.1B



Alternative 2031 Total Traffic Volumes AM Peak Hour



Alternative 2031 Total Traffic Volumes PM Peak Hour



The study area intersection operations analysis for the alternative future total traffic scenario followed the same methodology used for the 2031 total traffic conditions. **Table 5.5A-B** details the level of service conditions for the weekday AM and PM peak hours, respectively.

The study area intersections are forecast to operate within acceptable levels of service with the following critical movements noted:

- ▶ Geddes Street (Wellington Road 18) and James Street:
 - The westbound minor approach with LOS F and v/c ratio of 0.9 during the PM peak hour.
- ▶ East Mill Street (Wellington Road 18) and Irvine Street:
 - The southbound minor approach with LOS F and v/c ratio of 0.96 during the AM peak hour. LOS F and v/c ratio of 0.80 during the PM peak hour.

Appendix H contains the detailed Synchro reports.

It is recommended that the County of Wellington and Township of Centre Wellington include the above intersections in the study area for the traffic studies of both future developments to ensure the proper form of traffic control is identified for future traffic conditions.



TABLE 5.5A: 2031 TOTAL ALTERNATIVE SCENARIO OPERATIONS (AM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall		
				Eastbound				Westbound				Northbound				Southbound						
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach			
AM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.01 0	> > > >	A 0 0.32 0	> > > >	A 0 0 0	> > > >								B 15 0.04 1	> > > >	B 15 0 0	A 0	
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.03 1	> > > >	A 1 0 1	> > > >	< < < <	D 30 0.64 33	> > > >	D 30 0 1	> > > >	< < < <	B 14 0.03 1	> > > >	B 14 0 0	A 8		
	Irvine St & Bricker St/Keating Lands	TWSC	LOS Delay V/C Q	< < < <	B 13 0.12 3	> > > >	B 13 0.26 8	> > > >	B 14 0 0	> > > >	< < < <	A 1 0.02 0	> > > >	A 1 0 0	> > > >	< < < <	A 1 0.02 0	> > > >	A 1 0 0	A 5		
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q		A 0 0.02 0	> > > >	A 0 0.00 0	> > > >	A 0 0 0	> > > >	A 9 0.03 1				A 9 0 0							A 2
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.00 0	> > > >	A 1 0 0	> > > >	< < < <	A 10 0.03 1	> > > >	A 10 0 0	> > > >	< < < <	A 9 0.02 0	> > > >	A 9 0 0	A 5		
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q					C 23 0.70 42	> > > >	C 23 0 0	> > > >			A 0 0.11 0	> > > >	< < < <	A 6 0.19 5			A 6 0 0	B 13	
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	B 13 0.31 0	> > > >	B 13 0.63 0	> > > >	C 19 0 0	> > > >	< < < <	B 13 0.27 0	> > > >	B 13 0 0	> > > >	< < < <	D 31 0.82 0	> > > >	D 31 0 0	C 23		
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 2 0.05 1	> > > >	A 2 0.00 0	> > > >	A 0 0 0	> > > >	< < < <	C 21 0.02 1	> > > >	C 21 0 0	> > > >	< < < <	F 74 0.96 77	> > > >	F 74 0 0	C 22		
	Irvine St & Street A / Elora Sands	TWSC	LOS Delay V/C Q	< < < <	B 11 0.18 5	> > > >	B 11 0.28 9	> > > >	B 15 0 0	> > > >	< < < <	A 1 0.02 0	> > > >	A 1 0 0	> > > >	< < < <	A 1 0.01 0	> > > >	A 1 0 0	A 6		

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement



TABLE 5.5B: 2031 TOTAL ALTERNATIVE SCENARIO OPERATIONS (PM PEAK HOUR)

Analysis Period	Intersection	Control Type	MOE	Direction/Movement/Approach																Overall
				Eastbound				Westbound				Northbound				Southbound				
				Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	Left	Through	Right	Approach	
PM Peak Hour	Woolwich St & Milford Cres	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.22 0	> > > >	A 0 0 0	> > > >					C 16 0.04 1	> > > >	B 16 0 0	A 0 0 0		
	Irvine St & Woolwich St/Nichol Rd 15	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.06 1	> > > >	A 2 0 1	> > > >	< < < <	D 31 0.61 29	> > > >	D 31 0.03 1	> > > >	B 14 0 0	A 7 0 0			
	Irvine St & Bricker St / Keating	TWSC	LOS Delay V/C Q	< < < <	C 15 0.09 2	> > > >	C 15 0.25 8	> > > >	C 18 0 0	> > > >	< < < <	A 1 0.02 0	> > > >	A 1 0.07 2	> > > >	A 3 0 0	A 4 0 0			
	Clegg Rd & Bricker St	TWSC	LOS Delay V/C Q		A 0 0.02 0	> > > >	A 2 0.01 0	> > > >	A 2 0 0	> > > >	A 9 0.01 0	> > > >	A 9 0 0					A 2 0 0		
	Marr Dr & Bricker St	TWSC	LOS Delay V/C Q	< < < <	A 0 0.00 0	> > > >	A 0 0.01 0	> > > >	A 2 0 0	> > > >	< < < <	A 10 0.02 1	> > > >	A 10 0.01 0	> > > >	A 9 0 0	A 3 0 0			
	Geddes St (WR18) & James St	TWSC	LOS Delay V/C Q					F 77 0.99 90	> > > >	F 77 0 0	> > > >	A 0 0.09 0	> > > >	A 0 0.34 12	> > > >	A 8 0 0	A 8 0 0	D 30 0 0		
	Irvine St & Colborne St	AWSC	LOS Delay V/C Q	< < < <	D 33 0.78 0	> > > >	D 27 0.70 0	> > > >	D 27 0 0	> > > >	< < < <	D 29 0.71 0	> > > >	D 29 0.74 0	> > > >	D 31 0 0	D 30 0 0			
	East Mill St (WR18) & Irvine St	TWSC	LOS Delay V/C Q	< < < <	A 5 0.18 5	> > > >	A 5 0.00 0	> > > >	A 0 0 0	> > > >	< < < <	A 0 0.61 0	> > > >	A 0 0.80 46	> > > >	F 59 0 0	F 59 0 0	B 13 0 0		
	Irvine St & Street A / Elora Sands	TWSC	LOS Delay V/C Q	< < < <	B 13 0.17 5	> > > >	C 20 0.29 9	> > > >	C 20 0 0	> > > >	< < < <	A 2 0.07 2	> > > >	A 2 0.02 0	> > > >	A 1 0 0	A 5 0 0			

MOE - Measure of Effectiveness
 LOS - Level of Service
 Delay - Average Delay per Vehicle in Seconds
 V/C - Volume to Capacity Ratio
 Q - 95th Percentile Queue Length (m)
 TWSC - Two-Way Stop Control
 AWSC - All-Way Stop Control
 </> - Shared with through movement

6 Conclusions and Recommendations

6.1 Conclusions

Based on the investigations carried out, it is concluded that:

- ▶ **Existing Traffic Conditions:** The study area intersections are currently operating within acceptable levels of service with no specific problem movements during the AM and PM peak hours.
- ▶ **Development Trip Generation:** The residential development is forecast to generate approximately 174 and 227 trips during the AM and PM peak hours upon full build-out.
- ▶ **2026 Background Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements during the AM and PM peak hours.
- ▶ **2026 Total Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service during the AM and PM peak hours with no specific problem movements.
- ▶ The proposed municipal street connection to Irvine Street is forecast to operate within acceptable levels of service during the AM and PM peak hours.
- ▶ The addition of the site generated traffic increases the overall delay at the study area intersections by two second or less during the AM and PM peak hours.
- ▶ **2031 Background Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service with no specific problem movements during the AM and PM peak hours.
- ▶ **2031 Total Traffic Conditions:** The study area intersections are forecast to operate within acceptable levels of service during the AM and PM peak hours with no specific problem movements.
- ▶ The proposed municipal street connection to Irvine Street is forecast to operate within acceptable levels of service during the AM and PM peak hours.
- ▶ The addition of the site generated traffic increases the overall delay at the study area intersections by three second or less during the AM and PM peak hours.
- ▶ **Remedial Measures:** Left-turn lanes are not warranted at the following intersections:



- Westbound on Nichol Road 15 at Irvine;
 - Northbound on Irvine Street at Bricker Avenue;
 - Eastbound on East Mill Street (WR 18) at Irvine Street; and
 - Northbound on Irvine Street at Street A.
- ▶ **Alternative 2031 Scenario** with future Elora Sands and Keating developments identified critical movements at the following intersections under 2031 total traffic conditions:
- Geddes Street (Wellington Road 18) at James Street); and
 - East Mill Street (Wellington Road 18) at Irvine Street.

6.2 Recommendations

Based on the findings of this study, it is recommended that the development application be approved with no provision for off-site transportation network improvements.

It is recommended that the County of Wellington and Township of Centre Wellington include the intersections of Geddes Street at James Street and East Mill Street at Irvine Street for the traffic studies of both Elora Sands and Keating future developments to ensure the proper form of traffic control is identified for future traffic conditions.



Appendix A

Pre-Study Consultation



From: [Lee Wheildon](#)
To: [Andrew Evans](#)
Cc: [Colin Baker](#); [Pasquale Costanzo](#); [Ray Kirtz](#); [Dustin Lyttle](#); [Howard Wray](#); [Erica Bayley](#); [Chris Skelton](#)
Subject: RE: (210662) Residential Development, Woolwich Street & Irvine Street, Elora - Scope of Work
Date: December 3, 2021 11:29:29 AM
Attachments: [image002.jpg](#)
[image005.png](#)
[image001.jpg](#)

Andrew,

Please note that Triton Engineering Services Limited has completed a review of the Scope of Work outlined below on behalf of both the Township and County. Please see the redline comments below regarding the scope of work.

Should you have any questions or concerns, please do not hesitate to contact me.

Regards,

Lee Wheildon C.E.T.,rcca | Engineering Technologist - Development

Township of Centre Wellington | 1 MacDonald Square, Elora, ON N0B 1S0
519.846.9691 x253 [CentreWellington.ca](#)

From: Howard Wray <hwray@tritoneng.on.ca>
Sent: November 30, 2021 9:29 AM
To: Lee Wheildon <LWheildon@centrewellington.ca>
Cc: Colin Baker <CBaker@centrewellington.ca>; Pasquale Costanzo <pasqualec@wellington.ca>; Ray Kirtz <rkirtz@tritoneng.on.ca>; Dustin Lyttle <dlyttle@tritoneng.on.ca>
Subject: RE: (210662) Residential Development, Woolwich Street & Irvine Street, Elora - Scope of Work

You don't often get email from hwray@tritoneng.on.ca. [Learn why this is important](#)

Lee,

Pls see our comments on the study scope below **in red**.

Note that while we have attempted to identify the study requirements, the Township should reserve the right to require additional analysis should other issues be identified during the review.

Howard Wray, P. Eng.

Triton Engineering Services Limited
229 Broadway, Unit 1 Orangeville, ON L9W 1K4
Tel (519) 941-0330 ext 223 • Fax (519) 941-1830 • www.tritoneng.on.ca

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From: Lee Wheildon <LWheildon@centrewellington.ca>
Sent: November 29, 2021 2:55 PM
To: Howard Wray <hwray@tritoneng.on.ca>
Cc: Colin Baker <CBaker@centrewellington.ca>; Pasquale Costanzo <pasqualec@wellington.ca>
Subject: FW: (210662) Residential Development, Woolwich Street & Irvine Street, Elora - Scope of Work

Howard,

As discussed, are you available to provide some peer review comments for the Terms of Reference/Scope of Work on Paradigms proposed TIS Study below? Additionally, once the reporting is completed, do you have capacity to complete a peer review on the Township's behalf?

Both Township and County Staff have highlighted the need for the intersection of James Street/Geddes Street to be included into their scope of the study.

Should you have any questions or concerns, please do not hesitate to contact me.

Regards,

Lee Wheildon C.E.T.,rcca | Engineering Technologist - Development

Township of Centre Wellington | 1 MacDonald Square, Elora, ON N0B 1S0
519.846.9691 x253 CentreWellington.ca

From: Andrew Evans <aevans@ptsl.com>
Sent: November 15, 2021 9:52 AM
To: Lee Wheildon <LWheildon@centrewellington.ca>; pasqualec@wellington.ca
Cc: Erica Bayley <ebayley@ptsl.com>; Chris Skelton <cskelton@ptsl.com>
Subject: (210662) Residential Development, Woolwich Street & Irvine Street, Elora - Scope of Work

Greetings,

Paradigm was retained to undertake a Transportation Impact Study for a proposed residential development to be located on the southwest corner of Woolwich Street/Nichol Road 15 and Irvine Street, Township of Centre Wellington (Elora).

The property owner is proposing to develop the approximately 12.4 hectare block into 222 residential units, in a mix of townhouse (63 units) and single-detached homes (159 units).

Vehicle access is proposed via new street connections to Woolwich Street, Irvine Street, and Bricker Avenue.

Below is our scope of work for you review and approval:

Study Area Intersections:

- Woolwich Street/Nichol Road 15 & Irvine Street (unsignalized);
- Woolwich Street & Milford Crescent East (unsignalized);
- Irvine Street & Bricker Avenue (unsignalized);
- Bricker Avenue & Clegg Road (unsignalized);
- Bricker Avenue & Marr Drive (unsignalized); and
- Two new connections to Irvine Street (assumed unsignalized).

The following two intersections are required to be studied to identify the impact on the collector/arterial system

- Geddes/James
- Irvine/Colborne
- Irvine/East Mill Street

Planning Horizons:

- Five years from date of study (Year 2026) **Revise to year of anticipated build-out, plus 5 year horizon from build-out.**

Analysis Periods:

- Weekday AM and PM peak hours. **OK**

Existing Traffic:

- Derived from Turning Movement Counts at study area intersections **OK. Note that traffic counts should be representative of AADT (typically spring or fall). Counts taken outside this period should be adjusted. The impact of Covid-19 restrictions and work from home should be considered based on the conditions at the time the counts are taken. Although restrictions have largely been lifted, this should still be addressed in the report.**

Background Traffic:

- A background growth rate of 2.0% per annum (please confirm) **Rate is acceptable, in addition to traffic from adjacent approved and planned developments**
- Please provide any background developments from nearby approved and/or in-stream developments **Township to provide.**

Site Generated Traffic:

- ITE Trip Generation Manual (11th Edition) **OK**
- Trip Distribution based on Existing Traffic Patterns **Trip Distribution should be developed based on an analysis of anticipated destinations (work/shopping) as well as Existing Traffic Patterns. Convenient access to arterials and collectors should be considered in the analysis. The Consultant may provide their Trip**

Distribution assumptions for comment to the Township/Peer Reviewer prior to completion of the report if they wish.

Report

- We will document the study methodologies, findings, and conclusions in a report with appendices containing the detailed analysis results and any data collected.

The Report will include

- Site Plan and Map,
- Size & Number of Development Phases (if any)
- Existing Conditions (Study Area Intersections, Road Network, Pedestrian Routes, Cycling Routes, Transit Services, etc.)
- Existing Traffic Conditions (Site Operating Characteristics, Data Collection/Traffic Counts, Analysis Periods (5 years Ahead),
- Future Background Conditions (Horizon Years, Horizon Year Volumes)
- Background Traffic Demand Forecast (with acceptable growth rates)
- Site Generated Traffic (Transit Modal Split, Trip Generation/Distribution/Assignment)
- Future Total Traffic Demand,
- Capacity Analysis (by Intersection, with LOS, Avg. Delay, V/C ratios, 95th Queue length),
- Traffic Impacts (Tables – Total Traffic with/without Mitigation)
- Access Considerations – Existing, Proposed, Geometrics (turn lanes, sight lines),
- Recommendations - Identify required/recommended road improvements either as a result of the development impacts, or general non-development improvements.

A review of pedestrian and active transportation routes and objectives is required. Connectivity to existing and proposed pedestrian and active transportation routes shall be considered. Identify the potential need for pedestrian crossings (pedestrian signals or PXOs).

Thank you and regards.

Andrew Evans, M.Sc.

Transportation Planner



Paradigm Transportation Solutions Limited

5A-150 Pinebush Road Cambridge ON N1R 8J8

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Appendix B

Traffic Data





Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsI.com

Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 1

Turning Movement Data

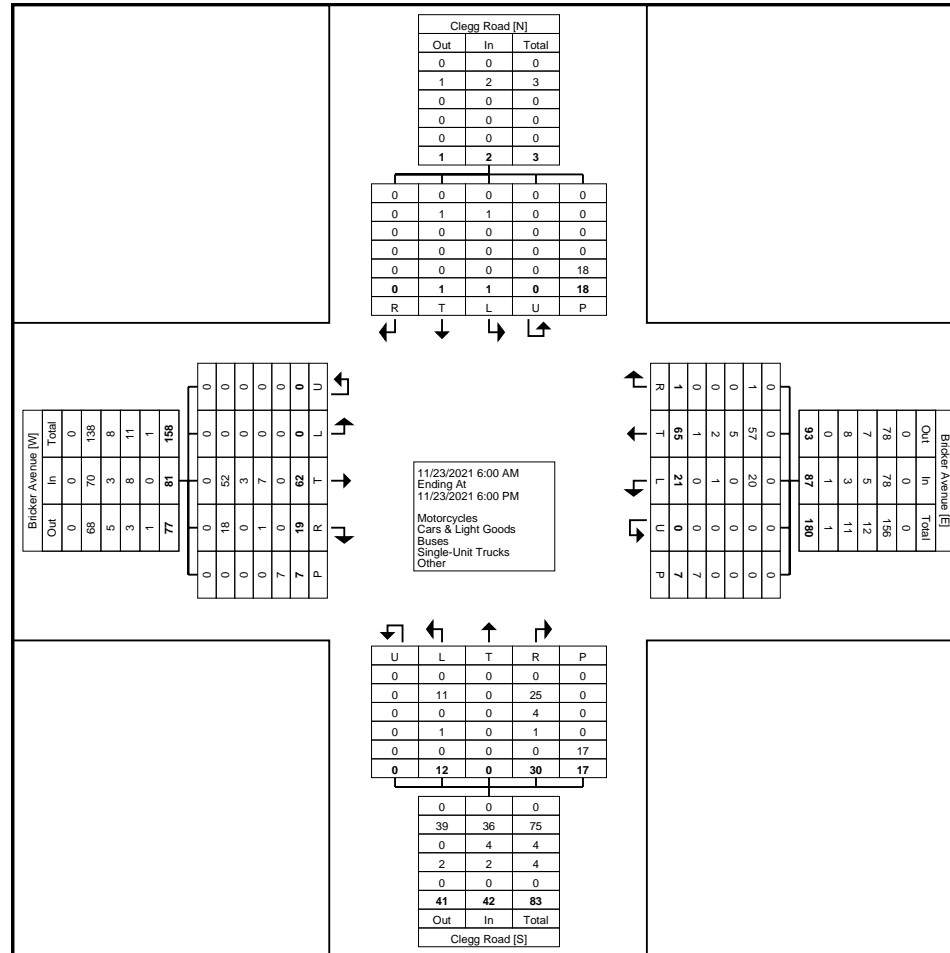
Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Clegg Road Northbound						Clegg Road Southbound						Int. Total	
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total		
6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	1
6:30 AM	0	0	0	0	0	0	1	0	0	0	0	1	2	0	0	0	0	2	0	0	0	0	1	0	3	
6:45 AM	0	0	0	0	1	0	0	3	0	0	1	3	0	0	0	0	0	0	0	0	0	0	2	0	3	
Hourly Total	0	0	0	0	1	0	1	3	0	0	1	4	2	0	1	0	0	3	0	0	0	0	3	0	7	
7:00 AM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	0	0	1	0	1	0	0	0	0	1	2	
7:15 AM	0	1	0	0	0	1	1	2	0	0	0	3	1	0	2	0	0	3	0	0	0	0	0	0	7	
7:30 AM	0	0	0	0	0	0	0	2	0	0	0	2	1	0	1	0	0	2	0	0	0	0	0	0	4	
7:45 AM	0	3	0	0	0	3	1	2	0	0	0	3	0	0	0	0	1	0	0	0	0	0	0	0	6	
Hourly Total	0	4	0	0	0	4	2	7	0	0	0	9	2	0	3	0	2	5	1	0	0	0	0	1	19	
8:00 AM	0	2	0	0	0	2	0	4	0	0	0	4	0	0	4	0	2	4	0	0	0	0	0	0	10	
8:15 AM	0	5	0	0	2	5	0	1	0	0	1	1	2	0	2	0	2	4	0	0	0	0	2	0	10	
8:30 AM	0	4	0	0	0	4	1	1	0	0	0	2	1	0	4	0	0	5	0	0	0	0	0	0	11	
8:45 AM	0	1	1	0	0	2	0	4	0	0	0	4	0	0	1	0	0	1	0	0	0	0	0	0	7	
Hourly Total	0	12	1	0	2	13	1	10	0	0	1	11	3	0	11	0	4	14	0	0	0	0	2	0	38	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
12:00 PM	0	3	0	0	0	3	1	1	0	0	0	2	0	0	0	0	1	0	0	0	0	0	0	0	5	
12:15 PM	0	1	0	0	0	1	0	1	0	0	0	1	0	0	1	0	0	1	0	0	0	0	0	0	3	
12:30 PM	0	1	1	0	0	2	0	2	0	0	0	2	1	0	0	0	1	1	0	0	0	0	1	0	5	
12:45 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2	0	0	2	0	0	0	0	0	0	3	
Hourly Total	0	5	1	0	0	6	1	5	0	0	0	6	1	0	3	0	2	4	0	0	0	0	1	0	16	
1:00 PM	0	3	1	0	0	4	0	2	0	0	0	2	0	0	1	0	2	1	0	0	0	0	0	0	7	
1:15 PM	0	1	0	0	0	1	0	4	0	0	0	4	1	0	0	0	0	1	0	0	0	0	1	0	6	
1:30 PM	0	2	0	0	0	2	2	0	0	0	0	2	0	0	0	0	1	0	0	0	0	0	1	0	4	
1:45 PM	0	3	0	0	2	3	0	0	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	4	
Hourly Total	0	9	1	0	2	10	2	6	0	0	0	8	2	0	1	0	3	3	0	0	0	0	2	0	21	
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
3:00 PM	0	0	1	0	0	1	2	1	0	0	0	3	0	0	1	0	0	1	0	0	0	0	0	0	5	
3:15 PM	0	1	2	0	0	3	3	5	0	0	0	8	0	0	4	0	0	4	0	0	0	0	2	0	15	
3:30 PM	0	4	1	0	1	5	2	6	0	0	4	8	0	0	0	0	0	0	0	0	0	0	3	0	13	
3:45 PM	0	6	2	0	0	8	2	2	0	0	0	4	1	0	2	0	2	3	0	1	0	0	0	1	16	
Hourly Total	0	11	6	0	1	17	9	14	0	0	4	23	1	0	7	0	2	8	0	1	0	0	5	1	49	
4:00 PM	0	1	1	0	0	2	0	4	0	0	0	4	0	0	1	0	0	1	0	0	0	0	1	0	7	
4:15 PM	0	0	2	0	1	2	1	1	0	0	0	2	0	0	0	0	1	0	0	0	0	0	0	0	4	
4:30 PM	0	3	2	0	0	5	2	3	0	0	0	5	1	0	1	0	2	2	0	0	0	0	1	0	12	



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5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@pts.com

Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 3



Turning Movement Data Plot



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsI.com

Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 4

Turning Movement Peak Hour Data (8:00 AM)

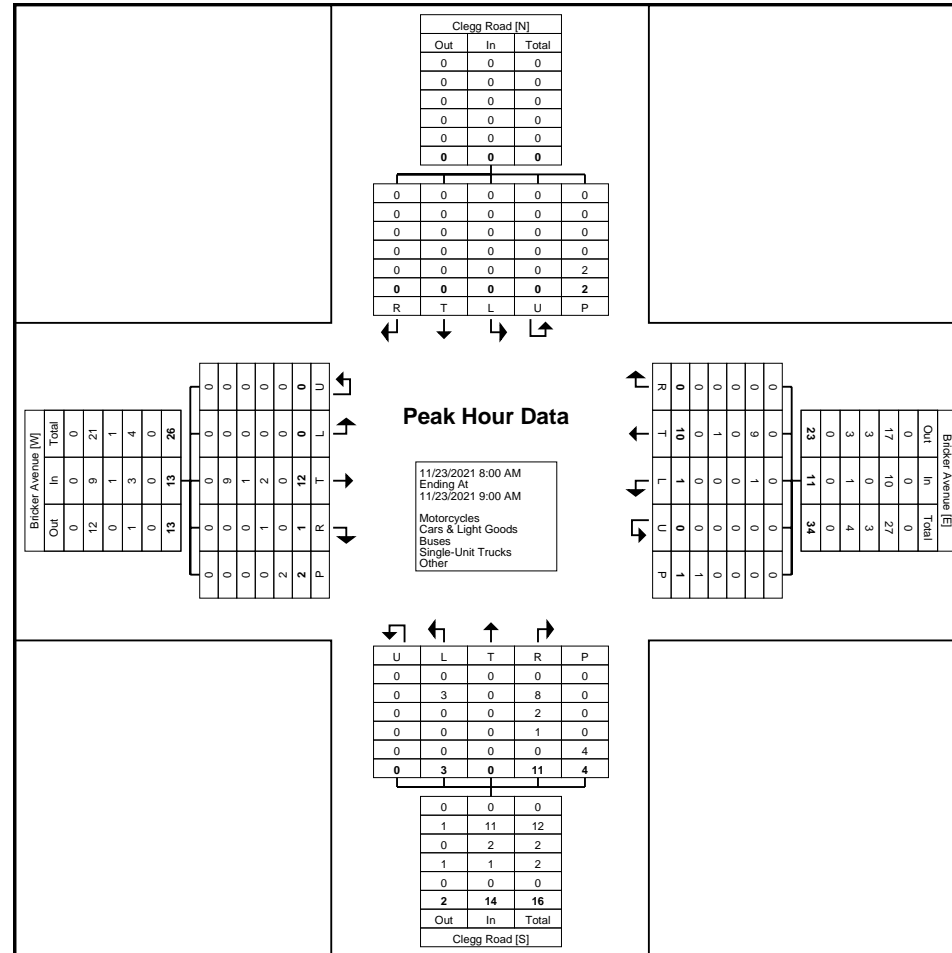
Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Clegg Road Northbound						Clegg Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
8:00 AM	0	2	0	0	0	2	0	4	0	0	0	4	0	0	4	0	2	4	0	0	0	0	0	0	10
8:15 AM	0	5	0	0	2	5	0	1	0	0	1	1	2	0	2	0	2	4	0	0	0	0	2	0	10
8:30 AM	0	4	0	0	0	4	1	1	0	0	0	2	1	0	4	0	0	5	0	0	0	0	0	0	11
8:45 AM	0	1	1	0	0	2	0	4	0	0	0	4	0	0	1	0	0	1	0	0	0	0	0	0	7
Total	0	12	1	0	2	13	1	10	0	0	1	11	3	0	11	0	4	14	0	0	0	0	2	0	38
Approach %	0.0	92.3	7.7	0.0	-	-	9.1	90.9	0.0	0.0	-	-	21.4	0.0	78.6	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	31.6	2.6	0.0	-	34.2	2.6	26.3	0.0	0.0	-	28.9	7.9	0.0	28.9	0.0	-	36.8	0.0	0.0	0.0	0.0	-	0.0	-
PHF	0.000	0.600	0.250	0.000	-	0.650	0.250	0.625	0.000	0.000	-	0.688	0.375	0.000	0.688	0.000	-	0.700	0.000	0.000	0.000	0.000	-	0.000	0.864
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Cars & Light Goods	0	9	0	0	-	9	1	9	0	0	-	10	3	0	8	0	-	11	0	0	0	0	-	0	30
% Cars & Light Goods	-	75.0	0.0	-	-	69.2	100.0	90.0	-	-	-	90.9	100.0	-	72.7	-	-	78.6	-	-	-	-	-	-	78.9
Buses	0	1	0	0	-	1	0	0	0	0	-	0	0	0	2	0	-	2	0	0	0	0	-	0	3
% Buses	-	8.3	0.0	-	-	7.7	0.0	0.0	-	-	-	0.0	0.0	-	18.2	-	-	14.3	-	-	-	-	-	-	7.9
Single-Unit Trucks	0	2	1	0	-	3	0	1	0	0	-	1	0	0	1	0	-	1	0	0	0	0	-	0	5
% Single-Unit Trucks	-	16.7	100.0	-	-	23.1	0.0	10.0	-	-	-	9.1	0.0	-	9.1	-	-	7.1	-	-	-	-	-	-	13.2
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	2	-	-	-	-	-	1	-	-	-	-	-	4	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



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Count Name: Bricker Avenue & Clegg Road
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Start Date: 11/23/2021
Page No: 5



Turning Movement Peak Hour Data Plot (8:00 AM)



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Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsl.com

Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 6

Turning Movement Peak Hour Data (12:30 PM)

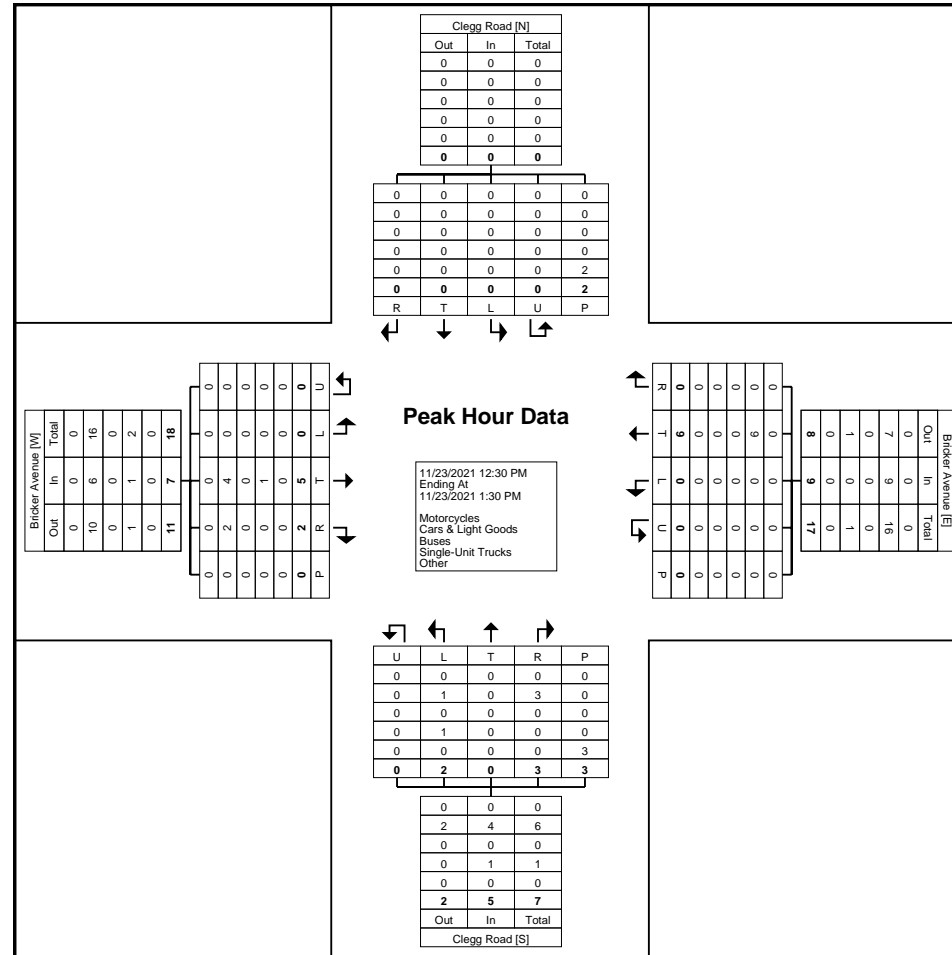
Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Clegg Road Northbound						Clegg Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:30 PM	0	1	1	0	0	2	0	2	0	0	0	2	1	0	0	0	1	1	0	0	0	0	1	0	5
12:45 PM	0	0	0	0	0	0	0	1	0	0	0	1	0	0	2	0	0	2	0	0	0	0	0	0	3
1:00 PM	0	3	1	0	0	4	0	2	0	0	0	2	0	0	1	0	2	1	0	0	0	0	0	0	7
1:15 PM	0	1	0	0	0	1	0	4	0	0	0	4	1	0	0	0	0	1	0	0	0	0	1	0	6
Total	0	5	2	0	0	7	0	9	0	0	0	9	2	0	3	0	3	5	0	0	0	0	2	0	21
Approach %	0.0	71.4	28.6	0.0	-	-	0.0	100.0	0.0	0.0	-	-	40.0	0.0	60.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	23.8	9.5	0.0	-	33.3	0.0	42.9	0.0	0.0	-	42.9	9.5	0.0	14.3	0.0	-	23.8	0.0	0.0	0.0	0.0	-	0.0	-
PHF	0.000	0.417	0.500	0.000	-	0.438	0.000	0.563	0.000	0.000	-	0.563	0.500	0.000	0.375	0.000	-	0.625	0.000	0.000	0.000	0.000	-	0.000	0.750
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Cars & Light Goods	0	4	2	0	-	6	0	9	0	0	-	9	1	0	3	0	-	4	0	0	0	0	-	0	19
% Cars & Light Goods	-	80.0	100.0	-	-	85.7	-	100.0	-	-	-	100.0	50.0	-	100.0	-	-	80.0	-	-	-	-	-	-	90.5
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Single-Unit Trucks	0	1	0	0	-	1	0	0	0	0	-	0	1	0	0	0	-	1	0	0	0	0	-	0	2
% Single-Unit Trucks	-	20.0	0.0	-	-	14.3	-	0.0	-	-	-	0.0	50.0	-	0.0	-	-	20.0	-	-	-	-	-	-	9.5
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	3	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 7



Turning Movement Peak Hour Data Plot (12:30 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 8

Turning Movement Peak Hour Data (3:15 PM)

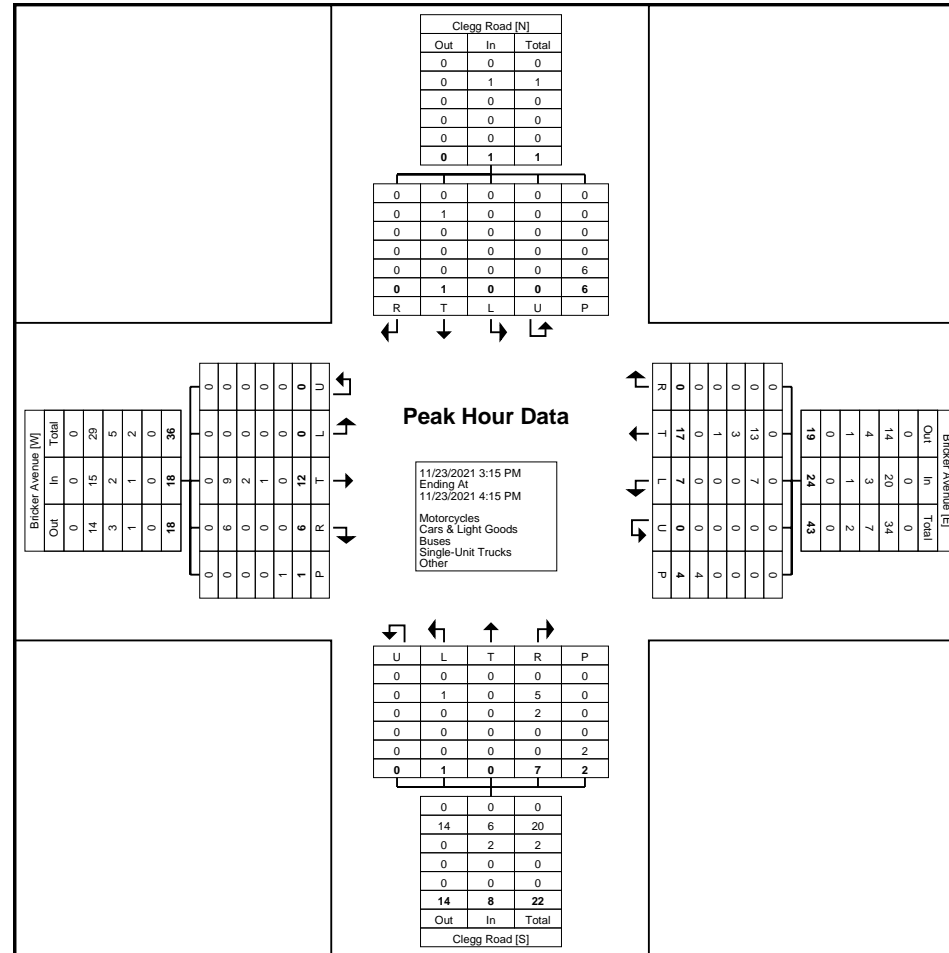
Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Clegg Road Northbound						Clegg Road Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
3:15 PM	0	1	2	0	0	3	3	5	0	0	0	8	0	0	4	0	0	4	0	0	0	0	2	0	15
3:30 PM	0	4	1	0	1	5	2	6	0	0	4	8	0	0	0	0	0	0	0	0	0	0	3	0	13
3:45 PM	0	6	2	0	0	8	2	2	0	0	0	4	1	0	2	0	2	3	0	1	0	0	0	1	16
4:00 PM	0	1	1	0	0	2	0	4	0	0	0	4	0	0	1	0	0	1	0	0	0	0	1	0	7
Total	0	12	6	0	1	18	7	17	0	0	4	24	1	0	7	0	2	8	0	1	0	0	6	1	51
Approach %	0.0	66.7	33.3	0.0	-	-	29.2	70.8	0.0	0.0	-	-	12.5	0.0	87.5	0.0	-	-	0.0	100.0	0.0	0.0	-	-	-
Total %	0.0	23.5	11.8	0.0	-	35.3	13.7	33.3	0.0	0.0	-	47.1	2.0	0.0	13.7	0.0	-	15.7	0.0	2.0	0.0	0.0	-	2.0	-
PHF	0.000	0.500	0.750	0.000	-	0.563	0.583	0.708	0.000	0.000	-	0.750	0.250	0.000	0.438	0.000	-	0.500	0.000	0.250	0.000	0.000	-	0.250	0.797
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0
Cars & Light Goods	0	9	6	0	-	15	7	13	0	0	-	20	1	0	5	0	-	6	0	1	0	0	-	1	42
% Cars & Light Goods	-	75.0	100.0	-	-	83.3	100.0	76.5	-	-	-	83.3	100.0	-	71.4	-	-	75.0	-	100.0	-	-	-	100.0	82.4
Buses	0	2	0	0	-	2	0	3	0	0	-	3	0	0	2	0	-	2	0	0	0	0	-	0	7
% Buses	-	16.7	0.0	-	-	11.1	0.0	17.6	-	-	-	12.5	0.0	-	28.6	-	-	25.0	-	0.0	-	-	-	0.0	13.7
Single-Unit Trucks	0	1	0	0	-	1	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	2
% Single-Unit Trucks	-	8.3	0.0	-	-	5.6	0.0	5.9	-	-	-	4.2	0.0	-	0.0	-	-	0.0	-	0.0	-	-	-	0.0	3.9
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	0.0	-	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	4	-	-	-	-	-	2	-	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Bricker Avenue & Clegg Road
Site Code: 210662
Start Date: 11/23/2021
Page No: 9



Turning Movement Peak Hour Data Plot (3:15 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@pts1.com

Count Name: Bricker Avenue & Marr Drive
Site Code: 210662
Start Date: 11/23/2021
Page No: 1

Turning Movement Data

Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Marr Drive Northbound						Marr Drive Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
6:30 AM	0	0	0	0	0	0	1	1	0	0	0	2	1	0	0	0	0	1	0	0	0	0	1	0	3
6:45 AM	0	0	0	0	1	0	1	2	0	0	0	3	0	0	0	0	0	0	0	0	0	2	0	0	3
Hourly Total	0	0	0	0	2	0	2	3	0	0	0	5	1	0	0	0	0	1	0	0	0	3	0	0	6
7:00 AM	0	0	1	0	0	1	1	1	0	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	3
7:15 AM	0	1	0	0	0	1	0	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	4
7:30 AM	0	0	0	0	0	0	0	3	0	0	0	3	1	0	0	0	0	1	0	0	0	0	0	0	4
7:45 AM	0	2	1	0	0	3	2	2	0	0	0	4	0	0	2	0	1	2	0	0	0	0	0	0	9
Hourly Total	0	3	2	0	0	5	3	9	0	0	0	12	1	0	2	0	1	3	0	0	0	0	0	0	20
8:00 AM	0	1	0	0	0	1	1	3	0	0	0	4	3	0	3	0	1	6	0	0	0	0	0	0	11
8:15 AM	0	3	1	0	0	4	0	3	0	0	1	3	2	0	2	0	0	4	0	0	0	0	4	0	11
8:30 AM	0	0	0	0	0	0	1	1	0	0	3	2	0	0	3	0	7	3	0	0	0	0	1	0	5
8:45 AM	0	1	1	0	1	2	1	4	0	0	0	5	0	1	2	0	1	3	0	0	0	0	0	0	10
Hourly Total	0	5	2	0	1	7	3	11	0	0	4	14	5	1	10	0	9	16	0	0	0	0	5	0	37
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	0	0	1	0	0	1	1	0	0	0	0	1	1	0	3	0	1	4	0	0	0	0	0	0	6
12:15 PM	0	0	2	0	0	2	0	1	0	0	0	1	2	0	1	0	0	3	0	0	0	0	0	0	6
12:30 PM	0	1	1	0	0	2	0	3	0	0	1	3	1	0	1	0	1	2	0	0	0	0	0	0	7
12:45 PM	0	0	3	0	1	3	0	1	0	0	0	1	1	0	0	0	0	1	0	0	0	0	1	0	5
Hourly Total	0	1	7	0	1	8	1	5	0	0	1	6	5	0	5	0	2	10	0	0	0	0	1	0	24
1:00 PM	0	2	2	0	0	4	2	0	0	0	0	2	0	0	2	0	0	2	0	0	0	0	1	0	8
1:15 PM	0	0	1	0	0	1	3	1	0	0	0	4	0	0	1	0	0	1	0	0	0	0	1	0	6
1:30 PM	0	1	2	0	0	3	0	0	0	0	0	0	1	0	1	0	0	2	0	0	0	0	2	0	5
1:45 PM	0	2	3	0	0	5	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	2	0	6
Hourly Total	0	5	8	0	0	13	5	1	0	0	0	6	1	0	5	0	0	6	0	0	0	0	6	0	25
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	0	1	2	0	0	3	1	0	0	0	0	1	0	0	0	0	1	0	0	0	0	0	0	0	4
3:15 PM	0	2	2	0	0	4	2	3	0	0	0	5	1	0	1	0	2	2	0	0	0	0	2	0	11
3:30 PM	0	4	3	0	0	7	1	4	0	0	0	5	1	0	1	0	9	2	0	0	0	0	0	0	14
3:45 PM	0	6	4	1	0	11	2	2	0	0	1	4	3	0	2	0	0	5	0	0	0	0	2	0	20
Hourly Total	0	13	11	1	0	25	6	9	0	0	1	15	5	0	4	0	12	9	0	0	0	0	4	0	49
4:00 PM	0	1	0	0	0	1	2	1	0	0	0	3	3	0	1	0	0	4	0	0	0	0	1	0	8
4:15 PM	0	1	0	0	0	1	0	2	0	0	0	2	1	0	1	0	1	2	0	0	0	0	0	0	5
4:30 PM	0	4	0	0	2	4	1	3	0	0	0	4	0	0	2	0	2	2	0	0	0	0	0	0	10

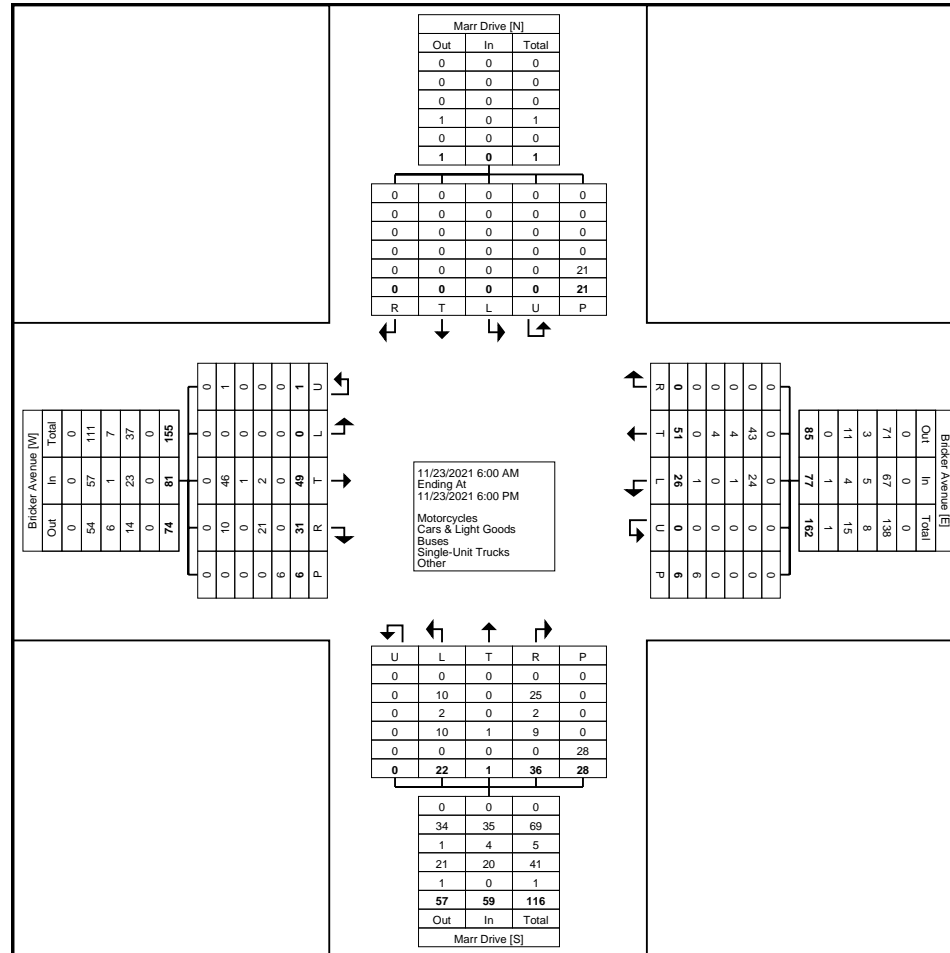
4:45 PM	0	3	1	0	0	4	1	2	0	0	0	3	0	0	0	0	0	0	0	0	0	0	7		
Hourly Total	0	9	1	0	2	10	4	8	0	0	0	12	4	0	4	0	3	8	0	0	0	1	0	30	
5:00 PM	0	7	0	0	0	7	1	1	0	0	0	2	0	0	2	0	1	2	0	0	0	0	1	0	11
5:15 PM	0	1	0	0	0	1	0	2	0	0	0	2	0	0	2	0	0	2	0	0	0	0	0	5	
5:30 PM	0	3	0	0	0	3	0	2	0	0	0	2	0	0	1	0	0	1	0	0	0	0	0	6	
5:45 PM	0	2	0	0	0	2	1	0	0	0	0	1	0	0	1	0	0	1	0	0	0	0	0	4	
Hourly Total	0	13	0	0	0	13	2	5	0	0	0	7	0	0	6	0	1	6	0	0	0	0	1	0	26
Grand Total	0	49	31	1	6	81	26	51	0	0	6	77	22	1	36	0	28	59	0	0	0	0	21	0	217
Approach %	0.0	60.5	38.3	1.2	-	-	33.8	66.2	0.0	0.0	-	-	37.3	1.7	61.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	22.6	14.3	0.5	-	37.3	12.0	23.5	0.0	0.0	-	35.5	10.1	0.5	16.6	0.0	-	27.2	0.0	0.0	0.0	0.0	-	0.0	-
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Cars & Light Goods	0	46	10	1	-	57	24	43	0	0	-	67	10	0	25	0	-	35	0	0	0	0	-	0	159
% Cars & Light Goods	-	93.9	32.3	100.0	-	70.4	92.3	84.3	-	-	-	87.0	45.5	0.0	69.4	-	-	59.3	-	-	-	-	-	-	73.3
Buses	0	1	0	0	-	1	1	4	0	0	-	5	2	0	2	0	-	4	0	0	0	0	-	0	10
% Buses	-	2.0	0.0	0.0	-	1.2	3.8	7.8	-	-	-	6.5	9.1	0.0	5.6	-	-	6.8	-	-	-	-	-	-	4.6
Single-Unit Trucks	0	2	21	0	-	23	0	4	0	0	-	4	10	1	9	0	-	20	0	0	0	0	-	0	47
% Single-Unit Trucks	-	4.1	67.7	0.0	-	28.4	0.0	7.8	-	-	-	5.2	45.5	100.0	25.0	-	-	33.9	-	-	-	-	-	-	21.7
Articulated Trucks	0	0	0	0	-	0	1	0	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	3.8	0.0	-	-	-	1.3	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.5
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	6	-	-	-	-	6	-	-	-	-	-	28	-	-	-	-	-	-	21	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Bricker Avenue & Marr Drive
Site Code: 210662
Start Date: 11/23/2021
Page No: 3



Turning Movement Data Plot



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Bricker Avenue & Marr Drive
Site Code: 210662
Start Date: 11/23/2021
Page No: 4

Turning Movement Peak Hour Data (8:00 AM)

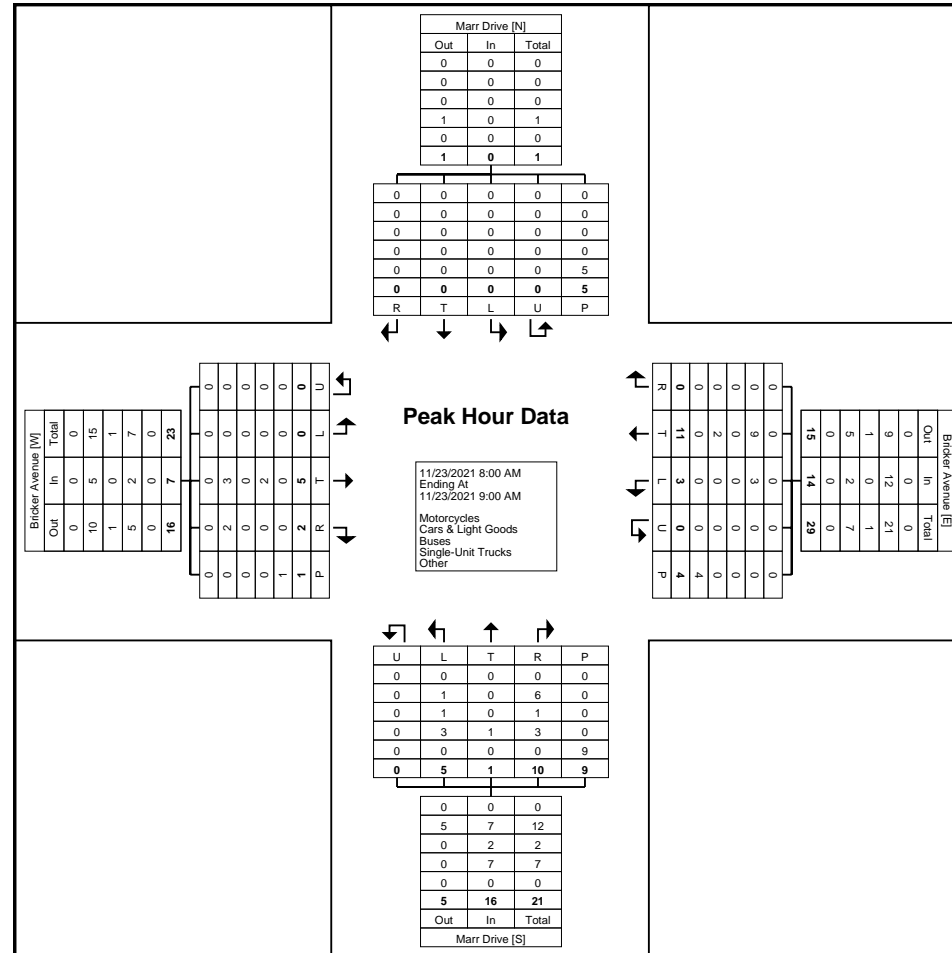
Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Marr Drive Northbound						Marr Drive Southbound						Int. Total						
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total							
8:00 AM	0	1	0	0	0	1	1	3	0	0	0	4	3	0	3	0	1	6	0	0	0	0	0	0	0	0	0	0	0	0	11
8:15 AM	0	3	1	0	0	4	0	3	0	0	1	3	2	0	2	0	0	4	0	0	0	0	4	0	0	0	0	0	4	0	11
8:30 AM	0	0	0	0	0	0	1	1	0	0	3	2	0	0	3	0	7	3	0	0	0	0	1	0	0	0	0	0	1	0	5
8:45 AM	0	1	1	0	1	2	1	4	0	0	0	5	0	1	2	0	1	3	0	0	0	0	0	0	0	0	0	0	0	0	10
Total	0	5	2	0	1	7	3	11	0	0	4	14	5	1	10	0	9	16	0	0	0	0	5	0	0	0	0	0	5	0	37
Approach %	0.0	71.4	28.6	0.0	-	-	21.4	78.6	0.0	0.0	-	-	31.3	6.3	62.5	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-	-	-	-	-	-	-
Total %	0.0	13.5	5.4	0.0	-	18.9	8.1	29.7	0.0	0.0	-	37.8	13.5	2.7	27.0	0.0	-	43.2	0.0	0.0	0.0	0.0	-	0.0	-	-	-	-	-	-	-
PHF	0.000	0.417	0.500	0.000	-	0.438	0.750	0.688	0.000	0.000	-	0.700	0.417	0.250	0.833	0.000	-	0.667	0.000	0.000	0.000	0.000	-	0.000	-	-	-	-	-	-	0.841
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	0.0
Cars & Light Goods	0	3	2	0	-	5	3	9	0	0	-	12	1	0	6	0	-	7	0	0	0	0	-	0	0	0	0	0	-	0	24
% Cars & Light Goods	-	60.0	100.0	-	-	71.4	100.0	81.8	-	-	-	85.7	20.0	0.0	60.0	-	-	43.8	-	-	-	-	-	-	-	-	-	-	-	-	64.9
Buses	0	0	0	0	-	0	0	0	0	0	-	0	1	0	1	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	2
% Buses	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	20.0	0.0	10.0	-	-	12.5	-	-	-	-	-	-	-	-	-	-	-	-	5.4
Single-Unit Trucks	0	2	0	0	-	2	0	2	0	0	-	2	3	1	3	0	-	7	0	0	0	0	-	0	0	0	0	0	-	0	11
% Single-Unit Trucks	-	40.0	0.0	-	-	28.6	0.0	18.2	-	-	-	14.3	60.0	100.0	30.0	-	-	43.8	-	-	-	-	-	-	-	-	-	-	-	-	29.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	4	-	-	-	-	-	9	-	-	-	-	-	5	-	-	-	-	-	5	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Bricker Avenue & Marr Drive
Site Code: 210662
Start Date: 11/23/2021
Page No: 5



Turning Movement Peak Hour Data Plot (8:00 AM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsI.com

Count Name: Bricker Avenue & Marr Drive
Site Code: 210662
Start Date: 11/23/2021
Page No: 6

Turning Movement Peak Hour Data (12:15 PM)

Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Marr Drive Northbound						Marr Drive Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:15 PM	0	0	2	0	0	2	0	1	0	0	0	1	2	0	1	0	0	3	0	0	0	0	0	0	6
12:30 PM	0	1	1	0	0	2	0	3	0	0	1	3	1	0	1	0	1	2	0	0	0	0	0	0	7
12:45 PM	0	0	3	0	1	3	0	1	0	0	0	1	1	0	0	0	0	1	0	0	0	0	1	0	5
1:00 PM	0	2	2	0	0	4	2	0	0	0	0	2	0	0	2	0	0	2	0	0	0	0	1	0	8
Total	0	3	8	0	1	11	2	5	0	0	1	7	4	0	4	0	1	8	0	0	0	0	2	0	26
Approach %	0.0	27.3	72.7	0.0	-	-	28.6	71.4	0.0	0.0	-	-	50.0	0.0	50.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	11.5	30.8	0.0	-	42.3	7.7	19.2	0.0	0.0	-	26.9	15.4	0.0	15.4	0.0	-	30.8	0.0	0.0	0.0	0.0	-	0.0	-
PHF	0.000	0.375	0.667	0.000	-	0.688	0.250	0.417	0.000	0.000	-	0.583	0.500	0.000	0.500	0.000	-	0.667	0.000	0.000	0.000	0.000	-	0.000	0.813
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Cars & Light Goods	0	3	0	0	-	3	2	4	0	0	-	6	2	0	2	0	-	4	0	0	0	0	-	0	13
% Cars & Light Goods	-	100.0	0.0	-	-	27.3	100.0	80.0	-	-	-	85.7	50.0	-	50.0	-	-	50.0	-	-	-	-	-	-	50.0
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Single-Unit Trucks	0	0	8	0	-	8	0	1	0	0	-	1	2	0	2	0	-	4	0	0	0	0	-	0	13
% Single-Unit Trucks	-	0.0	100.0	-	-	72.7	0.0	20.0	-	-	-	14.3	50.0	-	50.0	-	-	50.0	-	-	-	-	-	-	50.0
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	-	-	0	-	-	-	-	0	-	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	0.0	-	-	-	-	-	-	0.0	-	-	-	-	0.0	-	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	1	-	-	-	-	-	-	1	-	-	-	-	2	-	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Bricker Avenue & Marr Drive
Site Code: 210662
Start Date: 11/23/2021
Page No: 8

Turning Movement Peak Hour Data (3:15 PM)

Start Time	Bricker Avenue Eastbound						Bricker Avenue Westbound						Marr Drive Northbound						Marr Drive Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
3:15 PM	0	2	2	0	0	4	2	3	0	0	0	5	1	0	1	0	2	2	0	0	0	0	2	0	11
3:30 PM	0	4	3	0	0	7	1	4	0	0	0	5	1	0	1	0	9	2	0	0	0	0	0	0	14
3:45 PM	0	6	4	1	0	11	2	2	0	0	1	4	3	0	2	0	0	5	0	0	0	0	2	0	20
4:00 PM	0	1	0	0	0	1	2	1	0	0	0	3	3	0	1	0	0	4	0	0	0	0	1	0	8
Total	0	13	9	1	0	23	7	10	0	0	1	17	8	0	5	0	11	13	0	0	0	0	5	0	53
Approach %	0.0	56.5	39.1	4.3	-	-	41.2	58.8	0.0	0.0	-	-	61.5	0.0	38.5	0.0	-	-	0.0	0.0	0.0	0.0	-	-	-
Total %	0.0	24.5	17.0	1.9	-	43.4	13.2	18.9	0.0	0.0	-	32.1	15.1	0.0	9.4	0.0	-	24.5	0.0	0.0	0.0	0.0	-	0.0	-
PHF	0.000	0.542	0.563	0.250	-	0.523	0.875	0.625	0.000	0.000	-	0.850	0.667	0.000	0.625	0.000	-	0.650	0.000	0.000	0.000	0.000	-	0.000	0.663
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Cars & Light Goods	0	12	4	1	-	17	6	7	0	0	-	13	2	0	3	0	-	5	0	0	0	0	-	0	35
% Cars & Light Goods	-	92.3	44.4	100.0	-	73.9	85.7	70.0	-	-	-	76.5	25.0	-	60.0	-	-	38.5	-	-	-	-	-	-	66.0
Buses	0	1	0	0	-	1	1	2	0	0	-	3	1	0	1	0	-	2	0	0	0	0	-	0	6
% Buses	-	7.7	0.0	0.0	-	4.3	14.3	20.0	-	-	-	17.6	12.5	-	20.0	-	-	15.4	-	-	-	-	-	-	11.3
Single-Unit Trucks	0	0	5	0	-	5	0	1	0	0	-	1	5	0	1	0	-	6	0	0	0	0	-	0	12
% Single-Unit Trucks	-	0.0	55.6	0.0	-	21.7	0.0	10.0	-	-	-	5.9	62.5	-	20.0	-	-	46.2	-	-	-	-	-	-	22.6
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	-	-	-	-	-	-	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	11	-	-	-	-	-	5	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Geddes Street & James Street
Site Code: 210662
Start Date: 02/08/2022
Page No: 1

Turning Movement Data

Start Time	James Street Westbound					Geddes Street Northbound					Geddes Street Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	
7:00 AM	1	21	0	1	22	16	3	0	0	19	9	9	0	0	18	59
7:15 AM	1	31	0	0	32	18	4	0	0	22	14	12	0	0	26	80
7:30 AM	2	32	0	1	34	20	1	0	0	21	28	9	0	0	37	92
7:45 AM	7	31	0	0	38	11	6	0	0	17	23	13	0	0	36	91
Hourly Total	11	115	0	2	126	65	14	0	0	79	74	43	0	0	117	322
8:00 AM	5	28	0	0	33	18	6	0	0	24	23	13	0	0	36	93
8:15 AM	2	23	0	1	25	16	7	0	0	23	26	12	0	0	38	86
8:30 AM	5	20	0	0	25	16	6	0	0	22	25	24	0	0	49	96
8:45 AM	11	21	0	3	32	18	16	0	0	34	30	11	0	0	41	107
Hourly Total	23	92	0	4	115	68	35	0	0	103	104	60	0	0	164	382
9:00 AM	13	19	0	1	32	12	4	0	0	16	18	14	0	0	32	80
9:15 AM	5	17	0	0	22	15	1	0	0	16	2	9	0	0	11	49
9:30 AM	7	18	0	0	25	12	4	0	0	16	13	12	0	0	25	66
9:45 AM	3	7	0	2	10	11	2	0	0	13	19	9	0	0	28	51
Hourly Total	28	61	0	3	89	50	11	0	0	61	52	44	0	0	96	246
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	1	12	0	1	13	22	3	0	0	25	10	12	0	0	22	60
12:15 PM	5	14	0	0	19	7	3	0	0	10	18	13	0	2	31	60
12:30 PM	4	13	0	0	17	15	4	0	0	19	12	22	0	0	34	70
12:45 PM	4	17	0	0	21	17	7	0	0	24	17	9	0	0	26	71
Hourly Total	14	56	0	1	70	61	17	0	0	78	57	56	0	2	113	261
1:00 PM	11	13	0	0	24	7	8	0	0	15	8	12	0	0	20	59
1:15 PM	3	19	0	0	22	7	3	0	0	10	13	15	0	0	28	60
1:30 PM	7	17	0	0	24	12	6	0	0	18	20	21	0	0	41	83
1:45 PM	4	21	0	0	25	13	7	0	0	20	26	18	0	0	44	89
Hourly Total	25	70	0	0	95	39	24	0	0	63	67	66	0	0	133	291
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	4	23	0	0	27	9	12	0	0	21	23	18	0	0	41	89
3:15 PM	9	23	0	0	32	20	11	0	0	31	26	22	0	0	48	111
3:30 PM	14	24	0	6	38	16	5	0	0	21	19	20	0	0	39	98
3:45 PM	5	22	0	2	27	14	10	0	0	24	28	23	0	0	51	102
Hourly Total	32	92	0	8	124	59	38	0	0	97	96	83	0	0	179	400
4:00 PM	1	23	0	3	24	23	5	0	0	28	20	15	0	0	35	87
4:15 PM	9	21	0	2	30	17	3	0	0	20	35	20	0	0	55	105
4:30 PM	9	33	0	1	42	13	3	0	0	16	42	22	0	0	64	122
4:45 PM	12	32	0	1	44	17	7	0	0	24	39	30	0	0	69	137

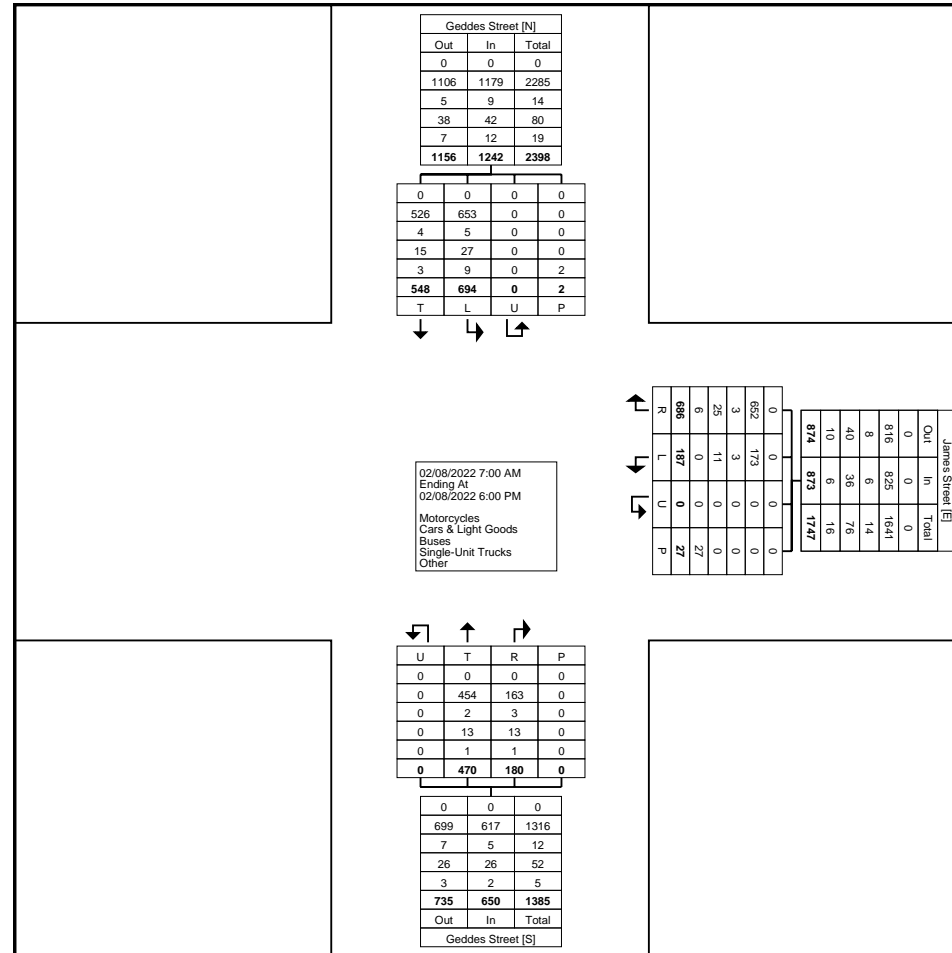
Hourly Total	31	109	0	7	140	70	18	0	0	88	136	87	0	0	223	451
5:00 PM	6	32	0	1	38	26	6	0	0	32	24	32	0	0	56	126
5:15 PM	7	23	0	1	30	11	6	0	0	17	27	29	0	0	56	103
5:30 PM	4	20	0	0	24	11	8	0	0	19	27	28	0	0	55	98
5:45 PM	6	16	0	0	22	10	3	0	0	13	30	20	0	0	50	85
Hourly Total	23	91	0	2	114	58	23	0	0	81	108	109	0	0	217	412
Grand Total	187	686	0	27	873	470	180	0	0	650	694	548	0	2	1242	2765
Approach %	21.4	78.6	0.0	-	-	72.3	27.7	0.0	-	-	55.9	44.1	0.0	-	-	-
Total %	6.8	24.8	0.0	-	31.6	17.0	6.5	0.0	-	23.5	25.1	19.8	0.0	-	44.9	-
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	173	652	0	-	825	454	163	0	-	617	653	526	0	-	1179	2621
% Cars & Light Goods	92.5	95.0	-	-	94.5	96.6	90.6	-	-	94.9	94.1	96.0	-	-	94.9	94.8
Buses	3	3	0	-	6	2	3	0	-	5	5	4	0	-	9	20
% Buses	1.6	0.4	-	-	0.7	0.4	1.7	-	-	0.8	0.7	0.7	-	-	0.7	0.7
Single-Unit Trucks	11	25	0	-	36	13	13	0	-	26	27	15	0	-	42	104
% Single-Unit Trucks	5.9	3.6	-	-	4.1	2.8	7.2	-	-	4.0	3.9	2.7	-	-	3.4	3.8
Articulated Trucks	0	6	0	-	6	1	1	0	-	2	9	3	0	-	12	20
% Articulated Trucks	0.0	0.9	-	-	0.7	0.2	0.6	-	-	0.3	1.3	0.5	-	-	1.0	0.7
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	27	-	-	-	-	0	-	-	-	-	2	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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519-896-3163 cbowness@ptsI.com

Count Name: Geddes Street & James Street
Site Code: 210662
Start Date: 02/08/2022
Page No: 3



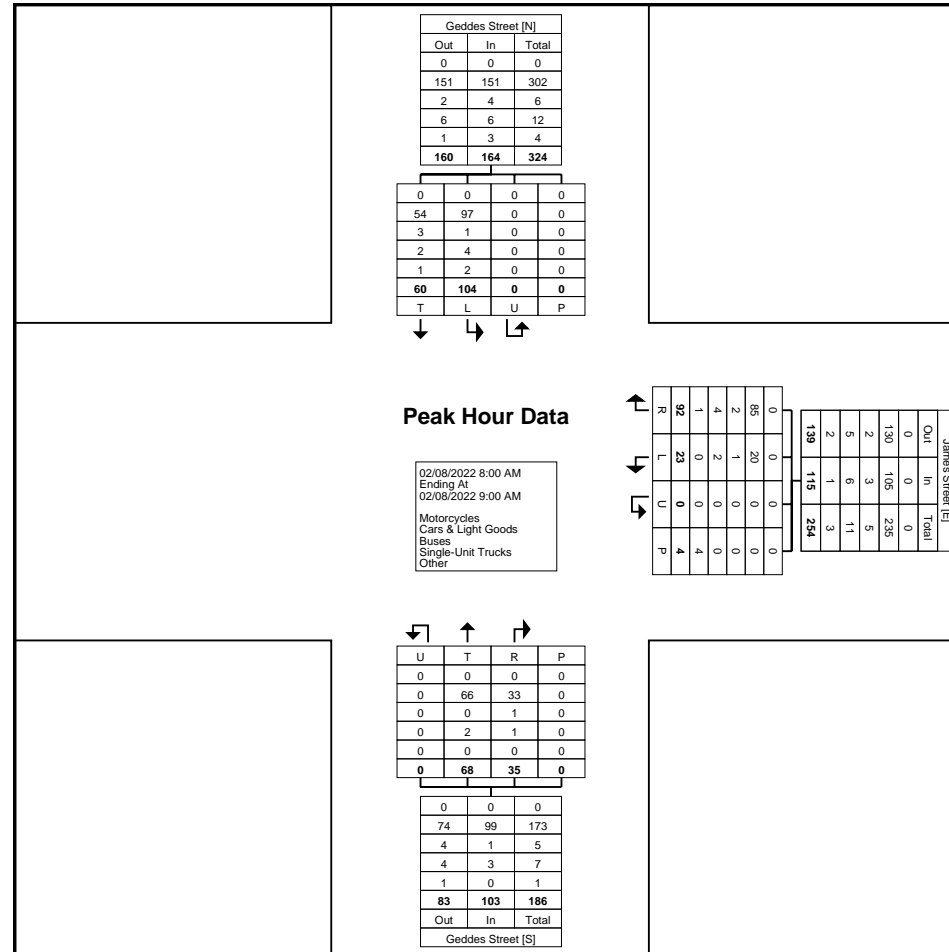
Turning Movement Data Plot



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Count Name: Geddes Street & James Street
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Page No: 5



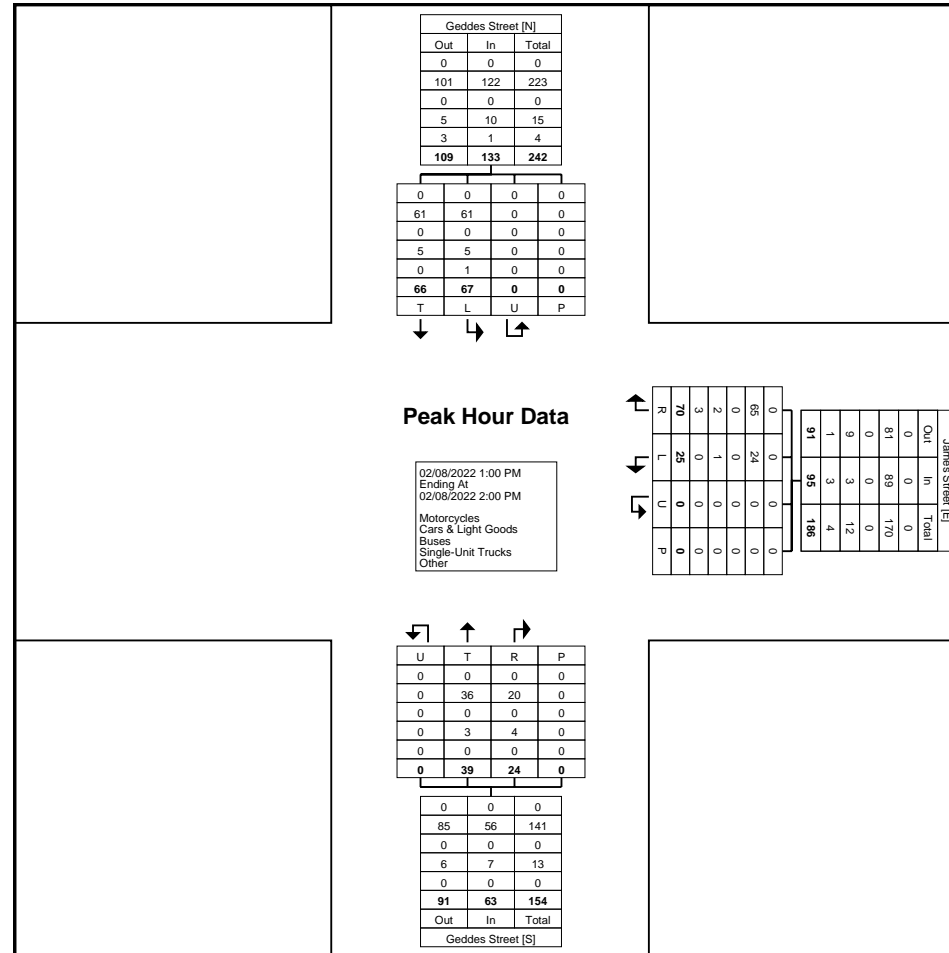
Turning Movement Peak Hour Data Plot (8:00 AM)



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Count Name: Geddes Street & James Street
Site Code: 210662
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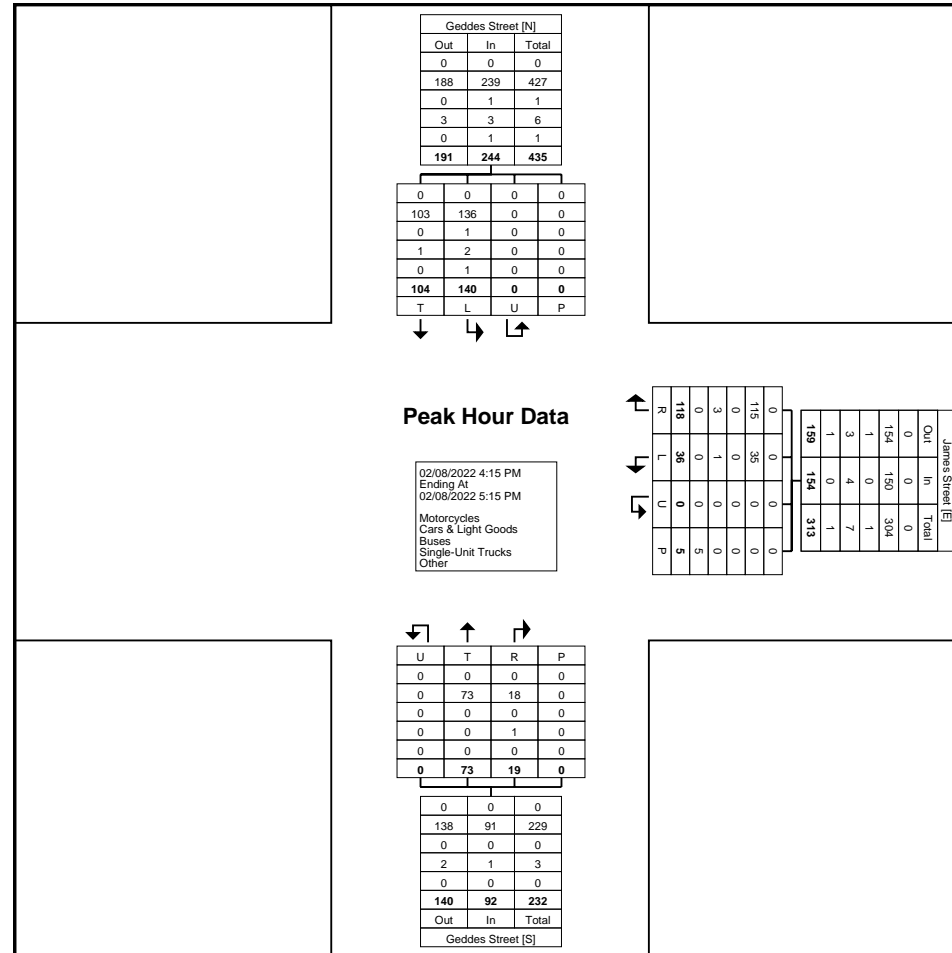
Turning Movement Peak Hour Data Plot (1:00 PM)



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Count Name: Geddes Street & James Street
Site Code: 210662
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Turning Movement Peak Hour Data Plot (4:15 PM)



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Count Name: Irvine Street & Bricker Avenue
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Data

Start Time	Bricker Avenue Eastbound					Irvine Street Northbound					Irvine Street Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0
6:15 AM	0	2	0	0	2	0	2	0	0	2	0	0	0	0	0	4
6:30 AM	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	2
6:45 AM	0	0	0	1	0	1	2	0	0	3	3	2	0	0	5	8
Hourly Total	0	2	0	1	2	1	6	0	0	7	3	2	0	1	5	14
7:00 AM	0	2	0	1	2	1	5	0	0	6	2	0	0	0	2	10
7:15 AM	1	2	0	0	3	0	6	0	0	6	3	1	0	0	4	13
7:30 AM	0	2	0	0	2	1	2	0	0	3	2	0	0	0	2	7
7:45 AM	2	1	0	0	3	0	8	0	0	8	8	2	0	0	10	21
Hourly Total	3	7	0	1	10	2	21	0	0	23	15	3	0	0	18	51
8:00 AM	5	3	0	1	8	1	3	0	0	4	2	1	0	0	3	15
8:15 AM	2	6	0	1	8	0	7	0	0	7	4	0	0	0	4	19
8:30 AM	5	4	0	0	9	1	9	0	0	10	7	1	0	0	8	27
8:45 AM	1	1	0	1	2	1	11	0	0	12	7	2	0	0	9	23
Hourly Total	13	14	0	3	27	3	30	0	0	33	20	4	0	0	24	84
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	2	0	0	0	2	1	2	0	0	3	3	0	0	0	3	8
12:15 PM	2	1	0	0	3	0	2	0	0	2	5	0	0	0	5	10
12:30 PM	1	0	0	0	1	4	9	1	0	14	3	0	0	0	3	18
12:45 PM	3	0	0	1	3	2	2	0	0	4	4	0	1	0	5	12
Hourly Total	8	1	0	1	9	7	15	1	0	23	15	0	1	0	16	48
1:00 PM	2	2	0	0	4	0	8	0	0	8	4	2	0	0	6	18
1:15 PM	0	1	0	0	1	2	1	0	0	3	2	3	0	0	5	9
1:30 PM	1	1	0	1	2	1	8	0	0	9	3	3	0	0	6	17
1:45 PM	1	1	0	0	2	0	3	0	0	3	0	0	0	0	0	5
Hourly Total	4	5	0	1	9	3	20	0	0	23	9	8	0	0	17	49
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	2	0	0	0	2	1	4	0	0	5	6	2	0	0	8	15
3:15 PM	2	3	0	6	5	3	10	0	0	13	4	4	0	0	8	26
3:30 PM	2	2	0	2	4	6	9	0	0	15	6	6	0	2	12	31
3:45 PM	5	2	0	0	7	0	5	0	0	5	6	4	0	0	10	22
Hourly Total	11	7	0	8	18	10	28	0	0	38	22	16	0	2	38	94
4:00 PM	0	1	0	0	1	0	12	0	0	12	4	4	0	0	8	21
4:15 PM	0	1	0	0	1	3	8	0	0	11	11	2	0	0	13	25
4:30 PM	0	4	0	0	4	4	9	0	0	13	4	3	0	0	7	24
4:45 PM	0	2	0	2	2	2	3	0	0	5	8	2	0	0	10	17

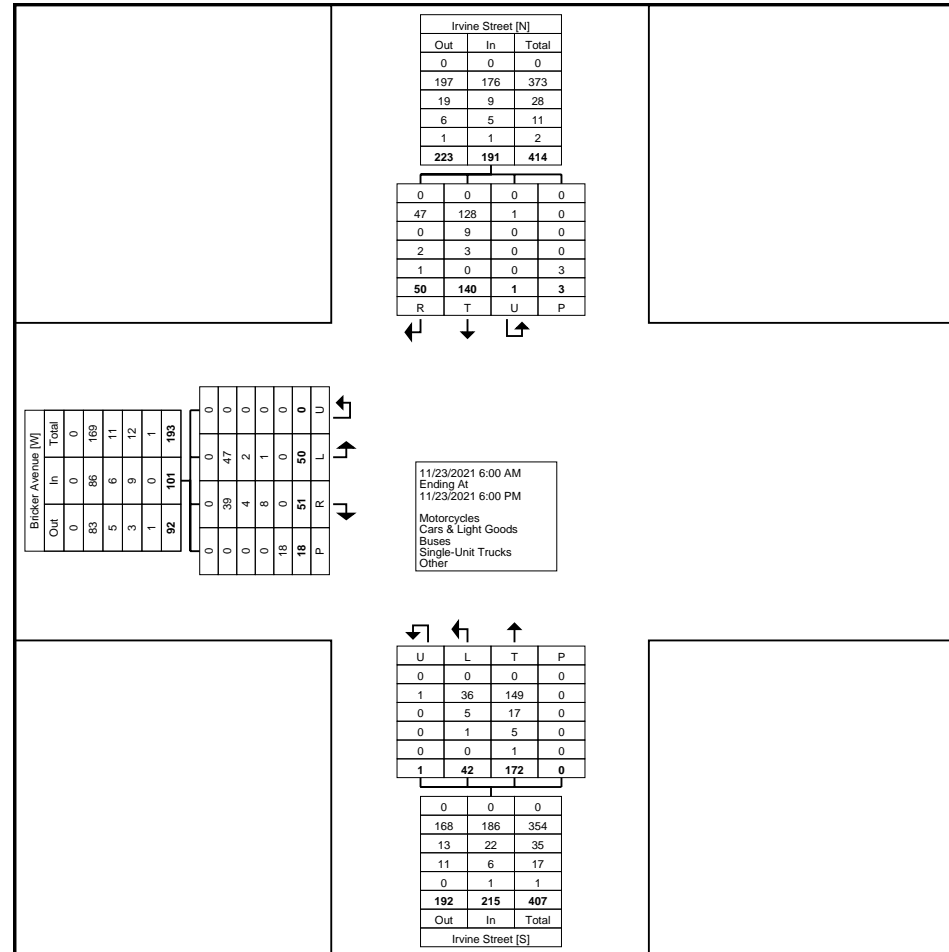
Hourly Total	0	8	0	2	8	9	32	0	0	41	27	11	0	0	38	87
5:00 PM	8	3	0	0	11	2	4	0	0	6	11	2	0	0	13	30
5:15 PM	0	1	0	1	1	0	5	0	0	5	11	2	0	0	13	19
5:30 PM	2	1	0	0	3	4	4	0	0	8	5	1	0	0	6	17
5:45 PM	1	2	0	0	3	1	7	0	0	8	2	1	0	0	3	14
Hourly Total	11	7	0	1	18	7	20	0	0	27	29	6	0	0	35	80
Grand Total	50	51	0	18	101	42	172	1	0	215	140	50	1	3	191	507
Approach %	49.5	50.5	0.0	-	-	19.5	80.0	0.5	-	-	73.3	26.2	0.5	-	-	-
Total %	9.9	10.1	0.0	-	19.9	8.3	33.9	0.2	-	42.4	27.6	9.9	0.2	-	37.7	-
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	0.0
Cars & Light Goods	47	39	0	-	86	36	149	1	-	186	128	47	1	-	176	448
% Cars & Light Goods	94.0	76.5	-	-	85.1	85.7	86.6	100.0	-	86.5	91.4	94.0	100.0	-	92.1	88.4
Buses	2	4	0	-	6	5	17	0	-	22	9	0	0	-	9	37
% Buses	4.0	7.8	-	-	5.9	11.9	9.9	0.0	-	10.2	6.4	0.0	0.0	-	4.7	7.3
Single-Unit Trucks	1	8	0	-	9	1	5	0	-	6	3	2	0	-	5	20
% Single-Unit Trucks	2.0	15.7	-	-	8.9	2.4	2.9	0.0	-	2.8	2.1	4.0	0.0	-	2.6	3.9
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	1	0	-	1	1
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0	2.0	0.0	-	0.5	0.2
Bicycles on Road	0	0	0	-	0	0	1	0	-	1	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.6	0.0	-	0.5	0.0	0.0	0.0	-	0.0	0.2
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	18	-	-	-	-	0	-	-	-	-	3	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Bricker Avenue
Site Code: 210662
Start Date: 11/23/2021
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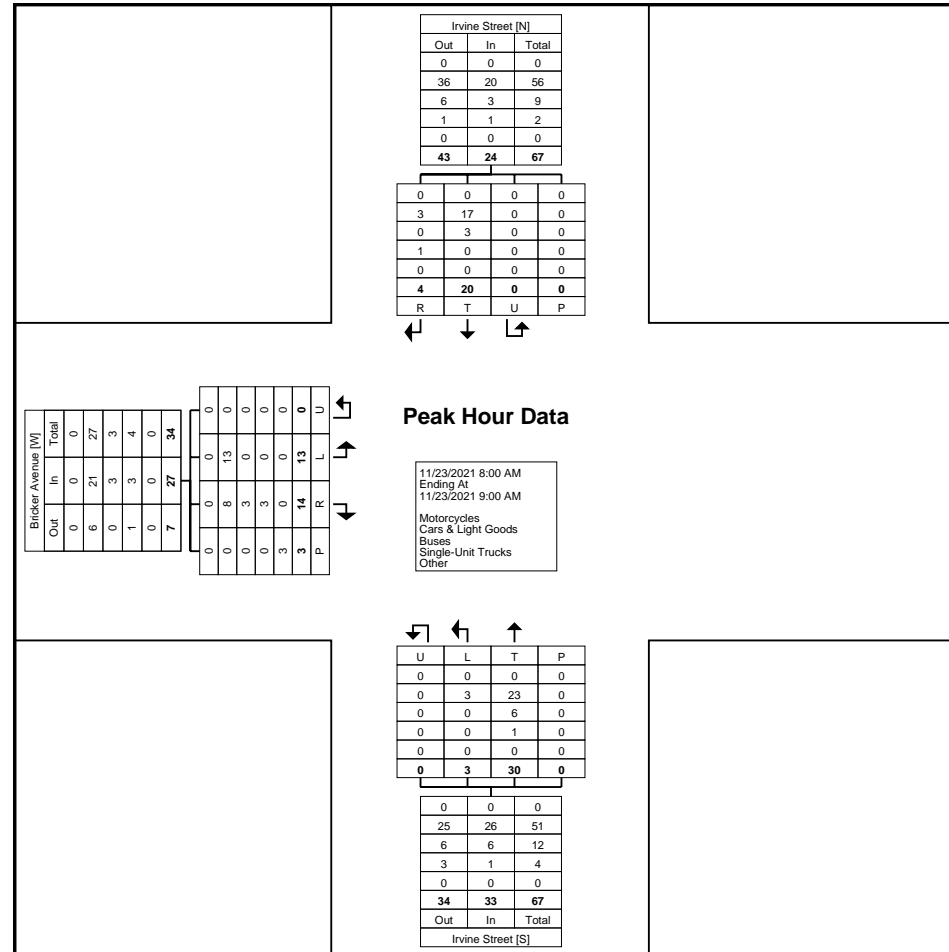
Turning Movement Data Plot



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Count Name: Irvine Street & Bricker Avenue
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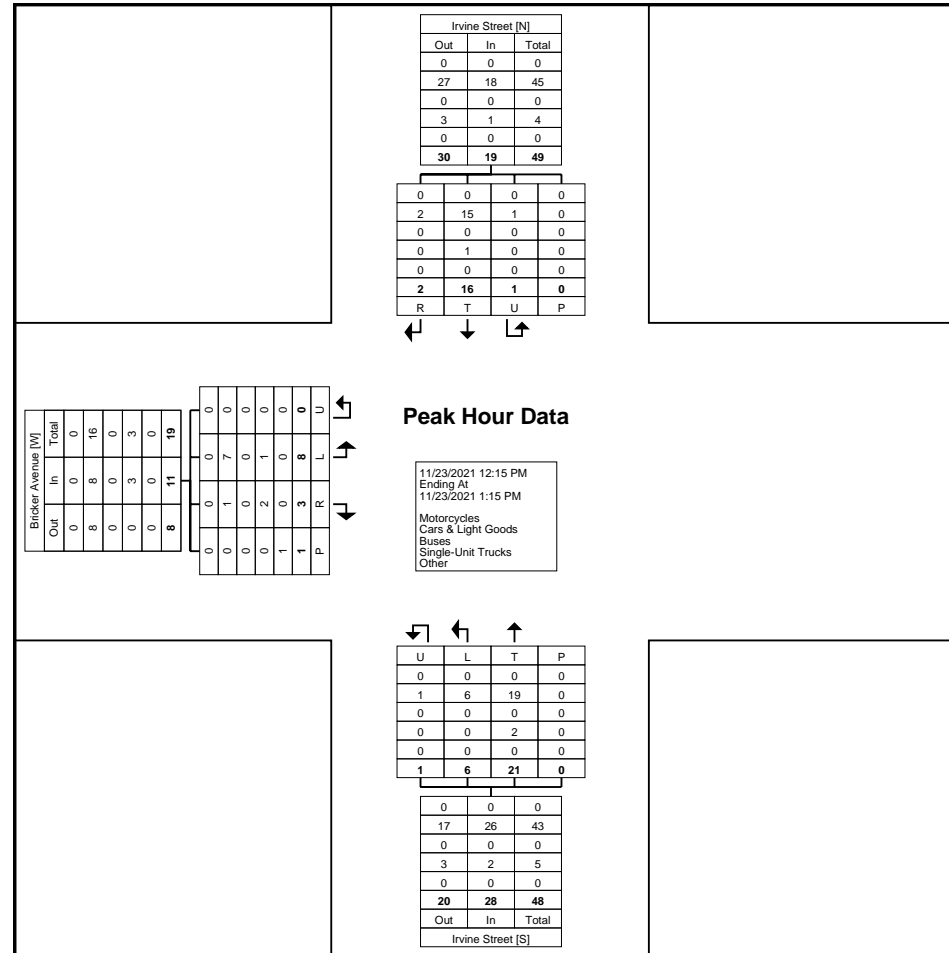
Turning Movement Peak Hour Data Plot (8:00 AM)



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Count Name: Irvine Street & Bricker Avenue
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Turning Movement Peak Hour Data Plot (12:15 PM)



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Count Name: Irvine Street & Bricker Avenue
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data (3:15 PM)

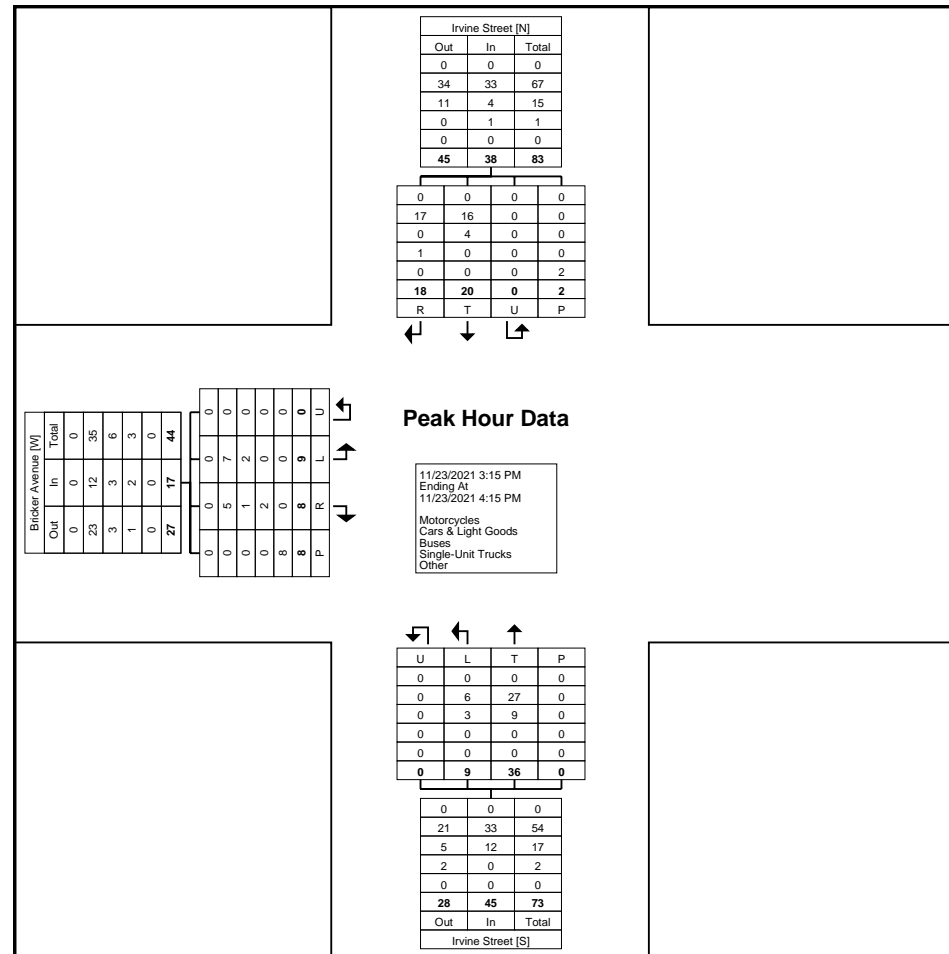
Start Time	Bricker Avenue Eastbound					Irvine Street Northbound					Irvine Street Southbound					Int. Total
	Left	Right	U-Turn	Peds	App. Total	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	
3:15 PM	2	3	0	6	5	3	10	0	0	13	4	4	0	0	8	26
3:30 PM	2	2	0	2	4	6	9	0	0	15	6	6	0	2	12	31
3:45 PM	5	2	0	0	7	0	5	0	0	5	6	4	0	0	10	22
4:00 PM	0	1	0	0	1	0	12	0	0	12	4	4	0	0	8	21
Total	9	8	0	8	17	9	36	0	0	45	20	18	0	2	38	100
Approach %	52.9	47.1	0.0	-	-	20.0	80.0	0.0	-	-	52.6	47.4	0.0	-	-	-
Total %	9.0	8.0	0.0	-	17.0	9.0	36.0	0.0	-	45.0	20.0	18.0	0.0	-	38.0	-
PHF	0.450	0.667	0.000	-	0.607	0.375	0.750	0.000	-	0.750	0.833	0.750	0.000	-	0.792	0.806
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	7	5	0	-	12	6	27	0	-	33	16	17	0	-	33	78
% Cars & Light Goods	77.8	62.5	-	-	70.6	66.7	75.0	-	-	73.3	80.0	94.4	-	-	86.8	78.0
Buses	2	1	0	-	3	3	9	0	-	12	4	0	0	-	4	19
% Buses	22.2	12.5	-	-	17.6	33.3	25.0	-	-	26.7	20.0	0.0	-	-	10.5	19.0
Single-Unit Trucks	0	2	0	-	2	0	0	0	-	0	0	1	0	-	1	3
% Single-Unit Trucks	0.0	25.0	-	-	11.8	0.0	0.0	-	-	0.0	0.0	5.6	-	-	2.6	3.0
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	8	-	-	-	-	0	-	-	-	-	2	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Bricker Avenue
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data Plot (3:15 PM)



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Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
Page No: 1

Turning Movement Data

Start Time	Colborne Street Eastbound						Colborne Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	1	4	1	0	0	6	0	14	1	0	1	15	0	2	1	0	0	3	5	1	0	0	0	6	30
7:15 AM	1	5	0	0	0	6	0	17	0	0	0	17	0	3	0	0	0	3	3	2	1	0	0	6	32
7:30 AM	0	10	0	0	0	10	0	21	3	0	1	24	0	3	1	0	0	4	3	4	0	0	1	7	45
7:45 AM	0	6	1	0	0	7	0	19	2	0	0	21	0	5	0	0	0	5	6	6	1	0	0	13	46
Hourly Total	2	25	2	0	0	29	0	71	6	0	2	77	0	13	2	0	0	15	17	13	2	0	1	32	153
8:00 AM	2	8	0	0	0	10	0	22	3	0	1	25	1	1	0	0	0	2	4	5	2	0	3	11	48
8:15 AM	2	14	1	0	4	17	0	21	6	0	3	27	0	3	3	0	0	6	9	9	3	0	5	21	71
8:30 AM	1	12	1	0	0	14	2	12	8	0	10	22	1	6	1	0	0	8	10	21	1	0	2	32	76
8:45 AM	1	15	3	0	0	19	1	35	3	0	1	39	2	12	3	0	0	17	5	11	3	0	6	19	94
Hourly Total	6	49	5	0	4	60	3	90	20	0	15	113	4	22	7	0	0	33	28	46	9	0	16	83	289
9:00 AM	1	11	0	0	0	12	0	17	6	0	0	23	2	2	1	0	0	5	4	2	0	0	0	6	46
9:15 AM	1	19	0	0	0	20	0	20	2	0	0	22	0	4	0	0	0	4	4	5	1	0	2	10	56
9:30 AM	0	8	0	0	0	8	1	21	4	0	1	26	1	1	1	0	0	3	7	3	2	0	1	12	49
9:45 AM	0	14	2	0	0	16	2	14	0	0	0	16	0	1	0	0	0	1	6	6	3	0	1	15	48
Hourly Total	2	52	2	0	0	56	3	72	12	0	1	87	3	8	2	0	0	13	21	16	6	0	4	43	199
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	1	23	0	0	0	24	0	14	9	0	0	23	0	4	2	0	0	6	8	8	0	0	2	16	69
12:15 PM	1	14	2	0	0	17	1	22	7	0	0	30	0	1	1	0	0	2	8	3	1	0	3	12	61
12:30 PM	0	17	1	0	0	18	0	21	7	0	0	28	0	4	2	0	0	6	4	7	1	0	0	12	64
12:45 PM	2	17	0	0	0	19	0	20	5	0	0	25	0	4	0	0	0	4	9	7	5	0	1	21	69
Hourly Total	4	71	3	0	0	78	1	77	28	0	0	106	0	13	5	0	0	18	29	25	7	0	6	61	263
1:00 PM	1	17	1	0	0	19	1	18	6	0	0	25	0	6	1	0	0	7	4	3	2	0	0	9	60
1:15 PM	1	19	1	0	0	21	0	20	6	0	0	26	0	3	1	0	0	4	9	4	2	0	1	15	66
1:30 PM	1	14	1	0	0	16	0	15	3	0	1	18	0	2	3	0	0	5	9	2	1	0	2	12	51
1:45 PM	2	24	0	0	0	26	0	22	3	0	2	25	2	2	2	0	0	6	5	1	2	0	1	8	65
Hourly Total	5	74	3	0	0	82	1	75	18	0	3	94	2	13	7	0	0	22	27	10	7	0	4	44	242
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	0	32	3	0	0	35	1	28	7	0	7	36	0	3	1	0	0	4	6	12	3	0	2	21	96
3:15 PM	1	26	1	0	0	28	0	23	8	0	11	31	1	10	5	0	0	16	7	12	3	0	3	22	97
3:30 PM	4	18	1	0	0	23	2	19	5	0	4	26	0	11	4	0	1	15	6	5	3	0	5	14	78
3:45 PM	3	34	1	0	0	38	0	21	9	0	0	30	0	5	1	0	0	6	13	3	1	0	5	17	91
Hourly Total	8	110	6	0	0	124	3	91	29	0	22	123	1	29	11	0	1	41	32	32	10	0	15	74	362
4:00 PM	1	25	1	0	0	27	1	28	11	0	1	40	0	3	0	0	0	3	4	8	1	0	2	13	83
4:15 PM	0	23	2	0	0	25	0	29	9	0	1	38	1	6	1	0	0	8	10	3	1	0	2	14	85
4:30 PM	1	27	0	0	0	28	0	21	13	0	3	34	2	4	1	0	0	7	9	5	4	0	2	18	87

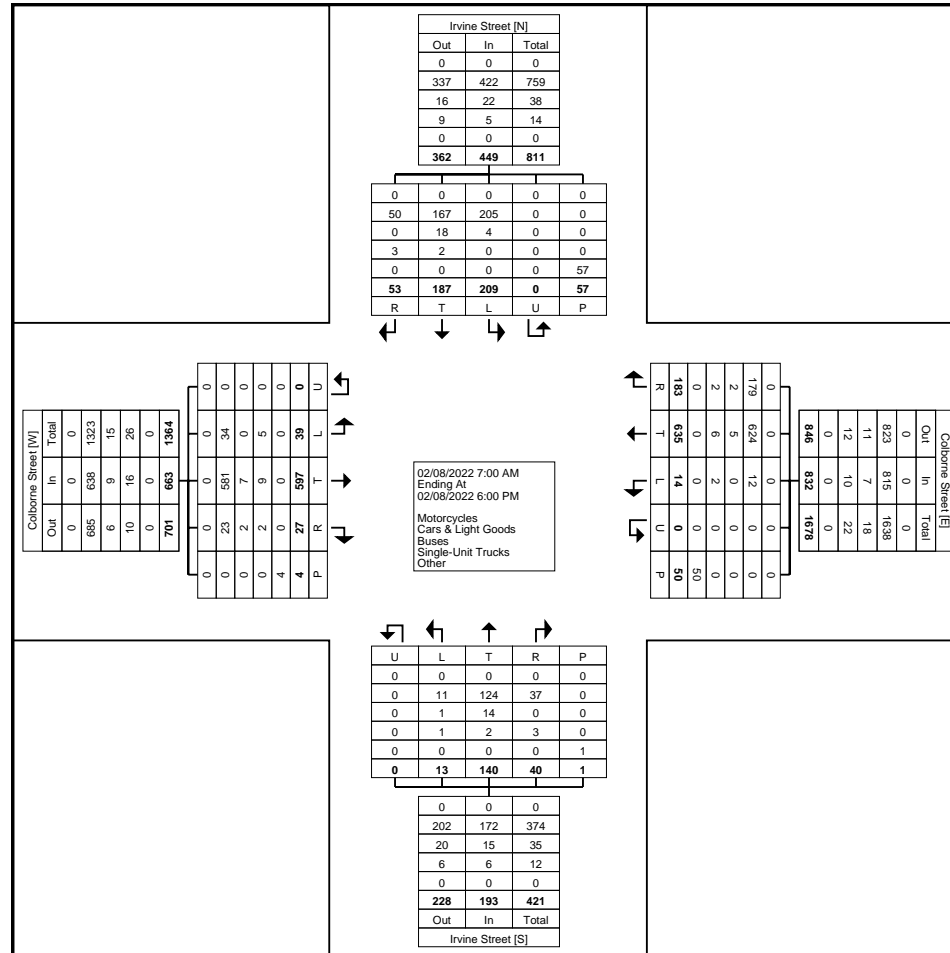
4:45 PM	4	31	0	0	0	35	2	16	9	0	0	27	0	9	1	0	0	10	9	8	3	0	0	20	92
Hourly Total	6	106	3	0	0	115	3	94	42	0	5	139	3	22	3	0	0	28	32	24	9	0	6	65	347
5:00 PM	2	31	0	0	0	33	0	21	10	0	2	31	0	4	2	0	0	6	5	7	0	0	1	12	82
5:15 PM	3	23	1	0	0	27	0	9	6	0	0	15	0	5	0	0	0	5	4	4	2	0	3	10	57
5:30 PM	1	28	1	0	0	30	0	18	8	0	0	26	0	5	0	0	0	5	7	5	0	0	0	12	73
5:45 PM	0	28	1	0	0	29	0	17	4	0	0	21	0	6	1	0	0	7	7	5	1	0	1	13	70
Hourly Total	6	110	3	0	0	119	0	65	28	0	2	93	0	20	3	0	0	23	23	21	3	0	5	47	282
Grand Total	39	597	27	0	4	663	14	635	183	0	50	832	13	140	40	0	1	193	209	187	53	0	57	449	2137
Approach %	5.9	90.0	4.1	0.0	-	-	1.7	76.3	22.0	0.0	-	-	6.7	72.5	20.7	0.0	-	-	46.5	41.6	11.8	0.0	-	-	-
Total %	1.8	27.9	1.3	0.0	-	31.0	0.7	29.7	8.6	0.0	-	38.9	0.6	6.6	1.9	0.0	-	9.0	9.8	8.8	2.5	0.0	-	21.0	-
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	34	581	23	0	-	638	12	624	179	0	-	815	11	124	37	0	-	172	205	167	50	0	-	422	2047
% Cars & Light Goods	87.2	97.3	85.2	-	-	96.2	85.7	98.3	97.8	-	-	98.0	84.6	88.6	92.5	-	-	89.1	98.1	89.3	94.3	-	-	94.0	95.8
Buses	0	7	2	0	-	9	0	5	2	0	-	7	1	14	0	0	-	15	4	18	0	0	-	22	53
% Buses	0.0	1.2	7.4	-	-	1.4	0.0	0.8	1.1	-	-	0.8	7.7	10.0	0.0	-	-	7.8	1.9	9.6	0.0	-	-	4.9	2.5
Single-Unit Trucks	5	9	2	0	-	16	2	6	2	0	-	10	1	2	3	0	-	6	0	2	3	0	-	5	37
% Single-Unit Trucks	12.8	1.5	7.4	-	-	2.4	14.3	0.9	1.1	-	-	1.2	7.7	1.4	7.5	-	-	3.1	0.0	1.1	5.7	-	-	1.1	1.7
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	4.0	-	-	-	-	-	0.0	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	4	-	-	-	-	48	-	-	-	-	-	1	-	-	-	-	-	-	57	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	96.0	-	-	-	-	-	100.0	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsll.com

Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
Page No: 4

Turning Movement Peak Hour Data (8:00 AM)

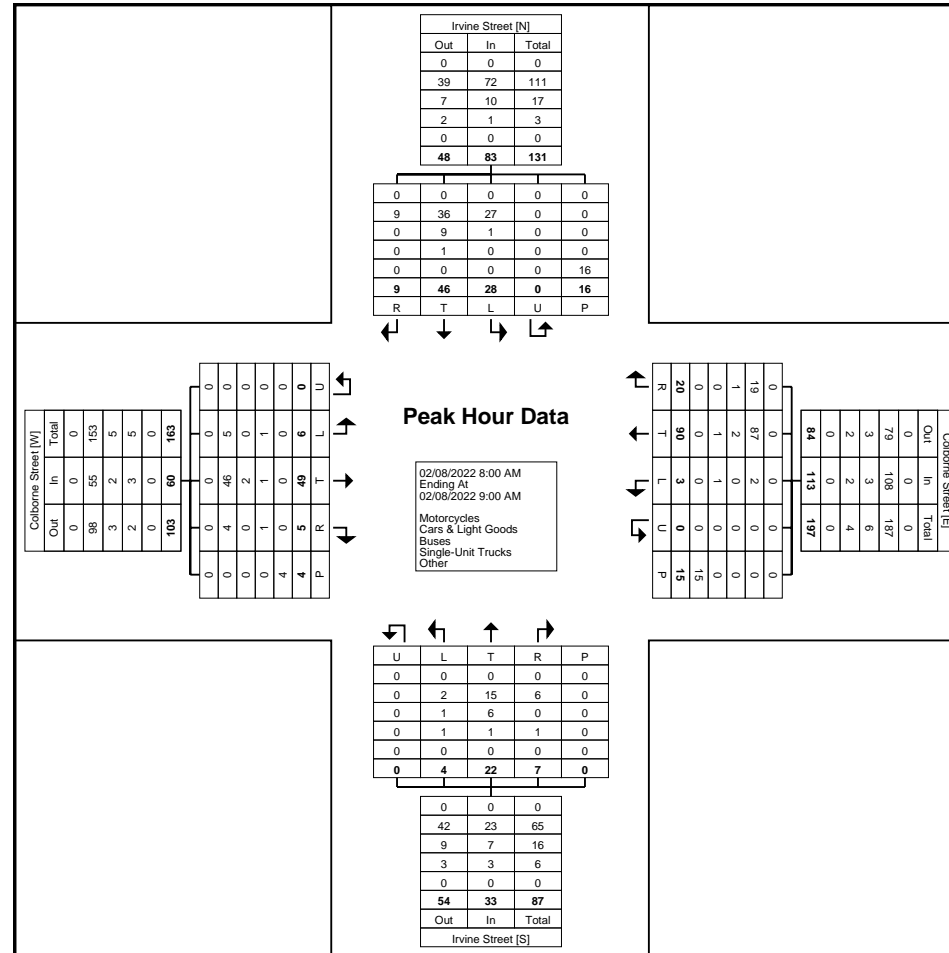
Start Time	Colborne Street Eastbound						Colborne Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
8:00 AM	2	8	0	0	0	10	0	22	3	0	1	25	1	1	0	0	0	2	4	5	2	0	3	11	48
8:15 AM	2	14	1	0	4	17	0	21	6	0	3	27	0	3	3	0	0	6	9	9	3	0	5	21	71
8:30 AM	1	12	1	0	0	14	2	12	8	0	10	22	1	6	1	0	0	8	10	21	1	0	2	32	76
8:45 AM	1	15	3	0	0	19	1	35	3	0	1	39	2	12	3	0	0	17	5	11	3	0	6	19	94
Total	6	49	5	0	4	60	3	90	20	0	15	113	4	22	7	0	0	33	28	46	9	0	16	83	289
Approach %	10.0	81.7	8.3	0.0	-	-	2.7	79.6	17.7	0.0	-	-	12.1	66.7	21.2	0.0	-	-	33.7	55.4	10.8	0.0	-	-	-
Total %	2.1	17.0	1.7	0.0	-	20.8	1.0	31.1	6.9	0.0	-	39.1	1.4	7.6	2.4	0.0	-	11.4	9.7	15.9	3.1	0.0	-	28.7	-
PHF	0.750	0.817	0.417	0.000	-	0.789	0.375	0.643	0.625	0.000	-	0.724	0.500	0.458	0.583	0.000	-	0.485	0.700	0.548	0.750	0.000	-	0.648	0.769
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	5	46	4	0	-	55	2	87	19	0	-	108	2	15	6	0	-	23	27	36	9	0	-	72	258
% Cars & Light Goods	83.3	93.9	80.0	-	-	91.7	66.7	96.7	95.0	-	-	95.6	50.0	68.2	85.7	-	-	69.7	96.4	78.3	100.0	-	-	86.7	89.3
Buses	0	2	0	0	-	2	0	2	1	0	-	3	1	6	0	0	-	7	1	9	0	0	-	10	22
% Buses	0.0	4.1	0.0	-	-	3.3	0.0	2.2	5.0	-	-	2.7	25.0	27.3	0.0	-	-	21.2	3.6	19.6	0.0	-	-	12.0	7.6
Single-Unit Trucks	1	1	1	0	-	3	1	1	0	0	-	2	1	1	1	0	-	3	0	1	0	0	-	1	9
% Single-Unit Trucks	16.7	2.0	20.0	-	-	5.0	33.3	1.1	0.0	-	-	1.8	25.0	4.5	14.3	-	-	9.1	0.0	2.2	0.0	-	-	1.2	3.1
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	4	-	-	-	-	-	15	-	-	-	-	-	0	-	-	-	-	-	16	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data Plot (8:00 AM)



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data (12:00 PM)

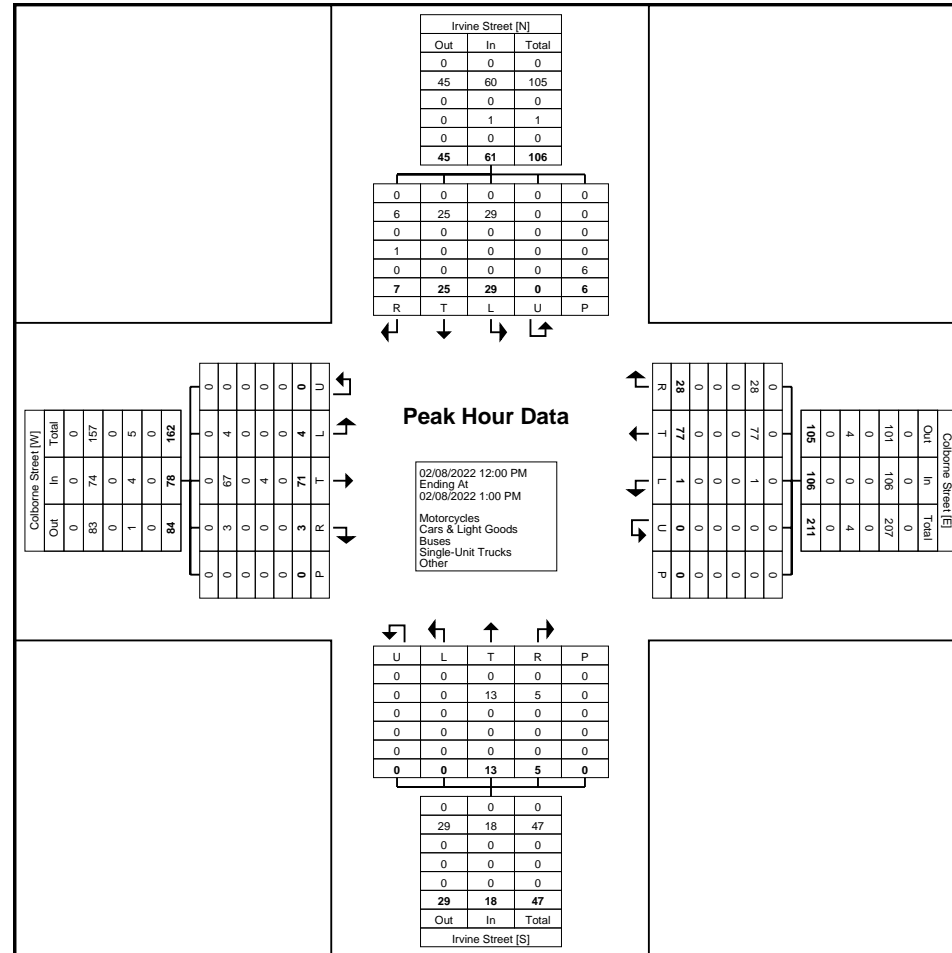
Start Time	Colborne Street Eastbound						Colborne Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:00 PM	1	23	0	0	0	24	0	14	9	0	0	23	0	4	2	0	0	6	8	8	0	0	2	16	69
12:15 PM	1	14	2	0	0	17	1	22	7	0	0	30	0	1	1	0	0	2	8	3	1	0	3	12	61
12:30 PM	0	17	1	0	0	18	0	21	7	0	0	28	0	4	2	0	0	6	4	7	1	0	0	12	64
12:45 PM	2	17	0	0	0	19	0	20	5	0	0	25	0	4	0	0	0	4	9	7	5	0	1	21	69
Total	4	71	3	0	0	78	1	77	28	0	0	106	0	13	5	0	0	18	29	25	7	0	6	61	263
Approach %	5.1	91.0	3.8	0.0	-	-	0.9	72.6	26.4	0.0	-	-	0.0	72.2	27.8	0.0	-	-	47.5	41.0	11.5	0.0	-	-	-
Total %	1.5	27.0	1.1	0.0	-	29.7	0.4	29.3	10.6	0.0	-	40.3	0.0	4.9	1.9	0.0	-	6.8	11.0	9.5	2.7	0.0	-	23.2	-
PHF	0.500	0.772	0.375	0.000	-	0.813	0.250	0.875	0.778	0.000	-	0.883	0.000	0.813	0.625	0.000	-	0.750	0.806	0.781	0.350	0.000	-	0.726	0.953
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	4	67	3	0	-	74	1	77	28	0	-	106	0	13	5	0	-	18	29	25	6	0	-	60	258
% Cars & Light Goods	100.0	94.4	100.0	-	-	94.9	100.0	100.0	100.0	-	-	100.0	-	100.0	100.0	-	-	100.0	100.0	100.0	85.7	-	-	98.4	98.1
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	4	0	0	-	4	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	5
% Single-Unit Trucks	0.0	5.6	0.0	-	-	5.1	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	14.3	-	-	1.6	1.9
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	-	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	6	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data Plot (12:00 PM)



Paradigm Transportation Solutions Limited
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Cambridge, Ontario, Canada N1R 8J8
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Count Name: Irvine Street & Colborne Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data (3:00 PM)

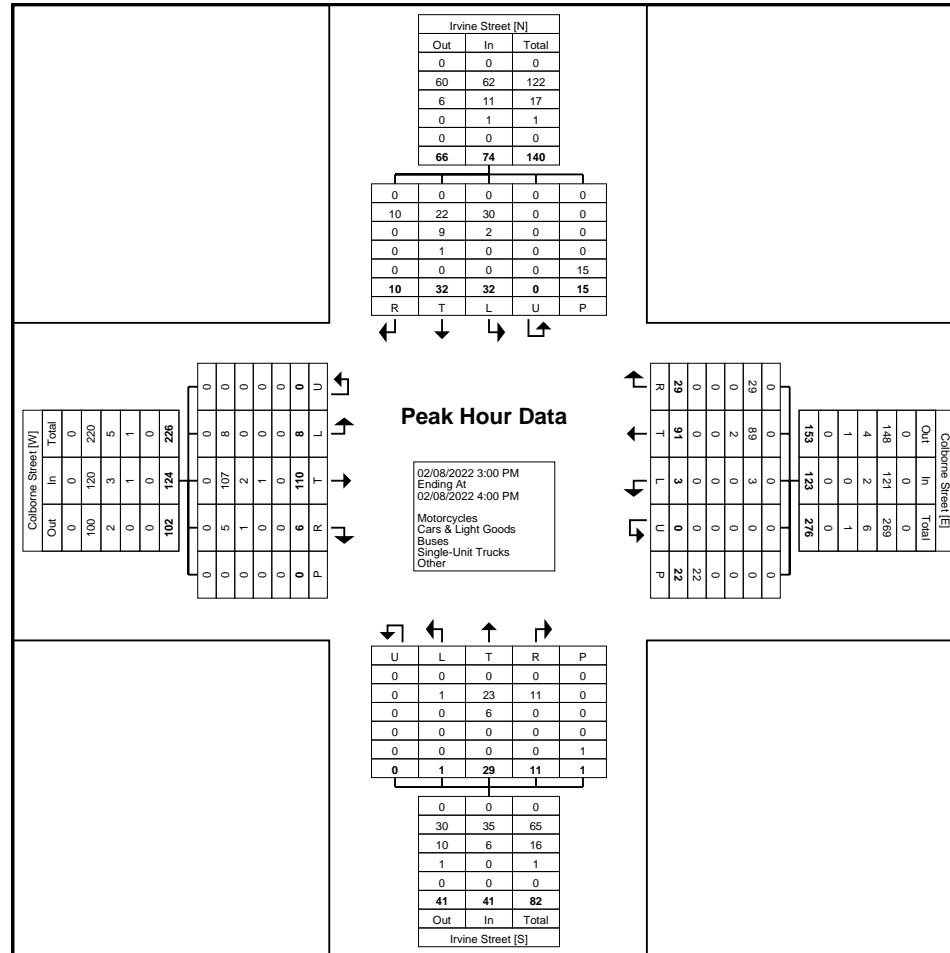
Start Time	Colborne Street Eastbound						Colborne Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
3:00 PM	0	32	3	0	0	35	1	28	7	0	7	36	0	3	1	0	0	4	6	12	3	0	2	21	96
3:15 PM	1	26	1	0	0	28	0	23	8	0	11	31	1	10	5	0	0	16	7	12	3	0	3	22	97
3:30 PM	4	18	1	0	0	23	2	19	5	0	4	26	0	11	4	0	1	15	6	5	3	0	5	14	78
3:45 PM	3	34	1	0	0	38	0	21	9	0	0	30	0	5	1	0	0	6	13	3	1	0	5	17	91
Total	8	110	6	0	0	124	3	91	29	0	22	123	1	29	11	0	1	41	32	32	10	0	15	74	362
Approach %	6.5	88.7	4.8	0.0	-	-	2.4	74.0	23.6	0.0	-	-	2.4	70.7	26.8	0.0	-	-	43.2	43.2	13.5	0.0	-	-	-
Total %	2.2	30.4	1.7	0.0	-	34.3	0.8	25.1	8.0	0.0	-	34.0	0.3	8.0	3.0	0.0	-	11.3	8.8	8.8	2.8	0.0	-	20.4	-
PHF	0.500	0.809	0.500	0.000	-	0.816	0.375	0.813	0.806	0.000	-	0.854	0.250	0.659	0.550	0.000	-	0.641	0.615	0.667	0.833	0.000	-	0.841	0.933
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	8	107	5	0	-	120	3	89	29	0	-	121	1	23	11	0	-	35	30	22	10	0	-	62	338
% Cars & Light Goods	100.0	97.3	83.3	-	-	96.8	100.0	97.8	100.0	-	-	98.4	100.0	79.3	100.0	-	-	85.4	93.8	68.8	100.0	-	-	83.8	93.4
Buses	0	2	1	0	-	3	0	2	0	0	-	2	0	6	0	0	-	6	2	9	0	0	-	11	22
% Buses	0.0	1.8	16.7	-	-	2.4	0.0	2.2	0.0	-	-	1.6	0.0	20.7	0.0	-	-	14.6	6.3	28.1	0.0	-	-	14.9	6.1
Single-Unit Trucks	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	1	0	0	-	1	2
% Single-Unit Trucks	0.0	0.9	0.0	-	-	0.8	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	3.1	0.0	-	-	1.4	0.6
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	2	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	9.1	-	-	-	-	-	0.0	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	20	-	-	-	-	-	1	-	-	-	-	-	15	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	90.9	-	-	-	-	-	100.0	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Colborne Street
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Turning Movement Peak Hour Data Plot (3:00 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsI.com

Count Name: Irvine Street & East Mill Street
Site Code: 210662
Start Date: 02/08/2022
Page No: 1

Turning Movement Data

Start Time	East Mill Street Eastbound						East Mill Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:00 AM	0	25	0	0	0	25	0	16	3	0	0	19	0	0	0	0	0	0	3	0	1	0	5	4	48
7:15 AM	0	16	0	0	0	16	0	23	3	0	0	26	0	0	0	0	0	0	2	0	0	0	0	2	44
7:30 AM	1	27	0	0	0	28	0	23	2	0	0	25	0	0	0	0	0	0	5	0	3	0	0	8	61
7:45 AM	1	32	0	0	0	33	0	18	2	0	0	20	0	0	0	0	0	0	5	0	2	0	1	7	60
Hourly Total	2	100	0	0	0	102	0	80	10	0	0	90	0	0	0	0	0	0	15	0	6	0	6	21	213
8:00 AM	0	41	0	0	0	41	0	26	2	0	0	28	0	0	0	0	0	0	7	0	3	0	0	10	79
8:15 AM	2	57	0	0	0	59	1	30	4	0	0	35	0	1	0	0	0	1	7	0	1	0	11	8	103
8:30 AM	0	72	1	0	0	73	0	33	7	0	0	40	1	1	0	0	0	2	22	0	3	0	23	25	140
8:45 AM	2	67	0	0	0	69	0	73	9	0	0	82	0	0	0	0	0	0	8	0	4	0	8	12	163
Hourly Total	4	237	1	0	0	242	1	162	22	0	0	185	1	2	0	0	0	3	44	0	11	0	42	55	485
9:00 AM	0	21	0	0	0	21	0	30	4	0	0	34	0	0	0	0	0	0	2	0	1	0	0	3	58
9:15 AM	0	32	0	0	0	32	0	43	4	0	0	47	0	0	0	0	0	0	5	0	0	0	4	5	84
9:30 AM	1	34	0	0	0	35	1	39	2	0	0	42	0	0	0	0	0	0	1	0	1	0	0	2	79
9:45 AM	0	38	0	0	0	38	0	39	2	0	0	41	0	0	0	0	0	0	6	0	4	0	1	10	89
Hourly Total	1	125	0	0	0	126	1	151	12	0	0	164	0	0	0	0	0	0	14	0	6	0	5	20	310
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	0	27	0	0	0	27	0	46	3	0	0	49	0	0	0	0	0	0	9	0	0	0	2	9	85
12:15 PM	0	29	0	0	0	29	0	42	0	0	0	42	0	0	0	0	0	0	6	0	2	0	2	8	79
12:30 PM	2	42	0	0	0	44	1	41	3	0	0	45	0	0	0	0	0	0	5	0	1	0	1	6	95
12:45 PM	1	54	0	0	0	55	0	34	5	0	0	39	0	0	0	0	0	0	7	0	0	0	0	7	101
Hourly Total	3	152	0	0	0	155	1	163	11	0	0	175	0	0	0	0	0	0	27	0	3	0	5	30	360
1:00 PM	0	29	0	0	0	29	0	42	4	0	0	46	0	0	0	0	0	0	3	0	1	0	0	4	79
1:15 PM	0	41	0	0	0	41	0	48	3	0	0	51	1	0	0	0	0	1	4	0	0	0	0	4	97
1:30 PM	1	43	1	0	0	45	0	41	3	0	0	44	0	0	0	0	0	0	3	0	1	0	2	4	93
1:45 PM	1	43	0	0	0	44	0	38	4	0	0	42	1	0	0	0	0	1	1	0	0	0	2	1	88
Hourly Total	2	156	1	0	0	159	0	169	14	0	0	183	2	0	0	0	0	2	11	0	2	0	4	13	357
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	1	66	0	0	0	67	0	41	2	0	0	43	0	0	0	0	0	0	13	0	3	0	2	16	126
3:15 PM	3	44	0	0	0	47	0	85	10	0	0	95	0	0	0	0	0	0	9	0	1	0	35	10	152
3:30 PM	9	38	0	0	0	47	0	51	8	0	0	59	0	0	0	0	0	0	4	0	5	0	16	9	115
3:45 PM	3	35	0	0	0	38	0	48	4	0	0	52	0	0	0	0	0	0	5	0	1	0	1	6	96
Hourly Total	16	183	0	0	0	199	0	225	24	0	0	249	0	0	0	0	0	0	31	0	10	0	54	41	489
4:00 PM	1	38	0	0	0	39	0	40	4	0	0	44	0	0	0	0	0	0	6	0	3	0	3	9	92
4:15 PM	3	55	0	0	0	58	0	42	5	0	0	47	0	0	1	0	0	1	2	0	4	0	1	6	112
4:30 PM	1	50	0	0	0	51	0	34	2	0	0	36	0	0	0	0	0	0	5	0	0	0	3	5	92

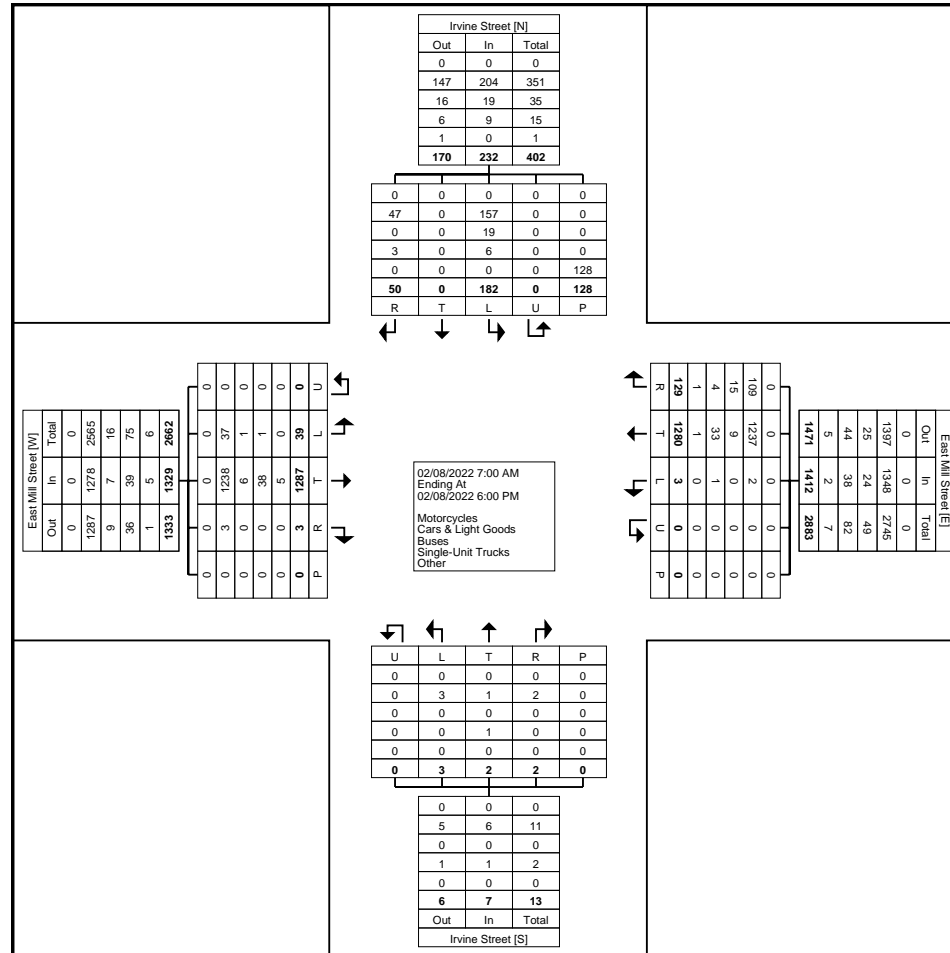
4:45 PM	3	54	0	0	0	57	0	52	5	0	0	57	0	0	0	0	0	6	0	1	0	0	7	121	
Hourly Total	8	197	0	0	0	205	0	168	16	0	0	184	0	0	1	0	0	1	19	0	8	0	7	27	417
5:00 PM	0	44	0	0	0	44	0	62	4	0	0	66	0	0	0	0	0	0	6	0	0	0	3	6	116
5:15 PM	1	38	0	0	0	39	0	46	6	0	0	52	0	0	0	0	0	0	3	0	4	0	1	7	98
5:30 PM	1	23	1	0	0	25	0	29	8	0	0	37	0	0	0	0	0	0	5	0	0	0	0	5	67
5:45 PM	1	32	0	0	0	33	0	25	2	0	0	27	0	0	1	0	0	1	7	0	0	0	1	7	68
Hourly Total	3	137	1	0	0	141	0	162	20	0	0	182	0	0	1	0	0	1	21	0	4	0	5	25	349
Grand Total	39	1287	3	0	0	1329	3	1280	129	0	0	1412	3	2	2	0	0	7	182	0	50	0	128	232	2980
Approach %	2.9	96.8	0.2	0.0	-	-	0.2	90.7	9.1	0.0	-	-	42.9	28.6	28.6	0.0	-	-	78.4	0.0	21.6	0.0	-	-	-
Total %	1.3	43.2	0.1	0.0	-	44.6	0.1	43.0	4.3	0.0	-	47.4	0.1	0.1	0.1	0.0	-	0.2	6.1	0.0	1.7	0.0	-	7.8	-
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Cars & Light Goods	37	1238	3	0	-	1278	2	1237	109	0	-	1348	3	1	2	0	-	6	157	0	47	0	-	204	2836
% Cars & Light Goods	94.9	96.2	100.0	-	-	96.2	66.7	96.6	84.5	-	-	95.5	100.0	50.0	100.0	-	-	85.7	86.3	-	94.0	-	-	87.9	95.2
Buses	1	6	0	0	-	7	0	9	15	0	-	24	0	0	0	0	-	0	19	0	0	0	-	19	50
% Buses	2.6	0.5	0.0	-	-	0.5	0.0	0.7	11.6	-	-	1.7	0.0	0.0	0.0	-	-	0.0	10.4	-	0.0	-	-	8.2	1.7
Single-Unit Trucks	1	38	0	0	-	39	1	33	4	0	-	38	0	1	0	0	-	1	6	0	3	0	-	9	87
% Single-Unit Trucks	2.6	3.0	0.0	-	-	2.9	33.3	2.6	3.1	-	-	2.7	0.0	50.0	0.0	-	-	14.3	3.3	-	6.0	-	-	3.9	2.9
Articulated Trucks	0	5	0	0	-	5	0	1	1	0	-	2	0	0	0	0	-	0	0	0	0	0	-	0	7
% Articulated Trucks	0.0	0.4	0.0	-	-	0.4	0.0	0.1	0.8	-	-	0.1	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	-	128	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Irvine Street & East Mill Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Irvine Street & East Mill Street
Site Code: 210662
Start Date: 02/08/2022
Page No: 4

Turning Movement Peak Hour Data (8:00 AM)

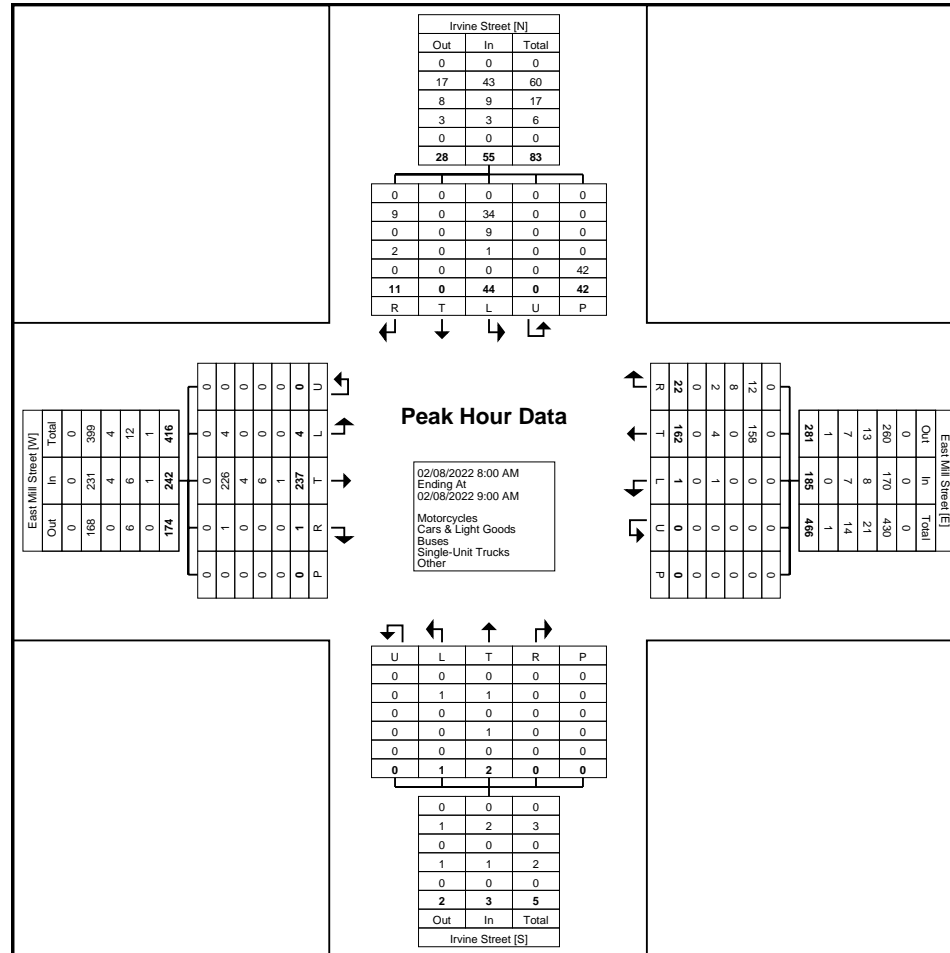
Start Time	East Mill Street Eastbound						East Mill Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
8:00 AM	0	41	0	0	0	41	0	26	2	0	0	28	0	0	0	0	0	0	7	0	3	0	0	10	79
8:15 AM	2	57	0	0	0	59	1	30	4	0	0	35	0	1	0	0	0	1	7	0	1	0	11	8	103
8:30 AM	0	72	1	0	0	73	0	33	7	0	0	40	1	1	0	0	0	2	22	0	3	0	23	25	140
8:45 AM	2	67	0	0	0	69	0	73	9	0	0	82	0	0	0	0	0	0	8	0	4	0	8	12	163
Total	4	237	1	0	0	242	1	162	22	0	0	185	1	2	0	0	0	3	44	0	11	0	42	55	485
Approach %	1.7	97.9	0.4	0.0	-	-	0.5	87.6	11.9	0.0	-	-	33.3	66.7	0.0	0.0	-	-	80.0	0.0	20.0	0.0	-	-	-
Total %	0.8	48.9	0.2	0.0	-	49.9	0.2	33.4	4.5	0.0	-	38.1	0.2	0.4	0.0	0.0	-	0.6	9.1	0.0	2.3	0.0	-	11.3	-
PHF	0.500	0.823	0.250	0.000	-	0.829	0.250	0.555	0.611	0.000	-	0.564	0.250	0.500	0.000	0.000	-	0.375	0.500	0.000	0.688	0.000	-	0.550	0.744
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Cars & Light Goods	4	226	1	0	-	231	0	158	12	0	-	170	1	1	0	0	-	2	34	0	9	0	-	43	446
% Cars & Light Goods	100.0	95.4	100.0	-	-	95.5	0.0	97.5	54.5	-	-	91.9	100.0	50.0	-	-	-	66.7	77.3	-	81.8	-	-	78.2	92.0
Buses	0	4	0	0	-	4	0	0	8	0	-	8	0	0	0	0	-	0	9	0	0	0	-	9	21
% Buses	0.0	1.7	0.0	-	-	1.7	0.0	0.0	36.4	-	-	4.3	0.0	0.0	-	-	-	0.0	20.5	-	0.0	-	-	16.4	4.3
Single-Unit Trucks	0	6	0	0	-	6	1	4	2	0	-	7	0	1	0	0	-	1	1	0	2	0	-	3	17
% Single-Unit Trucks	0.0	2.5	0.0	-	-	2.5	100.0	2.5	9.1	-	-	3.8	0.0	50.0	-	-	-	33.3	2.3	-	18.2	-	-	5.5	3.5
Articulated Trucks	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.4	0.0	-	-	0.4	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	42	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & East Mill Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data Plot (8:00 AM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Irvine Street & East Mill Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data (12:30 PM)

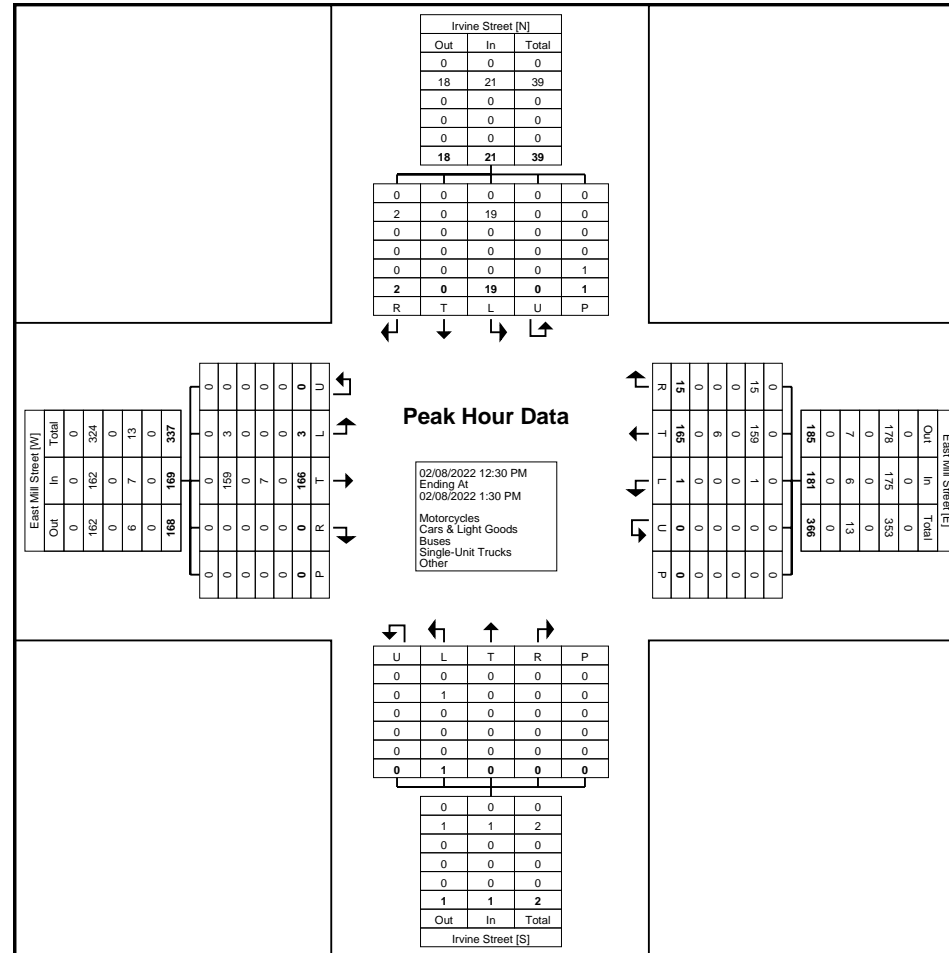
Start Time	East Mill Street Eastbound						East Mill Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:30 PM	2	42	0	0	0	44	1	41	3	0	0	45	0	0	0	0	0	0	5	0	1	0	1	6	95
12:45 PM	1	54	0	0	0	55	0	34	5	0	0	39	0	0	0	0	0	0	7	0	0	0	0	7	101
1:00 PM	0	29	0	0	0	29	0	42	4	0	0	46	0	0	0	0	0	0	3	0	1	0	0	4	79
1:15 PM	0	41	0	0	0	41	0	48	3	0	0	51	1	0	0	0	0	1	4	0	0	0	0	4	97
Total	3	166	0	0	0	169	1	165	15	0	0	181	1	0	0	0	0	1	19	0	2	0	1	21	372
Approach %	1.8	98.2	0.0	0.0	-	-	0.6	91.2	8.3	0.0	-	-	100.0	0.0	0.0	0.0	-	-	90.5	0.0	9.5	0.0	-	-	-
Total %	0.8	44.6	0.0	0.0	-	45.4	0.3	44.4	4.0	0.0	-	48.7	0.3	0.0	0.0	0.0	-	0.3	5.1	0.0	0.5	0.0	-	5.6	-
PHF	0.375	0.769	0.000	0.000	-	0.768	0.250	0.859	0.750	0.000	-	0.887	0.250	0.000	0.000	0.000	-	0.250	0.679	0.000	0.500	0.000	-	0.750	0.921
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Cars & Light Goods	3	159	0	0	-	162	1	159	15	0	-	175	1	0	0	0	-	1	19	0	2	0	-	21	359
% Cars & Light Goods	100.0	95.8	-	-	-	95.9	100.0	96.4	100.0	-	-	96.7	100.0	-	-	-	-	100.0	100.0	-	100.0	-	-	100.0	96.5
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	7	0	0	-	7	0	6	0	0	-	6	0	0	0	0	-	0	0	0	0	0	-	0	13
% Single-Unit Trucks	0.0	4.2	-	-	-	4.1	0.0	3.6	0.0	-	-	3.3	0.0	-	-	-	-	0.0	0.0	-	0.0	-	-	0.0	3.5
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Irvine Street & East Mill Street
Site Code: 210662
Start Date: 02/08/2022
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Turning Movement Peak Hour Data Plot (12:30 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Irvine Street & East Mill Street
Site Code: 210662
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Turning Movement Peak Hour Data (3:00 PM)

Start Time	East Mill Street Eastbound						East Mill Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
3:00 PM	1	66	0	0	0	67	0	41	2	0	0	43	0	0	0	0	0	0	13	0	3	0	2	16	126
3:15 PM	3	44	0	0	0	47	0	85	10	0	0	95	0	0	0	0	0	0	9	0	1	0	35	10	152
3:30 PM	9	38	0	0	0	47	0	51	8	0	0	59	0	0	0	0	0	0	4	0	5	0	16	9	115
3:45 PM	3	35	0	0	0	38	0	48	4	0	0	52	0	0	0	0	0	0	5	0	1	0	1	6	96
Total	16	183	0	0	0	199	0	225	24	0	0	249	0	0	0	0	0	0	31	0	10	0	54	41	489
Approach %	8.0	92.0	0.0	0.0	-	-	0.0	90.4	9.6	0.0	-	-	0.0	0.0	0.0	0.0	-	-	75.6	0.0	24.4	0.0	-	-	-
Total %	3.3	37.4	0.0	0.0	-	40.7	0.0	46.0	4.9	0.0	-	50.9	0.0	0.0	0.0	0.0	-	0.0	6.3	0.0	2.0	0.0	-	8.4	-
PHF	0.444	0.693	0.000	0.000	-	0.743	0.000	0.662	0.600	0.000	-	0.655	0.000	0.000	0.000	0.000	-	0.000	0.596	0.000	0.500	0.000	-	0.641	0.804
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	-	0.0	-	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	0.0
Cars & Light Goods	15	181	0	0	-	196	0	213	18	0	-	231	0	0	0	0	-	0	19	0	10	0	-	29	456
% Cars & Light Goods	93.8	98.9	-	-	-	98.5	-	94.7	75.0	-	-	92.8	-	-	-	-	-	-	61.3	-	100.0	-	-	70.7	93.3
Buses	1	1	0	0	-	2	0	6	6	0	-	12	0	0	0	0	-	0	9	0	0	0	-	9	23
% Buses	6.3	0.5	-	-	-	1.0	-	2.7	25.0	-	-	4.8	-	-	-	-	-	-	29.0	-	0.0	-	-	22.0	4.7
Single-Unit Trucks	0	1	0	0	-	1	0	5	0	0	-	5	0	0	0	0	-	0	3	0	0	0	-	3	9
% Single-Unit Trucks	0.0	0.5	-	-	-	0.5	-	2.2	0.0	-	-	2.0	-	-	-	-	-	-	9.7	-	0.0	-	-	7.3	1.8
Articulated Trucks	0	0	0	0	-	0	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.0	-	-	-	0.0	-	0.4	0.0	-	-	0.4	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	0.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	-	0.0	-	0.0	0.0	-	-	0.0	-	-	-	-	-	-	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	54	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Irvine Street & Woolwich Street
Site Code: 210662
Start Date: 11/23/2021
Page No: 1

Turning Movement Data

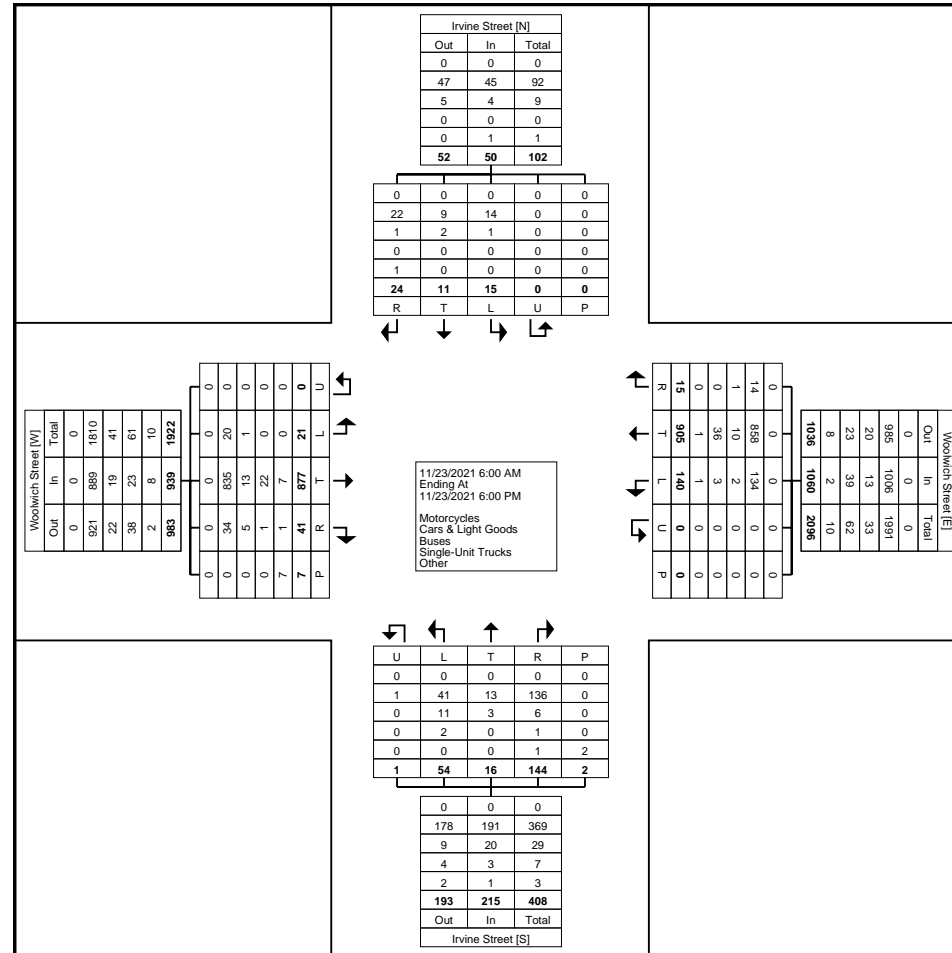
Start Time	Woolwich Street Eastbound						Woolwich Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
6:00 AM	0	4	0	0	0	4	0	17	0	0	0	17	0	0	0	0	0	0	0	0	0	0	0	0	21
6:15 AM	0	8	0	0	0	8	0	24	0	0	0	24	0	0	2	0	0	2	0	0	0	0	0	0	34
6:30 AM	1	9	0	0	1	10	0	16	1	0	0	17	0	0	1	0	0	1	0	0	0	0	0	0	28
6:45 AM	0	14	2	0	3	16	2	17	1	0	0	20	0	0	2	0	0	2	0	1	1	0	0	2	40
Hourly Total	1	35	2	0	4	38	2	74	2	0	0	78	0	0	5	0	0	5	0	1	1	0	0	2	123
7:00 AM	0	18	0	0	1	18	2	24	0	0	0	26	0	0	5	0	0	5	0	0	1	0	0	1	50
7:15 AM	0	15	0	0	0	15	2	27	0	0	0	29	0	0	7	0	0	7	1	0	2	0	0	3	54
7:30 AM	0	21	0	0	0	21	1	38	0	0	0	39	0	0	1	0	0	1	0	1	0	0	0	1	62
7:45 AM	0	40	1	0	0	41	10	34	1	0	0	45	1	2	7	0	0	10	0	1	1	0	0	2	98
Hourly Total	0	94	1	0	1	95	15	123	1	0	0	139	1	2	20	0	0	23	1	2	4	0	0	7	264
8:00 AM	0	31	0	0	0	31	2	47	0	0	0	49	2	1	5	0	1	8	0	0	0	0	0	0	88
8:15 AM	1	23	2	0	0	26	4	34	1	0	0	39	1	1	5	0	0	7	1	0	1	0	0	2	74
8:30 AM	2	25	2	0	0	29	3	47	1	0	0	51	6	0	7	0	0	13	1	0	2	0	0	3	96
8:45 AM	2	33	7	0	1	42	1	34	1	0	0	36	9	0	2	0	0	11	0	1	0	0	0	1	90
Hourly Total	5	112	11	0	1	128	10	162	3	0	0	175	18	2	19	0	1	39	2	1	3	0	0	6	348
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	0	24	0	0	0	24	2	23	1	0	0	26	0	0	4	0	0	4	1	1	1	0	0	3	57
12:15 PM	2	22	1	0	0	25	3	16	0	0	0	19	1	0	1	0	0	2	0	0	1	0	0	1	47
12:30 PM	1	27	0	0	0	28	3	20	0	0	0	23	2	2	6	0	0	10	2	0	0	0	0	2	63
12:45 PM	1	15	1	0	0	17	4	19	0	0	0	23	1	0	4	0	0	5	0	0	2	0	0	2	47
Hourly Total	4	88	2	0	0	94	12	78	1	0	0	91	4	2	15	0	0	21	3	1	4	0	0	8	214
1:00 PM	2	21	0	0	0	23	5	16	0	0	0	21	2	0	7	0	0	9	0	0	1	0	0	1	54
1:15 PM	0	23	0	0	0	23	5	22	1	0	0	28	2	0	0	0	0	2	1	0	0	0	0	1	54
1:30 PM	3	17	1	0	0	21	3	14	1	0	0	18	1	1	7	0	0	9	0	1	0	0	0	1	49
1:45 PM	0	20	0	0	0	20	1	18	1	0	0	20	1	0	3	0	0	4	0	0	1	0	0	1	45
Hourly Total	5	81	1	0	0	87	14	70	3	0	0	87	6	1	17	0	0	24	1	1	2	0	0	4	202
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	0	27	2	0	0	29	7	42	1	0	0	50	2	0	4	0	0	6	0	1	2	0	0	3	88
3:15 PM	0	44	6	0	0	50	2	30	0	0	0	32	5	0	8	0	0	13	1	0	1	0	0	2	97
3:30 PM	1	39	5	0	0	45	5	34	2	0	0	41	3	2	6	0	0	11	1	1	3	0	0	5	102
3:45 PM	1	38	2	0	1	41	7	32	0	0	0	39	2	2	6	0	1	10	0	1	1	0	0	2	92
Hourly Total	2	148	15	0	1	165	21	138	3	0	0	162	12	4	24	0	1	40	2	3	7	0	0	12	379
4:00 PM	1	41	2	0	0	44	8	32	1	0	0	41	1	2	9	1	0	13	1	0	1	0	0	2	100
4:15 PM	0	33	3	0	0	36	7	34	1	0	0	42	3	1	5	0	0	9	1	1	0	0	0	2	89
4:30 PM	0	47	1	0	0	48	6	38	0	0	0	44	2	0	7	0	0	9	0	0	0	0	0	0	101



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Woolwich Street
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Woolwich Street
Site Code: 210662
Start Date: 11/23/2021
Page No: 4

Turning Movement Peak Hour Data (7:45 AM)

Start Time	Woolwich Street Eastbound						Woolwich Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
7:45 AM	0	40	1	0	0	41	10	34	1	0	0	45	1	2	7	0	0	10	0	1	1	0	0	2	98
8:00 AM	0	31	0	0	0	31	2	47	0	0	0	49	2	1	5	0	1	8	0	0	0	0	0	0	88
8:15 AM	1	23	2	0	0	26	4	34	1	0	0	39	1	1	5	0	0	7	1	0	1	0	0	2	74
8:30 AM	2	25	2	0	0	29	3	47	1	0	0	51	6	0	7	0	0	13	1	0	2	0	0	3	96
Total	3	119	5	0	0	127	19	162	3	0	0	184	10	4	24	0	1	38	2	1	4	0	0	7	356
Approach %	2.4	93.7	3.9	0.0	-	-	10.3	88.0	1.6	0.0	-	-	26.3	10.5	63.2	0.0	-	-	28.6	14.3	57.1	0.0	-	-	-
Total %	0.8	33.4	1.4	0.0	-	35.7	5.3	45.5	0.8	0.0	-	51.7	2.8	1.1	6.7	0.0	-	10.7	0.6	0.3	1.1	0.0	-	2.0	-
PHF	0.375	0.744	0.625	0.000	-	0.774	0.475	0.862	0.750	0.000	-	0.902	0.417	0.500	0.857	0.000	-	0.731	0.500	0.250	0.500	0.000	-	0.583	0.908
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	3	115	4	0	-	122	14	150	2	0	-	166	8	4	23	0	-	35	2	1	3	0	-	6	329
% Cars & Light Goods	100.0	96.6	80.0	-	-	96.1	73.7	92.6	66.7	-	-	90.2	80.0	100.0	95.8	-	-	92.1	100.0	100.0	75.0	-	-	85.7	92.4
Buses	0	1	1	0	-	2	2	4	1	0	-	7	2	0	1	0	-	3	0	0	1	0	-	1	13
% Buses	0.0	0.8	20.0	-	-	1.6	10.5	2.5	33.3	-	-	3.8	20.0	0.0	4.2	-	-	7.9	0.0	0.0	25.0	-	-	14.3	3.7
Single-Unit Trucks	0	1	0	0	-	1	2	8	0	0	-	10	0	0	0	0	-	0	0	0	0	0	-	0	11
% Single-Unit Trucks	0.0	0.8	0.0	-	-	0.8	10.5	4.9	0.0	-	-	5.4	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	3.1
Articulated Trucks	0	2	0	0	-	2	1	0	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	3
% Articulated Trucks	0.0	1.7	0.0	-	-	1.6	5.3	0.0	0.0	-	-	0.5	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.8
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Woolwich Street
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data (12:30 PM)

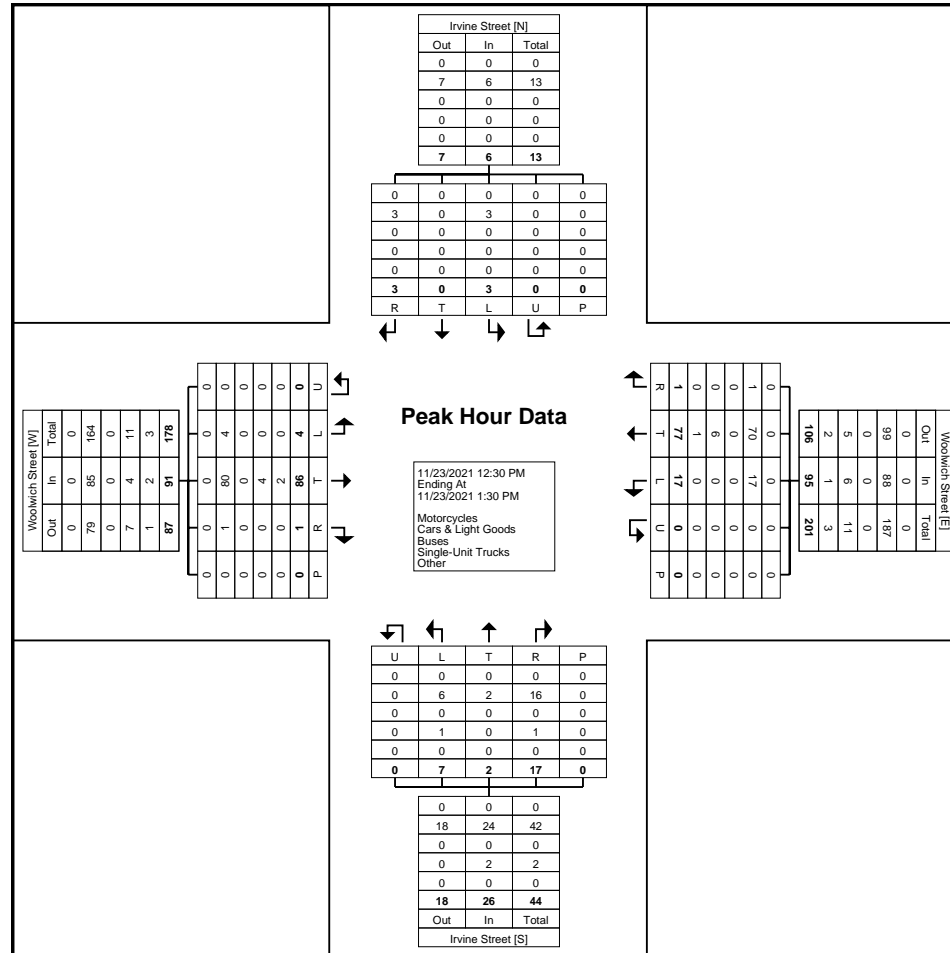
Start Time	Woolwich Street Eastbound						Woolwich Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
12:30 PM	1	27	0	0	0	28	3	20	0	0	0	23	2	2	6	0	0	10	2	0	0	0	0	2	63
12:45 PM	1	15	1	0	0	17	4	19	0	0	0	23	1	0	4	0	0	5	0	0	2	0	0	2	47
1:00 PM	2	21	0	0	0	23	5	16	0	0	0	21	2	0	7	0	0	9	0	0	1	0	0	1	54
1:15 PM	0	23	0	0	0	23	5	22	1	0	0	28	2	0	0	0	0	2	1	0	0	0	0	1	54
Total	4	86	1	0	0	91	17	77	1	0	0	95	7	2	17	0	0	26	3	0	3	0	0	6	218
Approach %	4.4	94.5	1.1	0.0	-	-	17.9	81.1	1.1	0.0	-	-	26.9	7.7	65.4	0.0	-	-	50.0	0.0	50.0	0.0	-	-	-
Total %	1.8	39.4	0.5	0.0	-	41.7	7.8	35.3	0.5	0.0	-	43.6	3.2	0.9	7.8	0.0	-	11.9	1.4	0.0	1.4	0.0	-	2.8	-
PHF	0.500	0.796	0.250	0.000	-	0.813	0.850	0.875	0.250	0.000	-	0.848	0.875	0.250	0.607	0.000	-	0.650	0.375	0.000	0.375	0.000	-	0.750	0.865
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Cars & Light Goods	4	80	1	0	-	85	17	70	1	0	-	88	6	2	16	0	-	24	3	0	3	0	-	6	203
% Cars & Light Goods	100.0	93.0	100.0	-	-	93.4	100.0	90.9	100.0	-	-	92.6	85.7	100.0	94.1	-	-	92.3	100.0	-	100.0	-	-	100.0	93.1
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Single-Unit Trucks	0	4	0	0	-	4	0	6	0	0	-	6	1	0	1	0	-	2	0	0	0	0	-	0	12
% Single-Unit Trucks	0.0	4.7	0.0	-	-	4.4	0.0	7.8	0.0	-	-	6.3	14.3	0.0	5.9	-	-	7.7	0.0	-	0.0	-	-	0.0	5.5
Articulated Trucks	0	2	0	0	-	2	0	1	0	0	-	1	0	0	0	0	-	0	0	0	0	0	-	0	3
% Articulated Trucks	0.0	2.3	0.0	-	-	2.2	0.0	1.3	0.0	-	-	1.1	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	1.4
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	-	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Paradigm Transportation Solutions Limited
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Count Name: Irvine Street & Woolwich Street
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Start Date: 11/23/2021
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Turning Movement Peak Hour Data Plot (12:30 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Irvine Street & Woolwich Street
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data (3:15 PM)

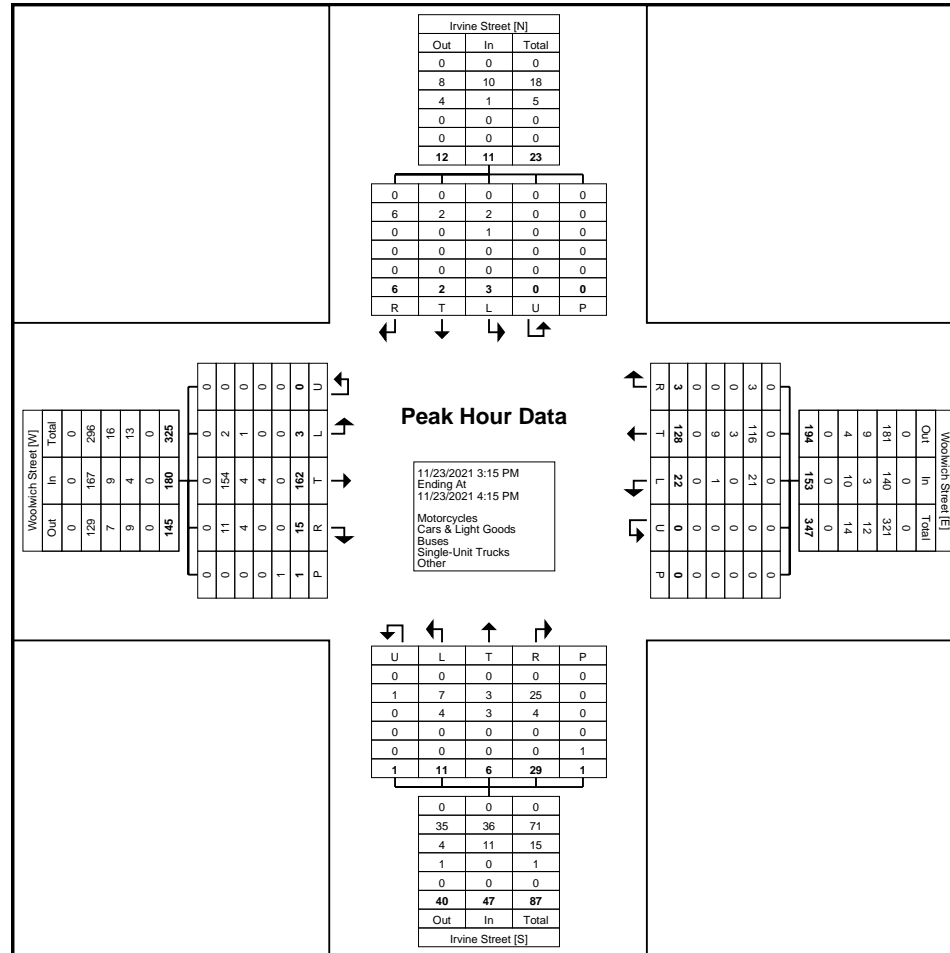
Start Time	Woolwich Street Eastbound						Woolwich Street Westbound						Irvine Street Northbound						Irvine Street Southbound						Int. Total
	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	Left	Thru	Right	U-Turn	Peds	App. Total	
3:15 PM	0	44	6	0	0	50	2	30	0	0	0	32	5	0	8	0	0	13	1	0	1	0	0	2	97
3:30 PM	1	39	5	0	0	45	5	34	2	0	0	41	3	2	6	0	0	11	1	1	3	0	0	5	102
3:45 PM	1	38	2	0	1	41	7	32	0	0	0	39	2	2	6	0	1	10	0	1	1	0	0	2	92
4:00 PM	1	41	2	0	0	44	8	32	1	0	0	41	1	2	9	1	0	13	1	0	1	0	0	2	100
Total	3	162	15	0	1	180	22	128	3	0	0	153	11	6	29	1	1	47	3	2	6	0	0	11	391
Approach %	1.7	90.0	8.3	0.0	-	-	14.4	83.7	2.0	0.0	-	-	23.4	12.8	61.7	2.1	-	-	27.3	18.2	54.5	0.0	-	-	-
Total %	0.8	41.4	3.8	0.0	-	46.0	5.6	32.7	0.8	0.0	-	39.1	2.8	1.5	7.4	0.3	-	12.0	0.8	0.5	1.5	0.0	-	2.8	-
PHF	0.750	0.920	0.625	0.000	-	0.900	0.688	0.941	0.375	0.000	-	0.933	0.550	0.750	0.806	0.250	-	0.904	0.750	0.500	0.500	0.000	-	0.550	0.958
Motorcycles	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	2	154	11	0	-	167	21	116	3	0	-	140	7	3	25	1	-	36	2	2	6	0	-	10	353
% Cars & Light Goods	66.7	95.1	73.3	-	-	92.8	95.5	90.6	100.0	-	-	91.5	63.6	50.0	86.2	100.0	-	76.6	66.7	100.0	100.0	-	-	90.9	90.3
Buses	1	4	4	0	-	9	0	3	0	0	-	3	4	3	4	0	-	11	1	0	0	0	-	1	24
% Buses	33.3	2.5	26.7	-	-	5.0	0.0	2.3	0.0	-	-	2.0	36.4	50.0	13.8	0.0	-	23.4	33.3	0.0	0.0	-	-	9.1	6.1
Single-Unit Trucks	0	4	0	0	-	4	1	9	0	0	-	10	0	0	0	0	-	0	0	0	0	0	-	0	14
% Single-Unit Trucks	0.0	2.5	0.0	-	-	2.2	4.5	7.0	0.0	-	-	6.5	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	3.6
Articulated Trucks	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	0.0	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-	-	-	-	-	-	-
Pedestrians	-	-	-	-	1	-	-	-	-	-	0	-	-	-	-	-	1	-	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	100.0	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-	-	-	-	-	-	-



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Irvine Street & Woolwich Street
Site Code: 210662
Start Date: 11/23/2021
Page No: 9



Turning Movement Peak Hour Data Plot (3:15 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
519-896-3163 cbowness@ptsl.com

Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
Page No: 1

Turning Movement Data

Start Time	Woolwich Street Eastbound					Woolwich Street Westbound					Milford Cres Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
6:00 AM	0	4	0	0	4	19	0	0	0	19	0	0	0	0	0	23
6:15 AM	0	8	0	0	8	24	0	0	0	24	0	1	0	3	1	33
6:30 AM	0	8	0	0	8	15	1	0	0	16	2	0	0	0	2	26
6:45 AM	0	15	0	0	15	17	0	0	0	17	1	0	0	0	1	33
Hourly Total	0	35	0	0	35	75	1	0	0	76	3	1	0	3	4	115
7:00 AM	0	19	0	0	19	25	0	0	0	25	0	0	0	0	0	44
7:15 AM	0	11	0	0	11	28	0	0	0	28	2	0	0	0	2	41
7:30 AM	0	21	0	0	21	36	0	0	0	36	0	0	0	0	0	57
7:45 AM	1	38	0	0	39	37	1	0	0	38	2	1	0	0	3	80
Hourly Total	1	89	0	0	90	126	1	0	0	127	4	1	0	0	5	222
8:00 AM	0	31	0	0	31	48	0	0	0	48	0	2	0	1	2	81
8:15 AM	0	24	0	0	24	34	2	0	0	36	2	1	0	0	3	63
8:30 AM	4	28	0	0	32	49	6	0	0	55	2	1	0	0	3	90
8:45 AM	1	40	0	0	41	30	13	0	0	43	1	0	0	1	1	85
Hourly Total	5	123	0	0	128	161	21	0	0	182	5	4	0	2	9	319
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
12:00 PM	2	24	0	0	26	23	1	0	0	24	0	0	0	0	0	50
12:15 PM	1	23	0	0	24	18	0	0	0	18	2	0	0	0	2	44
12:30 PM	0	28	0	0	28	22	0	0	1	22	0	0	0	1	0	50
12:45 PM	2	17	0	0	19	22	0	0	0	22	0	0	0	0	0	41
Hourly Total	5	92	0	0	97	85	1	0	1	86	2	0	0	1	2	185
1:00 PM	0	23	0	0	23	17	1	0	0	18	0	1	0	0	1	42
1:15 PM	0	22	0	0	22	23	1	0	0	24	0	1	0	0	1	47
1:30 PM	0	20	0	0	20	14	1	0	0	15	1	0	0	0	1	36
1:45 PM	1	19	0	1	20	20	0	0	0	20	1	0	0	0	1	41
Hourly Total	1	84	0	1	85	74	3	0	0	77	2	2	0	0	4	166
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
3:00 PM	3	27	0	0	30	36	9	0	0	45	2	1	0	0	3	78
3:15 PM	1	50	0	0	51	32	6	0	0	38	1	0	0	1	1	90
3:30 PM	0	42	0	0	42	38	2	0	0	40	2	2	0	0	4	86
3:45 PM	1	39	0	0	40	33	3	1	0	37	1	0	0	1	1	78
Hourly Total	5	158	0	0	163	139	20	1	0	160	6	3	0	2	9	332
4:00 PM	2	41	0	0	43	32	1	0	0	33	3	0	0	0	3	79
4:15 PM	0	35	0	0	35	37	1	0	0	38	2	0	0	0	2	75
4:30 PM	0	47	0	0	47	38	2	0	0	40	0	0	0	0	0	87
4:45 PM	0	46	0	0	46	33	0	0	0	33	2	0	0	1	2	81

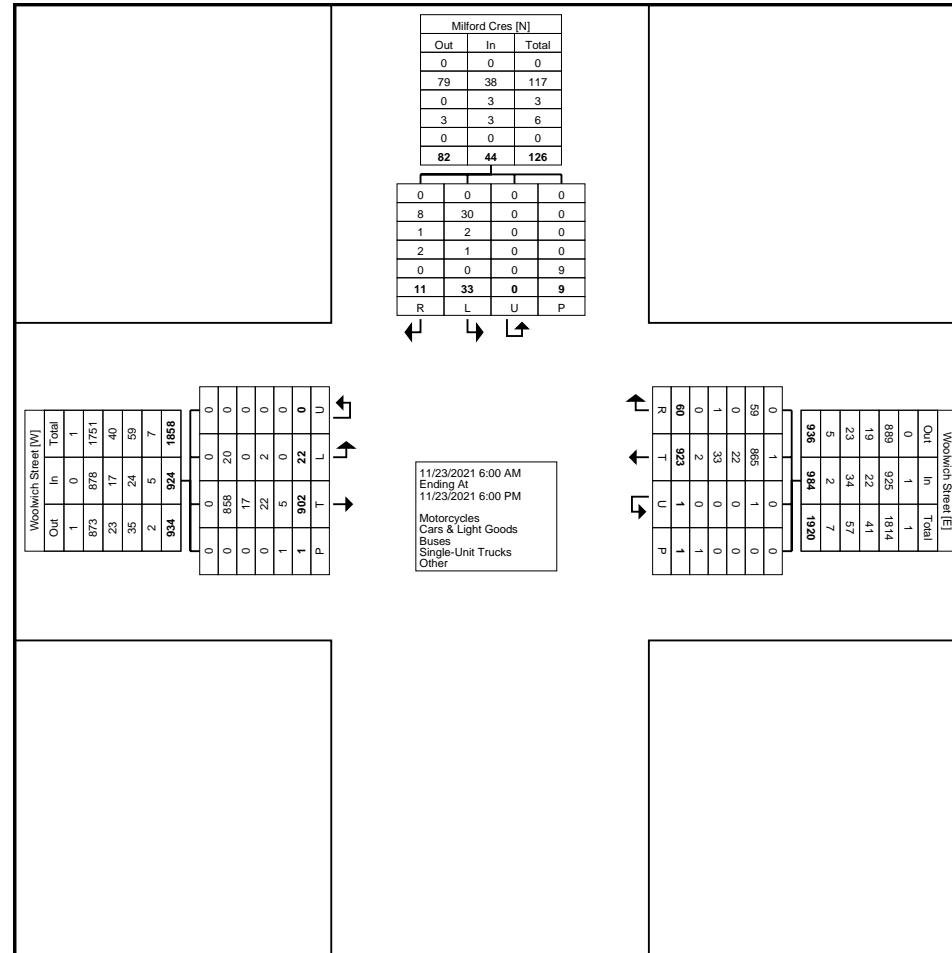
Hourly Total	2	169	0	0	171	140	4	0	0	144	7	0	0	1	7	322
5:00 PM	0	41	0	0	41	36	2	0	0	38	2	0	0	0	2	81
5:15 PM	1	31	0	0	32	40	2	0	0	42	0	0	0	0	0	74
5:30 PM	1	37	0	0	38	26	3	0	0	29	0	0	0	0	0	67
5:45 PM	1	43	0	0	44	21	2	0	0	23	2	0	0	0	2	69
Hourly Total	3	152	0	0	155	123	9	0	0	132	4	0	0	0	4	291
Grand Total	22	902	0	1	924	923	60	1	1	984	33	11	0	9	44	1952
Approach %	2.4	97.6	0.0	-	-	93.8	6.1	0.1	-	-	75.0	25.0	0.0	-	-	-
Total %	1.1	46.2	0.0	-	47.3	47.3	3.1	0.1	-	50.4	1.7	0.6	0.0	-	2.3	-
Motorcycles	0	0	0	-	0	1	0	0	-	1	0	0	0	-	0	1
% Motorcycles	0.0	0.0	-	-	0.0	0.1	0.0	0.0	-	0.1	0.0	0.0	-	-	0.0	0.1
Cars & Light Goods	20	858	0	-	878	865	59	1	-	925	30	8	0	-	38	1841
% Cars & Light Goods	90.9	95.1	-	-	95.0	93.7	98.3	100.0	-	94.0	90.9	72.7	-	-	86.4	94.3
Buses	0	17	0	-	17	22	0	0	-	22	2	1	0	-	3	42
% Buses	0.0	1.9	-	-	1.8	2.4	0.0	0.0	-	2.2	6.1	9.1	-	-	6.8	2.2
Single-Unit Trucks	2	22	0	-	24	33	1	0	-	34	1	2	0	-	3	61
% Single-Unit Trucks	9.1	2.4	-	-	2.6	3.6	1.7	0.0	-	3.5	3.0	18.2	-	-	6.8	3.1
Articulated Trucks	0	5	0	-	5	1	0	0	-	1	0	0	0	-	0	6
% Articulated Trucks	0.0	0.6	-	-	0.5	0.1	0.0	0.0	-	0.1	0.0	0.0	-	-	0.0	0.3
Bicycles on Road	0	0	0	-	0	1	0	0	-	1	0	0	0	-	0	1
% Bicycles on Road	0.0	0.0	-	-	0.0	0.1	0.0	0.0	-	0.1	0.0	0.0	-	-	0.0	0.1
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	0.0	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	1	-	-	-	-	1	-	-	-	-	9	-	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
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Turning Movement Data Plot



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data (8:00 AM)

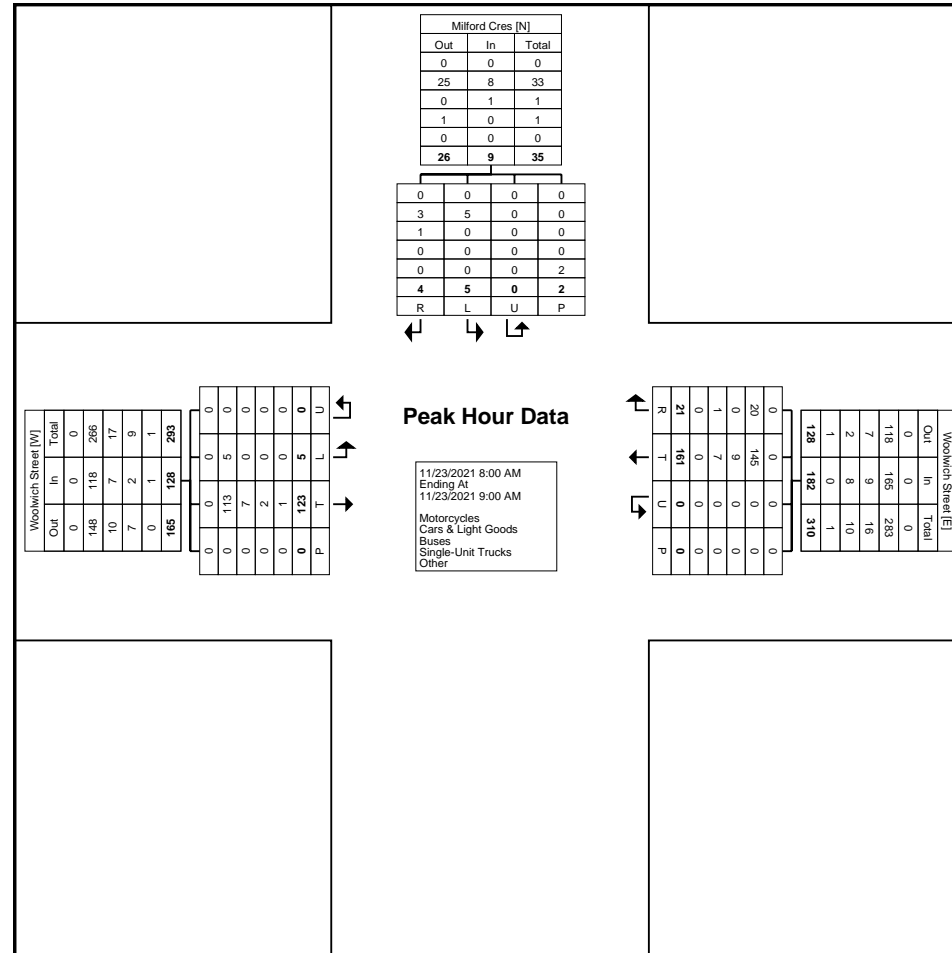
Start Time	Woolwich Street Eastbound					Woolwich Street Westbound					Milford Cres Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
8:00 AM	0	31	0	0	31	48	0	0	0	48	0	2	0	1	2	81
8:15 AM	0	24	0	0	24	34	2	0	0	36	2	1	0	0	3	63
8:30 AM	4	28	0	0	32	49	6	0	0	55	2	1	0	0	3	90
8:45 AM	1	40	0	0	41	30	13	0	0	43	1	0	0	1	1	85
Total	5	123	0	0	128	161	21	0	0	182	5	4	0	2	9	319
Approach %	3.9	96.1	0.0	-	-	88.5	11.5	0.0	-	-	55.6	44.4	0.0	-	-	-
Total %	1.6	38.6	0.0	-	40.1	50.5	6.6	0.0	-	57.1	1.6	1.3	0.0	-	2.8	-
PHF	0.313	0.769	0.000	-	0.780	0.821	0.404	0.000	-	0.827	0.625	0.500	0.000	-	0.750	0.886
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	5	113	0	-	118	145	20	0	-	165	5	3	0	-	8	291
% Cars & Light Goods	100.0	91.9	-	-	92.2	90.1	95.2	-	-	90.7	100.0	75.0	-	-	88.9	91.2
Buses	0	7	0	-	7	9	0	0	-	9	0	1	0	-	1	17
% Buses	0.0	5.7	-	-	5.5	5.6	0.0	-	-	4.9	0.0	25.0	-	-	11.1	5.3
Single-Unit Trucks	0	2	0	-	2	7	1	0	-	8	0	0	0	-	0	10
% Single-Unit Trucks	0.0	1.6	-	-	1.6	4.3	4.8	-	-	4.4	0.0	0.0	-	-	0.0	3.1
Articulated Trucks	0	1	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Articulated Trucks	0.0	0.8	-	-	0.8	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
Page No: 5



Turning Movement Peak Hour Data Plot (8:00 AM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
Page No: 6

Turning Movement Peak Hour Data (12:00 PM)

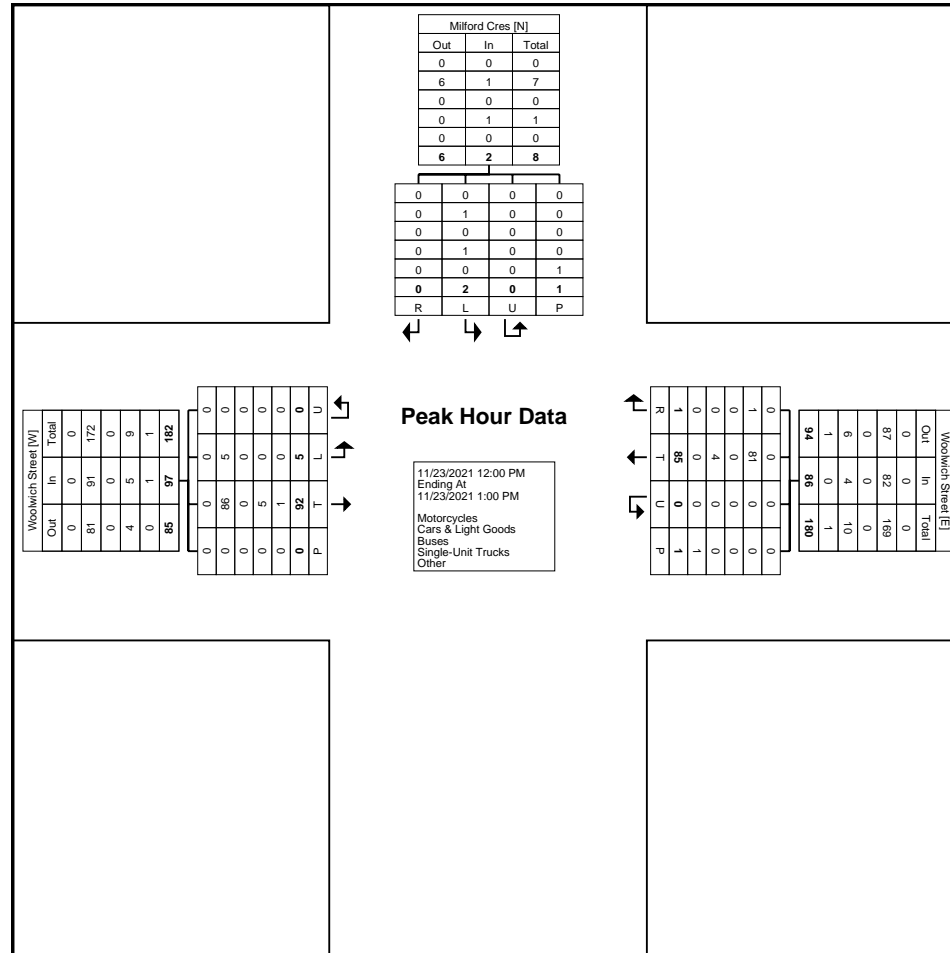
Start Time	Woolwich Street Eastbound					Woolwich Street Westbound					Milford Cres Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
12:00 PM	2	24	0	0	26	23	1	0	0	24	0	0	0	0	0	50
12:15 PM	1	23	0	0	24	18	0	0	0	18	2	0	0	0	2	44
12:30 PM	0	28	0	0	28	22	0	0	1	22	0	0	0	1	0	50
12:45 PM	2	17	0	0	19	22	0	0	0	22	0	0	0	0	0	41
Total	5	92	0	0	97	85	1	0	1	86	2	0	0	1	2	185
Approach %	5.2	94.8	0.0	-	-	98.8	1.2	0.0	-	-	100.0	0.0	0.0	-	-	-
Total %	2.7	49.7	0.0	-	52.4	45.9	0.5	0.0	-	46.5	1.1	0.0	0.0	-	1.1	-
PHF	0.625	0.821	0.000	-	0.866	0.924	0.250	0.000	-	0.896	0.250	0.000	0.000	-	0.250	0.925
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	0.0	0.0
Cars & Light Goods	5	86	0	-	91	81	1	0	-	82	1	0	0	-	1	174
% Cars & Light Goods	100.0	93.5	-	-	93.8	95.3	100.0	-	-	95.3	50.0	-	-	-	50.0	94.1
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	0.0	0.0
Single-Unit Trucks	0	5	0	-	5	4	0	0	-	4	1	0	0	-	1	10
% Single-Unit Trucks	0.0	5.4	-	-	5.2	4.7	0.0	-	-	4.7	50.0	-	-	-	50.0	5.4
Articulated Trucks	0	1	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Articulated Trucks	0.0	1.1	-	-	1.0	0.0	0.0	-	-	0.0	0.0	-	-	-	0.0	0.5
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	-	-	0.0	0.0	-	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	0.0	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	1	-	-	-	-	1	-	-
% Pedestrians	-	-	-	-	-	-	-	-	100.0	-	-	-	-	100.0	-	-



Paradigm Transportation Solutions Limited
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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data Plot (12:00 PM)



Paradigm Transportation Solutions Limited
5A-150 Pinebush Rd

Cambridge, Ontario, Canada N1R 8J8
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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
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Turning Movement Peak Hour Data (3:15 PM)

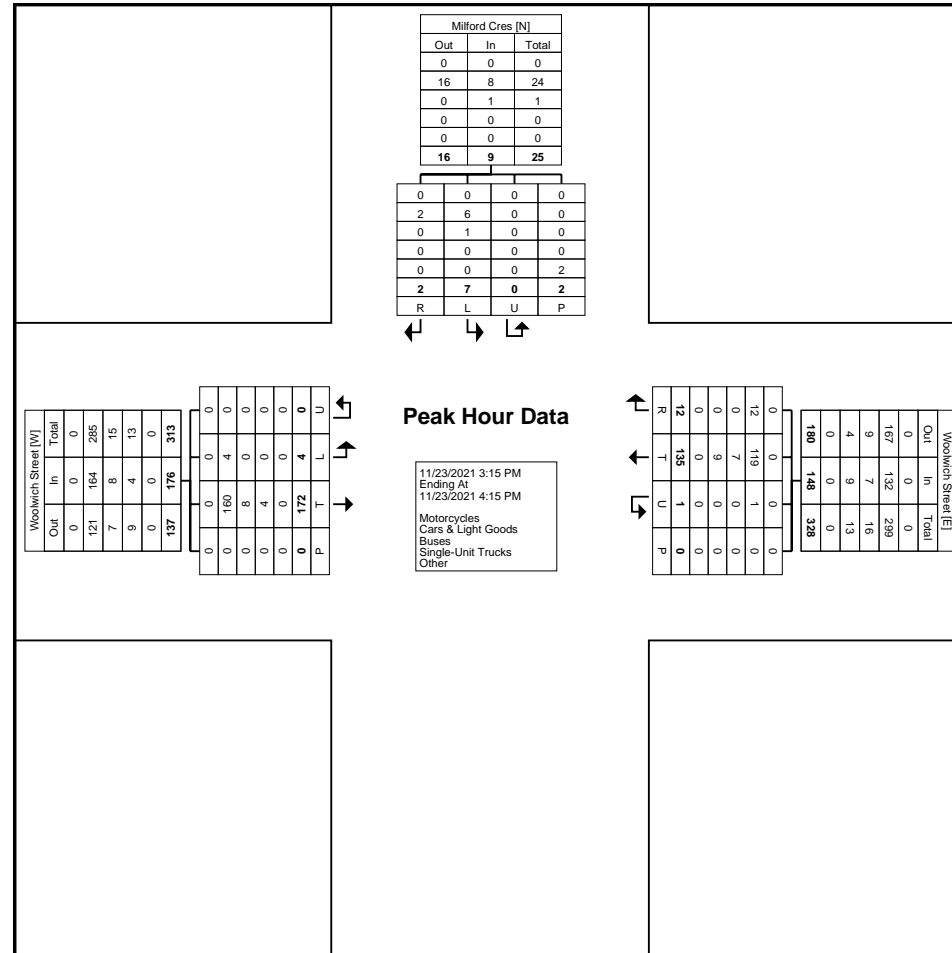
Start Time	Woolwich Street Eastbound					Woolwich Street Westbound					Milford Cres Southbound					Int. Total
	Left	Thru	U-Turn	Peds	App. Total	Thru	Right	U-Turn	Peds	App. Total	Left	Right	U-Turn	Peds	App. Total	
3:15 PM	1	50	0	0	51	32	6	0	0	38	1	0	0	1	1	90
3:30 PM	0	42	0	0	42	38	2	0	0	40	2	2	0	0	4	86
3:45 PM	1	39	0	0	40	33	3	1	0	37	1	0	0	1	1	78
4:00 PM	2	41	0	0	43	32	1	0	0	33	3	0	0	0	3	79
Total	4	172	0	0	176	135	12	1	0	148	7	2	0	2	9	333
Approach %	2.3	97.7	0.0	-	-	91.2	8.1	0.7	-	-	77.8	22.2	0.0	-	-	-
Total %	1.2	51.7	0.0	-	52.9	40.5	3.6	0.3	-	44.4	2.1	0.6	0.0	-	2.7	-
PHF	0.500	0.860	0.000	-	0.863	0.888	0.500	0.250	-	0.925	0.583	0.250	0.000	-	0.563	0.925
Motorcycles	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Motorcycles	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	0.0	0.0
Cars & Light Goods	4	160	0	-	164	119	12	1	-	132	6	2	0	-	8	304
% Cars & Light Goods	100.0	93.0	-	-	93.2	88.1	100.0	100.0	-	89.2	85.7	100.0	-	-	88.9	91.3
Buses	0	8	0	-	8	7	0	0	-	7	1	0	0	-	1	16
% Buses	0.0	4.7	-	-	4.5	5.2	0.0	0.0	-	4.7	14.3	0.0	-	-	11.1	4.8
Single-Unit Trucks	0	4	0	-	4	9	0	0	-	9	0	0	0	-	0	13
% Single-Unit Trucks	0.0	2.3	-	-	2.3	6.7	0.0	0.0	-	6.1	0.0	0.0	-	-	0.0	3.9
Articulated Trucks	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Articulated Trucks	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	0.0	0.0	-	-	0.0	0.0	0.0	0.0	-	0.0	0.0	0.0	-	-	0.0	0.0
Bicycles on Crosswalk	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	0.0	-	-
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-



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Count Name: Woolwich Street & Milford Cres
Site Code: 210662
Start Date: 11/23/2021
Page No: 9



Turning Movement Peak Hour Data Plot (3:15 PM)

Appendix C

Base Year Operation Synchro Reports



Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Base Year (2022)
AM Peak Hour

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	7	161	212	28	7	5
Future Volume (vph)	7	161	212	28	7	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.984		0.948	
Flt Protected		0.998			0.970	
Satd. Flow (prot)	0	1861	1796	0	1747	0
Flt Permitted		0.998			0.970	
Satd. Flow (perm)	0	1861	1796	0	1747	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	8	175	230	30	8	5
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	183	260	0	13	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	24.2%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Base Year (2022)
AM Peak Hour

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	7	161	212	28	7	5
Future Volume (Veh/h)	7	161	212	28	7	5
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	175	230	30	8	5
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	262				438	247
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	262				438	247
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				99	99
cM capacity (veh/h)	1312				575	795
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	183	260	13			
Volume Left	8	0	8			
Volume Right	0	30	5			
eSH	1312	1700	644			
Volume to Capacity	0.01	0.15	0.02			
Queue Length 95th (m)	0.1	0.0	0.5			
Control Delay (s)	0.4	0.0	10.7			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	10.7			
Approach LOS			B			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization	24.2%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Base Year (2022)

AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	4	157	7	25	220	4	15	6	35	3	1	5
Future Volume (vph)	4	157	7	25	220	4	15	6	35	3	1	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.994			0.998			0.916			0.925	
Fit Protected		0.999			0.995			0.987			0.984	
Satd. Flow (prot)	0	1835	0	0	1857	0	0	1718	0	0	1729	0
Fit Permitted		0.999			0.995			0.987			0.984	
Satd. Flow (perm)	0	1835	0	0	1857	0	0	1718	0	0	1729	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			450.4			484.2	
Travel Time (s)		13.7			48.8			32.4			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	171	8	27	239	4	16	7	38	3	1	5
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	183	0	0	270	0	0	61	0	0	9	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	35.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Base Year (2022)

AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	4	157	7	25	220	4	15	6	35	3	1	5
Future Volume (Veh/h)	4	157	7	25	220	4	15	6	35	3	1	5
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	171	8	27	239	4	16	7	38	3	1	5
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	243			180			484	481	176	520	483	241
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	243			180			484	481	176	520	483	241
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			97	99	96	99	100	99
cM capacity (veh/h)	1335			1314			483	476	872	436	474	803
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	183	270	61	9								
Volume Left	4	27	16	3								
Volume Right	8	4	38	5								
cSH	1335	1314	667	592								
Volume to Capacity	0.00	0.02	0.09	0.02								
Queue Length 95th (m)	0.1	0.5	2.3	0.4								
Control Delay (s)	0.2	0.9	10.9	11.2								
Lane LOS	A	A	B	B								
Approach Delay (s)	0.2	0.9	10.9	11.2								
Approach LOS			B	B								

Intersection Summary

Average Delay	2.0
Intersection Capacity Utilization	35.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Base Year (2022)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	17	18	10	39	28	5
Future Volume (vph)	17	18	10	39	28	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.929				0.981	
Flt Protected	0.977			0.990		
Satd. Flow (prot)	1553	0	0	1837	1800	0
Flt Permitted	0.977			0.990		
Satd. Flow (perm)	1553	0	0	1837	1800	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	450.4	
Travel Time (s)	11.7			20.9	32.4	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	18	20	11	42	30	5
Shared Lane Traffic (%)						
Lane Group Flow (vph)	38	0	0	53	35	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	19.3%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Base Year (2022)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	17	18	10	39	28	5
Future Volume (Veh/h)	17	18	10	39	28	5
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	18	20	11	42	30	5
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	100	36	38			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	100	36	38			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	98	98	99			
cM capacity (veh/h)	896	983	1581			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	38	53	35			
Volume Left	18	11	0			
Volume Right	20	0	5			
sSH	940	1581	1700			
Volume to Capacity	0.04	0.01	0.02			
Queue Length 95th (m)	1.0	0.2	0.0			
Control Delay (s)	9.0	1.6	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.0	1.6	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			3.4			
Intersection Capacity Utilization	19.3%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	20	1	1	14	0	4	0	15	0	0	0
Future Volume (vph)	0	20	1	1	14	0	4	0	15	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.994						0.892				
Fit Protected					0.997			0.990				
Satd. Flow (prot)	0	1566	0	0	1732	0	0	1565	0	0	1900	0
Fit Permitted					0.997			0.990				
Satd. Flow (perm)	0	1566	0	0	1732	0	0	1565	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		88.4			162.4			207.7			36.3	
Travel Time (s)		6.4			11.7			15.0			2.6	
Confl. Peds. (#/hr)	2		4	4		2	2		1	1		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	17%	100%	0%	10%	0%	0%	0%	9%	0%	0%	0%
Adj. Flow (vph)	0	22	1	1	15	0	4	0	16	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	23	0	0	16	0	0	20	0	0	0	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	15.2%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	20	1	1	14	0	4	0	15	0	0	0
Future Volume (Veh/h)	0	20	1	1	14	0	4	0	15	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	22	1	1	15	0	4	0	16	0	0	0
Pedestrians		2			1			4			2	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	17			27			46	46	28	58	46	19
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	17			27			46	46	28	58	46	19
tC, single (s)	4.1			4.1			7.1	6.5	6.3	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.4	3.5	4.0	3.3
p0 queue free %	100			100			100	100	98	100	100	100
cM capacity (veh/h)	1611			1595			952	845	1024	922	845	1061
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	23	16	20	0								
Volume Left	0	1	4	0								
Volume Right	1	0	16	0								
eSH	1611	1595	1009	1700								
Volume to Capacity	0.00	0.00	0.02	0.00								
Queue Length 95th (m)	0.0	0.0	0.5	0.0								
Control Delay (s)	0.0	0.5	8.6	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	0.5	8.6	0.0								
Approach LOS			A	A								
Intersection Summary												
Average Delay				3.1								
Intersection Capacity Utilization			15.2%				ICU Level of Service			A		
Analysis Period (min)			15									

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	8	3	4	14	0	7	0	13	0	0	0
Future Volume (vph)	0	8	3	4	14	0	7	0	13	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.966						0.914				
Fit Protected					0.990			0.982				
Satd. Flow (prot)	0	1412	0	0	1647	0	0	920	0	0	1900	0
Fit Permitted					0.990			0.982				
Satd. Flow (perm)	0	1412	0	0	1647	0	0	920	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	9	3	4	15	0	8	0	14	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	12	0	0	19	0	0	22	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	17.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	8	3	4	14	0	7	0	13	0	0	0
Future Volume (Veh/h)	0	8	3	4	14	0	7	0	13	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	9	3	4	15	0	8	0	14	0	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	20			21			44	48	24	56	49	21
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	20			21			44	48	24	56	49	21
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			100			99	100	98	100	100	100
cM capacity (veh/h)	1603			1596			816	836	820	913	834	1057

Direction, Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	12	19	22	0
Volume Left	0	4	8	0
Volume Right	3	0	14	0
eSH	1603	1596	819	1700
Volume to Capacity	0.00	0.00	0.03	0.00
Queue Length 95th (m)	0.0	0.1	0.6	0.0
Control Delay (s)	0.0	1.5	9.5	0.0
Lane LOS		A	A	A
Approach Delay (s)	0.0	1.5	9.5	0.0
Approach LOS		A	A	

Intersection Summary	
Average Delay	4.5
Intersection Capacity Utilization	17.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Base Year (2022)
AM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	30	121	89	46	136	79
Future Volume (vph)	30	121	89	46	136	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.892	0.954				
Flt Protected	0.990					0.969
Satd. Flow (prot)	1539	0	1743	0	0	1703
Flt Permitted	0.990					0.969
Satd. Flow (perm)	1539	0	1743	0	0	1703
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	8%	3%	6%	7%	10%
Adj. Flow (vph)	33	132	97	50	148	86
Shared Lane Traffic (%)						
Lane Group Flow (vph)	165	0	147	0	0	234
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	39.2%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Base Year (2022)
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	30	121	89	46	136	79
Future Volume (Veh/h)	30	121	89	46	136	79
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	33	132	97	50	148	86
Pedestrians	4					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	508	126			151	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	508	126			151	
tC, single (s)	6.5	6.3			4.2	
tC, 2 stage (s)						
tF (s)	3.6	3.4			2.3	
p0 queue free %	93	85			89	
cM capacity (veh/h)	451	905			1395	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	165	147	234			
Volume Left	33	0	148			
Volume Right	132	50	0			
eSH	753	1700	1395			
Volume to Capacity	0.22	0.09	0.11			
Queue Length 95th (m)	6.3	0.0	2.7			
Control Delay (s)	11.1	0.0	5.3			
Lane LOS	B		A			
Approach Delay (s)	11.1	0.0	5.3			
Approach LOS	B					
Intersection Summary						
Average Delay		5.6				
Intersection Capacity Utilization	39.2%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	8	64	7	4	118	26	5	29	9	37	60	12
Future Volume (vph)	8	64	7	4	118	26	5	29	9	37	60	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.988			0.976			0.971			0.985	
Flt Protected		0.995			0.999			0.995			0.983	
Satd. Flow (prot)	0	1723	0	0	1780	0	0	1411	0	0	1621	0
Flt Permitted		0.995			0.999			0.995			0.983	
Satd. Flow (perm)	0	1723	0	0	1780	0	0	1411	0	0	1621	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	57		1	1		57	4		50	50		4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	6%	20%	33%	3%	5%	50%	32%	14%	4%	22%	0%
Adj. Flow (vph)	9	70	8	4	128	28	5	32	10	40	65	13
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	87	0	0	160	0	0	47	0	0	118	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	32.1%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	8	64	7	4	118	26	5	29	9	37	60	12
Future Volume (vph)	8	64	7	4	118	26	5	29	9	37	60	12
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	70	8	4	128	28	5	32	10	40	65	13
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	87	160	47	118								
Volume Left (vph)	9	4	5	40								
Volume Right (vph)	8	28	10	13								
Hadj (s)	0.11	-0.03	0.41	0.23								
Departure Headway (s)	4.6	4.4	5.0	4.8								
Degree Utilization, x	0.11	0.20	0.07	0.16								
Capacity (veh/h)	745	778	670	708								
Control Delay (s)	8.2	8.5	8.4	8.7								
Approach Delay (s)	8.2	8.5	8.4	8.7								
Approach LOS	A	A	A	A								
Intersection Summary												
Delay	8.5											
Level of Service	A											
Intersection Capacity Utilization	32.1%				ICU Level of Service				A			
Analysis Period (min)	15											

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	5	310	1	1	212	29	1	3	0	58	0	14
Future Volume (vph)	5	310	1	1	212	29	1	3	0	58	0	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.984							0.974	
Flt Protected		0.999						0.988			0.961	
Satd. Flow (prot)	0	1826	0	0	1806	0	0	1877	0	0	1778	0
Flt Permitted		0.999						0.988			0.961	
Satd. Flow (perm)	0	1826	0	0	1806	0	0	1877	0	0	1778	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)	42					42						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	5	337	1	1	230	32	1	3	0	63	0	15
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	343	0	0	263	0	0	4	0	0	78	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	37.3%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Base Year (2022)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	5	310	1	1	212	29	1	3	0	58	0	14
Future Volume (Veh/h)	5	310	1	1	212	29	1	3	0	58	0	14
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	337	1	1	230	32	1	3	0	63	0	15
Pedestrians												42
Lane Width (m)												3.6
Walking Speed (m/s)												1.2
Percent Blockage												4
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	304			338			610	654	338	639	638	288
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	304			338			610	654	338	639	638	288
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			100			100	99	100	83	100	98
cM capacity (veh/h)	1224			1232			389	374	709	364	381	729
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	343	263	4	78								
Volume Left	5	1	1	63								
Volume Right	1	32	0	15								
eSH	1224	1232	377	403								
Volume to Capacity	0.00	0.00	0.01	0.19								
Queue Length 95th (m)	0.1	0.0	0.2	5.4								
Control Delay (s)	0.2	0.0	14.6	16.1								
Lane LOS	A	A	B	C								
Approach Delay (s)	0.2	0.0	14.6	16.1								
Approach LOS			B	C								
Intersection Summary												
Average Delay				2.0								
Intersection Capacity Utilization	37.3%			ICU Level of Service	A							
Analysis Period (min)	15											

Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Base Year (2022)
PM Peak Hour

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	4	185	144	13	7	2
Future Volume (vph)	4	185	144	13	7	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.989		0.973	
Flt Protected		0.999			0.962	
Satd. Flow (prot)	0	1862	1805	0	1778	0
Flt Permitted		0.999			0.962	
Satd. Flow (perm)	0	1862	1805	0	1778	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	4	201	157	14	8	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	205	171	0	10	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	22.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Base Year (2022)
PM Peak Hour

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	4	185	144	13	7	2
Future Volume (Veh/h)	4	185	144	13	7	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	201	157	14	8	2
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	173				375	166
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	173				375	166
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				99	100
cM capacity (veh/h)	1414				627	882
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	205	171	10			
Volume Left	4	0	8			
Volume Right	0	14	2			
eSH	1414	1700	666			
Volume to Capacity	0.00	0.10	0.02			
Queue Length 95th (m)	0.1	0.0	0.3			
Control Delay (s)	0.2	0.0	10.5			
Lane LOS	A		B			
Approach Delay (s)	0.2	0.0	10.5			
Approach LOS			B			
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization	22.9%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Base Year (2022)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕				↕	
Traffic Volume (vph)	3	173	16	24	139	3	12	6	31	3	2	6	
Future Volume (vph)	3	173	16	24	139	3	12	6	31	3	2	6	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor													
Frt		0.989			0.998			0.915			0.921		
Flt Protected		0.999			0.993			0.988			0.988		
Satd. Flow (prot)	0	1828	0	0	1840	0	0	1718	0	0	1729	0	
Flt Permitted		0.999			0.993			0.988			0.988		
Satd. Flow (perm)	0	1828	0	0	1840	0	0	1718	0	0	1729	0	
Link Speed (k/h)		40			40			50			50		
Link Distance (m)		152.7			542.4			450.4			484.2		
Travel Time (s)		13.7			48.8			32.4			34.9		
Confl. Peds. (#/hr)			1	1									
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	3	188	17	26	151	3	13	7	34	3	2	7	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	208	0	0	180	0	0	54	0	0	12	0	
Sign Control		Free			Free			Stop			Stop		

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	32.4%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Base Year (2022)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕				↕	
Traffic Volume (veh/h)	3	173	16	24	139	3	12	6	31	3	2	6	
Future Volume (Veh/h)	3	173	16	24	139	3	12	6	31	3	2	6	
Sign Control		Free			Free			Stop			Stop		
Grade		0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	3	188	17	26	151	3	13	7	34	3	2	7	
Pedestrians								1					
Lane Width (m)								3.6					
Walking Speed (m/s)								1.2					
Percent Blockage								0					
Right turn flare (veh)													
Median type		None			None								
Median storage (veh)													
Upstream signal (m)													
pX, platoon unblocked													
vC, conflicting volume	154			206			416	410	198	444	416	152	
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	154			206			416	410	198	444	416	152	
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2	
tC, 2 stage (s)													
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3	
p0 queue free %	100			98			98	99	96	99	100	99	
cM capacity (veh/h)	1439			1285			535	522	848	492	518	899	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total	208	180	54	12									
Volume Left	3	26	13	3									
Volume Right	17	3	34	7									
cSH	1439	1285	694	676									
Volume to Capacity	0.00	0.02	0.08	0.02									
Queue Length 95th (m)	0.0	0.5	1.9	0.4									
Control Delay (s)	0.1	1.3	10.6	10.4									
Lane LOS	A	A	B	B									
Approach Delay (s)	0.1	1.3	10.6	10.4									
Approach LOS			B	B									

Intersection Summary

Average Delay	2.1
Intersection Capacity Utilization	32.4%
ICU Level of Service A	
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Base Year (2022)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	10	10	10	39	23	19
Future Volume (vph)	10	10	10	39	23	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932				0.938	
Flt Protected	0.976			0.990		
Satd. Flow (prot)	1564	0	0	1837	1600	0
Flt Permitted	0.976			0.990		
Satd. Flow (perm)	1564	0	0	1837	1600	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	450.4	
Travel Time (s)	11.7			20.9	32.4	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	11	11	11	42	25	21
Shared Lane Traffic (%)						
Lane Group Flow (vph)	22	0	0	53	46	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	19.3%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Base Year (2022)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	10	10	10	39	23	19
Future Volume (Veh/h)	10	10	10	39	23	19
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	11	11	42	25	21
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	102	38	49			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	102	38	49			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	99	99	99			
cM capacity (veh/h)	892	979	1567			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	22	53	46			
Volume Left	11	11	0			
Volume Right	11	0	21			
eSH	934	1567	1700			
Volume to Capacity	0.02	0.01	0.03			
Queue Length 95th (m)	0.5	0.2	0.0			
Control Delay (s)	8.9	1.6	0.0			
Lane LOS	A	A				
Approach Delay (s)	8.9	1.6	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utilization	19.3%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Base Year (2022)
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	13	6	8	21	0	1	0	7	0	0	0
Future Volume (vph)	0	13	6	8	21	0	1	0	7	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.955						0.880				
Flt Protected					0.986			0.994				
Satd. Flow (prot)	0	1254	0	0	1748	0	0	1539	0	0	1900	0
Flt Permitted					0.986			0.994				
Satd. Flow (perm)	0	1254	0	0	1748	0	0	1539	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		88.4			162.4			207.7			36.3	
Travel Time (s)		6.4			11.7			15.0			2.6	
Confl. Peds. (#/hr)	2		4	4		2	2		1	1		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	17%	100%	0%	10%	0%	0%	0%	9%	0%	0%	0%
Adj. Flow (vph)	0	14	7	9	23	0	1	0	8	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	21	0	0	32	0	0	9	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.1% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Base Year (2022)
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	13	6	8	21	0	1	0	7	0	0	0
Future Volume (Veh/h)	0	13	6	8	21	0	1	0	7	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	14	7	9	23	0	1	0	8	0	0	0
Pedestrians		2			1			4			2	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	25			25			64	64	22	70	68	27
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	25			25			64	64	22	70	68	27
tC, single (s)	4.1			4.1			7.1	6.5	6.3	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.4	3.5	4.0	3.3
p0 queue free %	100			99			100	100	99	100	100	100
cM capacity (veh/h)	1600			1597			922	821	1030	911	818	1051

Direction, Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	21	32	9	0
Volume Left	0	9	1	0
Volume Right	7	0	8	0
cSH	1600	1597	1017	1700
Volume to Capacity	0.00	0.01	0.01	0.00
Queue Length 95th (m)	0.0	0.1	0.2	0.0
Control Delay (s)	0.0	2.1	8.6	0.0
Lane LOS		A	A	A
Approach Delay (s)	0.0	2.1	8.6	0.0
Approach LOS		A	A	

Intersection Summary	
Average Delay	2.3
Intersection Capacity Utilization	19.1% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Base Year (2022)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	14	10	7	15	0	9	0	5	0	0	0
Future Volume (vph)	0	14	10	7	15	0	9	0	5	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.943						0.955				
Flt Protected					0.984			0.968				
Satd. Flow (prot)	0	1456	0	0	1669	0	0	1013	0	0	1900	0
Flt Permitted					0.984			0.968				
Satd. Flow (perm)	0	1456	0	0	1669	0	0	1013	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	15	11	8	16	0	10	0	5	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	26	0	0	24	0	0	15	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Base Year (2022)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	14	10	7	15	0	9	0	5	0	0	0
Future Volume (Veh/h)	0	14	10	7	15	0	9	0	5	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	15	11	8	16	0	10	0	5	0	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	21			35			62	66	34	66	72	22
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	21			35			62	66	34	66	72	22
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			99			99	100	99	100	100	100
cM capacity (veh/h)	1601			1577			790	814	809	907	809	1056
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	26	24	15	0								
Volume Left	0	8	10	0								
Volume Right	11	0	5	0								
cSH	1601	1577	796	1700								
Volume to Capacity	0.00	0.01	0.02	0.00								
Queue Length 95th (m)	0.0	0.1	0.4	0.0								
Control Delay (s)	0.0	2.5	9.6	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	2.5	9.6	0.0								
Approach LOS			A	A								

Intersection Summary	
Average Delay	3.1
Intersection Capacity Utilization	19.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Base Year (2022)
PM Peak Hour

	↙	↖	↑	↗	↘	↓
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		T			T
Traffic Volume (vph)	39	126	78	20	150	111
Future Volume (vph)	39	126	78	20	150	111
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.897		0.972			
Flt Protected	0.988					0.972
Satd. Flow (prot)	1635	0	1828	0	0	1808
Flt Permitted	0.988					0.972
Satd. Flow (perm)	1635	0	1828	0	0	1808
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				5	5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	3%	0%	5%	3%	1%
Adj. Flow (vph)	42	137	85	22	163	121
Shared Lane Traffic (%)						
Lane Group Flow (vph)	179	0	107	0	0	284
Sign Control	Stop		Free			Free

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	37.4%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Base Year (2022)
PM Peak Hour

	↙	↖	↑	↗	↘	↓
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		T			T
Traffic Volume (veh/h)	39	126	78	20	150	111
Future Volume (Veh/h)	39	126	78	20	150	111
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	42	137	85	22	163	121
Pedestrians	5					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	548	101			112	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	548	101			112	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	90	86			89	
cM capacity (veh/h)	439	948			1465	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	179	107	284
Volume Left	42	0	163
Volume Right	137	22	0
eSH	745	1700	1465
Volume to Capacity	0.24	0.06	0.11
Queue Length 95th (m)	7.1	0.0	2.8
Control Delay (s)	11.4	0.0	4.9
Lane LOS	B		A
Approach Delay (s)	11.4	0.0	4.9
Approach LOS	B		

Intersection Summary			
Average Delay		6.0	
Intersection Capacity Utilization	37.4%	ICU Level of Service	A
Analysis Period (min)	15		

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Base Year (2022)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	9	118	6	3	97	31	1	31	12	34	34	11
Future Volume (vph)	9	118	6	3	97	31	1	31	12	34	34	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.993			0.968			0.963			0.981	
Flt Protected		0.997			0.999			0.999			0.979	
Satd. Flow (prot)	0	1818	0	0	1811	0	0	1591	0	0	1574	0
Flt Permitted		0.997			0.999			0.999			0.979	
Satd. Flow (perm)	0	1818	0	0	1811	0	0	1591	0	0	1574	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	1		15	15		1			22	22		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	17%	0%	2%	0%	0%	21%	0%	6%	31%	0%
Adj. Flow (vph)	10	128	7	3	105	34	1	34	13	37	37	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	145	0	0	142	0	0	48	0	0	86	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	29.7%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Base Year (2022)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Sign Control		Stop			Stop			Stop			Stop		
Traffic Volume (vph)	9	118	6	3	97	31	1	31	12	34	34	11	
Future Volume (vph)	9	118	6	3	97	31	1	31	12	34	34	11	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	10	128	7	3	105	34	1	34	13	37	37	12	
Direction, Lane #													
	EB 1	WB 1	NB 1	SB 1									
Volume Total (vph)	145	142	48	86									
Volume Left (vph)	10	3	1	37									
Volume Right (vph)	7	34	13	12									
Hadj (s)	0.04	-0.11	0.09	0.27									
Departure Headway (s)	4.4	4.3	4.8	4.9									
Degree Utilization, x	0.18	0.17	0.06	0.12									
Capacity (veh/h)	781	798	703	686									
Control Delay (s)	8.4	8.2	8.1	8.5									
Approach Delay (s)	8.4	8.2	8.1	8.5									
Approach LOS	A	A	A	A									
Intersection Summary													
Delay				8.3									
Level of Service				A									
Intersection Capacity Utilization	29.7%			ICU Level of Service						A			
Analysis Period (min)				15									

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Base Year (2022)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	17	196	0	0	241	26	0	0	0	33	0	11
Future Volume (vph)	17	196	0	0	241	26	0	0	0	33	0	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.987						0.966	
Flt Protected		0.996									0.964	
Satd. Flow (prot)	0	1866	0	0	1754	0	0	1900	0	0	1369	0
Flt Permitted		0.996									0.964	
Satd. Flow (perm)	0	1866	0	0	1754	0	0	1900	0	0	1369	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)			54	54								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	1%	0%	0%	5%	25%	0%	0%	0%	39%	0%	0%
Adj. Flow (vph)	18	213	0	0	262	28	0	0	0	36	0	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	231	0	0	290	0	0	0	0	48	0	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	34.4%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

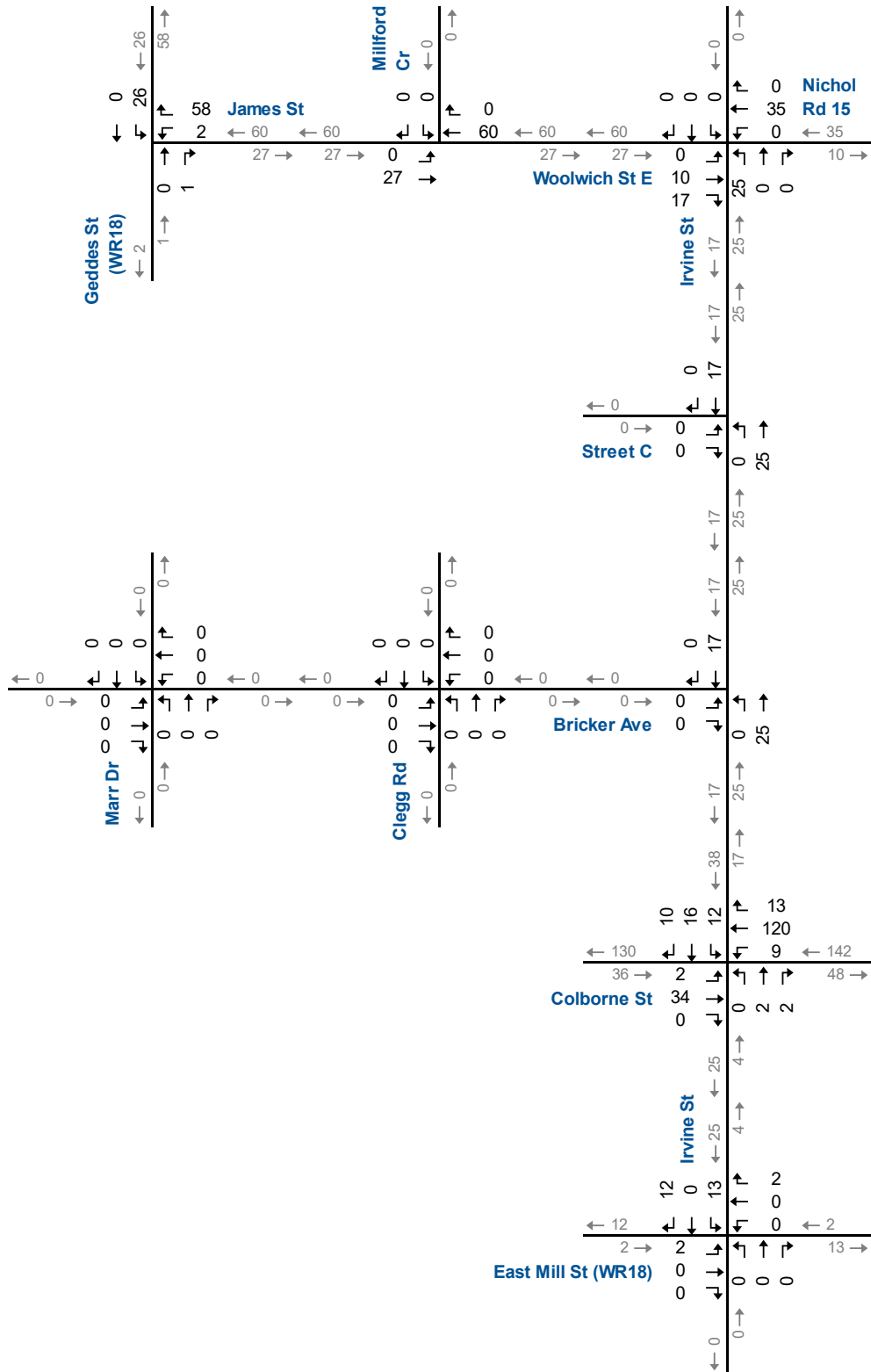
Base Year (2022)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	17	196	0	0	241	26	0	0	0	33	0	11
Future Volume (Veh/h)	17	196	0	0	241	26	0	0	0	33	0	11
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	18	213	0	0	262	28	0	0	0	36	0	12
Pedestrians											54	
Lane Width (m)											3.6	
Walking Speed (m/s)											1.2	
Percent Blockage											5	
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	290			267			591	593	267	525	579	276
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	290			267			591	593	267	525	579	276
tC, single (s)	4.2			4.1			7.1	6.5	6.2	7.5	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.5	4.0	3.3	3.9	4.0	3.3
p0 queue free %	99			100			100	100	100	91	100	98
cM capacity (veh/h)	1249			1250			378	396	742	391	404	768
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	231	290	0	48								
Volume Left	18	0	0	36								
Volume Right	0	28	0	12								
eSH	1249	1250	1700	446								
Volume to Capacity	0.01	0.00	0.00	0.11								
Queue Length 95th (m)	0.3	0.0	0.0	2.7								
Control Delay (s)	0.7	0.0	0.0	14.0								
Lane LOS	A			A			B				B	
Approach Delay (s)	0.7	0.0	0.0	14.0								
Approach LOS				A			B					
Intersection Summary												
Average Delay				1.5								
Intersection Capacity Utilization	34.4%			ICU Level of Service			A					
Analysis Period (min)	15											

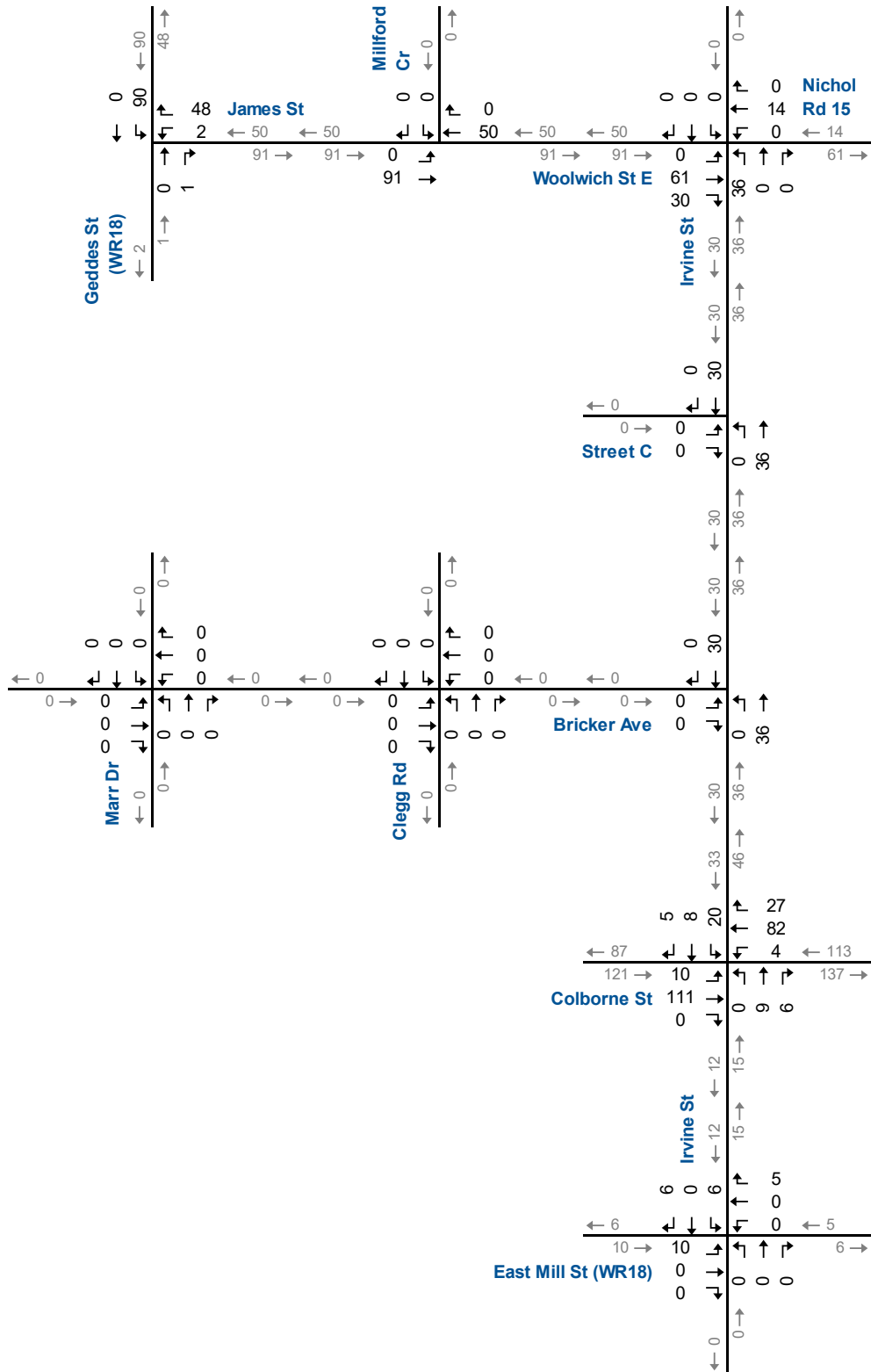
Appendix D

Background Developments Traffic Volumes





Background Developments Traffic Volumes AM Peak Hour



Background Developments Traffic Volumes PM Peak Hour

Appendix E1

2026 Background Operation Synchro Reports



Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Background (2026)
AM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	8	205	294	31	8	6
Future Volume (vph)	8	205	294	31	8	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.987		0.941	
Frt Protected		0.998			0.973	
Satd. Flow (prot)	0	1860	1802	0	1740	0
Frt Permitted		0.998			0.973	
Satd. Flow (perm)	0	1860	1802	0	1740	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	9	223	320	34	9	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	232	354	0	16	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	27.4%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Background (2026)
AM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	8	205	294	31	8	6
Future Volume (Veh/h)	8	205	294	31	8	6
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	223	320	34	9	7
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked					580	339
vC, conflicting volume	356				580	339
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	356				580	339
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	99
cM capacity (veh/h)	1212				476	707
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	232	354	16			
Volume Left	9	0	9			
Volume Right	0	34	7			
eSH	1212	1700	555			
Volume to Capacity	0.01	0.21	0.03			
Queue Length 95th (m)	0.2	0.0	0.7			
Control Delay (s)	0.4	0.0	11.7			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	11.7			
Approach LOS			B			
Intersection Summary						
Average Delay		0.5				
Intersection Capacity Utilization	27.4%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2026)

AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	4	183	25	28	278	4	42	7	39	3	1	6
Future Volume (vph)	4	183	25	28	278	4	42	7	39	3	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.984			0.998			0.941			0.914	
Fit Protected		0.999			0.996			0.977			0.987	
Satd. Flow (prot)	0	1820	0	0	1862	0	0	1747	0	0	1714	0
Fit Permitted		0.999			0.996			0.977			0.987	
Satd. Flow (perm)	0	1820	0	0	1862	0	0	1747	0	0	1714	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			450.4			484.2	
Travel Time (s)		13.7			48.8			32.4			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	199	27	30	302	4	46	8	42	3	1	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	230	0	0	336	0	0	96	0	0	11	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	45.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2026)

AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	4	183	25	28	278	4	42	7	39	3	1	6
Future Volume (Veh/h)	4	183	25	28	278	4	42	7	39	3	1	6
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	199	27	30	302	4	46	8	42	3	1	7
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	306			227			593	588	214	630	599	304
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	306			227			593	588	214	630	599	304
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			98			89	98	95	99	100	99
cM capacity (veh/h)	1266			1262			406	413	831	363	406	740
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	230	336	96	11								
Volume Left	4	30	46	3								
Volume Right	27	4	42	7								
eSH	1266	1262	524	545								
Volume to Capacity	0.00	0.02	0.18	0.02								
Queue Length 95th (m)	0.1	0.6	5.0	0.5								
Control Delay (s)	0.2	0.9	13.4	11.7								
Lane LOS	A	A	B	B								
Approach Delay (s)	0.2	0.9	13.4	11.7								
Approach LOS			B	B								

Intersection Summary

Average Delay	2.6
Intersection Capacity Utilization	45.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Background (2026)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	19	20	11	68	48	6
Future Volume (vph)	19	20	11	68	48	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.931				0.984	
Flt Protected	0.976			0.993		
Satd. Flow (prot)	1559	0	0	1839	1816	0
Flt Permitted	0.976			0.993		
Satd. Flow (perm)	1559	0	0	1839	1816	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	450.4	
Travel Time (s)	11.7			20.9	32.4	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	21	22	12	74	52	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	43	0	0	86	59	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	20.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Background (2026)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	19	20	11	68	48	6
Future Volume (Veh/h)	19	20	11	68	48	6
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	22	12	74	52	7
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	156	58	62			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	156	58	62			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	97	98	99			
cM capacity (veh/h)	831	954	1550			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	43	86	59			
Volume Left	21	12	0			
Volume Right	22	0	7			
sSH	890	1550	1700			
Volume to Capacity	0.05	0.01	0.03			
Queue Length 95th (m)	1.2	0.2	0.0			
Control Delay (s)	9.3	1.1	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.3	1.1	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			2.6			
Intersection Capacity Utilization	20.9%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Background (2026)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	22	1	1	15	0	4	0	17	0	0	0
Future Volume (vph)	0	22	1	1	15	0	4	0	17	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.995						0.890				
Fit Protected					0.997			0.991				
Satd. Flow (prot)	0	1571	0	0	1731	0	0	1561	0	0	1900	0
Fit Permitted					0.997			0.991				
Satd. Flow (perm)	0	1571	0	0	1731	0	0	1561	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		88.4			162.4			207.7			36.3	
Travel Time (s)		6.4			11.7			15.0			2.6	
Confl. Peds. (#/hr)	2		4	4		2	2		1	1		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	17%	100%	0%	10%	0%	0%	0%	9%	0%	0%	0%
Adj. Flow (vph)	0	24	1	1	16	0	4	0	18	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	25	0	0	17	0	0	22	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	15.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Background (2026)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	22	1	1	15	0	4	0	17	0	0	0
Future Volume (Veh/h)	0	22	1	1	15	0	4	0	17	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	24	1	1	16	0	4	0	18	0	0	0
Pedestrians		2			1			4			2	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	18			29			48	48	30	64	49	20
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	18			29			48	48	30	64	49	20
tC, single (s)	4.1			4.1			7.1	6.5	6.3	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.4	3.5	4.0	3.3
p0 queue free %	100			100			100	100	98	100	100	100
cM capacity (veh/h)	1609			1592			948	842	1021	913	842	1060

Direction, Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	25	17	22	0
Volume Left	0	1	4	0
Volume Right	1	0	18	0
sSH	1609	1592	1007	1700
Volume to Capacity	0.00	0.00	0.02	0.00
Queue Length 95th (m)	0.0	0.0	0.5	0.0
Control Delay (s)	0.0	0.4	8.7	0.0
Lane LOS	A	A	A	A
Approach Delay (s)	0.0	0.4	8.7	0.0
Approach LOS		A	A	

Intersection Summary	
Average Delay	3.1
Intersection Capacity Utilization	15.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Background (2026)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	9	3	4	15	0	8	0	14	0	0	0
Future Volume (vph)	0	9	3	4	15	0	8	0	14	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.969						0.916				
Flt Protected					0.990			0.982				
Satd. Flow (prot)	0	1408	0	0	1644	0	0	924	0	0	1900	0
Flt Permitted					0.990			0.982				
Satd. Flow (perm)	0	1408	0	0	1644	0	0	924	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	10	3	4	16	0	9	0	15	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	20	0	0	24	0	0	0	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	17.2%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Background (2026)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↔			↔			↔				↔	
Traffic Volume (veh/h)	0	9	3	4	15	0	8	0	14	0	0	0	
Future Volume (Veh/h)	0	9	3	4	15	0	8	0	14	0	0	0	
Sign Control	Free			Free			Stop			Stop			
Grade	0%			0%			0%			0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	0	10	3	4	16	0	9	0	15	0	0	0	
Pedestrians		1			4			9			5		
Lane Width (m)	3.6			3.6			3.6			3.6			
Walking Speed (m/s)	1.2			1.2			1.2			1.2			
Percent Blockage	0			0			1			0			
Right turn flare (veh)													
Median type	None			None									
Median storage (veh)													
Upstream signal (m)													
pX, platoon unblocked													
vC, conflicting volume	21			22			46	50	24	60	51	22	
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	21			22			46	50	24	60	51	22	
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2	
tC, 2 stage (s)													
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3	
p0 queue free %	100			100			99	100	98	100	100	100	
cM capacity (veh/h)	1601			1595			814	834	819	907	832	1056	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total	13	20	24	0									
Volume Left	0	4	9	0									
Volume Right	3	0	15	0									
eSH	1601	1595	817	1700									
Volume to Capacity	0.00	0.00	0.03	0.00									
Queue Length 95th (m)	0.0	0.1	0.7	0.0									
Control Delay (s)	0.0	1.5	9.5	0.0									
Lane LOS		A	A	A									
Approach Delay (s)	0.0	1.5	9.5	0.0									
Approach LOS		A	A										
Intersection Summary													
Average Delay				4.5									
Intersection Capacity Utilization	17.2%			ICU Level of Service						A			
Analysis Period (min)	15												

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Background (2026)
AM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	35	192	98	52	176	87
Future Volume (vph)	35	192	98	52	176	87
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.886		0.953			
Flt Protected	0.992					0.968
Satd. Flow (prot)	1535	0	1740	0	0	1703
Flt Permitted	0.992					0.968
Satd. Flow (perm)	1535	0	1740	0	0	1703
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	8%	3%	6%	7%	10%
Adj. Flow (vph)	38	209	107	57	191	95
Shared Lane Traffic (%)						
Lane Group Flow (vph)	247	0	164	0	0	286
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	47.2%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Background (2026)
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	35	192	98	52	176	87
Future Volume (Veh/h)	35	192	98	52	176	87
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	209	107	57	191	95
Pedestrians	4					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	616	140			168	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	616	140			168	
tC, single (s)	6.5	6.3			4.2	
tC, 2 stage (s)						
tF (s)	3.6	3.4			2.3	
p0 queue free %	90	77			86	
cM capacity (veh/h)	375	890			1375	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	247	164	286			
Volume Left	38	0	191			
Volume Right	209	57	0			
eSH	734	1700	1375			
Volume to Capacity	0.34	0.10	0.14			
Queue Length 95th (m)	11.3	0.0	3.7			
Control Delay (s)	12.4	0.0	5.8			
Lane LOS	B		A			
Approach Delay (s)	12.4	0.0	5.8			
Approach LOS	B					
Intersection Summary						
Average Delay			6.7			
Intersection Capacity Utilization	47.2%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Background (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	11	105	8	13	250	42	6	34	12	53	82	23
Future Volume (vph)	11	105	8	13	250	42	6	34	12	53	82	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.991			0.981			0.969			0.980	
Fit Protected		0.996			0.998			0.994			0.983	
Satd. Flow (prot)	0	1738	0	0	1779	0	0	1407	0	0	1624	0
Fit Permitted		0.996			0.998			0.994			0.983	
Satd. Flow (perm)	0	1738	0	0	1779	0	0	1407	0	0	1624	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	57		1	1		57	4		50	50		4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	6%	20%	33%	3%	5%	50%	32%	14%	4%	22%	0%
Adj. Flow (vph)	12	114	9	14	272	46	7	37	13	58	89	25
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	135	0	0	332	0	0	57	0	0	172	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	41.4%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Background (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	11	105	8	13	250	42	6	34	12	53	82	23
Future Volume (vph)	11	105	8	13	250	42	6	34	12	53	82	23
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	114	9	14	272	46	7	37	13	58	89	25
Direction, Lane #												
	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	135	332	57	172								
Volume Left (vph)	12	14	7	58								
Volume Right (vph)	9	46	13	25								
Hadj (s)	0.11	0.00	0.40	0.20								
Departure Headway (s)	5.1	4.7	5.8	5.4								
Degree Utilization, x	0.19	0.44	0.09	0.26								
Capacity (veh/h)	655	725	561	617								
Control Delay (s)	9.3	11.4	9.3	10.2								
Approach Delay (s)	9.3	11.4	9.3	10.2								
Approach LOS	A	B	A	B								
Intersection Summary												
Delay	10.5											
Level of Service	B											
Intersection Capacity Utilization	41.4%				ICU Level of Service				A			
Analysis Period (min)	15											

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Background (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	8	342	1	1	234	34	1	3	0	77	0	27
Future Volume (vph)	8	342	1	1	234	34	1	3	0	77	0	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.983							0.965	
Flt Protected		0.999						0.988			0.964	
Satd. Flow (prot)	0	1827	0	0	1805	0	0	1877	0	0	1767	0
Flt Permitted		0.999						0.988			0.964	
Satd. Flow (perm)	0	1827	0	0	1805	0	0	1877	0	0	1767	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)	42					42						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	9	372	1	1	254	37	1	3	0	84	0	29
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	382	0	0	292	0	0	4	0	0	113	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	43.1%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Background (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Traffic Volume (veh/h)	8	342	1	1	234	34	1	3	0	77	0	27	
Future Volume (Veh/h)	8	342	1	1	234	34	1	3	0	77	0	27	
Sign Control	Free			Free			Stop			Stop			
Grade	0%			0%			0%			0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	9	372	1	1	254	37	1	3	0	84	0	29	
Pedestrians												42	
Lane Width (m)												3.6	
Walking Speed (m/s)												1.2	
Percent Blockage												4	
Right turn flare (veh)													
Median type	None			None									
Median storage (veh)													
Upstream signal (m)													
pX, platoon unblocked													
vC, conflicting volume	333			373			694	726	372	708	708	314	
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	333			373			694	726	372	708	708	314	
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2	
tC, 2 stage (s)													
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3	
p0 queue free %	99			100			100	99	100	74	100	96	
cM capacity (veh/h)	1194			1197			334	339	678	326	347	705	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total	382	292	4	113									
Volume Left	9	1	1	84									
Volume Right	1	37	0	29									
eSH	1194	1197	337	378									
Volume to Capacity	0.01	0.00	0.01	0.30									
Queue Length 95th (m)	0.2	0.0	0.3	9.4									
Control Delay (s)	0.3	0.0	15.8	18.5									
Lane LOS	A	A	C	C									
Approach Delay (s)	0.3	0.0	15.8	18.5									
Approach LOS			C	C									
Intersection Summary													
Average Delay	2.9												
Intersection Capacity Utilization	43.1%				ICU Level of Service				A				
Analysis Period (min)	15												

Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Background (2026)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	4	295	209	14	8	2
Future Volume (vph)	4	295	209	14	8	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.992		0.975	
Flt Protected		0.999			0.961	
Satd. Flow (prot)	0	1861	1811	0	1780	0
Flt Permitted		0.999			0.961	
Satd. Flow (perm)	0	1861	1811	0	1780	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	4	321	227	15	9	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	325	242	0	11	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	28.7%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Background (2026)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	4	295	209	14	8	2
Future Volume (Veh/h)	4	295	209	14	8	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	321	227	15	9	2
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	244				566	236
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	244				566	236
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				98	100
cM capacity (veh/h)	1332				487	806
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	325	242	11			
Volume Left	4	0	9			
Volume Right	0	15	2			
eSH	1332	1700	525			
Volume to Capacity	0.00	0.14	0.02			
Queue Length 95th (m)	0.1	0.0	0.5			
Control Delay (s)	0.1	0.0	12.0			
Lane LOS	A		B			
Approach Delay (s)	0.1	0.0	12.0			
Approach LOS			B			
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization	28.7%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2026)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕				↕	
Traffic Volume (vph)	3	252	49	26	167	3	49	7	34	3	2	7	
Future Volume (vph)	3	252	49	26	167	3	49	7	34	3	2	7	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor													
Frt		0.978			0.998			0.949			0.917		
Flt Protected					0.993			0.974			0.989		
Satd. Flow (prot)	0	1813	0	0	1844	0	0	1756	0	0	1723	0	
Flt Permitted					0.993			0.974			0.989		
Satd. Flow (perm)	0	1813	0	0	1844	0	0	1756	0	0	1723	0	
Link Speed (k/h)		40			40			50			50		
Link Distance (m)		152.7			542.4			450.4			484.2		
Travel Time (s)		13.7			48.8			32.4			34.9		
Confl. Peds. (#/hr)			1	1									
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	3	274	53	28	182	3	53	8	37	3	2	8	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	330	0	0	213	0	0	98	0	0	13	0	
Sign Control		Free			Free			Stop			Stop		

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	43.4%
ICU Level of Service	A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2026)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕				↕	
Traffic Volume (veh/h)	3	252	49	26	167	3	49	7	34	3	2	7	
Future Volume (Veh/h)	3	252	49	26	167	3	49	7	34	3	2	7	
Sign Control		Free			Free			Stop			Stop		
Grade		0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	3	274	53	28	182	3	53	8	37	3	2	8	
Pedestrians								1					
Lane Width (m)								3.6					
Walking Speed (m/s)								1.2					
Percent Blockage								0					
Right turn flare (veh)													
Median type		None			None								
Median storage (veh)													
Upstream signal (m)													
pX, platoon unblocked													
vC, conflicting volume	185			328			556	548	302	587	574	184	
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	185			328			556	548	302	587	574	184	
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2	
tC, 2 stage (s)													
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3	
p0 queue free %	100			98			88	98	95	99	100	99	
cM capacity (veh/h)	1402			1156			430	434	742	389	420	864	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total	330	213	98	13									
Volume Left	3	28	53	3									
Volume Right	53	3	37	8									
eSH	1402	1156	511	598									
Volume to Capacity	0.00	0.02	0.19	0.02									
Queue Length 95th (m)	0.0	0.6	5.3	0.5									
Control Delay (s)	0.1	1.3	13.7	11.1									
Lane LOS	A	A	B	B									
Approach Delay (s)	0.1	1.3	13.7	11.1									
Approach LOS			B	B									

Intersection Summary

Average Delay	2.7
Intersection Capacity Utilization	43.4%
ICU Level of Service	A
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Background (2026)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	11	11	11	79	55	21
Future Volume (vph)	11	11	11	79	55	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932				0.963	
Flt Protected	0.976			0.994		
Satd. Flow (prot)	1564	0	0	1840	1711	0
Flt Permitted	0.976			0.994		
Satd. Flow (perm)	1564	0	0	1840	1711	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	450.4	
Travel Time (s)	11.7			20.9	32.4	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	12	12	12	86	60	23
Shared Lane Traffic (%)						
Lane Group Flow (vph)	24	0	0	98	83	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	21.4%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Background (2026)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	11	11	11	79	55	21
Future Volume (Veh/h)	11	11	11	79	55	21
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	12	12	86	60	23
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	184	74	86			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	184	74	86			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	99	99	99			
cM capacity (veh/h)	801	934	1519			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	24	98	83			
Volume Left	12	12	0			
Volume Right	12	0	23			
sSH	863	1519	1700			
Volume to Capacity	0.03	0.01	0.05			
Queue Length 95th (m)	0.7	0.2	0.0			
Control Delay (s)	9.3	1.0	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.3	1.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			1.5			
Intersection Capacity Utilization	21.4%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Background (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	14	7	9	23	0	1	0	8	0	0	0
Future Volume (vph)	0	14	7	9	23	0	1	0	8	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.953						0.878				
Flt Protected					0.986			0.995				
Satd. Flow (prot)	0	1241	0	0	1749	0	0	1535	0	0	1900	0
Flt Permitted					0.986			0.995				
Satd. Flow (perm)	0	1241	0	0	1749	0	0	1535	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		88.4			162.4			207.7			36.3	
Travel Time (s)		6.4			11.7			15.0			2.6	
Confl. Peds. (#/hr)	2		4	4		2	2		1	1		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	17%	100%	0%	10%	0%	0%	0%	9%	0%	0%	0%
Adj. Flow (vph)	0	15	8	10	25	0	1	0	9	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	23	0	0	35	0	0	10	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.3% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Background (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	14	7	9	23	0	1	0	8	0	0	0
Future Volume (Veh/h)	0	14	7	9	23	0	1	0	8	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	15	8	10	25	0	1	0	9	0	0	0
Pedestrians		2			1			4			2	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked							70	70	24	76	74	29
vC, conflicting volume	27			27			70	70	24	76	74	29
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	27			27			70	70	24	76	74	29
tC, single (s)	4.1			4.1			7.1	6.5	6.3	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.4	3.5	4.0	3.3
p0 queue free %	100			99			100	100	99	100	100	100
cM capacity (veh/h)	1597			1595			914	815	1028	900	811	1048
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	23	35	10	0								
Volume Left	0	10	1	0								
Volume Right	8	0	9	0								
cSH	1597	1595	1016	1700								
Volume to Capacity	0.00	0.01	0.01	0.00								
Queue Length 95th (m)	0.0	0.1	0.2	0.0								
Control Delay (s)	0.0	2.1	8.6	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	2.1	8.6	0.0								
Approach LOS		A	A	A								

Intersection Summary	
Average Delay	2.3
Intersection Capacity Utilization	19.3% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Background (2026)
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔				↔			↔	
Traffic Volume (vph)	0	15	11	8	17	0	10	0	6	0	0	0
Future Volume (vph)	0	15	11	8	17	0	10	0	6	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.942						0.947				
Flt Protected					0.984			0.970				
Satd. Flow (prot)	0	1457	0	0	1669	0	0	994	0	0	1900	0
Flt Permitted					0.984			0.970				
Satd. Flow (perm)	0	1457	0	0	1669	0	0	994	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	16	12	9	18	0	11	0	7	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	28	0	0	27	0	0	18	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	20.0% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Background (2026)
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔				↔			↔	
Traffic Volume (veh/h)	0	15	11	8	17	0	10	0	6	0	0	0
Future Volume (Veh/h)	0	15	11	8	17	0	10	0	6	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	16	12	9	18	0	11	0	7	0	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	23			37			68	72	35	74	78	24
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	23			37			68	72	35	74	78	24
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			99			99	100	99	100	100	100
cM capacity (veh/h)	1599			1575			783	808	807	894	802	1053
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	28	27	18	0								
Volume Left	0	9	11	0								
Volume Right	12	0	7	0								
cSH	1599	1575	792	1700								
Volume to Capacity	0.00	0.01	0.02	0.00								
Queue Length 95th (m)	0.0	0.1	0.5	0.0								
Control Delay (s)	0.0	2.5	9.7	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	2.5	9.7	0.0								
Approach LOS			A	A								

Intersection Summary	
Average Delay	3.3
Intersection Capacity Utilization	20.0% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Background (2026)
PM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	45	187	86	23	256	123
Future Volume (vph)	45	187	86	23	256	123
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.891	0.971				
Flt Protected	0.990					0.967
Satd. Flow (prot)	1627	0	1826	0	0	1795
Flt Permitted	0.990					0.967
Satd. Flow (perm)	1627	0	1826	0	0	1795
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				5	5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	3%	0%	5%	3%	1%
Adj. Flow (vph)	49	203	93	25	278	134
Shared Lane Traffic (%)						
Lane Group Flow (vph)	252	0	118	0	0	412
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	48.0%				ICU Level of Service A	
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Background (2026)
PM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	45	187	86	23	256	123
Future Volume (Veh/h)	45	187	86	23	256	123
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	203	93	25	278	134
Pedestrians	5					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	800	110			123	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	800	110			123	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	83	78			81	
cM capacity (veh/h)	284	936			1452	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	252	118	412			
Volume Left	49	0	278			
Volume Right	203	25	0			
eSH	647	1700	1452			
Volume to Capacity	0.39	0.07	0.19			
Queue Length 95th (m)	14.0	0.0	5.4			
Control Delay (s)	14.1	0.0	6.0			
Lane LOS	B		A			
Approach Delay (s)	14.1	0.0	6.0			
Approach LOS	B					
Intersection Summary						
Average Delay			7.7			
Intersection Capacity Utilization	48.0%			ICU Level of Service	A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Background (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	20	241	7	7	189	61	1	43	19	58	46	17
Future Volume (vph)	20	241	7	7	189	61	1	43	19	58	46	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.996			0.968			0.959			0.981	
Flt Protected		0.996			0.999			0.999			0.977	
Satd. Flow (prot)	0	1827	0	0	1811	0	0	1592	0	0	1587	0
Flt Permitted		0.996			0.999			0.999			0.977	
Satd. Flow (perm)	0	1827	0	0	1811	0	0	1592	0	0	1587	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	1		15	15		1			22	22		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	17%	0%	2%	0%	0%	21%	0%	6%	31%	0%
Adj. Flow (vph)	22	262	8	8	205	66	1	47	21	63	50	18
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	292	0	0	279	0	0	69	0	0	131	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	42.6%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Background (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Sign Control		Stop			Stop			Stop			Stop		
Traffic Volume (vph)	20	241	7	7	189	61	1	43	19	58	46	17	
Future Volume (vph)	20	241	7	7	189	61	1	43	19	58	46	17	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	22	262	8	8	205	66	1	47	21	63	50	18	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total (vph)	292	279	69	131									
Volume Left (vph)	22	8	1	63									
Volume Right (vph)	8	66	21	18									
Hadj (s)	0.05	-0.11	0.06	0.26									
Departure Headway (s)	4.9	4.8	5.6	5.7									
Degree Utilization, x	0.40	0.37	0.11	0.21									
Capacity (veh/h)	697	717	557	570									
Control Delay (s)	11.1	10.5	9.3	10.2									
Approach Delay (s)	11.1	10.5	9.3	10.2									
Approach LOS	B	B	A	B									
Intersection Summary													
Delay	10.6												
Level of Service	B												
Intersection Capacity Utilization	42.6%				ICU Level of Service				A				
Analysis Period (min)	15												

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Background (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	29	216	0	0	266	34	0	0	0	42	0	18
Future Volume (vph)	29	216	0	0	266	34	0	0	0	42	0	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.985						0.959	
Flt Protected		0.994									0.966	
Satd. Flow (prot)	0	1859	0	0	1745	0	0	1900	0	0	1384	0
Flt Permitted		0.994									0.966	
Satd. Flow (perm)	0	1859	0	0	1745	0	0	1900	0	0	1384	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)			54	54								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	1%	0%	0%	5%	25%	0%	0%	0%	39%	0%	0%
Adj. Flow (vph)	32	235	0	0	289	37	0	0	0	46	0	20
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	267	0	0	326	0	0	0	0	66	0	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	42.8%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Background (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Traffic Volume (veh/h)	29	216	0	0	266	34	0	0	0	42	0	18	
Future Volume (Veh/h)	29	216	0	0	266	34	0	0	0	42	0	18	
Sign Control	Free			Free			Stop			Stop			
Grade	0%			0%			0%			0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	32	235	0	0	289	37	0	0	0	46	0	20	
Pedestrians												54	
Lane Width (m)												3.6	
Walking Speed (m/s)												1.2	
Percent Blockage												5	
Right turn flare (veh)													
Median type	None			None									
Median storage (veh)													
Upstream signal (m)													
pX, platoon unblocked													
vC, conflicting volume	326			289			680	679	289	606	660	308	
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	326			289			680	679	289	606	660	308	
tC, single (s)	4.2			4.1			7.1	6.5	6.2	7.5	6.5	6.2	
tC, 2 stage (s)													
tF (s)	2.3			2.2			3.5	4.0	3.3	3.9	4.0	3.3	
p0 queue free %	97			100			100	100	100	86	100	97	
cM capacity (veh/h)	1211			1227			323	350	721	340	358	737	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total	267	326	0	66									
Volume Left	32	0	0	46									
Volume Right	0	37	0	20									
eSH	1211	1227	1700	407									
Volume to Capacity	0.03	0.00	0.00	0.16									
Queue Length 95th (m)	0.6	0.0	0.0	4.4									
Control Delay (s)	1.2	0.0	0.0	15.6									
Lane LOS	A		A	C									
Approach Delay (s)	1.2	0.0	0.0	15.6									
Approach LOS			A	C									
Intersection Summary													
Average Delay												2.0	
Intersection Capacity Utilization	42.8%				ICU Level of Service				A				
Analysis Period (min)												15	


Appendix E2

2031 Background Operation Synchro Reports



Lanes, Volumes, Timings
101: Woolwich St & Milford Cres


Background (2031)
AM Peak Hour



Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	9	223	318	34	9	6
Future Volume (vph)	9	223	318	34	9	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.987		0.944	
Flt Protected		0.998			0.971	
Satd. Flow (prot)	0	1860	1801	0	1742	0
Flt Permitted		0.998			0.971	
Satd. Flow (perm)	0	1860	1801	0	1742	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	10	242	346	37	10	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	252	383	0	17	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	29.0%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Background (2031)
AM Peak Hour



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	9	223	318	34	9	6
Future Volume (Veh/h)	9	223	318	34	9	6
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	242	346	37	10	7
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	385				628	366
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	385				628	366
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	99
cM capacity (veh/h)	1183				445	682
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	252	383	17			
Volume Left	10	0	10			
Volume Right	0	37	7			
eSH	1183	1700	519			
Volume to Capacity	0.01	0.23	0.03			
Queue Length 95th (m)	0.2	0.0	0.8			
Control Delay (s)	0.4	0.0	12.2			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	12.2			
Approach LOS			B			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization	29.0%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2031)

AM Peak Hour

	↖	→	↘	↙	←	↖	↘	↙	↗	↘	↙	↗	↘	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		↕			↕			↕			↕			
Traffic Volume (vph)	5	201	26	30	303	5	43	7	43	4	1	6		
Future Volume (vph)	5	201	26	30	303	5	43	7	43	4	1	6		
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		
Ped Bike Factor														
Frt		0.985			0.998			0.938			0.921			
Flt Protected		0.999			0.996			0.977			0.984			
Satd. Flow (prot)	0	1822	0	0	1862	0	0	1741	0	0	1722	0		
Flt Permitted		0.999			0.996			0.977			0.984			
Satd. Flow (perm)	0	1822	0	0	1862	0	0	1741	0	0	1722	0		
Link Speed (k/h)		40			40			50			50			
Link Distance (m)		152.7			542.4			450.4			484.2			
Travel Time (s)		13.7			48.8			32.4			34.9			
Confl. Peds. (#/hr)			1	1										
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%		
Adj. Flow (vph)	5	218	28	33	329	5	47	8	47	4	1	7		
Shared Lane Traffic (%)														
Lane Group Flow (vph)	0	251	0	0	367	0	0	102	0	0	12	0		
Sign Control		Free			Free			Stop			Stop			

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	46.4% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2031)

AM Peak Hour

	↖	→	↘	↙	←	↖	↘	↙	↗	↘	↙	↗	↘	↙
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		↕			↕			↕			↕			
Traffic Volume (veh/h)	5	201	26	30	303	5	43	7	43	4	1	6		
Future Volume (Veh/h)	5	201	26	30	303	5	43	7	43	4	1	6		
Sign Control		Free			Free			Stop			Stop			
Grade		0%			0%			0%			0%			
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	5	218	28	33	329	5	47	8	47	4	1	7		
Pedestrians								1						
Lane Width (m)								3.6						
Walking Speed (m/s)								1.2						
Percent Blockage								0						
Right turn flare (veh)														
Median type		None			None									
Median storage (veh)														
Upstream signal (m)														
pX, platoon unblocked														
vC, conflicting volume	334			247			648	643	233	690	654	332		
vC1, stage 1 conf vol														
vC2, stage 2 conf vol														
vCu, unblocked vol	334			247			648	643	233	690	654	332		
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2		
tC, 2 stage (s)														
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3		
p0 queue free %	100			97			87	98	94	99	100	99		
cM capacity (veh/h)	1237			1240			372	382	810	327	376	715		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1										
Volume Total	251	367	102	12										
Volume Left	5	33	47	4										
Volume Right	28	5	47	7										
cSH	1237	1240	497	486										
Volume to Capacity	0.00	0.03	0.21	0.02										
Queue Length 95th (m)	0.1	0.6	5.8	0.6										
Control Delay (s)	0.2	1.0	14.1	12.6										
Lane LOS	A	A	B	B										
Approach Delay (s)	0.2	1.0	14.1	12.6										
Approach LOS			B	B										

Intersection Summary	
Average Delay	2.7
Intersection Capacity Utilization	46.4% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Background (2031)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	21	22	12	73	51	6
Future Volume (vph)	21	22	12	73	51	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.931				0.985	
Flt Protected	0.976			0.993		
Satd. Flow (prot)	1559	0	0	1839	1820	0
Flt Permitted	0.976			0.993		
Satd. Flow (perm)	1559	0	0	1839	1820	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	450.4	
Travel Time (s)	11.7			20.9	32.4	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	23	24	13	79	55	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	47	0	0	92	62	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	21.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Background (2031)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	21	22	12	73	51	6
Future Volume (Veh/h)	21	22	12	73	51	6
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	23	24	13	79	55	7
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	166	62	65			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	166	62	65			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	97	97	99			
cM capacity (veh/h)	820	950	1546			

Direction, Lane #	EB 1	NB 1	SB 1
Volume Total	47	92	62
Volume Left	23	13	0
Volume Right	24	0	7
sSH	881	1546	1700
Volume to Capacity	0.05	0.01	0.04
Queue Length 95th (m)	1.3	0.2	0.0
Control Delay (s)	9.3	1.1	0.0
Lane LOS	A	A	
Approach Delay (s)	9.3	1.1	0.0
Approach LOS	A		

Intersection Summary			
Average Delay		2.7	
Intersection Capacity Utilization	21.2%	ICU Level of Service	A
Analysis Period (min)		15	

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Background (2031)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	24	1	1	17	0	5	0	18	0	0	0
Future Volume (vph)	0	24	1	1	17	0	5	0	18	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.995						0.892				
Flt Protected					0.997			0.990				
Satd. Flow (prot)	0	1574	0	0	1730	0	0	1565	0	0	1900	0
Flt Permitted					0.997			0.990				
Satd. Flow (perm)	0	1574	0	0	1730	0	0	1565	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		88.4			162.4			207.7			36.3	
Travel Time (s)		6.4			11.7			15.0			2.6	
Confl. Peds. (#/hr)	2		4	4		2	2		1	1		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	17%	100%	0%	10%	0%	0%	0%	9%	0%	0%	0%
Adj. Flow (vph)	0	26	1	1	18	0	5	0	20	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	27	0	0	19	0	0	25	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	15.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Background (2031)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	24	1	1	17	0	5	0	18	0	0	0
Future Volume (Veh/h)	0	24	1	1	17	0	5	0	18	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	26	1	1	18	0	5	0	20	0	0	0
Pedestrians		2			1			4			2	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	20			31			52	52	32	70	53	22
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	20			31			52	52	32	70	53	22
tC, single (s)	4.1			4.1			7.1	6.5	6.3	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.4	3.5	4.0	3.3
p0 queue free %	100			100			99	100	98	100	100	100
cM capacity (veh/h)	1607			1589			942	838	1018	903	837	1057

Direction, Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	27	19	25	0
Volume Left	0	1	5	0
Volume Right	1	0	20	0
sSH	1607	1589	1002	1700
Volume to Capacity	0.00	0.00	0.02	0.00
Queue Length 95th (m)	0.0	0.0	0.6	0.0
Control Delay (s)	0.0	0.4	8.7	0.0
Lane LOS		A	A	A
Approach Delay (s)	0.0	0.4	8.7	0.0
Approach LOS		A	A	

Intersection Summary	
Average Delay	3.2
Intersection Capacity Utilization	15.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Background (2031)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	10	4	5	17	0	9	0	16	0	0	0
Future Volume (vph)	0	10	4	5	17	0	9	0	16	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.964						0.915				
Fit Protected					0.989			0.982				
Satd. Flow (prot)	0	1416	0	0	1647	0	0	922	0	0	1900	0
Fit Permitted					0.989			0.982				
Satd. Flow (perm)	0	1416	0	0	1647	0	0	922	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	11	4	5	18	0	10	0	17	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	15	0	0	23	0	0	27	0	0	0	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	17.7%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Background (2031)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	10	4	5	17	0	9	0	16	0	0	0
Future Volume (Veh/h)	0	10	4	5	17	0	9	0	16	0	0	0
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	11	4	5	18	0	10	0	17	0	0	0
Pedestrians		1			4			9			5	
Lane Width (m)	3.6			3.6			3.6			3.6		
Walking Speed (m/s)	1.2			1.2			1.2			1.2		
Percent Blockage	0			0			1			0		
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	23			24			51	55	26	67	57	24
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	23			24			51	55	26	67	57	24
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			100			99	100	98	100	100	100
cM capacity (veh/h)	1599			1592			806	828	817	895	826	1053
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	15	23	27	0								
Volume Left	0	5	10	0								
Volume Right	4	0	17	0								
eSH	1599	1592	813	1700								
Volume to Capacity	0.00	0.00	0.03	0.00								
Queue Length 95th (m)	0.0	0.1	0.8	0.0								
Control Delay (s)	0.0	1.6	9.6	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	1.6	9.6	0.0								
Approach LOS		A	A									
Intersection Summary												
Average Delay				4.5								
Intersection Capacity Utilization	17.7%			ICU Level of Service				A				
Analysis Period (min)	15											

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Background (2031)
AM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	39	205	108	57	192	96
Future Volume (vph)	39	205	108	57	192	96
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.886		0.953			
Flt Protected	0.992					0.968
Satd. Flow (prot)	1535	0	1740	0	0	1703
Flt Permitted	0.992					0.968
Satd. Flow (perm)	1535	0	1740	0	0	1703
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	8%	3%	6%	7%	10%
Adj. Flow (vph)	42	223	117	62	209	104
Shared Lane Traffic (%)						
Lane Group Flow (vph)	265	0	179	0	0	313
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	50.3%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Background (2031)
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	39	205	108	57	192	96
Future Volume (Veh/h)	39	205	108	57	192	96
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	42	223	117	62	209	104
Pedestrians	4					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	674	152			183	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	674	152			183	
tC, single (s)	6.5	6.3			4.2	
tC, 2 stage (s)						
tF (s)	3.6	3.4			2.3	
p0 queue free %	88	75			85	
cM capacity (veh/h)	340	876			1358	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	265	179	313			
Volume Left	42	0	209			
Volume Right	223	62	0			
sSH	701	1700	1358			
Volume to Capacity	0.38	0.11	0.15			
Queue Length 95th (m)	13.4	0.0	4.1			
Control Delay (s)	13.2	0.0	5.9			
Lane LOS	B		A			
Approach Delay (s)	13.2	0.0	5.9			
Approach LOS	B					
Intersection Summary						
Average Delay			7.1			
Intersection Capacity Utilization	50.3%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Background (2031)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↗	↘	↓	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	12	112	9	14	264	45	6	37	13	57	89	25
Future Volume (vph)	12	112	9	14	264	45	6	37	13	57	89	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.991			0.981			0.969			0.980	
Flt Protected		0.996			0.998			0.994			0.984	
Satd. Flow (prot)	0	1737	0	0	1779	0	0	1408	0	0	1624	0
Flt Permitted		0.996			0.998			0.994			0.984	
Satd. Flow (perm)	0	1737	0	0	1779	0	0	1408	0	0	1624	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	57		1	1		57	4		50	50		4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	6%	20%	33%	3%	5%	50%	32%	14%	4%	22%	0%
Adj. Flow (vph)	13	122	10	15	287	49	7	40	14	62	97	27
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	145	0	0	351	0	0	61	0	0	186	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	43.2%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Background (2031)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↗	↘	↓	↖
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	12	112	9	14	264	45	6	37	13	57	89	25
Future Volume (vph)	12	112	9	14	264	45	6	37	13	57	89	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	13	122	10	15	287	49	7	40	14	62	97	27
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	145	351	61	186								
Volume Left (vph)	13	15	7	62								
Volume Right (vph)	10	49	14	27								
Hadj (s)	0.11	0.00	0.39	0.20								
Departure Headway (s)	5.2	4.8	5.9	5.5								
Degree Utilization, x	0.21	0.47	0.10	0.28								
Capacity (veh/h)	640	713	536	605								
Control Delay (s)	9.6	12.1	9.6	10.6								
Approach Delay (s)	9.6	12.1	9.6	10.6								
Approach LOS	A	B	A	B								
Intersection Summary												
Delay				11.0								
Level of Service				B								
Intersection Capacity Utilization				43.2%				ICU Level of Service				A
Analysis Period (min)				15								

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Background (2031)
AM Peak Hour

	↖	→	↗	↙	←	↖	↙	↑	↗	↘	↓	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	8	378	1	1	258	37	1	4	0	84	0	29
Future Volume (vph)	8	378	1	1	258	37	1	4	0	84	0	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.983						0.965	
Flt Protected		0.999						0.990			0.964	
Satd. Flow (prot)	0	1827	0	0	1805	0	0	1881	0	0	1767	0
Flt Permitted		0.999						0.990			0.964	
Satd. Flow (perm)	0	1827	0	0	1805	0	0	1881	0	0	1767	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)	42					42						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	9	411	1	1	280	40	1	4	0	91	0	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	421	0	0	321	0	0	5	0	0	123	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	45.6%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Background (2031)
AM Peak Hour

	↖	→	↗	↙	←	↖	↙	↑	↗	↘	↓	↖
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	8	378	1	1	258	37	1	4	0	84	0	29
Future Volume (Veh/h)	8	378	1	1	258	37	1	4	0	84	0	29
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	411	1	1	280	40	1	4	0	91	0	32
Pedestrians												42
Lane Width (m)												3.6
Walking Speed (m/s)												1.2
Percent Blockage												4
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	362			412			764	794	412	776	774	342
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	362			412			764	794	412	776	774	342
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			100			100	99	100	69	100	95
cM capacity (veh/h)	1165			1158			298	309	645	293	317	680
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	421	321	5	123								
Volume Left	9	1	1	91								
Volume Right	1	40	0	32								
eSH	1165	1158	307	344								
Volume to Capacity	0.01	0.00	0.02	0.36								
Queue Length 95th (m)	0.2	0.0	0.4	12.0								
Control Delay (s)	0.3	0.0	16.9	21.2								
Lane LOS	A	A	C	C								
Approach Delay (s)	0.3	0.0	16.9	21.2								
Approach LOS			C	C								
Intersection Summary												
Average Delay				3.2								
Intersection Capacity Utilization	45.6%			ICU Level of Service	A							
Analysis Period (min)	15											

Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Background (2031)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	5	317	226	16	9	2
Future Volume (vph)	5	317	226	16	9	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.991		0.977	
Flt Protected		0.999			0.960	
Satd. Flow (prot)	0	1861	1809	0	1782	0
Flt Permitted		0.999			0.960	
Satd. Flow (perm)	0	1861	1809	0	1782	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	5	345	246	17	10	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	350	263	0	12	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	30.7%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Background (2031)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	5	317	226	16	9	2
Future Volume (Veh/h)	5	317	226	16	9	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	345	246	17	10	2
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked					612	256
vC, conflicting volume	265				612	256
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	265				612	256
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				98	100
cM capacity (veh/h)	1309				458	786
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	350	263	12			
Volume Left	5	0	10			
Volume Right	0	17	2			
eSH	1309	1700	492			
Volume to Capacity	0.00	0.15	0.02			
Queue Length 95th (m)	0.1	0.0	0.6			
Control Delay (s)	0.1	0.0	12.5			
Lane LOS	A		B			
Approach Delay (s)	0.1	0.0	12.5			
Approach LOS			B			
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization		30.7%		ICU Level of Service	A	
Analysis Period (min)		15				

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2031)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	4	272	50	29	183	4	51	7	38	4	2	7
Future Volume (vph)	4	272	50	29	183	4	51	7	38	4	2	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.979			0.998			0.947			0.923	
Flt Protected		0.999			0.993			0.974			0.986	
Satd. Flow (prot)	0	1813	0	0	1843	0	0	1753	0	0	1729	0
Flt Permitted		0.999			0.993			0.974			0.986	
Satd. Flow (perm)	0	1813	0	0	1843	0	0	1753	0	0	1729	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			450.4			484.2	
Travel Time (s)		13.7			48.8			32.4			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	296	54	32	199	4	55	8	41	4	2	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	354	0	0	235	0	0	104	0	0	14	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	44.6% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Background (2031)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	4	272	50	29	183	4	51	7	38	4	2	7
Future Volume (Veh/h)	4	272	50	29	183	4	51	7	38	4	2	7
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	296	54	32	199	4	55	8	41	4	2	8
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	203			351			606	599	324	641	624	201
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	203			351			606	599	324	641	624	201
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			86	98	94	99	99	99
cM capacity (veh/h)	1381			1133			396	405	721	354	392	845
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	354	235	104	14								
Volume Left	4	32	55	4								
Volume Right	54	4	41	8								
cSH	1381	1133	483	541								
Volume to Capacity	0.00	0.03	0.22	0.03								
Queue Length 95th (m)	0.1	0.7	6.2	0.6								
Control Delay (s)	0.1	1.4	14.5	11.8								
Lane LOS	A	A	B	B								
Approach Delay (s)	0.1	1.4	14.5	11.8								
Approach LOS			B	B								

Intersection Summary	
Average Delay	2.9
Intersection Capacity Utilization	44.6% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Background (2031)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	12	12	12	84	58	23
Future Volume (vph)	12	12	12	84	58	23
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.932				0.962	
Flt Protected	0.976			0.994		
Satd. Flow (prot)	1564	0	0	1840	1707	0
Flt Permitted	0.976			0.994		
Satd. Flow (perm)	1564	0	0	1840	1707	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	450.4	
Travel Time (s)	11.7			20.9	32.4	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	13	13	13	91	63	25
Shared Lane Traffic (%)						
Lane Group Flow (vph)	26	0	0	104	88	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	21.8%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Background (2031)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	12	12	12	84	58	23
Future Volume (Veh/h)	12	12	12	84	58	23
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	13	13	13	91	63	25
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	196	78	91			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	196	78	91			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	98	99	99			
cM capacity (veh/h)	789	929	1513			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	26	104	88			
Volume Left	13	13	0			
Volume Right	13	0	25			
sSH	853	1513	1700			
Volume to Capacity	0.03	0.01	0.05			
Queue Length 95th (m)	0.7	0.2	0.0			
Control Delay (s)	9.4	1.0	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.4	1.0	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			1.6			
Intersection Capacity Utilization	21.8%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Background (2031)
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	16	7	10	26	0	1	0	9	0	0	0
Future Volume (vph)	0	16	7	10	26	0	1	0	9	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.957						0.877				
Fit Protected					0.986			0.995				
Satd. Flow (prot)	0	1267	0	0	1748	0	0	1533	0	0	1900	0
Fit Permitted					0.986			0.995				
Satd. Flow (perm)	0	1267	0	0	1748	0	0	1533	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		88.4			162.4			207.7			36.3	
Travel Time (s)		6.4			11.7			15.0			2.6	
Confl. Peds. (#/hr)	2		4	4		2	2		1	1		2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	17%	100%	0%	10%	0%	0%	0%	9%	0%	0%	0%
Adj. Flow (vph)	0	17	8	11	28	0	1	0	10	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	25	0	0	39	0	0	11	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.5% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Background (2031)
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	16	7	10	26	0	1	0	9	0	0	0
Future Volume (Veh/h)	0	16	7	10	26	0	1	0	9	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	17	8	11	28	0	1	0	10	0	0	0
Pedestrians		2			1			4			2	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			0			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked							77	77	26	84	81	32
vC, conflicting volume	30			29			77	77	26	84	81	32
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	30			29			77	77	26	84	81	32
tC, single (s)	4.1			4.1			7.1	6.5	6.3	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.4	3.5	4.0	3.3
p0 queue free %	100			99			100	100	99	100	100	100
cM capacity (veh/h)	1593			1592			904	807	1026	888	803	1044

Direction, Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	25	39	11	0
Volume Left	0	11	1	0
Volume Right	8	0	10	0
sSH	1593	1592	1013	1700
Volume to Capacity	0.00	0.01	0.01	0.00
Queue Length 95th (m)	0.0	0.2	0.3	0.0
Control Delay (s)	0.0	2.1	8.6	0.0
Lane LOS		A	A	A
Approach Delay (s)	0.0	2.1	8.6	0.0
Approach LOS		A	A	

Intersection Summary	
Average Delay	2.3
Intersection Capacity Utilization	19.5% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Background (2031)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	17	12	9	18	0	11	0	6	0	0	0
Future Volume (vph)	0	17	12	9	18	0	11	0	6	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.943						0.950				
Flt Protected					0.984			0.969				
Satd. Flow (prot)	0	1454	0	0	1669	0	0	1001	0	0	1900	0
Flt Permitted					0.984			0.969				
Satd. Flow (perm)	0	1454	0	0	1669	0	0	1001	0	0	1900	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	18	13	10	20	0	12	0	7	0	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	31	0	0	30	0	0	19	0	0	0	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	20.2% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Background (2031)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	17	12	9	18	0	11	0	6	0	0	0
Future Volume (Veh/h)	0	17	12	9	18	0	11	0	6	0	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	18	13	10	20	0	12	0	7	0	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	25			40			74	78	38	80	85	26
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	25			40			74	78	38	80	85	26
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			99			98	100	99	100	100	100
cM capacity (veh/h)	1596			1571			775	801	804	885	794	1050
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	31	30	19	0								
Volume Left	0	10	12	0								
Volume Right	13	0	7	0								
cSH	1596	1571	785	1700								
Volume to Capacity	0.00	0.01	0.02	0.00								
Queue Length 95th (m)	0.0	0.1	0.6	0.0								
Control Delay (s)	0.0	2.5	9.7	0.0								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	2.5	9.7	0.0								
Approach LOS			A	A								

Intersection Summary	
Average Delay	3.2
Intersection Capacity Utilization	20.2% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Background (2031)
PM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	50	202	95	25	273	135
Future Volume (vph)	50	202	95	25	273	135
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.892	0.972				
Flt Protected	0.990				0.968	
Satd. Flow (prot)	1629	0	1828	0	0	1797
Flt Permitted	0.990				0.968	
Satd. Flow (perm)	1629	0	1828	0	0	1797
Link Speed (k/h)	50	50			50	
Link Distance (m)	111.8	38.3			111.4	
Travel Time (s)	8.0	2.8			8.0	
Confl. Peds. (#/hr)				5	5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	3%	0%	5%	3%	1%
Adj. Flow (vph)	54	220	103	27	297	147
Shared Lane Traffic (%)						
Lane Group Flow (vph)	274	0	130	0	0	444
Sign Control	Stop		Free			Free

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	50.8% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Background (2031)
PM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	50	202	95	25	273	135
Future Volume (Veh/h)	50	202	95	25	273	135
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	54	220	103	27	297	147
Pedestrians	5					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	862	122			135	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	862	122			135	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	79	76			79	
cM capacity (veh/h)	256	923			1437	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	274	130	444
Volume Left	54	0	297
Volume Right	220	27	0
sSH	610	1700	1437
Volume to Capacity	0.45	0.08	0.21
Queue Length 95th (m)	17.6	0.0	5.9
Control Delay (s)	15.6	0.0	6.1
Lane LOS	C		A
Approach Delay (s)	15.6	0.0	6.1
Approach LOS	C		

Intersection Summary			
Average Delay		8.2	
Intersection Capacity Utilization	50.8%	ICU Level of Service	A
Analysis Period (min)	15		

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Background (2031)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	21	255	7	8	200	65	1	47	21	61	49	18
Future Volume (vph)	21	255	7	8	200	65	1	47	21	61	49	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.996			0.968			0.959			0.981	
Fit Protected		0.996			0.998			0.999			0.977	
Satd. Flow (prot)	0	1827	0	0	1809	0	0	1593	0	0	1588	0
Fit Permitted		0.996			0.998			0.999			0.977	
Satd. Flow (perm)	0	1827	0	0	1809	0	0	1593	0	0	1588	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	1		15	15		1			22	22		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	17%	0%	2%	0%	0%	21%	0%	6%	31%	0%
Adj. Flow (vph)	23	277	8	9	217	71	1	51	23	66	53	20
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	308	0	0	297	0	0	75	0	0	139	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	43.9%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Background (2031)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘	
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Sign Control		Stop			Stop			Stop			Stop		
Traffic Volume (vph)	21	255	7	8	200	65	1	47	21	61	49	18	
Future Volume (vph)	21	255	7	8	200	65	1	47	21	61	49	18	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	23	277	8	9	217	71	1	51	23	66	53	20	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total (vph)	308	297	75	139									
Volume Left (vph)	23	9	1	66									
Volume Right (vph)	8	71	23	20									
Hadj (s)	0.05	-0.11	0.06	0.26									
Departure Headway (s)	5.0	4.9	5.8	5.8									
Degree Utilization, x	0.43	0.40	0.12	0.22									
Capacity (veh/h)	684	705	540	557									
Control Delay (s)	11.7	11.1	9.5	10.5									
Approach Delay (s)	11.7	11.1	9.5	10.5									
Approach LOS	B	B	A	B									
Intersection Summary													
Delay	11.1												
Level of Service	B												
Intersection Capacity Utilization	43.9%				ICU Level of Service				A				
Analysis Period (min)	15												

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Background (2031)
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	31	239	0	0	294	37	0	0	0	46	0	19
Future Volume (vph)	31	239	0	0	294	37	0	0	0	46	0	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.985							0.960	
Flt Protected		0.994									0.966	
Satd. Flow (prot)	0	1859	0	0	1745	0	0	1900	0	0	1382	0
Flt Permitted		0.994									0.966	
Satd. Flow (perm)	0	1859	0	0	1745	0	0	1900	0	0	1382	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)			54	54								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	1%	0%	0%	5%	25%	0%	0%	0%	39%	0%	0%
Adj. Flow (vph)	34	260	0	0	320	40	0	0	0	50	0	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	294	0	0	360	0	0	0	0	0	71	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	45.7%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Background (2031)
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	31	239	0	0	294	37	0	0	0	46	0	19
Future Volume (Veh/h)	31	239	0	0	294	37	0	0	0	46	0	19
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	260	0	0	320	40	0	0	0	50	0	21
Pedestrians												54
Lane Width (m)												3.6
Walking Speed (m/s)												1.2
Percent Blockage												5
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	360			314			743	742	314	668	722	340
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	360			314			743	742	314	668	722	340
tC, single (s)	4.2			4.1			7.1	6.5	6.2	7.5	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.5	4.0	3.3	3.9	4.0	3.3
p0 queue free %	97			100			100	100	100	84	100	97
cM capacity (veh/h)	1177			1201			292	321	698	308	330	707
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	294	360	0	71								
Volume Left	34	0	0	50								
Volume Right	0	40	0	21								
eSH	1177	1201	1700	370								
Volume to Capacity	0.03	0.00	0.00	0.19								
Queue Length 95th (m)	0.7	0.0	0.0	5.3								
Control Delay (s)	1.2	0.0	0.0	17.0								
Lane LOS	A			C								
Approach Delay (s)	1.2	0.0	0.0	17.0								
Approach LOS			A	C								
Intersection Summary												
Average Delay				2.2								
Intersection Capacity Utilization	45.7%			ICU Level of Service	A							
Analysis Period (min)	15											

Appendix F1

2026 Total Operation Synchro Reports



Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Total (2026)
AM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	8	217	328	31	8	6
Future Volume (vph)	8	217	328	31	8	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.988		0.941		
Flt Protected		0.998		0.973		
Satd. Flow (prot)	0	1860	1803	0	1740	0
Flt Permitted		0.998		0.973		
Satd. Flow (perm)	0	1860	1803	0	1740	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	9	236	357	34	9	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	245	391	0	16	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	29.2%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Total (2026)
AM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	8	217	328	31	8	6
Future Volume (Veh/h)	8	217	328	31	8	6
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	9	236	357	34	9	7
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked					630	376
vC, conflicting volume	393					
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	393				630	376
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	99
cM capacity (veh/h)	1175				445	674
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	245	391	16			
Volume Left	9	0	9			
Volume Right	0	34	7			
eSH	1175	1700	522			
Volume to Capacity	0.01	0.23	0.03			
Queue Length 95th (m)	0.2	0.0	0.7			
Control Delay (s)	0.4	0.0	12.1			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	12.1			
Approach LOS			B			
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization	29.2%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2026)

AM Peak Hour

	↖	→	↘	↙	←	↖	↘	↙	↖	↘	↙	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	4	183	37	36	278	4	79	7	59	3	1	6
Future Volume (vph)	4	183	37	36	278	4	79	7	59	3	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.978			0.998			0.945			0.914	
Flt Protected		0.999			0.994			0.974			0.987	
Satd. Flow (prot)	0	1812	0	0	1851	0	0	1749	0	0	1714	0
Flt Permitted		0.999			0.994			0.974			0.987	
Satd. Flow (perm)	0	1812	0	0	1851	0	0	1749	0	0	1714	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			206.4			484.2	
Travel Time (s)		13.7			48.8			14.9			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	199	40	39	302	4	86	8	64	3	1	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	243	0	0	345	0	0	158	0	0	11	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	53.8%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2026)

AM Peak Hour

	↖	→	↘	↙	←	↖	↘	↙	↖	↘	↙	↖
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	4	183	37	36	278	4	79	7	59	3	1	6
Future Volume (Veh/h)	4	183	37	36	278	4	79	7	59	3	1	6
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	199	40	39	302	4	86	8	64	3	1	7
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	306			240			618	612	220	677	630	304
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	306			240			618	612	220	677	630	304
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			78	98	92	99	100	99
cM capacity (veh/h)	1266			1248			389	396	824	326	387	740
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	243	345	158	11								
Volume Left	4	39	86	3								
Volume Right	40	4	64	7								
eSH	1266	1248	495	518								
Volume to Capacity	0.00	0.03	0.32	0.02								
Queue Length 95th (m)	0.1	0.7	10.3	0.5								
Control Delay (s)	0.2	1.2	15.6	12.1								
Lane LOS	A	A	C	B								
Approach Delay (s)	0.2	1.2	15.6	12.1								
Approach LOS			C	B								

Intersection Summary

Average Delay	4.0
Intersection Capacity Utilization	53.8%
ICU Level of Service A	
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Total (2026)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	24	28	17	90	125	10
Future Volume (vph)	24	28	17	90	125	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.928				0.990	
Flt Protected	0.977			0.992		
Satd. Flow (prot)	1548	0	0	1838	1846	0
Flt Permitted	0.977			0.992		
Satd. Flow (perm)	1548	0	0	1838	1846	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	244.0	
Travel Time (s)	11.7			20.9	17.6	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	26	30	18	98	136	11
Shared Lane Traffic (%)						
Lane Group Flow (vph)	56	0	0	116	147	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	26.8%		ICU Level of Service A			
Analysis Period (min)	15					


HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Total (2026)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	24	28	17	90	125	10
Future Volume (Veh/h)	24	28	17	90	125	10
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	26	30	18	98	136	11
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	278	144	150			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	278	144	150			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	96	96	99			
cM capacity (veh/h)	705	853	1440			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	56	116	147			
Volume Left	26	18	0			
Volume Right	30	0	11			
eSH	777	1440	1700			
Volume to Capacity	0.07	0.01	0.09			
Queue Length 95th (m)	1.8	0.3	0.0			
Control Delay (s)	10.0	1.3	0.0			
Lane LOS	A	A				
Approach Delay (s)	10.0	1.3	0.0			
Approach LOS	A					
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization	26.8%		ICU Level of Service	A		
Analysis Period (min)	15					


Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Total (2026)
AM Peak Hour

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	35	1	1	25	4	17
Future Volume (vph)	35	1	1	25	4	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.997				0.890	
Flt Protected				0.998	0.991	
Satd. Flow (prot)	1590	0	0	1729	1561	0
Flt Permitted				0.998	0.991	
Satd. Flow (perm)	1590	0	0	1729	1561	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	88.4			162.4	207.7	
Travel Time (s)	6.4			11.7	15.0	
Confl. Peds. (#/hr)		4	4		2	1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	100%	0%	10%	0%	9%
Adj. Flow (vph)	38	1	1	27	4	18
Shared Lane Traffic (%)						
Lane Group Flow (vph)	39	0	0	28	22	0
Sign Control	Free			Free	Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	14.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Total (2026)
AM Peak Hour

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	35	1	1	25	4	17
Future Volume (Veh/h)	35	1	1	25	4	17
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	1	1	27	4	18
Pedestrians	2			1	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			43		74	44
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			43		74	44
tC, single (s)			4.1		6.4	6.3
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.4
p0 queue free %			100		100	98
cM capacity (veh/h)			1573		930	1003
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	39	28	22			
Volume Left	0	1	4			
Volume Right	1	0	18			
sSH	1700	1573	989			
Volume to Capacity	0.02	0.00	0.02			
Queue Length 95th (m)	0.0	0.0	0.5			
Control Delay (s)	0.0	0.3	8.7			
Lane LOS		A	A			
Approach Delay (s)	0.0	0.3	8.7			
Approach LOS			A			
Intersection Summary						
Average Delay			2.2			
Intersection Capacity Utilization	14.9%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Total (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	9	3	4	15	10	8	0	14	13	0	0
Future Volume (vph)	0	9	3	4	15	10	8	0	14	13	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.969			0.952			0.916				
Flt Protected					0.994			0.982			0.950	
Satd. Flow (prot)	0	1408	0	0	1645	0	0	924	0	0	1805	0
Flt Permitted					0.994			0.982			0.950	
Satd. Flow (perm)	0	1408	0	0	1645	0	0	924	0	0	1805	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	10	3	4	16	11	9	0	15	14	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	13	0	0	31	0	0	24	0	0	14	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	17.7% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Total (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	9	3	4	15	10	8	0	14	13	0	0
Future Volume (Veh/h)	0	9	3	4	15	10	8	0	14	13	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	10	3	4	16	11	9	0	15	14	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	32			22			51	60	24	65	56	28
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	32			22			51	60	24	65	56	28
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			100			99	100	98	98	100	100
cM capacity (veh/h)	1587			1595			806	823	819	900	827	1048
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	13	31	24	14								
Volume Left	0	4	9	14								
Volume Right	3	11	15	0								
eSH	1587	1595	814	900								
Volume to Capacity	0.00	0.00	0.03	0.02								
Queue Length 95th (m)	0.0	0.1	0.7	0.4								
Control Delay (s)	0.0	1.0	9.6	9.1								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	1.0	9.6	9.1								
Approach LOS			A	A								

Intersection Summary	
Average Delay	4.7
Intersection Capacity Utilization	17.7% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Total (2026)
AM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	46	215	98	57	183	87
Future Volume (vph)	46	215	98	57	183	87
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.889	0.950				
Flt Protected	0.991					0.967
Satd. Flow (prot)	1537	0	1734	0	0	1702
Flt Permitted	0.991					0.967
Satd. Flow (perm)	1537	0	1734	0	0	1702
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	8%	3%	6%	7%	10%
Adj. Flow (vph)	50	234	107	62	199	95
Shared Lane Traffic (%)						
Lane Group Flow (vph)	284	0	169	0	0	294
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	49.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Total (2026)
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	46	215	98	57	183	87
Future Volume (Veh/h)	46	215	98	57	183	87
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	50	234	107	62	199	95
Pedestrians	4					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type			None		None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	635	142			173	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	635	142			173	
tC, single (s)	6.5	6.3			4.2	
tC, 2 stage (s)						
tF (s)	3.6	3.4			2.3	
p0 queue free %	86	74			85	
cM capacity (veh/h)	362	887			1369	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	284	169	294			
Volume Left	50	0	199			
Volume Right	234	62	0			
eSH	707	1700	1369			
Volume to Capacity	0.40	0.10	0.15			
Queue Length 95th (m)	14.8	0.0	3.9			
Control Delay (s)	13.5	0.0	5.9			
Lane LOS	B		A			
Approach Delay (s)	13.5	0.0	5.9			
Approach LOS	B					
Intersection Summary						
Average Delay			7.4			
Intersection Capacity Utilization	49.9%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Total (2026)
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	14	105	8	13	250	47	6	54	12	64	142	37
Future Volume (vph)	14	105	8	13	250	47	6	54	12	64	142	37
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.991			0.980			0.978			0.980	
Flt Protected		0.995			0.998			0.996			0.987	
Satd. Flow (prot)	0	1733	0	0	1777	0	0	1417	0	0	1614	0
Flt Permitted		0.995			0.998			0.996			0.987	
Satd. Flow (perm)	0	1733	0	0	1777	0	0	1417	0	0	1614	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	57		1	1		57	4		50	50		4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	6%	20%	33%	3%	5%	50%	32%	14%	4%	22%	0%
Adj. Flow (vph)	15	114	9	14	272	51	7	59	13	70	154	40
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	138	0	0	337	0	0	79	0	0	264	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	45.9%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Total (2026)
AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↔			↔			↔			↔		
Sign Control		Stop			Stop			Stop			Stop		
Traffic Volume (vph)	14	105	8	13	250	47	6	54	12	64	142	37	
Future Volume (vph)	14	105	8	13	250	47	6	54	12	64	142	37	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	15	114	9	14	272	51	7	59	13	70	154	40	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total (vph)	138	337	79	264									
Volume Left (vph)	15	14	7	70									
Volume Right (vph)	9	51	13	40									
Hadj (s)	0.12	-0.01	0.44	0.20									
Departure Headway (s)	5.6	5.1	6.1	5.5									
Degree Utilization, x	0.21	0.48	0.13	0.41									
Capacity (veh/h)	591	664	516	607									
Control Delay (s)	10.1	12.8	10.1	12.3									
Approach Delay (s)	10.1	12.8	10.1	12.3									
Approach LOS	B	B	B	B									
Intersection Summary													
Delay	11.9												
Level of Service	B												
Intersection Capacity Utilization	45.9%				ICU Level of Service				A				
Analysis Period (min)	15												

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Total (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	19	342	1	1	234	43	1	3	0	114	0	50
Future Volume (vph)	19	342	1	1	234	43	1	3	0	114	0	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.979							0.959	
Flt Protected		0.997						0.988			0.966	
Satd. Flow (prot)	0	1825	0	0	1800	0	0	1877	0	0	1760	0
Flt Permitted		0.997						0.988			0.966	
Satd. Flow (perm)	0	1825	0	0	1800	0	0	1877	0	0	1760	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)	42					42						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	21	372	1	1	254	47	1	3	0	124	0	54
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	394	0	0	302	0	0	4	0	0	178	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	54.8%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Total (2026)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘		
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		↕			↕			↕			↕			
Traffic Volume (veh/h)	19	342	1	1	234	43	1	3	0	114	0	50		
Future Volume (Veh/h)	19	342	1	1	234	43	1	3	0	114	0	50		
Sign Control	Free			Free			Stop			Stop				
Grade	0%			0%			0%			0%				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	21	372	1	1	254	47	1	3	0	124	0	54		
Pedestrians												42		
Lane Width (m)												3.6		
Walking Speed (m/s)												1.2		
Percent Blockage												4		
Right turn flare (veh)														
Median type	None			None										
Median storage (veh)														
Upstream signal (m)														
pX, platoon unblocked														
vC, conflicting volume	343				373				748	760	372	738	736	320
vC1, stage 1 conf vol														
vC2, stage 2 conf vol														
vCu, unblocked vol	343				373				748	760	372	738	736	320
tC, single (s)	4.1				4.1				7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)														
tF (s)	2.2				2.2				3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	98				100				100	99	100	60	100	92
cM capacity (veh/h)	1184				1197				293	320	678	309	330	700
Direction, Lane #	EB 1	WB 1	NB 1	SB 1										
Volume Total	394	302	4	178										
Volume Left	21	1	1	124										
Volume Right	1	47	0	54										
eSH	1184	1197	313	372										
Volume to Capacity	0.02	0.00	0.01	0.48										
Queue Length 95th (m)	0.4	0.0	0.3	18.9										
Control Delay (s)	0.6	0.0	16.6	23.2										
Lane LOS	A	A	C	C										
Approach Delay (s)	0.6	0.0	16.6	23.2										
Approach LOS			C	C										
Intersection Summary														
Average Delay												5.1		
Intersection Capacity Utilization	54.8%				ICU Level of Service				A					
Analysis Period (min)	15													

Lanes, Volumes, Timings
201: Irvine St & Street A

Total (2026)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	49	77	22	92	57	16
Future Volume (vph)	49	77	22	92	57	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Flt	0.917			0.971		
Flt Protected	0.981			0.990		
Satd. Flow (prot)	1709	0	0	1851	1816	0
Flt Permitted	0.981			0.990		
Satd. Flow (perm)	1709	0	0	1851	1816	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	230.9			244.0	206.4	
Travel Time (s)	16.6			17.6	14.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Adj. Flow (vph)	53	84	24	100	62	17
Shared Lane Traffic (%)						
Lane Group Flow (vph)	137	0	0	124	79	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	26.8%
ICU Level of Service	A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
201: Irvine St & Street A

Total (2026)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	49	77	22	92	57	16
Future Volume (Veh/h)	49	77	22	92	57	16
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	53	84	24	100	62	17
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	218	70	79			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	218	70	79			
tC, 2 stage (s)	6.4	6.2	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	92	98			
cM capacity (veh/h)	762	998	1532			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	137	124	79			
Volume Left	53	24	0			
Volume Right	84	0	17			
sSH	891	1532	1700			
Volume to Capacity	0.15	0.02	0.05			
Queue Length 95th (m)	4.1	0.4	0.0			
Control Delay (s)	9.8	1.5	0.0			
Lane LOS	A	A				
Approach Delay (s)	9.8	1.5	0.0			
Approach LOS	A					

Intersection Summary			
Average Delay		4.5	
Intersection Capacity Utilization	26.8%	ICU Level of Service	A
Analysis Period (min)		15	

Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Total (2026)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	4	338	233	14	8	2
Future Volume (vph)	4	338	233	14	8	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.992		0.975	
Flt Protected		0.999			0.961	
Satd. Flow (prot)	0	1861	1811	0	1780	0
Flt Permitted		0.999			0.961	
Satd. Flow (perm)	0	1861	1811	0	1780	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	4	367	253	15	9	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	371	268	0	11	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	31.0%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Total (2026)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	4	338	233	14	8	2
Future Volume (Veh/h)	4	338	233	14	8	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	367	253	15	9	2
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	270				638	262
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	270				638	262
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				98	100
cM capacity (veh/h)	1303				442	780
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	371	268	11			
Volume Left	4	0	9			
Volume Right	0	15	2			
eSH	1303	1700	480			
Volume to Capacity	0.00	0.16	0.02			
Queue Length 95th (m)	0.1	0.0	0.5			
Control Delay (s)	0.1	0.0	12.7			
Lane LOS	A		B			
Approach Delay (s)	0.1	0.0	12.7			
Approach LOS			B			
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization	31.0%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2026)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Traffic Volume (vph)	3	252	91	46	167	3	73	7	49	3	2	7	
Future Volume (vph)	3	252	91	46	167	3	73	7	49	3	2	7	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Ped Bike Factor													
Frt		0.964			0.998			0.949			0.917		
Flt Protected					0.989			0.973			0.989		
Satd. Flow (prot)	0	1792	0	0	1814	0	0	1754	0	0	1723	0	
Flt Permitted					0.989			0.973			0.989		
Satd. Flow (perm)	0	1792	0	0	1814	0	0	1754	0	0	1723	0	
Link Speed (k/h)		40			40			50			50		
Link Distance (m)		152.7			542.4			202.8			484.2		
Travel Time (s)		13.7			48.8			14.6			34.9		
Confl. Peds. (#/hr)			1	1									
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%	
Adj. Flow (vph)	3	274	99	50	182	3	79	8	53	3	2	8	
Shared Lane Traffic (%)													
Lane Group Flow (vph)	0	376	0	0	235	0	0	140	0	0	13	0	
Sign Control		Free			Free			Stop			Stop		

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	54.6%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2026)

PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕			↕			↕			↕		
Traffic Volume (veh/h)	3	252	91	46	167	3	73	7	49	3	2	7	
Future Volume (Veh/h)	3	252	91	46	167	3	73	7	49	3	2	7	
Sign Control		Free			Free			Stop			Stop		
Grade		0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	3	274	99	50	182	3	79	8	53	3	2	8	
Pedestrians								1					
Lane Width (m)								3.6					
Walking Speed (m/s)								1.2					
Percent Blockage								0					
Right turn flare (veh)													
Median type		None			None								
Median storage (veh)													
Upstream signal (m)													
pX, platoon unblocked													
vC, conflicting volume	185			374			623	616	324	670	664	184	
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	185			374			623	616	324	670	664	184	
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2	
tC, 2 stage (s)													
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3	
p0 queue free %	100			95			79	98	93	99	99	99	
cM capacity (veh/h)	1402			1111			381	389	721	328	366	864	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1									
Volume Total	376	235	140	13									
Volume Left	3	50	79	3									
Volume Right	99	3	53	8									
eSH	1402	1111	465	545									
Volume to Capacity	0.00	0.05	0.30	0.02									
Queue Length 95th (m)	0.0	1.1	9.5	0.6									
Control Delay (s)	0.1	2.1	16.1	11.8									
Lane LOS	A	A	C	B									
Approach Delay (s)	0.1	2.1	16.1	11.8									
Approach LOS			C	B									

Intersection Summary

Average Delay	3.8
Intersection Capacity Utilization	54.6%
ICU Level of Service A	
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Total (2026)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	15	16	20	159	103	27
Future Volume (vph)	15	16	20	159	103	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.930				0.972	
Flt Protected	0.976			0.994		
Satd. Flow (prot)	1556	0	0	1840	1756	0
Flt Permitted	0.976			0.994		
Satd. Flow (perm)	1556	0	0	1840	1756	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	247.6	
Travel Time (s)	11.7			20.9	17.8	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	16	17	22	173	112	29
Shared Lane Traffic (%)						
Lane Group Flow (vph)	33	0	0	195	141	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	30.5%		ICU Level of Service A			
Analysis Period (min)	15					

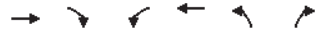
HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Total (2026)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	15	16	20	159	103	27
Future Volume (Veh/h)	15	16	20	159	103	27
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	17	22	173	112	29
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	346	130	144			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	346	130	144			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	98	98	98			
cM capacity (veh/h)	643	870	1447			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	33	195	141			
Volume Left	16	22	0			
Volume Right	17	0	29			
eSH	743	1447	1700			
Volume to Capacity	0.04	0.02	0.08			
Queue Length 95th (m)	1.1	0.4	0.0			
Control Delay (s)	10.1	1.0	0.0			
Lane LOS	B	A				
Approach Delay (s)	10.1	1.0	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			1.4			
Intersection Capacity Utilization	30.5%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St


Total (2026)
PM Peak Hour

						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↘	↙	↔	↙	↘
Traffic Volume (vph)	23	7	9	38	1	8
Future Volume (vph)	23	7	9	38	1	8
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.967				0.878	
Flt Protected				0.990	0.995	
Satd. Flow (prot)	1340	0	0	1741	1535	0
Flt Permitted				0.990	0.995	
Satd. Flow (perm)	1340	0	0	1741	1535	0
Link Speed (k/h)	50				50	50
Link Distance (m)	88.4			162.4	207.7	
Travel Time (s)	6.4			11.7	15.0	
Confl. Peds. (#/hr)	4		4			2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	100%	0%	10%	0%	9%
Adj. Flow (vph)	25	8	10	41	1	9
Shared Lane Traffic (%)						
Lane Group Flow (vph)	33	0	0	51	10	0
Sign Control	Free		Free		Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.5% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Total (2026)
PM Peak Hour

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↘	↙	↔	↙	↘
Traffic Volume (veh/h)	23	7	9	38	1	8
Future Volume (Veh/h)	23	7	9	38	1	8
Sign Control	Free			Free Stop		
Grade	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	25	8	10	41	1	9
Pedestrians	2		1		4	
Lane Width (m)	3.6		3.6		3.6	
Walking Speed (m/s)	1.2		1.2		1.2	
Percent Blockage	0		0		0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume				37	96	34
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				37	96	34
tC, single (s)				4.1	6.4	6.3
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.4
p0 queue free %				99	100	99
cM capacity (veh/h)				1581	898	1015

Direction, Lane #	EB 1	WB 1	NB 1
Volume Total	33	51	10
Volume Left	0	10	1
Volume Right	8	0	9
sSH	1700	1581	1002
Volume to Capacity	0.02	0.01	0.01
Queue Length 95th (m)	0.0	0.1	0.2
Control Delay (s)	0.0	1.5	8.6
Lane LOS	A		
Approach Delay (s)	0.0	1.5	8.6
Approach LOS	A		

Intersection Summary			
Average Delay			1.7
Intersection Capacity Utilization	19.5%	ICU Level of Service	A
Analysis Period (min)			15

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Total (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	15	11	8	17	15	10	0	6	9	0	0
Future Volume (vph)	0	15	11	8	17	15	10	0	6	9	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.942			0.950			0.947				
Flt Protected					0.990			0.970			0.950	
Satd. Flow (prot)	0	1457	0	0	1662	0	0	994	0	0	1805	0
Flt Permitted					0.990			0.970			0.950	
Satd. Flow (perm)	0	1457	0	0	1662	0	0	994	0	0	1805	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	16	12	9	18	16	11	0	7	10	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	28	0	0	43	0	0	18	0	0	10	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	21.0% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Total (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	15	11	8	17	15	10	0	6	9	0	0
Future Volume (Veh/h)	0	15	11	8	17	15	10	0	6	9	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	16	12	9	18	16	11	0	7	10	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	39			37			76	88	35	82	86	32
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	39			37			76	88	35	82	86	32
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			99			99	100	99	99	100	100
cM capacity (veh/h)	1577			1575			773	792	807	884	794	1042
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	28	43	18	10								
Volume Left	0	9	11	10								
Volume Right	12	16	7	0								
cSH	1577	1575	786	884								
Volume to Capacity	0.00	0.01	0.02	0.01								
Queue Length 95th (m)	0.0	0.1	0.5	0.3								
Control Delay (s)	0.0	1.6	9.7	9.1								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	1.6	9.7	9.1								
Approach LOS			A	A								

Intersection Summary	
Average Delay	3.4
Intersection Capacity Utilization	21.0% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Total (2026)
PM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	55	201	86	35	287	123
Future Volume (vph)	55	201	86	35	287	123
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.894	0.961				
Flt Protected	0.989				0.966	
Satd. Flow (prot)	1631	0	1800	0	0	1792
Flt Permitted	0.989				0.966	
Satd. Flow (perm)	1631	0	1800	0	0	1792
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				5	5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	3%	0%	5%	3%	1%
Adj. Flow (vph)	60	218	93	38	312	134
Shared Lane Traffic (%)						
Lane Group Flow (vph)	278	0	131	0	0	446
Sign Control	Stop		Free			Free

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	55.6% ICU Level of Service B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Total (2026)
PM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	55	201	86	35	287	123
Future Volume (Veh/h)	55	201	86	35	287	123
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	60	218	93	38	312	134
Pedestrians	5					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	875	117			136	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	875	117			136	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	76	77			78	
cM capacity (veh/h)	248	928			1436	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	278	131	446
Volume Left	60	0	312
Volume Right	218	38	0
sSH	583	1700	1436
Volume to Capacity	0.48	0.08	0.22
Queue Length 95th (m)	19.4	0.0	6.3
Control Delay (s)	16.7	0.0	6.3
Lane LOS	C		A
Approach Delay (s)	16.7	0.0	6.3
Approach LOS	C		

Intersection Summary			
Average Delay		8.7	
Intersection Capacity Utilization	55.6%	ICU Level of Service	B
Analysis Period (min)	15		

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Total (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	36	241	7	7	189	77	1	100	19	69	80	25
Future Volume (vph)	36	241	7	7	189	77	1	100	19	69	80	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.997			0.962			0.978			0.981	
Flt Protected		0.994			0.999						0.981	
Satd. Flow (prot)	0	1828	0	0	1801	0	0	1582	0	0	1567	0
Flt Permitted		0.994			0.999						0.981	
Satd. Flow (perm)	0	1828	0	0	1801	0	0	1582	0	0	1567	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	1		15	15		1			22	22		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	17%	0%	2%	0%	0%	21%	0%	6%	31%	0%
Adj. Flow (vph)	39	262	8	8	205	84	1	109	21	75	87	27
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	309	0	0	297	0	0	131	0	0	189	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	54.2%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Total (2026)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	36	241	7	7	189	77	1	100	19	69	80	25
Future Volume (vph)	36	241	7	7	189	77	1	100	19	69	80	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	39	262	8	8	205	84	1	109	21	75	87	27
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	309	297	131	189								
Volume Left (vph)	39	8	1	75								
Volume Right (vph)	8	84	21	27								
Hadj (s)	0.06	-0.14	0.20	0.28								
Departure Headway (s)	5.5	5.3	6.2	6.1								
Degree Utilization, x	0.47	0.44	0.22	0.32								
Capacity (veh/h)	620	638	509	531								
Control Delay (s)	13.2	12.3	10.9	11.9								
Approach Delay (s)	13.2	12.3	10.9	11.9								
Approach LOS	B	B	B	B								
Intersection Summary												
Delay	12.3											
Level of Service	B											
Intersection Capacity Utilization	54.2%				ICU Level of Service				A			
Analysis Period (min)	15											

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Total (2026)
PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	55	216	0	0	266	65	0	0	0	58	0	36
Future Volume (vph)	55	216	0	0	266	65	0	0	0	58	0	36
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.973							0.948	
Flt Protected		0.990									0.970	
Satd. Flow (prot)	0	1844	0	0	1697	0	0	1900	0	0	1408	0
Flt Permitted		0.990									0.970	
Satd. Flow (perm)	0	1844	0	0	1697	0	0	1900	0	0	1408	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)			54	54								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	1%	0%	0%	5%	25%	0%	0%	0%	39%	0%	0%
Adj. Flow (vph)	60	235	0	0	289	71	0	0	0	63	0	39
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	295	0	0	360	0	0	0	0	102	0	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	47.8%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Total (2026)
PM Peak Hour

	↖	→	↗	↙	←	↖	↗	↙	↘	↖	↗	↙
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	55	216	0	0	266	65	0	0	0	58	0	36
Future Volume (Veh/h)	55	216	0	0	266	65	0	0	0	58	0	36
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	60	235	0	0	289	71	0	0	0	63	0	39
Pedestrians											54	
Lane Width (m)											3.6	
Walking Speed (m/s)											1.2	
Percent Blockage											5	
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	360			289			772	769	289	680	734	324
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	360			289			772	769	289	680	734	324
tC, single (s)	4.2			4.1			7.1	6.5	6.2	7.5	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.5	4.0	3.3	3.9	4.0	3.3
p0 queue free %	95			100			100	100	100	79	100	95
cM capacity (veh/h)	1177			1227			267	303	721	297	317	721
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	295	360	0	102								
Volume Left	60	0	0	63								
Volume Right	0	71	0	39								
eSH	1177	1227	1700	383								
Volume to Capacity	0.05	0.00	0.00	0.27								
Queue Length 95th (m)	1.2	0.0	0.0	8.0								
Control Delay (s)	2.1	0.0	0.0	17.8								
Lane LOS	A			C								
Approach Delay (s)	2.1	0.0	0.0	17.8								
Approach LOS				C								
Intersection Summary												
Average Delay				3.2								
Intersection Capacity Utilization	47.8%			ICU Level of Service	A							
Analysis Period (min)	15											

Lanes, Volumes, Timings
201: Irvine St & Street A

Total (2026)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	35	48	80	94	82	57
Future Volume (vph)	35	48	80	94	82	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Flt	0.922				0.945	
Flt Protected	0.979			0.978		
Satd. Flow (prot)	1715	0	0	1838	1775	0
Flt Permitted	0.979			0.978		
Satd. Flow (perm)	1715	0	0	1838	1775	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.6			247.6	202.8	
Travel Time (s)	18.5			17.8	14.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Adj. Flow (vph)	38	52	87	102	89	62
Shared Lane Traffic (%)						
Lane Group Flow (vph)	90	0	0	189	151	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	32.1%
ICU Level of Service	A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
201: Irvine St & Street A

Total (2026)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	35	48	80	94	82	57
Future Volume (Veh/h)	35	48	80	94	82	57
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	52	87	102	89	62
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	396	120	151			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	396	120	151			
tC, 2 stage (s)	6.4	6.2	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	94	94			
cM capacity (veh/h)	576	937	1442			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	90	189	151			
Volume Left	38	87	0			
Volume Right	52	0	62			
sSH	741	1442	1700			
Volume to Capacity	0.12	0.06	0.09			
Queue Length 95th (m)	3.1	1.5	0.0			
Control Delay (s)	10.5	3.8	0.0			
Lane LOS	B	A				
Approach Delay (s)	10.5	3.8	0.0			
Approach LOS	B					

Intersection Summary

Average Delay	3.9
Intersection Capacity Utilization	32.1%
ICU Level of Service	A
Analysis Period (min)	15

Appendix F2

2031 Total Operation Synchro Reports



Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Total (2031)
AM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	9	235	352	34	9	6
Future Volume (vph)	9	235	352	34	9	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.988		0.944	
Flt Protected		0.998			0.971	
Satd. Flow (prot)	0	1860	1803	0	1742	0
Flt Permitted		0.998			0.971	
Satd. Flow (perm)	0	1860	1803	0	1742	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	10	255	383	37	10	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	265	420	0	17	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	30.6%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Total (2031)
AM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	9	235	352	34	9	6
Future Volume (Veh/h)	9	235	352	34	9	6
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	255	383	37	10	7
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked					678	404
vC, conflicting volume	422				678	404
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	422				678	404
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				98	99
cM capacity (veh/h)	1146				416	650
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	265	420	17			
Volume Left	10	0	10			
Volume Right	0	37	7			
eSH	1146	1700	489			
Volume to Capacity	0.01	0.25	0.03			
Queue Length 95th (m)	0.2	0.0	0.8			
Control Delay (s)	0.4	0.0	12.6			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	12.6			
Approach LOS			B			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization	30.6%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031)

AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	5	201	38	38	303	5	77	7	63	4	1	6
Future Volume (vph)	5	201	38	38	303	5	77	7	63	4	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.979			0.998			0.943			0.921	
Flt Protected		0.999			0.995			0.974			0.984	
Satd. Flow (prot)	0	1813	0	0	1854	0	0	1745	0	0	1722	0
Flt Permitted		0.999			0.995			0.974			0.984	
Satd. Flow (perm)	0	1813	0	0	1854	0	0	1745	0	0	1722	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			206.4			484.2	
Travel Time (s)		13.7			48.8			14.9			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	5	218	41	41	329	5	84	8	68	4	1	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	264	0	0	375	0	0	160	0	0	12	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	54.6%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031)

AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	5	201	38	38	303	5	77	7	63	4	1	6
Future Volume (Veh/h)	5	201	38	38	303	5	77	7	63	4	1	6
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	218	41	41	329	5	84	8	68	4	1	7
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	334			260			670	666	240	734	684	332
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	334			260			670	666	240	734	684	332
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			77	98	92	99	100	99
cM capacity (veh/h)	1237			1226			358	368	804	296	360	715
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	264	375	160	12								
Volume Left	5	41	84	4								
Volume Right	41	5	68	7								
eSH	1237	1226	469	460								
Volume to Capacity	0.00	0.03	0.34	0.03								
Queue Length 95th (m)	0.1	0.8	11.4	0.6								
Control Delay (s)	0.2	1.2	16.6	13.0								
Lane LOS	A	A	C	B								
Approach Delay (s)	0.2	1.2	16.6	13.0								
Approach LOS			C	B								

Intersection Summary

Average Delay	4.1
Intersection Capacity Utilization	54.6%
ICU Level of Service A	
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Total (2031)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	26	30	18	95	128	10
Future Volume (vph)	26	30	18	95	128	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.927				0.990	
Flt Protected	0.978			0.992		
Satd. Flow (prot)	1547	0	0	1839	1847	0
Flt Permitted	0.978			0.992		
Satd. Flow (perm)	1547	0	0	1839	1847	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	244.0	
Travel Time (s)	11.7			20.9	17.6	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	28	33	20	103	139	11
Shared Lane Traffic (%)						
Lane Group Flow (vph)	61	0	0	123	150	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	27.3%		ICU Level of Service A			
Analysis Period (min)	15					


HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Total (2031)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	26	30	18	95	128	10
Future Volume (Veh/h)	26	30	18	95	128	10
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	28	33	20	103	139	11
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	290	148	153			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	290	148	153			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	96	96	99			
cM capacity (veh/h)	693	849	1436			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	61	123	150			
Volume Left	28	20	0			
Volume Right	33	0	11			
sSH	770	1436	1700			
Volume to Capacity	0.08	0.01	0.09			
Queue Length 95th (m)	2.0	0.3	0.0			
Control Delay (s)	10.1	1.3	0.0			
Lane LOS	B	A				
Approach Delay (s)	10.1	1.3	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay			2.3			
Intersection Capacity Utilization	27.3%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Total (2031)
AM Peak Hour


						
Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔		↔	
Traffic Volume (vph)	37	1	1	27	5	18
Future Volume (vph)	37	1	1	27	5	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.997			0.892		
Flt Protected				0.998 0.990		
Satd. Flow (prot)	1592	0	0	1729	1565	0
Flt Permitted				0.998 0.990		
Satd. Flow (perm)	1592	0	0	1729	1565	0
Link Speed (k/h)	50			50		
Link Distance (m)	88.4			162.4		207.7
Travel Time (s)	6.4			11.7		15.0
Confl. Peds. (#/hr)	4		4		2 1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	100%	0%	10%	0%	9%
Adj. Flow (vph)	40	1	1	29	5	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	41	0	0	30	25	0
Sign Control	Free			Free Stop		

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	14.9%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Total (2031)
AM Peak Hour

						
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔		↔	
Traffic Volume (veh/h)	37	1	1	27	5	18
Future Volume (Veh/h)	37	1	1	27	5	18
Sign Control	Free			Free Stop		
Grade	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	40	1	1	29	5	20
Pedestrians	2		1		4	
Lane Width (m)	3.6			3.6		3.6
Walking Speed (m/s)	1.2			1.2		1.2
Percent Blockage	0			0		0
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume				45		78 46
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				45		78 46
tC, single (s)				4.1		6.4 6.3
tC, 2 stage (s)						
tF (s)				2.2		3.5 3.4
p0 queue free %				100		99 98
cM capacity (veh/h)				1571		925 1000

Direction, Lane #	EB 1	WB 1	NB 1
Volume Total	41	30	25
Volume Left	0	1	5
Volume Right	1	0	20
sSH	1700	1571	984
Volume to Capacity	0.02	0.00	0.03
Queue Length 95th (m)	0.0	0.0	0.6
Control Delay (s)	0.0	0.2	8.8
Lane LOS	A A A		
Approach Delay (s)	0.0	0.2	8.8
Approach LOS	A		

Intersection Summary

Average Delay	2.4		
Intersection Capacity Utilization	14.9%	ICU Level of Service	A
Analysis Period (min)	15		

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Total (2031)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	10	4	5	17	10	9	0	16	13	0	0
Future Volume (vph)	0	10	4	5	17	10	9	0	16	13	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.964			0.956			0.915				
Flt Protected					0.993			0.982				0.950
Satd. Flow (prot)	0	1416	0	0	1647	0	0	922	0	0	1805	0
Flt Permitted					0.993			0.982				0.950
Satd. Flow (perm)	0	1416	0	0	1647	0	0	922	0	0	1805	0
Link Speed (k/h)		50			50			50				50
Link Distance (m)		123.8			88.4			134.2				45.0
Travel Time (s)		8.9			6.4			9.7				3.2
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	11	4	5	18	11	10	0	17	14	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	15	0	0	34	0	0	27	0	0	14	0
Sign Control		Free			Free			Stop				Stop

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	18.5% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Total (2031)
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	10	4	5	17	10	9	0	16	13	0	0
Future Volume (Veh/h)	0	10	4	5	17	10	9	0	16	13	0	0
Sign Control		Free			Free			Stop				Stop
Grade		0%			0%			0%				0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	11	4	5	18	11	10	0	17	14	0	0
Pedestrians		1			4			9				5
Lane Width (m)		3.6			3.6			3.6				3.6
Walking Speed (m/s)		1.2			1.2			1.2				1.2
Percent Blockage		0			0			1				0
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	34			24			56	66	26	72	62	30
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	34			24			56	66	26	72	62	30
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			100			99	100	98	98	100	100
cM capacity (veh/h)	1584			1592			799	816	817	887	820	1046

Direction, Lane #	EB 1	WB 1	NB 1	SB 1
Volume Total	15	34	27	14
Volume Left	0	5	10	14
Volume Right	4	11	17	0
cSH	1584	1592	810	887
Volume to Capacity	0.00	0.00	0.03	0.02
Queue Length 95th (m)	0.0	0.1	0.8	0.4
Control Delay (s)	0.0	1.1	9.6	9.1
Lane LOS		A	A	A
Approach Delay (s)	0.0	1.1	9.6	9.1
Approach LOS			A	A

Intersection Summary	
Average Delay	4.7
Intersection Capacity Utilization	18.5% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Total (2031)
AM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	50	228	108	62	199	96
Future Volume (vph)	50	228	108	62	199	96
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.889	0.951				
Flt Protected	0.991					0.967
Satd. Flow (prot)	1537	0	1736	0	0	1702
Flt Permitted	0.991					0.967
Satd. Flow (perm)	1537	0	1736	0	0	1702
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	8%	3%	6%	7%	10%
Adj. Flow (vph)	54	248	117	67	216	104
Shared Lane Traffic (%)						
Lane Group Flow (vph)	302	0	184	0	0	320
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	53.0%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Total (2031)
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	50	228	108	62	199	96
Future Volume (Veh/h)	50	228	108	62	199	96
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	54	248	117	67	216	104
Pedestrians	4					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	690	154			188	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	690	154			188	
tC, single (s)	6.5	6.3			4.2	
tC, 2 stage (s)						
tF (s)	3.6	3.4			2.3	
p0 queue free %	84	72			84	
cM capacity (veh/h)	330	873			1352	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	302	184	320			
Volume Left	54	0	216			
Volume Right	248	67	0			
eSH	675	1700	1352			
Volume to Capacity	0.45	0.11	0.16			
Queue Length 95th (m)	17.6	0.0	4.3			
Control Delay (s)	14.6	0.0	6.0			
Lane LOS	B		A			
Approach Delay (s)	14.6	0.0	6.0			
Approach LOS	B					
Intersection Summary						
Average Delay		7.8				
Intersection Capacity Utilization	53.0%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Total (2031)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	15	112	9	14	264	50	6	57	13	68	149	39
Future Volume (vph)	15	112	9	14	264	50	6	57	13	68	149	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.991			0.980			0.977			0.980	
Flt Protected		0.995			0.998			0.996			0.987	
Satd. Flow (prot)	0	1733	0	0	1777	0	0	1417	0	0	1614	0
Flt Permitted		0.995			0.998			0.996			0.987	
Satd. Flow (perm)	0	1733	0	0	1777	0	0	1417	0	0	1614	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	57		1	1		57	4		50	50		4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	6%	20%	33%	3%	5%	50%	32%	14%	4%	22%	0%
Adj. Flow (vph)	16	122	10	15	287	54	7	62	14	74	162	42
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	148	0	0	356	0	0	83	0	0	278	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	47.8%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Total (2031)
AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	15	112	9	14	264	50	6	57	13	68	149	39
Future Volume (vph)	15	112	9	14	264	50	6	57	13	68	149	39
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	16	122	10	15	287	54	7	62	14	74	162	42
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	148	356	83	278								
Volume Left (vph)	16	15	7	74								
Volume Right (vph)	10	54	14	42								
Hadj (s)	0.12	0.00	0.43	0.20								
Departure Headway (s)	5.7	5.2	6.3	5.7								
Degree Utilization, x	0.23	0.52	0.14	0.44								
Capacity (veh/h)	575	652	501	594								
Control Delay (s)	10.4	13.7	10.3	13.0								
Approach Delay (s)	10.4	13.7	10.3	13.0								
Approach LOS	B	B	B	B								
Intersection Summary												
Delay	12.6											
Level of Service	B											
Intersection Capacity Utilization	47.8%				ICU Level of Service				A			
Analysis Period (min)	15											

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Total (2031)
AM Peak Hour

	↖	→	↗	↙	←	↖	↙	↑	↗	↘	↓	↖
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	19	378	1	1	258	46	1	4	0	121	0	52
Future Volume (vph)	19	378	1	1	258	46	1	4	0	121	0	52
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.980							0.959	
Flt Protected		0.998						0.990			0.966	
Satd. Flow (prot)	0	1827	0	0	1801	0	0	1881	0	0	1760	0
Flt Permitted		0.998						0.990			0.966	
Satd. Flow (perm)	0	1827	0	0	1801	0	0	1881	0	0	1760	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)	42					42						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	21	411	1	1	280	50	1	4	0	132	0	57
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	433	0	0	331	0	0	5	0	0	189	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	57.3%				ICU Level of Service B							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Total (2031)
AM Peak Hour

	↖	→	↗	↙	←	↖	↙	↑	↗	↘	↓	↖												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR												
Lane Configurations		↕			↕			↕			↕													
Traffic Volume (veh/h)	19	378	1	1	258	46	1	4	0	121	0	52												
Future Volume (Veh/h)	19	378	1	1	258	46	1	4	0	121	0	52												
Sign Control	Free			Free			Stop			Stop														
Grade	0%			0%			0%			0%														
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92												
Hourly flow rate (vph)	21	411	1	1	280	50	1	4	0	132	0	57												
Pedestrians												42												
Lane Width (m)												3.6												
Walking Speed (m/s)												1.2												
Percent Blockage												4												
Right turn flare (veh)																								
Median type	None			None																				
Median storage (veh)																								
Upstream signal (m)																								
pX, platoon unblocked																								
vC, conflicting volume	372				412				818	828	412	804	803	347										
vC1, stage 1 conf vol																								
vC2, stage 2 conf vol																								
vCu, unblocked vol	372				412				818	828	412	804	803	347										
tC, single (s)	4.1				4.1				7.1	6.5	6.2	7.1	6.5	6.2										
tC, 2 stage (s)																								
tF (s)	2.2				2.2				3.5	4.0	3.3	3.5	4.0	3.3										
p0 queue free %	98				100				100	99	100	53	100	92										
cM capacity (veh/h)	1156			1158			261			292			645			278			302			676		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1																				
Volume Total	433	331	5	189																				
Volume Left	21	1	1	132																				
Volume Right	1	50	0	57																				
cSH	1156	1158	286	338																				
Volume to Capacity	0.02	0.00	0.02	0.56																				
Queue Length 95th (m)	0.4	0.0	0.4	24.6																				
Control Delay (s)	0.6	0.0	17.8	28.4																				
Lane LOS	A	A	C	D																				
Approach Delay (s)	0.6	0.0	17.8	28.4																				
Approach LOS	C			D																				
Intersection Summary																								
Average Delay													6.0											
Intersection Capacity Utilization	57.3%				ICU Level of Service				B															
Analysis Period (min)													15											

Lanes, Volumes, Timings
201: Irvine St & Street A

Total (2031)
AM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	49	77	22	98	61	16
Future Volume (vph)	49	77	22	98	61	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.917				0.972	
Fit Protected	0.981			0.991		
Satd. Flow (prot)	1709	0	0	1853	1818	0
Fit Permitted	0.981			0.991		
Satd. Flow (perm)	1709	0	0	1853	1818	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	230.9			244.0	206.4	
Travel Time (s)	16.6			17.6	14.9	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Adj. Flow (vph)	53	84	24	107	66	17
Shared Lane Traffic (%)						
Lane Group Flow (vph)	137	0	0	131	83	0
Sign Control	Stop			Free	Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	27.2%
ICU Level of Service	A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
201: Irvine St & Street A

Total (2031)
AM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	49	77	22	98	61	16
Future Volume (Veh/h)	49	77	22	98	61	16
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	53	84	24	107	66	17
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	230	74	83			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	230	74	83			
tC, 2 stage (s)	6.4	6.2	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	92	98			
cM capacity (veh/h)	751	993	1527			

Direction, Lane #	EB 1	NB 1	SB 1
Volume Total	137	131	83
Volume Left	53	24	0
Volume Right	84	0	17
sSH	883	1527	1700
Volume to Capacity	0.16	0.02	0.05
Queue Length 95th (m)	4.2	0.4	0.0
Control Delay (s)	9.8	1.5	0.0
Lane LOS	A	A	
Approach Delay (s)	9.8	1.5	0.0
Approach LOS	A		

Intersection Summary	
Average Delay	4.4
Intersection Capacity Utilization	27.2%
ICU Level of Service	A
Analysis Period (min)	15

Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Total (2031)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (vph)	5	360	250	16	9	2
Future Volume (vph)	5	360	250	16	9	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.992		0.977	
Flt Protected		0.999			0.960	
Satd. Flow (prot)	0	1861	1811	0	1782	0
Flt Permitted		0.999			0.960	
Satd. Flow (perm)	0	1861	1811	0	1782	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	5	391	272	17	10	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	396	289	0	12	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	32.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Total (2031)
PM Peak Hour

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	
Traffic Volume (veh/h)	5	360	250	16	9	2
Future Volume (Veh/h)	5	360	250	16	9	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	391	272	17	10	2
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked					684	282
vC, conflicting volume	291					
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	291				684	282
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				98	100
cM capacity (veh/h)	1280				415	760
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	396	289	12			
Volume Left	5	0	10			
Volume Right	0	17	2			
eSH	1280	1700	449			
Volume to Capacity	0.00	0.17	0.03			
Queue Length 95th (m)	0.1	0.0	0.6			
Control Delay (s)	0.1	0.0	13.2			
Lane LOS	A		B			
Approach Delay (s)	0.1	0.0	13.2			
Approach LOS			B			
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization	32.9%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031)

PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	4	272	93	49	183	4	75	7	53	4	2	7
Future Volume (vph)	4	272	93	49	183	4	75	7	53	4	2	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.966			0.998			0.947			0.923	
Flt Protected					0.990			0.973			0.986	
Satd. Flow (prot)	0	1796	0	0	1817	0	0	1751	0	0	1729	0
Flt Permitted					0.990			0.973			0.986	
Satd. Flow (perm)	0	1796	0	0	1817	0	0	1751	0	0	1729	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			202.8			484.2	
Travel Time (s)		13.7			48.8			14.6			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	296	101	53	199	4	82	8	58	4	2	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	401	0	0	256	0	0	148	0	0	14	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	56.5%
ICU Level of Service	B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031)

PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	4	272	93	49	183	4	75	7	53	4	2	7
Future Volume (Veh/h)	4	272	93	49	183	4	75	7	53	4	2	7
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	296	101	53	199	4	82	8	58	4	2	8
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	203			398			672	664	348	724	713	201
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	203			398			672	664	348	724	713	201
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			95			77	98	92	99	99	99
cM capacity (veh/h)	1381			1088			352	363	700	298	341	845
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	401	256	148	14								
Volume Left	4	53	82	4								
Volume Right	101	4	58	8								
cSH	1381	1088	438	487								
Volume to Capacity	0.00	0.05	0.34	0.03								
Queue Length 95th (m)	0.1	1.2	11.2	0.7								
Control Delay (s)	0.1	2.1	17.3	12.6								
Lane LOS	A	A	C	B								
Approach Delay (s)	0.1	2.1	17.3	12.6								
Approach LOS			C	B								

Intersection Summary

Average Delay	4.1
Intersection Capacity Utilization	56.5%
ICU Level of Service	B
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St

Total (2031)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	16	17	21	164	106	29
Future Volume (vph)	16	17	21	164	106	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.931				0.971	
Flt Protected	0.976			0.994		
Satd. Flow (prot)	1558	0	0	1840	1750	0
Flt Permitted	0.976			0.994		
Satd. Flow (perm)	1558	0	0	1840	1750	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	162.4			289.9	247.6	
Travel Time (s)	11.7			20.9	17.8	
Confl. Peds. (#/hr)			3			3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	21%	0%	3%	0%	25%
Adj. Flow (vph)	17	18	23	178	115	32
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	0	201	147	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	31.1%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St

Total (2031)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	16	17	21	164	106	29
Future Volume (Veh/h)	16	17	21	164	106	29
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	17	18	23	178	115	32
Pedestrians	3					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	358	134	150			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	358	134	150			
tC, single (s)	6.4	6.4	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.5	2.2			
p0 queue free %	97	98	98			
cM capacity (veh/h)	633	865	1440			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	35	201	147			
Volume Left	17	23	0			
Volume Right	18	0	32			
eSH	734	1440	1700			
Volume to Capacity	0.05	0.02	0.09			
Queue Length 95th (m)	1.1	0.4	0.0			
Control Delay (s)	10.2	1.0	0.0			
Lane LOS	B	A				
Approach Delay (s)	10.2	1.0	0.0			
Approach LOS	B					
Intersection Summary						
Average Delay		1.4				
Intersection Capacity Utilization	31.1%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Total (2031)
PM Peak Hour

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	25	7	10	41	1	9
Future Volume (vph)	25	7	10	41	1	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.969				0.877	
Flt Protected				0.990	0.995	
Satd. Flow (prot)	1354	0	0	1741	1533	0
Flt Permitted				0.990	0.995	
Satd. Flow (perm)	1354	0	0	1741	1533	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	88.4			162.4	207.7	
Travel Time (s)	6.4			11.7	15.0	
Confl. Peds. (#/hr)		4	4		2	1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	100%	0%	10%	0%	9%
Adj. Flow (vph)	27	8	11	45	1	10
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	0	56	11	0
Sign Control	Free			Free	Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	19.7%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Total (2031)
PM Peak Hour

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	25	7	10	41	1	9
Future Volume (Veh/h)	25	7	10	41	1	9
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	8	11	45	1	10
Pedestrians	2			1	4	
Lane Width (m)	3.6			3.6	3.6	
Walking Speed (m/s)	1.2			1.2	1.2	
Percent Blockage	0			0	0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			39		104	36
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			39		104	36
tC, single (s)			4.1		6.4	6.3
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.4
p0 queue free %			99		100	99
cM capacity (veh/h)			1579		888	1013

Direction, Lane #	EB 1	WB 1	NB 1
Volume Total	35	56	11
Volume Left	0	11	1
Volume Right	8	0	10
sSH	1700	1579	1000
Volume to Capacity	0.02	0.01	0.01
Queue Length 95th (m)	0.0	0.2	0.3
Control Delay (s)	0.0	1.5	8.6
Lane LOS		A	A
Approach Delay (s)	0.0	1.5	8.6
Approach LOS			A

Intersection Summary

Average Delay		1.7	
Intersection Capacity Utilization	19.7%	ICU Level of Service	A
Analysis Period (min)		15	

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Total (2031)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	0	17	12	9	18	15	11	0	6	9	0	0
Future Volume (vph)	0	17	12	9	18	15	11	0	6	9	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.943			0.953			0.950				
Flt Protected					0.989			0.969			0.950	
Satd. Flow (prot)	0	1454	0	0	1661	0	0	1001	0	0	1805	0
Flt Permitted					0.989			0.969			0.950	
Satd. Flow (perm)	0	1454	0	0	1661	0	0	1001	0	0	1805	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		123.8			88.4			134.2			45.0	
Travel Time (s)		8.9			6.4			9.7			3.2	
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	18	13	10	20	16	12	0	7	10	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	31	0	0	46	0	0	19	0	0	10	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	21.1% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Total (2031)
PM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	0	17	12	9	18	15	11	0	6	9	0	0
Future Volume (Veh/h)	0	17	12	9	18	15	11	0	6	9	0	0
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	18	13	10	20	16	12	0	7	10	0	0
Pedestrians		1			4			9			5	
Lane Width (m)		3.6			3.6			3.6			3.6	
Walking Speed (m/s)		1.2			1.2			1.2			1.2	
Percent Blockage		0			0			1			0	
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	41			40			82	94	38	88	93	34
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	41			40			82	94	38	88	93	34
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			99			98	100	99	99	100	100
cM capacity (veh/h)	1575			1571			765	785	804	875	787	1040
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	31	46	19	10								
Volume Left	0	10	12	10								
Volume Right	13	16	7	0								
eSH	1575	1571	779	875								
Volume to Capacity	0.00	0.01	0.02	0.01								
Queue Length 95th (m)	0.0	0.1	0.6	0.3								
Control Delay (s)	0.0	1.6	9.7	9.2								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	1.6	9.7	9.2								
Approach LOS			A	A								

Intersection Summary	
Average Delay	3.3
Intersection Capacity Utilization	21.1% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Total (2031)
PM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	60	216	95	37	304	135
Future Volume (vph)	60	216	95	37	304	135
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.894	0.962				
Flt Protected	0.989				0.967	
Satd. Flow (prot)	1631	0	1803	0	0	1795
Flt Permitted	0.989				0.967	
Satd. Flow (perm)	1631	0	1803	0	0	1795
Link Speed (k/h)	50		50		50	
Link Distance (m)	111.8		38.3		111.4	
Travel Time (s)	8.0		2.8		8.0	
Confl. Peds. (#/hr)				5	5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	3%	0%	5%	3%	1%
Adj. Flow (vph)	65	235	103	40	330	147
Shared Lane Traffic (%)						
Lane Group Flow (vph)	300	0	143	0	0	477
Sign Control	Stop		Free			Free

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	58.9% ICU Level of Service B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Total (2031)
PM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	60	216	95	37	304	135
Future Volume (Veh/h)	60	216	95	37	304	135
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	65	235	103	40	330	147
Pedestrians	5					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	935	128			148	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	935	128			148	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	71	74			77	
cM capacity (veh/h)	224	916			1422	

Direction, Lane #	WB 1	NB 1	SB 1
Volume Total	300	143	477
Volume Left	65	0	330
Volume Right	235	40	0
sSH	549	1700	1422
Volume to Capacity	0.55	0.08	0.23
Queue Length 95th (m)	24.9	0.0	6.8
Control Delay (s)	19.2	0.0	6.4
Lane LOS	C		A
Approach Delay (s)	19.2	0.0	6.4
Approach LOS	C		

Intersection Summary			
Average Delay		9.6	
Intersection Capacity Utilization	58.9%	ICU Level of Service	B
Analysis Period (min)	15		

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Total (2031)
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	37	255	7	8	200	81	1	104	21	72	83	26
Future Volume (vph)	37	255	7	8	200	81	1	104	21	72	83	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.997			0.962			0.977			0.981	
Flt Protected		0.994			0.999						0.980	
Satd. Flow (prot)	0	1829	0	0	1801	0	0	1582	0	0	1566	0
Flt Permitted		0.994			0.999						0.980	
Satd. Flow (perm)	0	1829	0	0	1801	0	0	1582	0	0	1566	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	1		15	15		1			22	22		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	17%	0%	2%	0%	0%	21%	0%	6%	31%	0%
Adj. Flow (vph)	40	277	8	9	217	88	1	113	23	78	90	28
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	325	0	0	314	0	0	137	0	0	196	0
Sign Control		Stop			Stop			Stop			Stop	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	62.2% ICU Level of Service B
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Total (2031)
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	37	255	7	8	200	81	1	104	21	72	83	26
Future Volume (vph)	37	255	7	8	200	81	1	104	21	72	83	26
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	40	277	8	9	217	88	1	113	23	78	90	28
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	325	314	137	196								
Volume Left (vph)	40	9	1	78								
Volume Right (vph)	8	88	23	28								
Hadj (s)	0.06	-0.14	0.20	0.28								
Departure Headway (s)	5.6	5.4	6.3	6.2								
Degree Utilization, x	0.50	0.47	0.24	0.34								
Capacity (veh/h)	608	625	495	517								
Control Delay (s)	14.0	13.1	11.3	12.4								
Approach Delay (s)	14.0	13.1	11.3	12.4								
Approach LOS	B	B	B	B								
Intersection Summary												
Delay		13.0										
Level of Service		B										
Intersection Capacity Utilization		62.2%		ICU Level of Service						B		
Analysis Period (min)		15										

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Total (2031)
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR				
Lane Configurations		↔			↔			↔			↔					
Traffic Volume (vph)	57	239	0	0	294	68	0	0	0	62	0	37				
Future Volume (vph)	57	239	0	0	294	68	0	0	0	62	0	37				
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900				
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Ped Bike Factor																
Frt	0.975															
Flt Protected	0.990															
Satd. Flow (prot)	0	1845	0	0	1703	0	0	1900	0	0	1407	0				
Flt Permitted	0.990															
Satd. Flow (perm)	0	1845	0	0	1703	0	0	1900	0	0	1407	0				
Link Speed (k/h)	50															
Link Distance (m)	497.2				520.7				62.2				427.7			
Travel Time (s)	35.8				37.5				4.5				30.8			
Confl. Peds. (#/hr)	54			54												
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92				
Heavy Vehicles (%)	6%	1%	0%	0%	5%	25%	0%	0%	0%	39%	0%	0%				
Adj. Flow (vph)	62	260	0	0	320	74	0	0	0	67	0	40				
Shared Lane Traffic (%)																
Lane Group Flow (vph)	0	322	0	0	394	0	0	0	0	0	107	0				
Sign Control	Free			Free			Stop			Stop						
Intersection Summary																
Area Type:	Other															
Control Type:	Unsignalized															
Intersection Capacity Utilization	51.0%				ICU Level of Service A											
Analysis Period (min)	15															

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Total (2031)
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	57	239	0	0	294	68	0	0	0	62	0	37
Future Volume (Veh/h)	57	239	0	0	294	68	0	0	0	62	0	37
Sign Control	Free			Free			Stop			Stop		
Grade	0%											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	62	260	0	0	320	74	0	0	0	67	0	40
Pedestrians	54											
Lane Width (m)	3.6											
Walking Speed (m/s)	1.2											
Percent Blockage	5											
Right turn flare (veh)												
Median type	None						None					
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	394			314			835	832	314	741	795	357
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	394			314			835	832	314	741	795	357
tC, single (s)	4.2			4.1			7.1	6.5	6.2	7.5	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.3			2.2			3.5	4.0	3.3	3.9	4.0	3.3
p0 queue free %	95			100			100	100	100	75	100	94
cM capacity (veh/h)	1143			1201			241	277	698	268	291	692
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	322	394	0	107								
Volume Left	62	0	0	67								
Volume Right	0	74	0	40								
eSH	1143	1201	1700	348								
Volume to Capacity	0.05	0.00	0.00	0.31								
Queue Length 95th (m)	1.3	0.0	0.0	9.7								
Control Delay (s)	2.0	0.0	0.0	19.9								
Lane LOS	A			C								
Approach Delay (s)	2.0	0.0	0.0	19.9								
Approach LOS			A	C								
Intersection Summary												
Average Delay	3.4											
Intersection Capacity Utilization	51.0%				ICU Level of Service				A			
Analysis Period (min)	15											

Lanes, Volumes, Timings
201: Irvine St & Street A

Total (2031)
PM Peak Hour

Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (vph)	35	48	80	100	87	57
Future Volume (vph)	35	48	80	100	87	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.922				0.947	
Flt Protected	0.979			0.978		
Satd. Flow (prot)	1715	0	0	1838	1778	0
Flt Permitted	0.979			0.978		
Satd. Flow (perm)	1715	0	0	1838	1778	0
Link Speed (k/h)	50			50	50	
Link Distance (m)	256.6			247.6	202.8	
Travel Time (s)	18.5			17.8	14.6	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Adj. Flow (vph)	38	52	87	109	95	62
Shared Lane Traffic (%)						
Lane Group Flow (vph)	90	0	0	196	157	0
Sign Control	Stop			Free	Free	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	32.6%
ICU Level of Service	A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
201: Irvine St & Street A

Total (2031)
PM Peak Hour

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Volume (veh/h)	35	48	80	100	87	57
Future Volume (Veh/h)	35	48	80	100	87	57
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	52	87	109	95	62
Pedestrians						
Lane Width (m)						
Walking Speed (m/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	409	126	157			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	409	126	157			
tC, 2 stage (s)	6.4	6.2	4.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	93	94	94			
cM capacity (veh/h)	566	930	1435			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	90	196	157			
Volume Left	38	87	0			
Volume Right	52	0	62			
sSH	731	1435	1700			
Volume to Capacity	0.12	0.06	0.09			
Queue Length 95th (m)	3.2	1.5	0.0			
Control Delay (s)	10.6	3.7	0.0			
Lane LOS	B	A				
Approach Delay (s)	10.6	3.7	0.0			
Approach LOS	B					

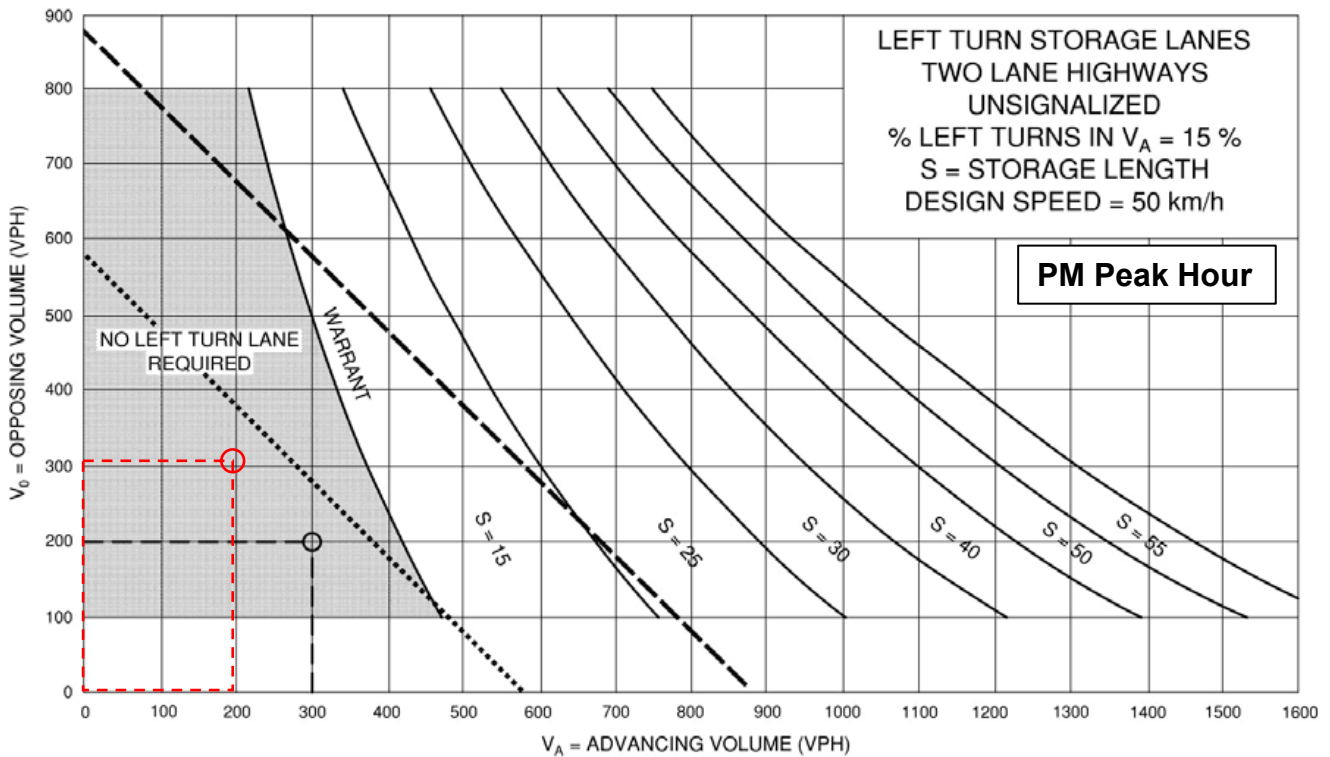
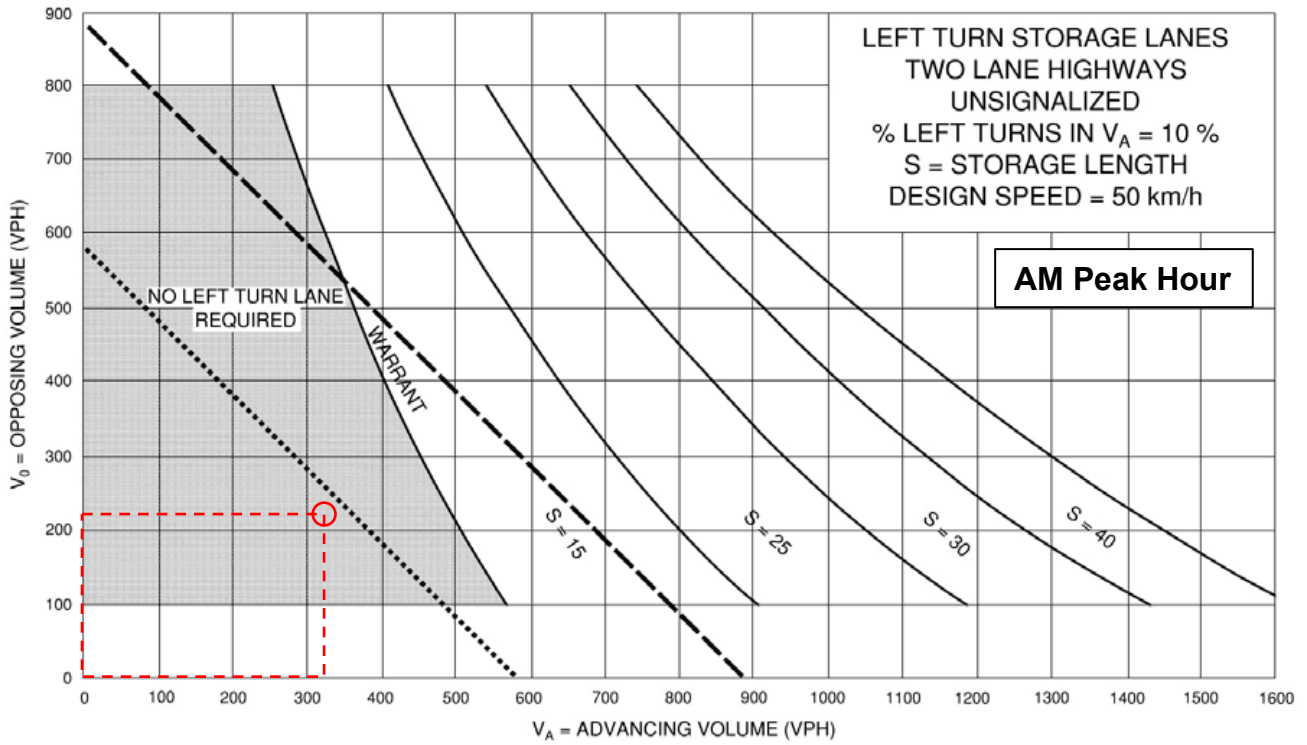
Intersection Summary

Average Delay	3.8
Intersection Capacity Utilization	32.6%
ICU Level of Service	A
Analysis Period (min)	15

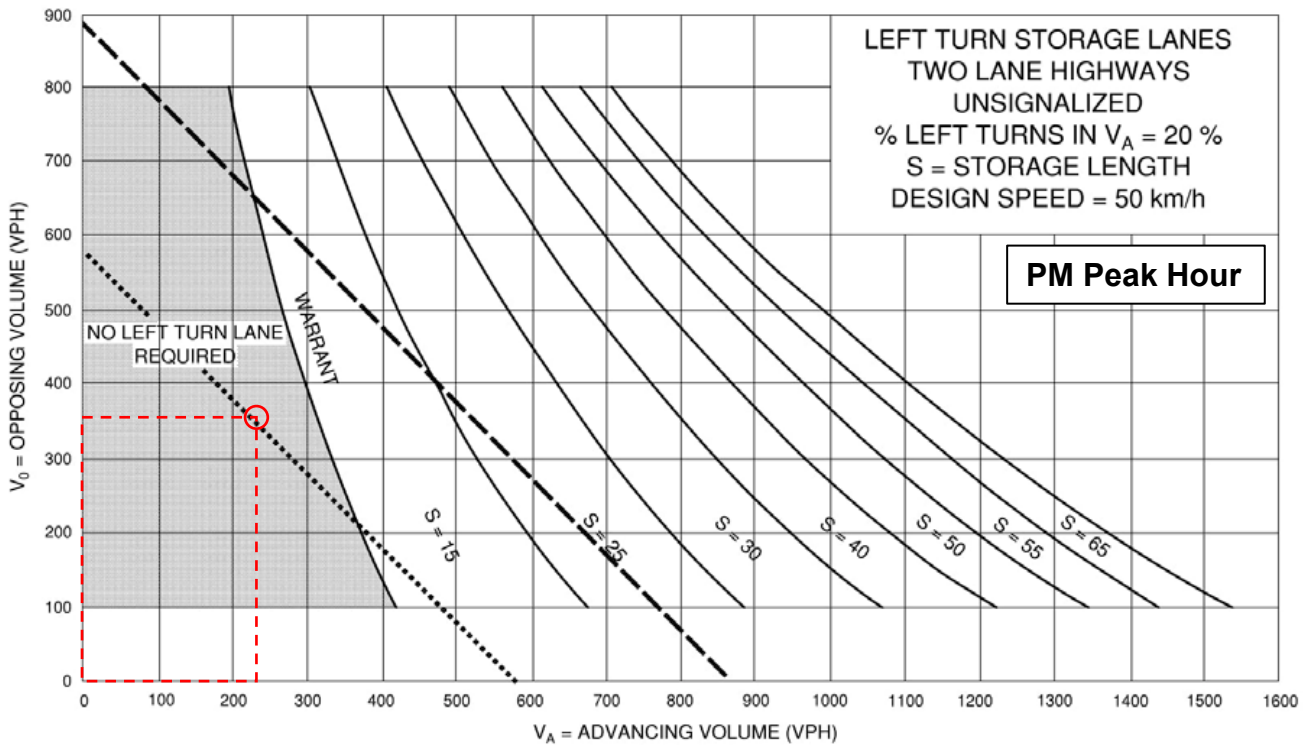
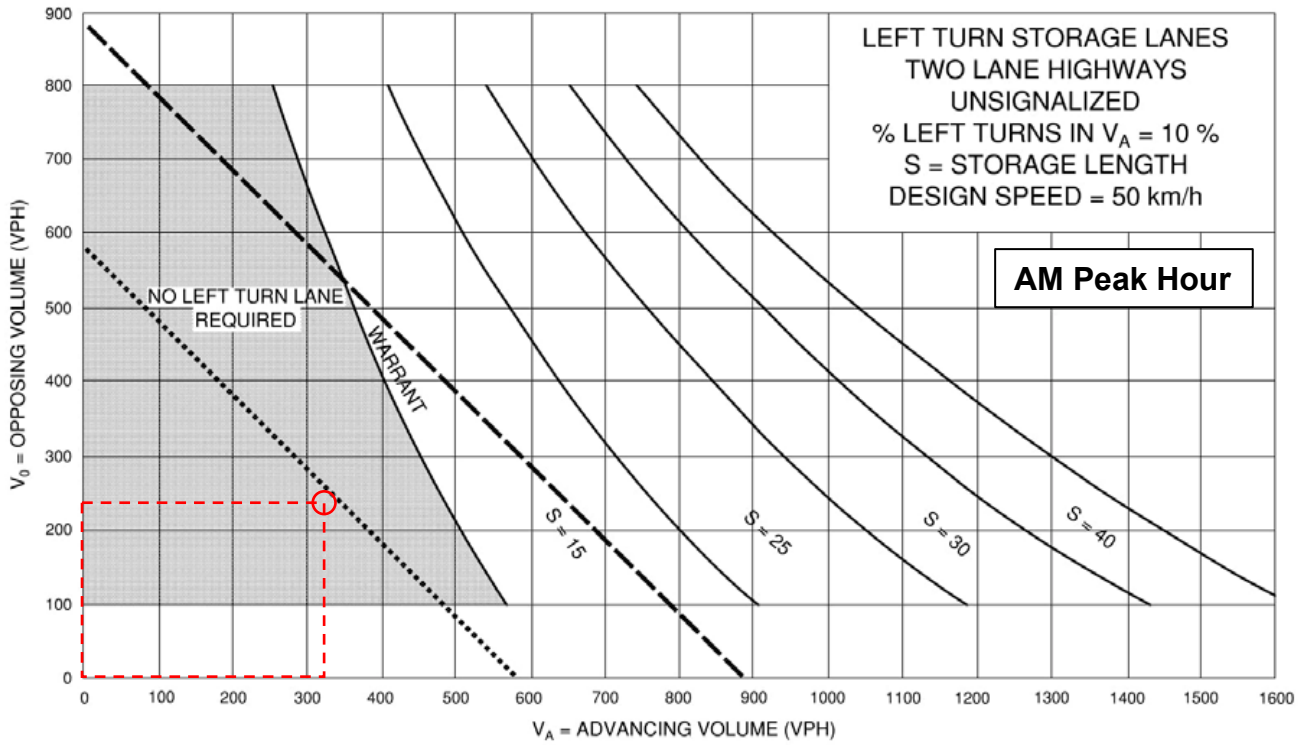
Appendix G

Left-Turn Lane Warrant Nomographs

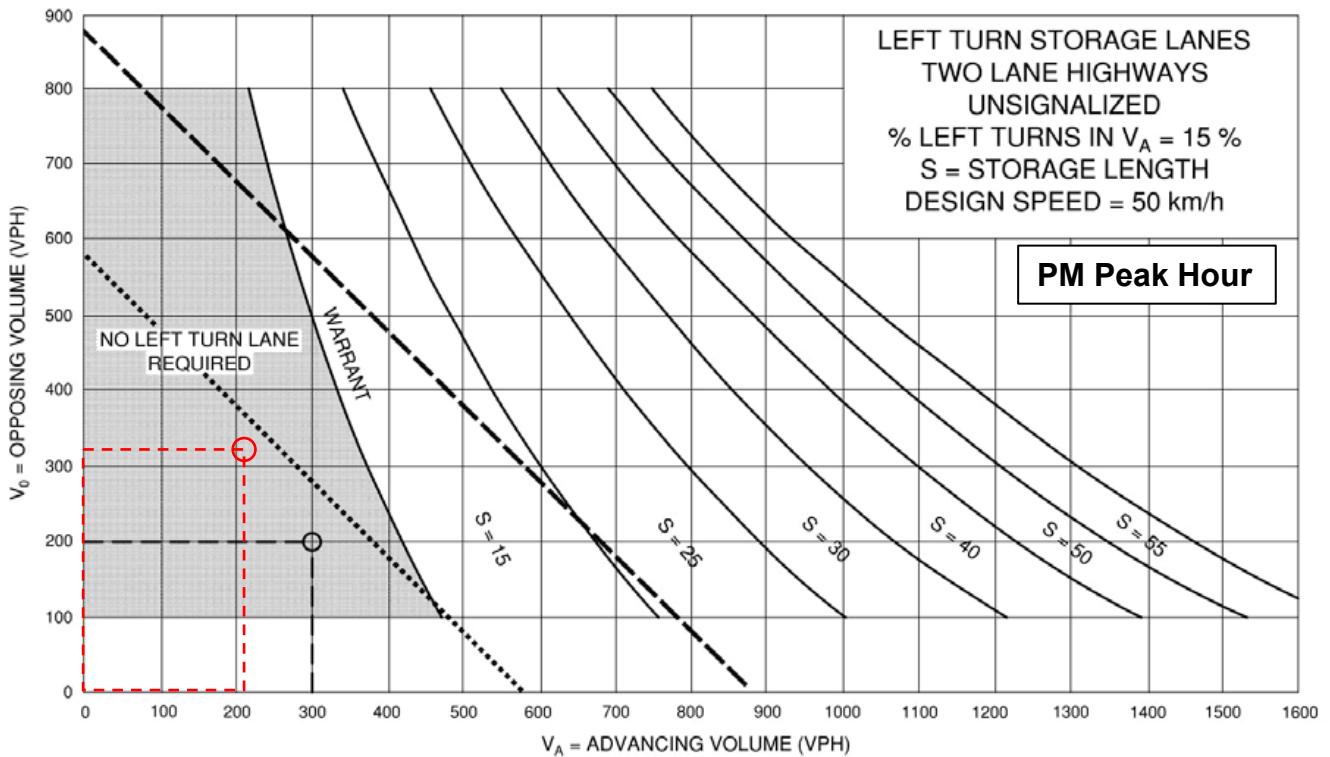
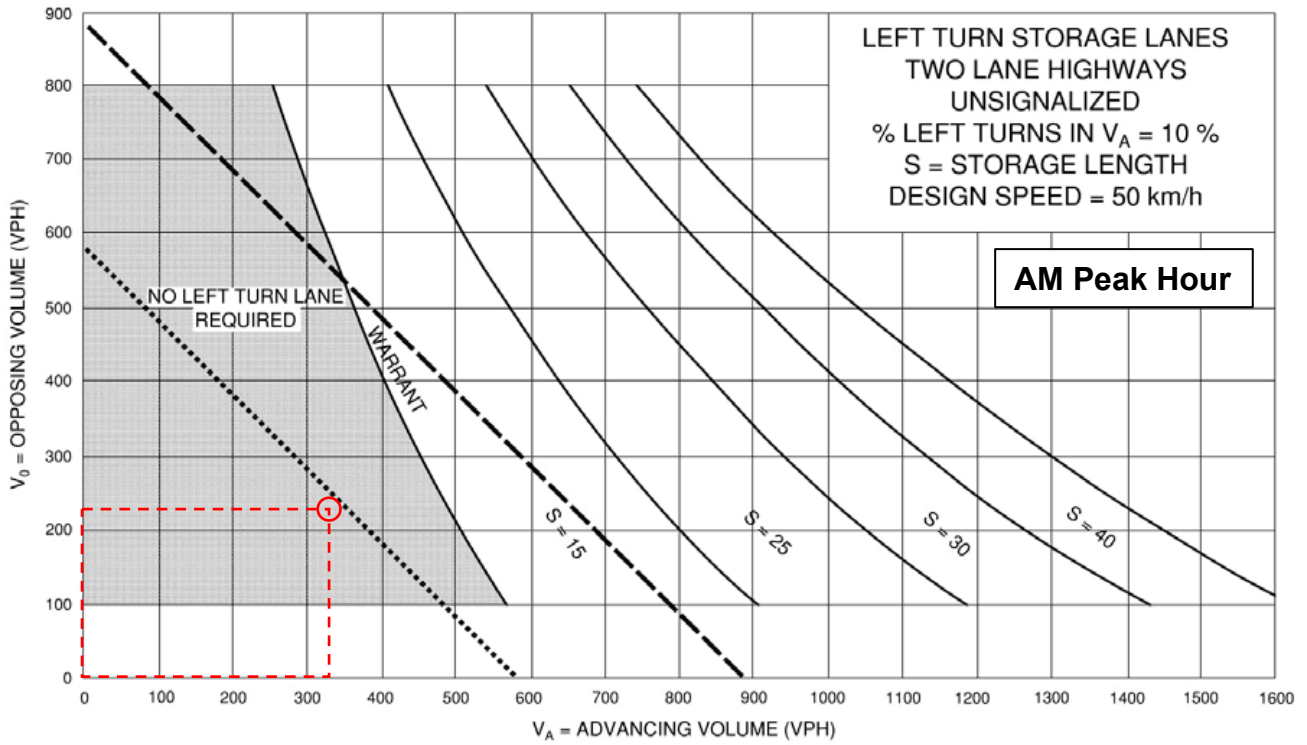




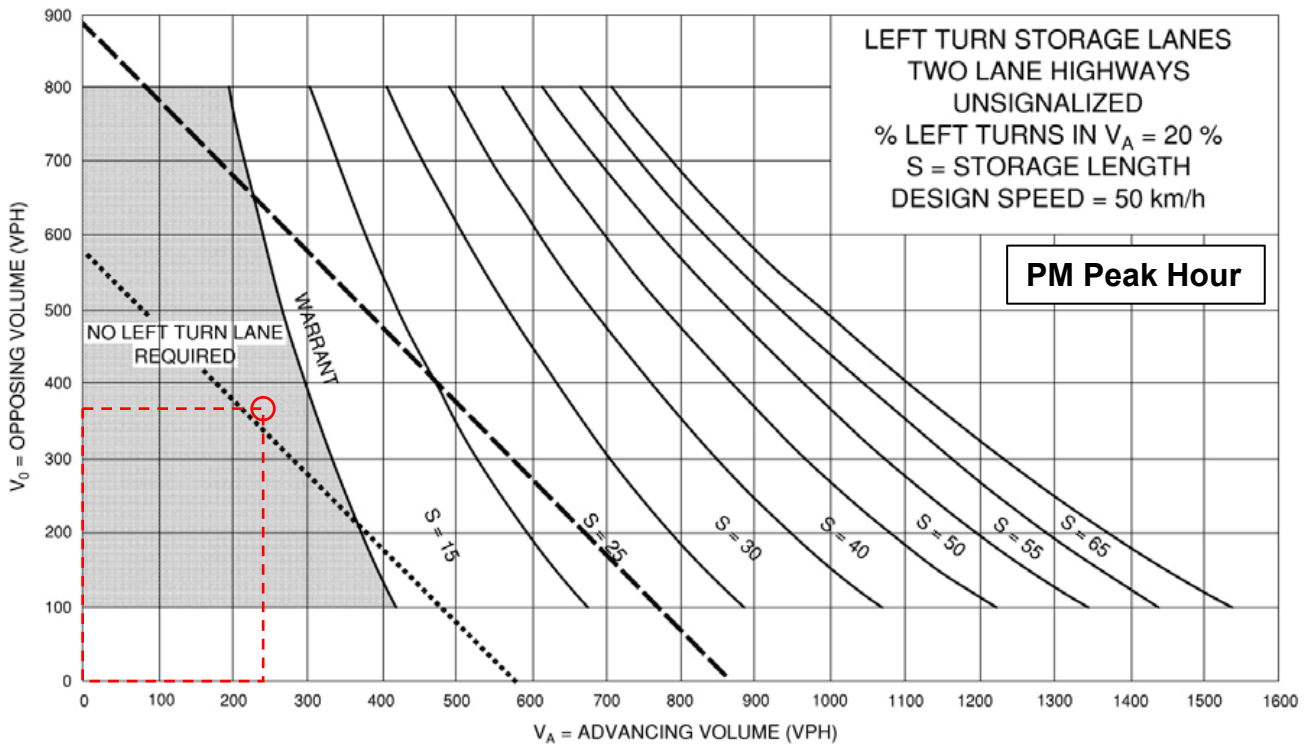
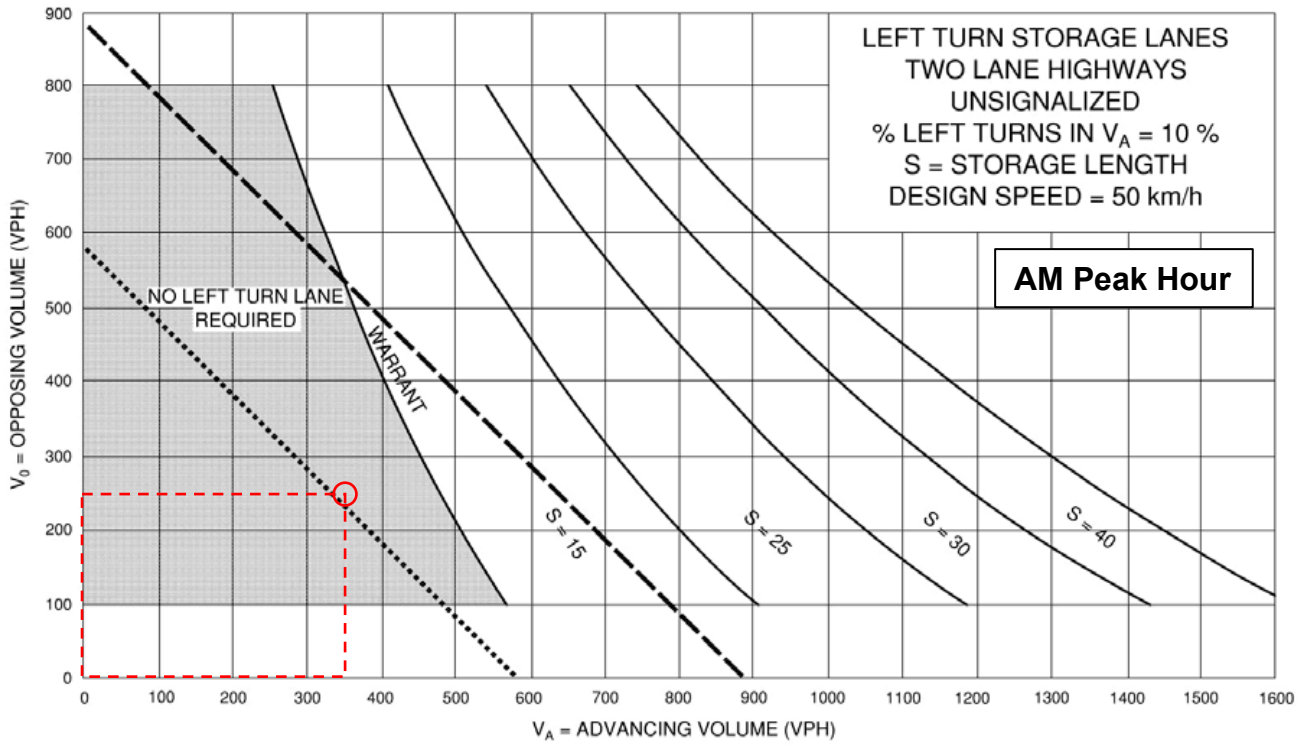
Westbound Left-Turn Lane Woolwich St/Nichol Rd 15 & Irvine St 2026 Background



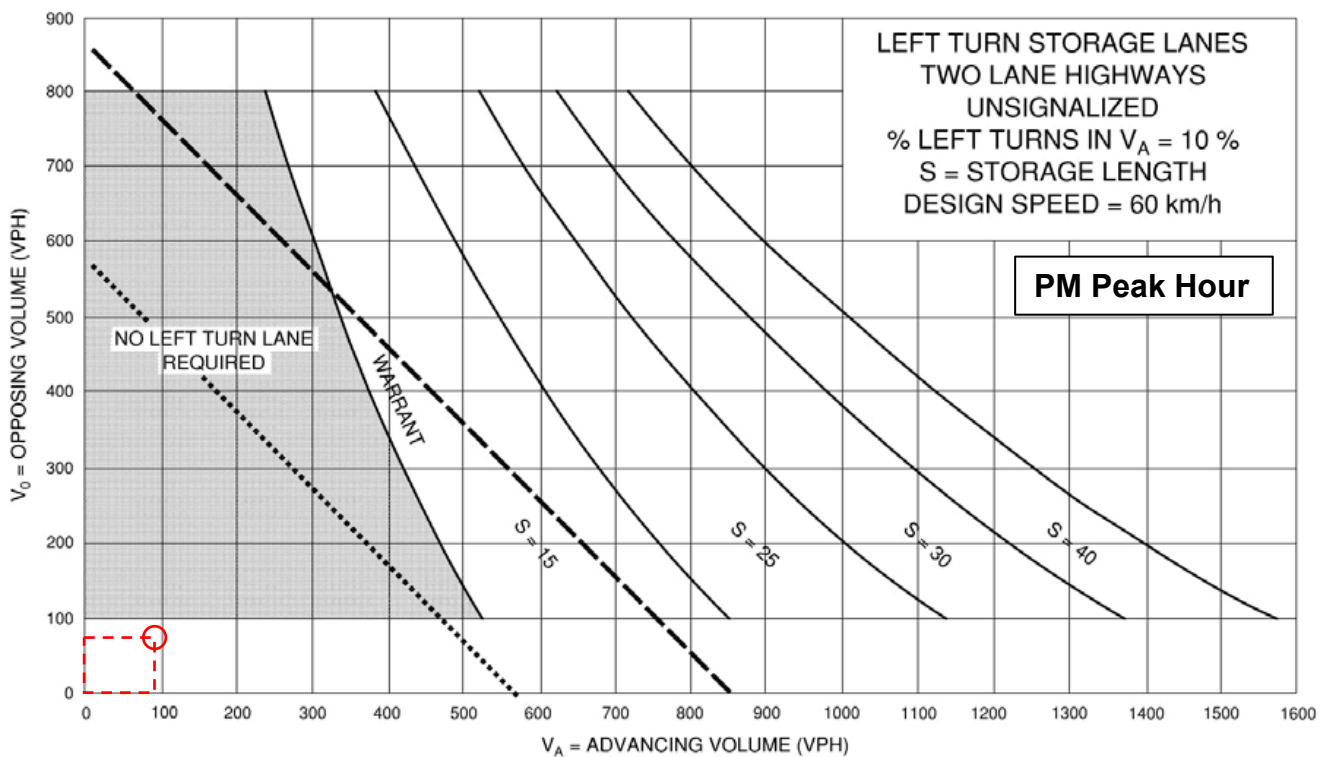
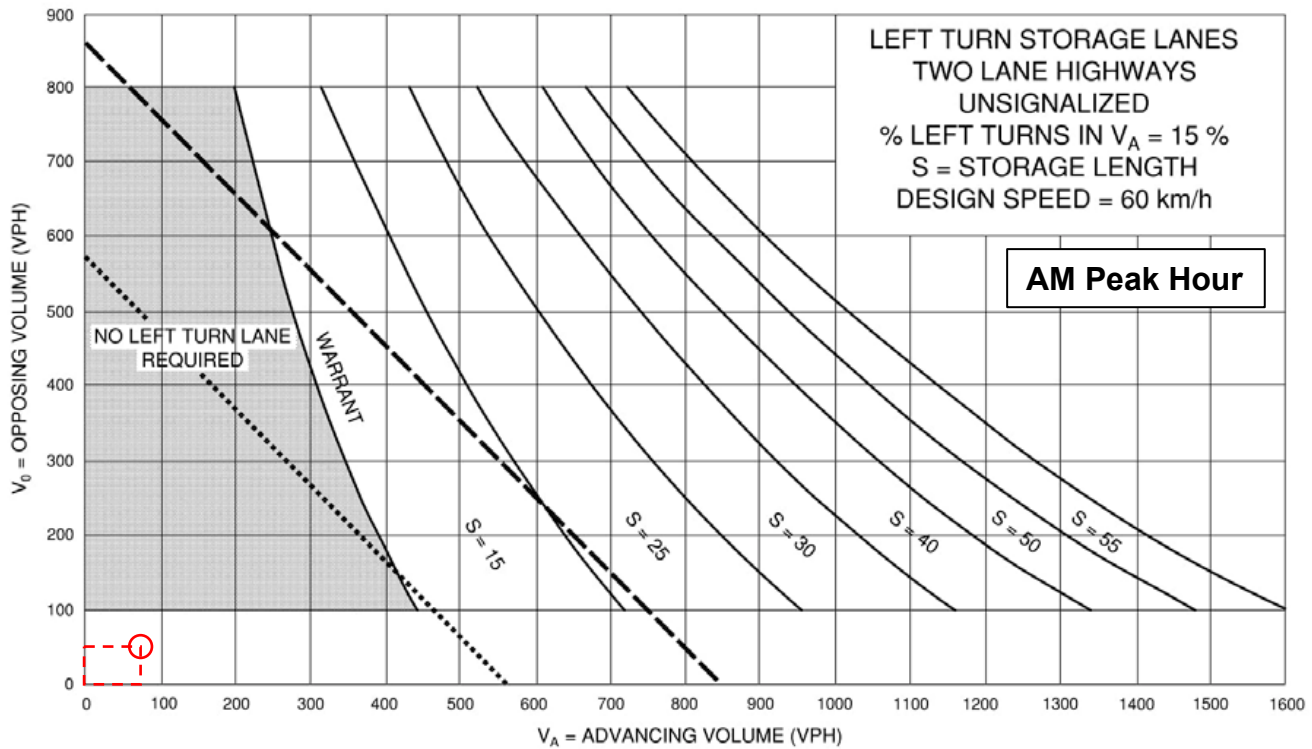
Westbound Left-Turn Lane Woolwich St/Nichol Rd 15 & Irvine St 2026 Total



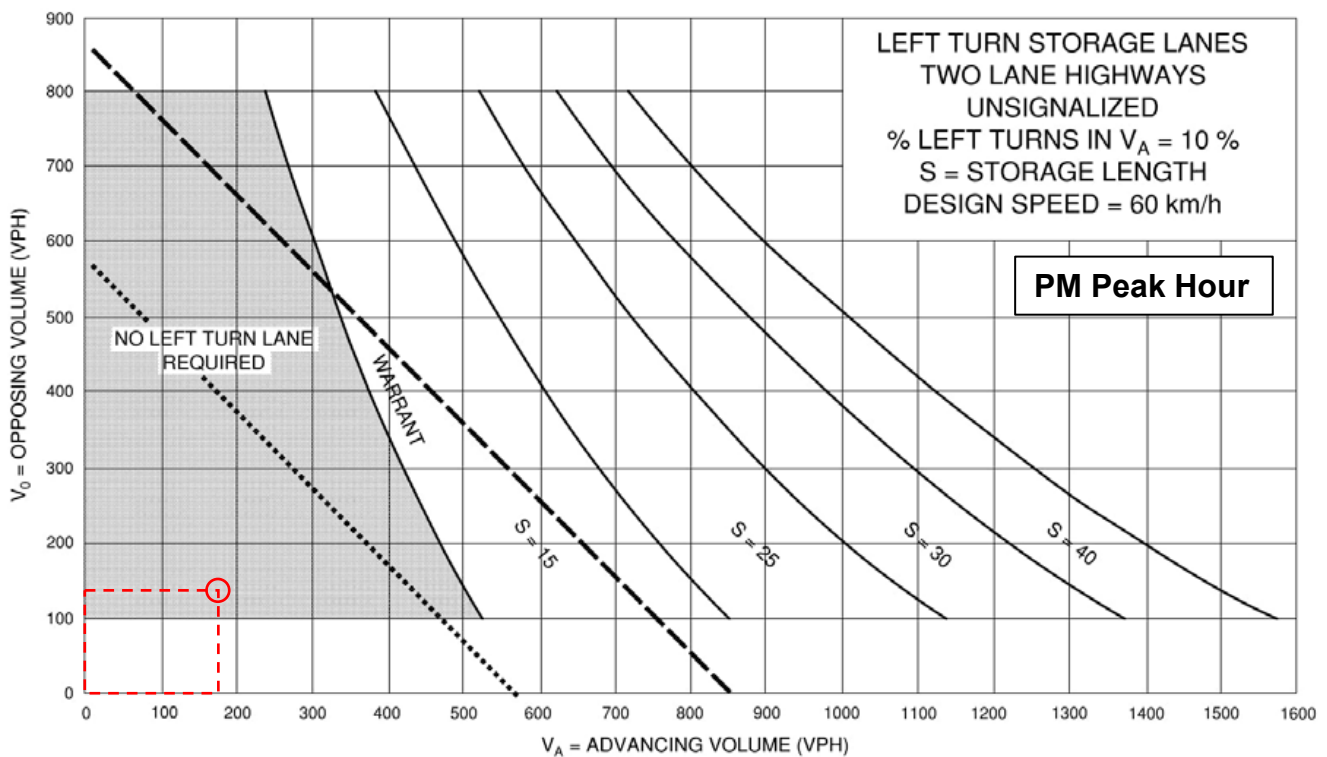
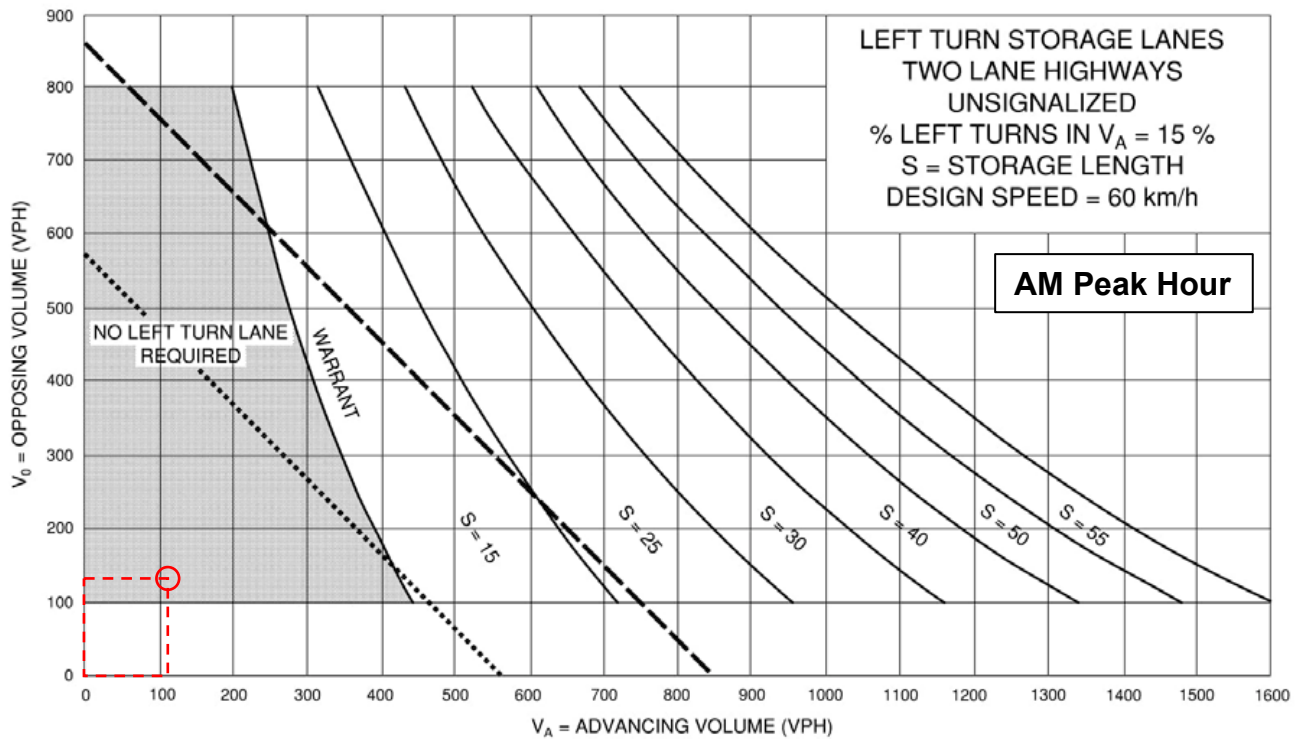
Westbound Left-Turn Lane Woolwich St/Nichol Rd 15 & Irvine St 2031 Background



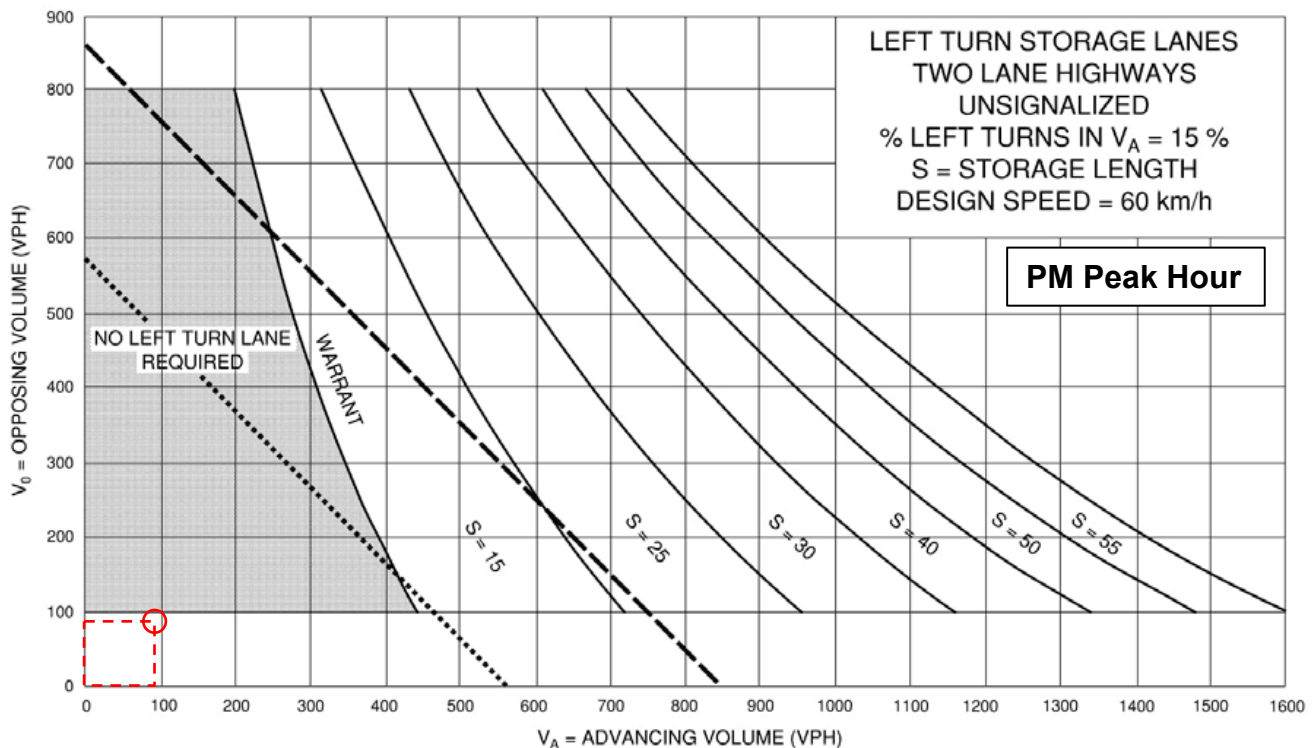
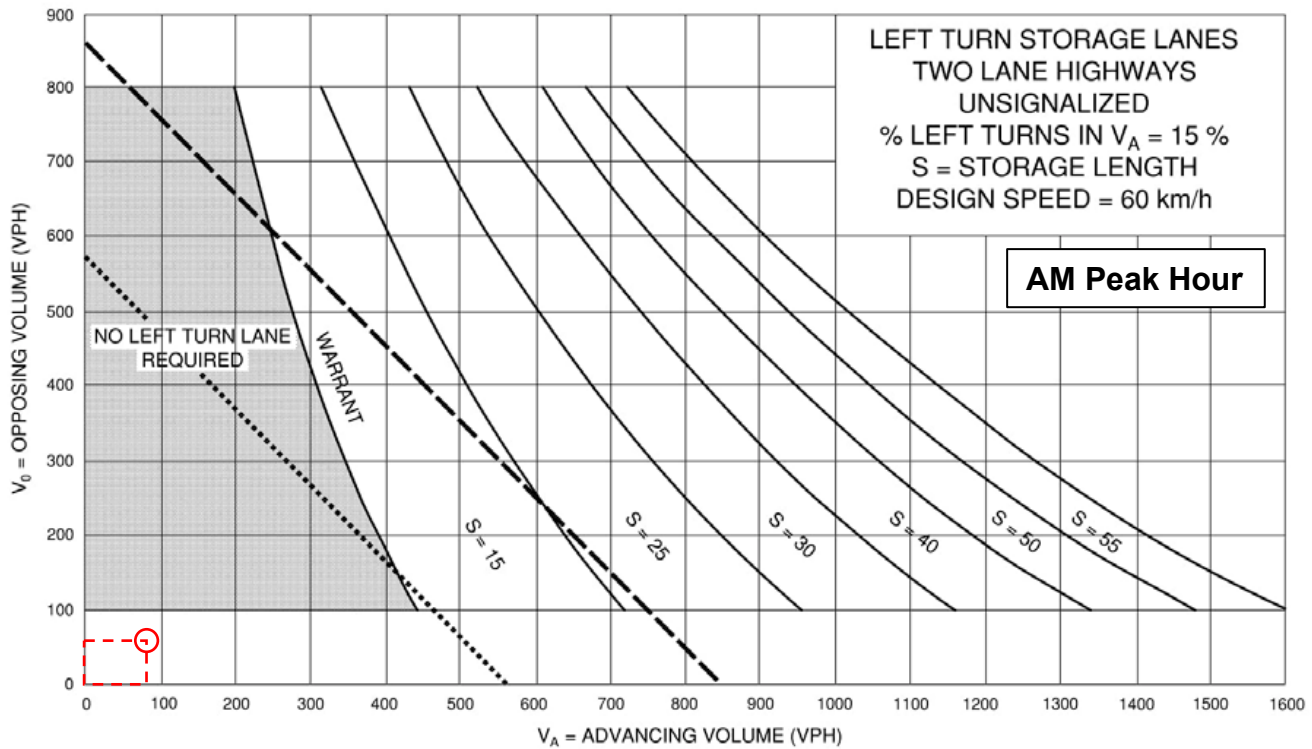
Westbound Left-Turn Lane Woolwich St/Nichol Rd 15 & Irvine St 2031 Total



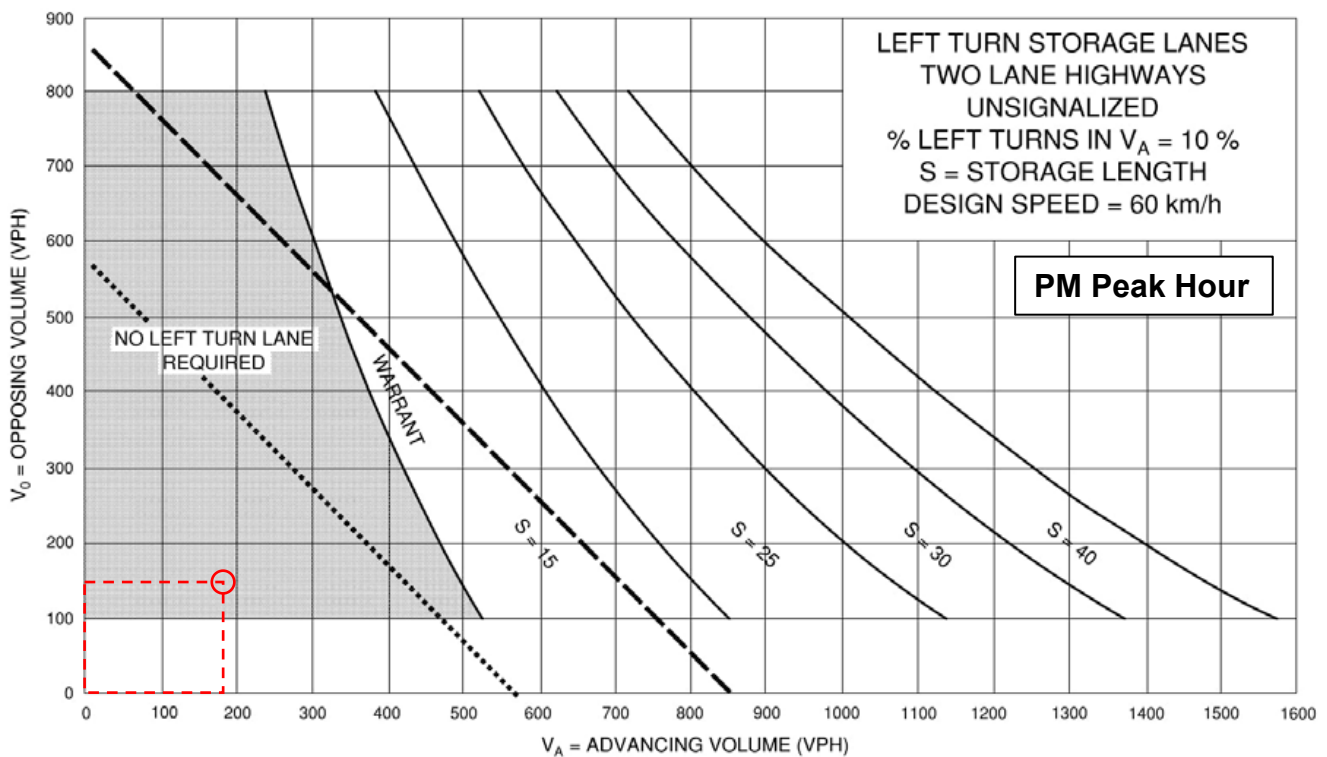
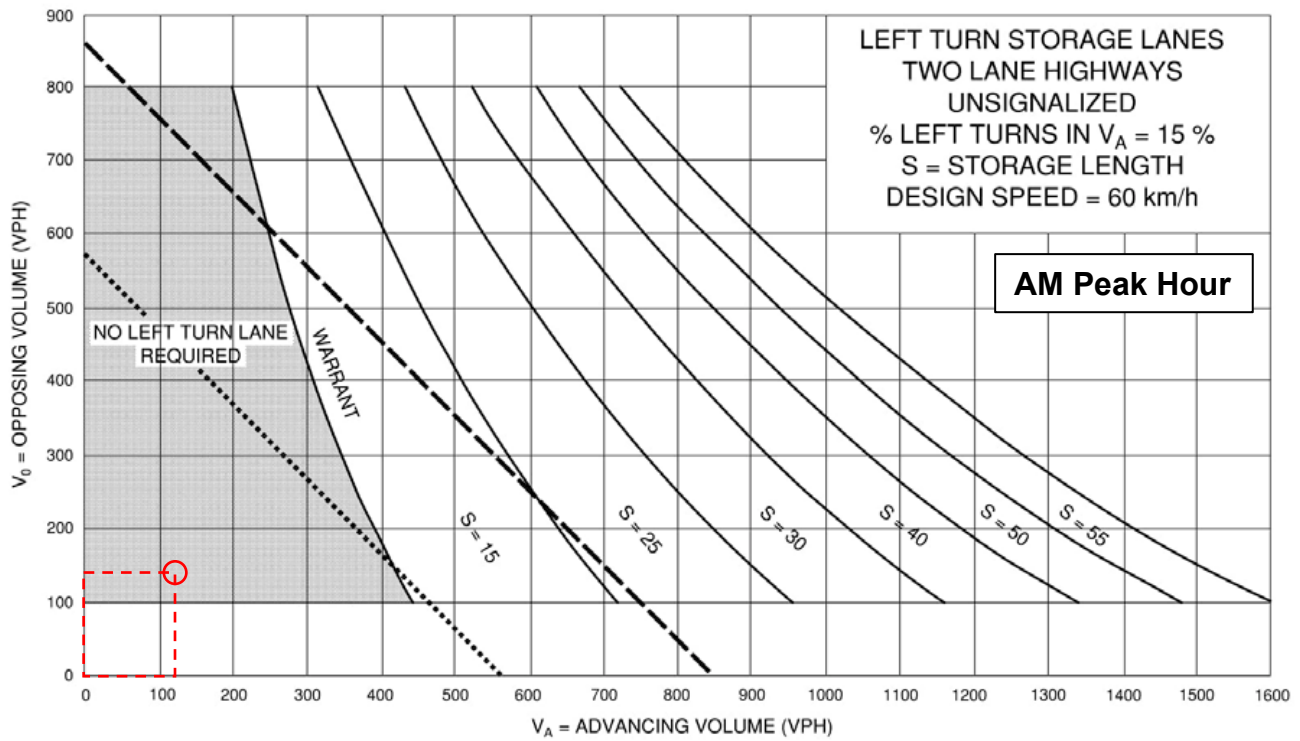
Northbound Left-Turn Lane Irvine St & Bricker Ave 2026 Background



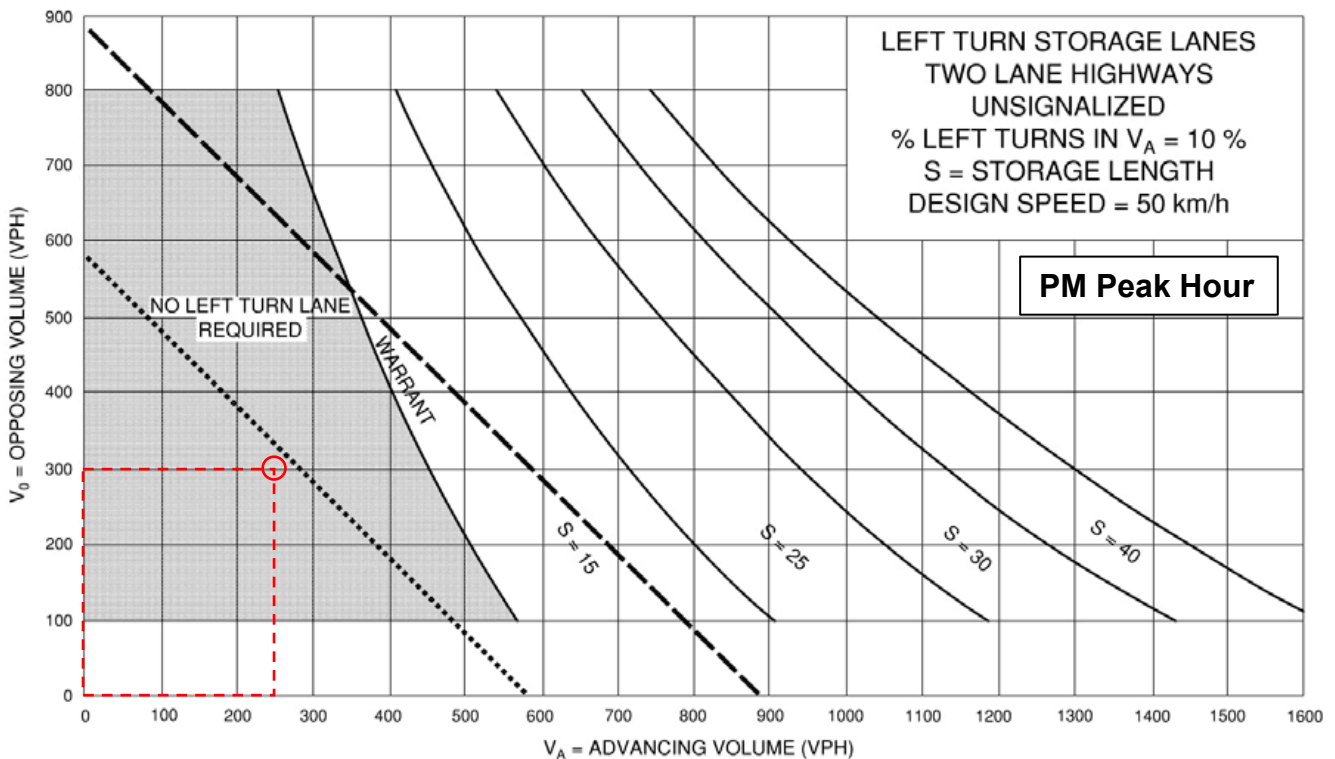
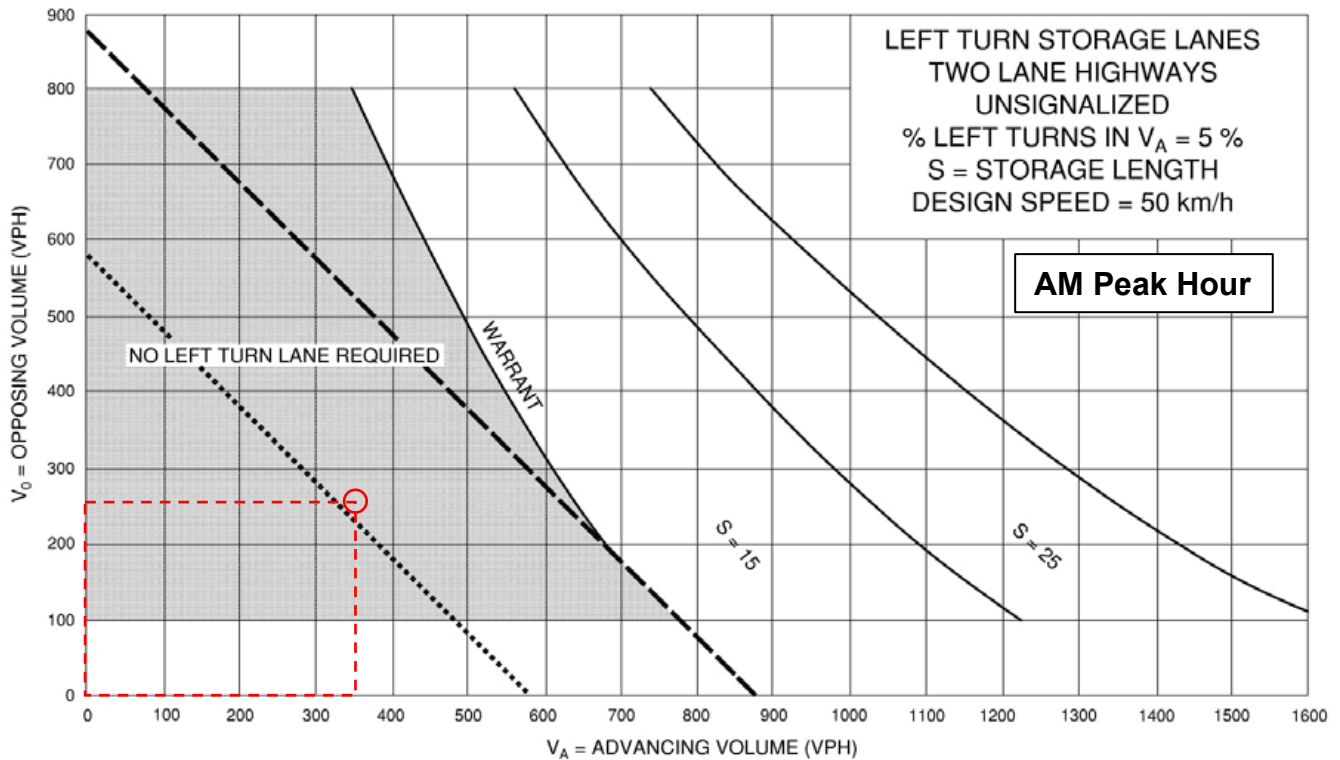
Northbound Left-Turn Lane Irvine St & Bricker Ave 2026 Total



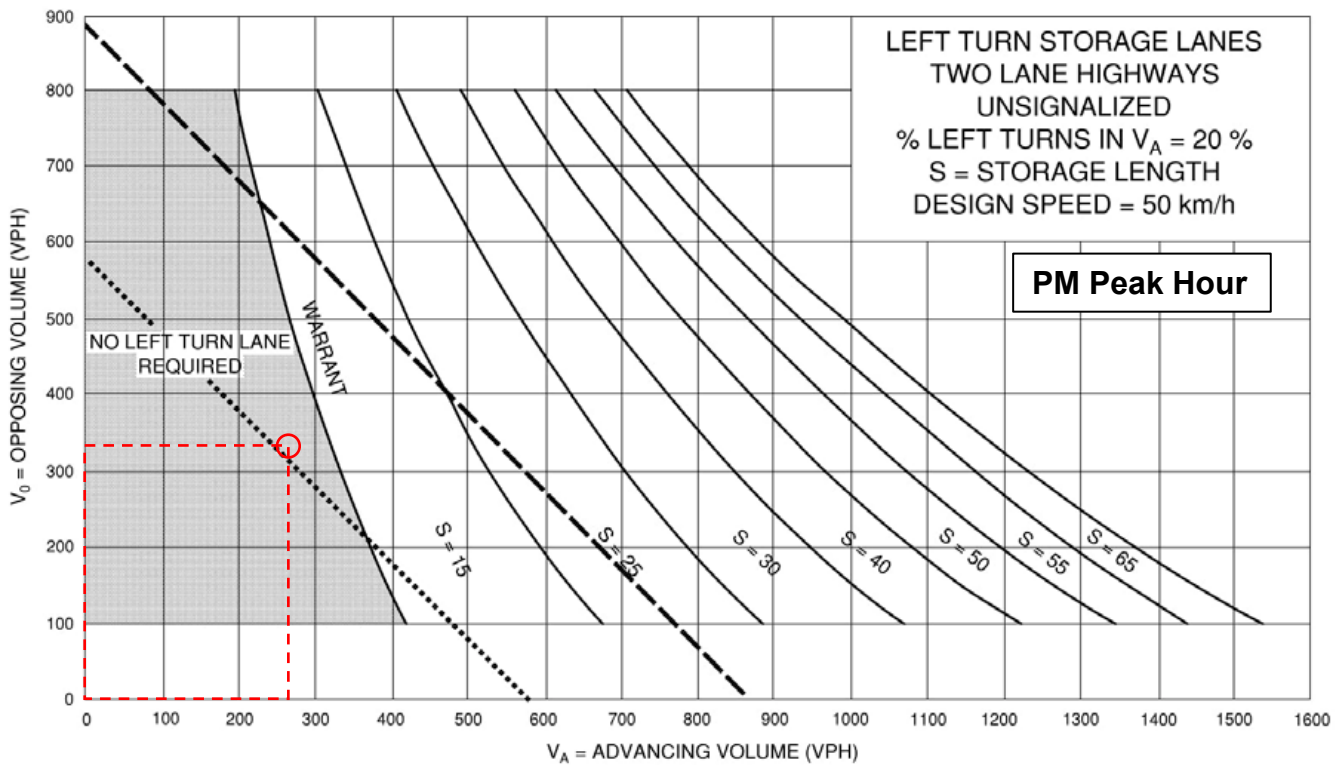
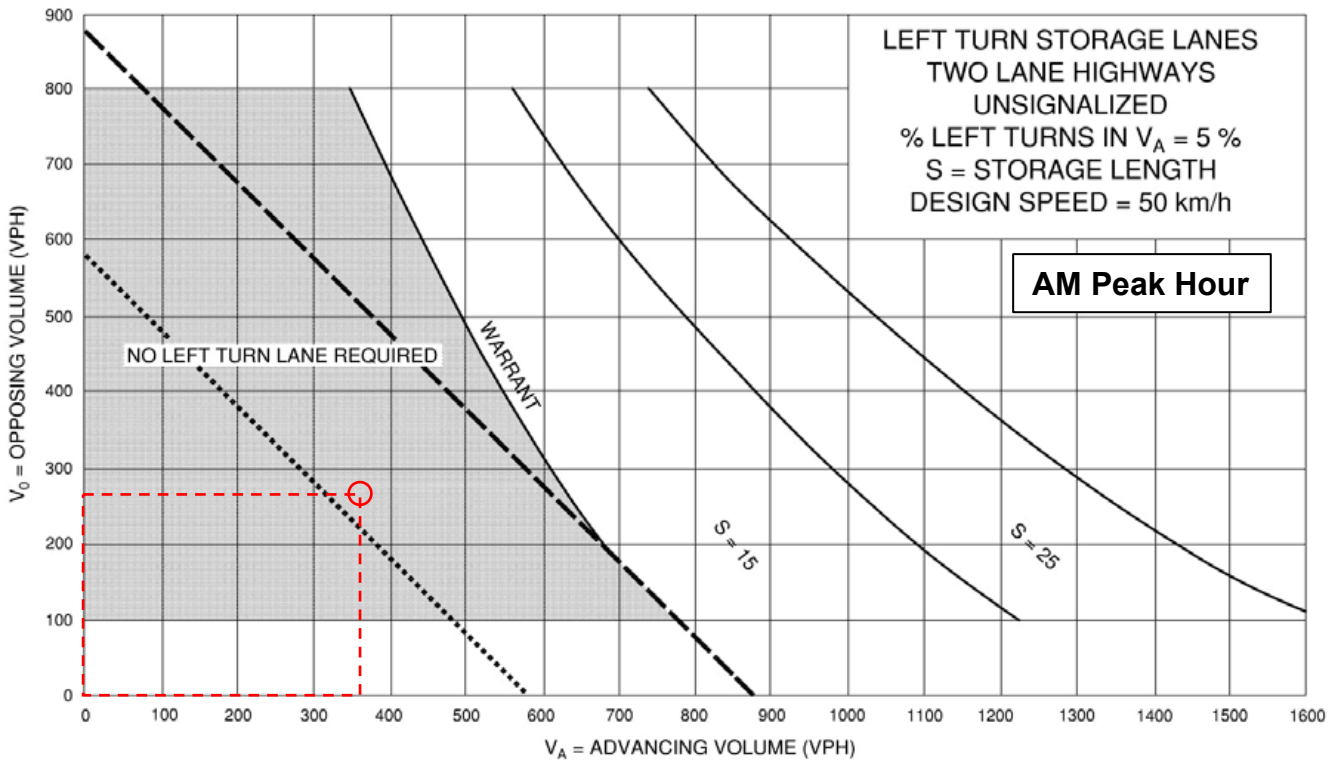
Northbound Left-Turn Lane Irvine St & Bricker Ave 2031 Background



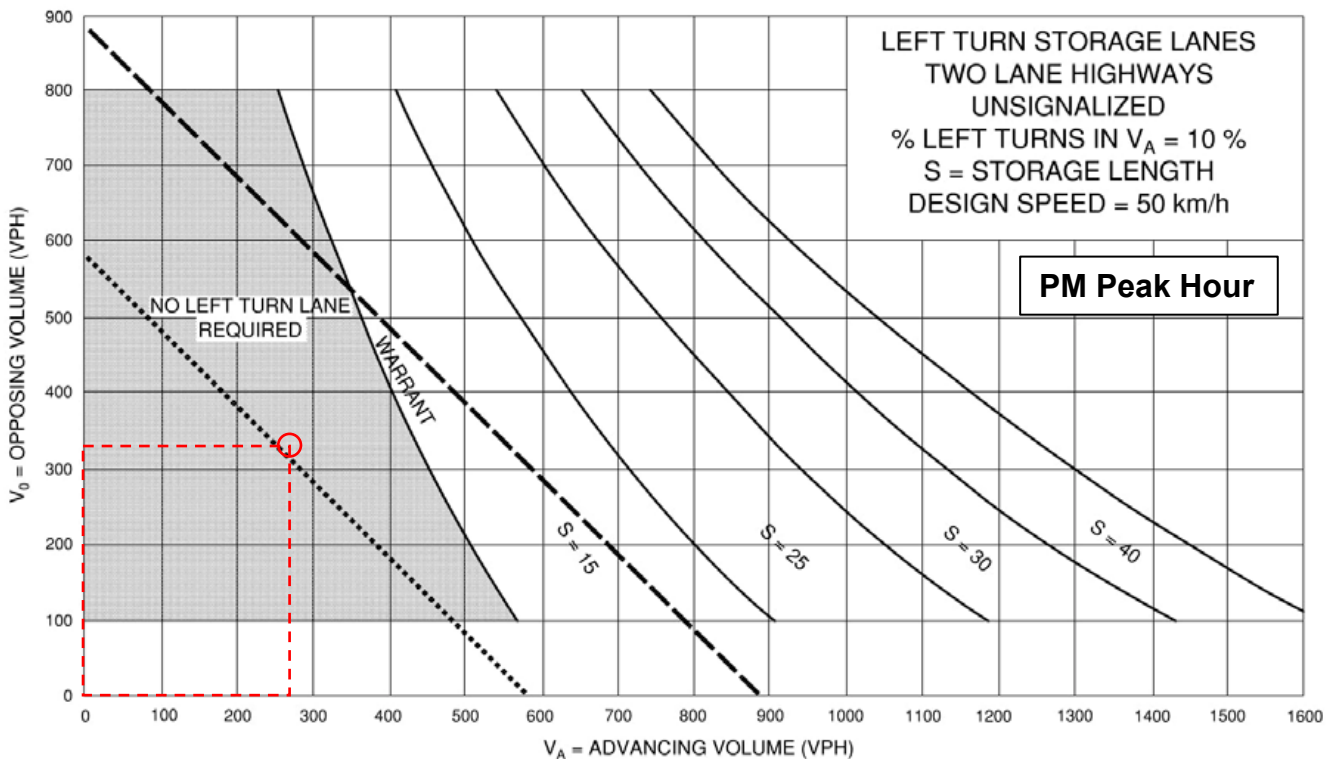
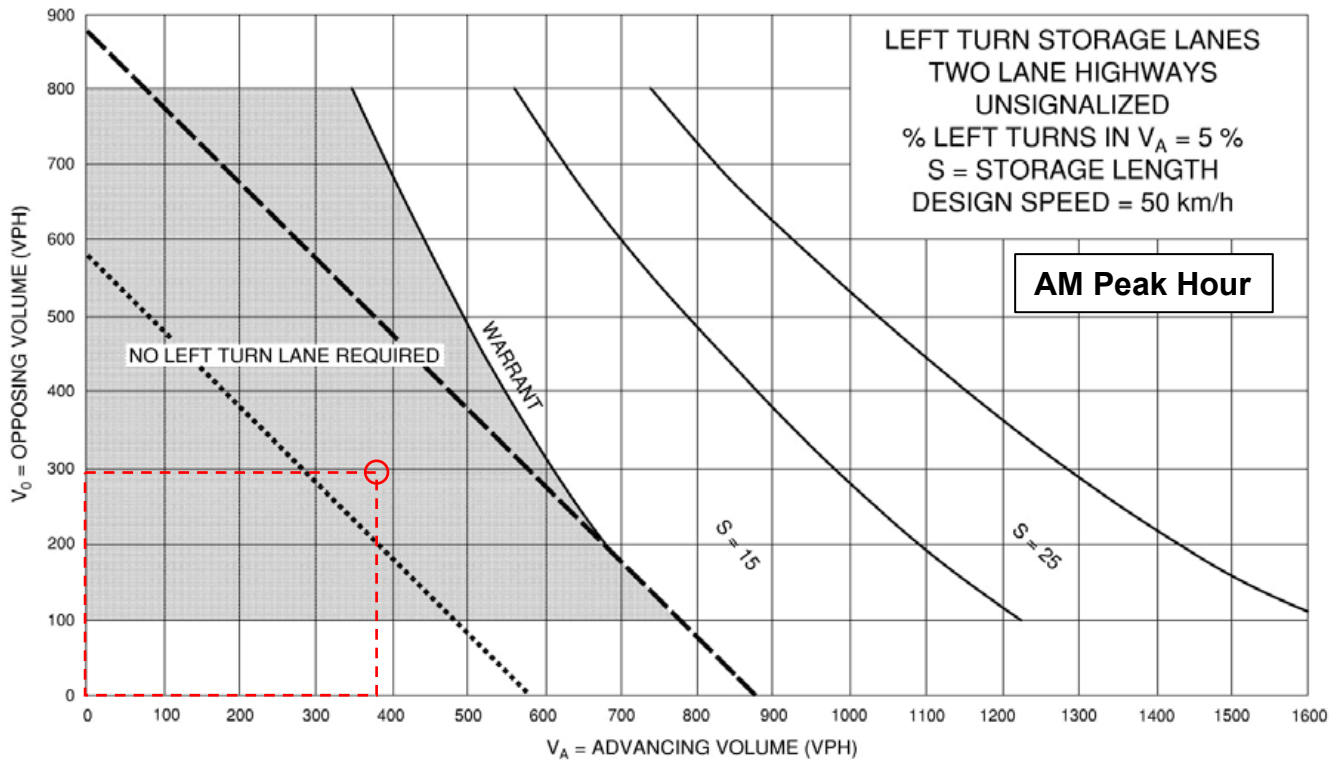
Northbound Left-Turn Lane Irvine St & Bricker Ave 2031 Total



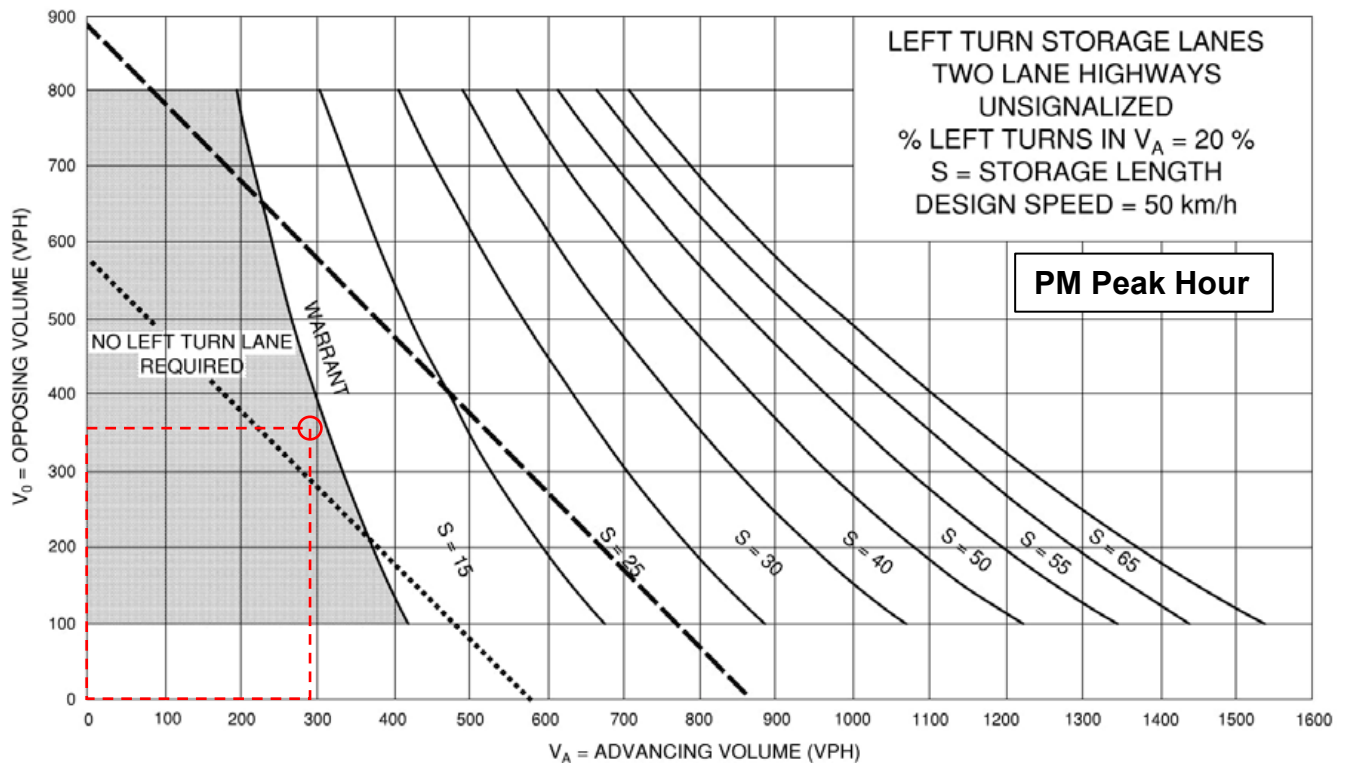
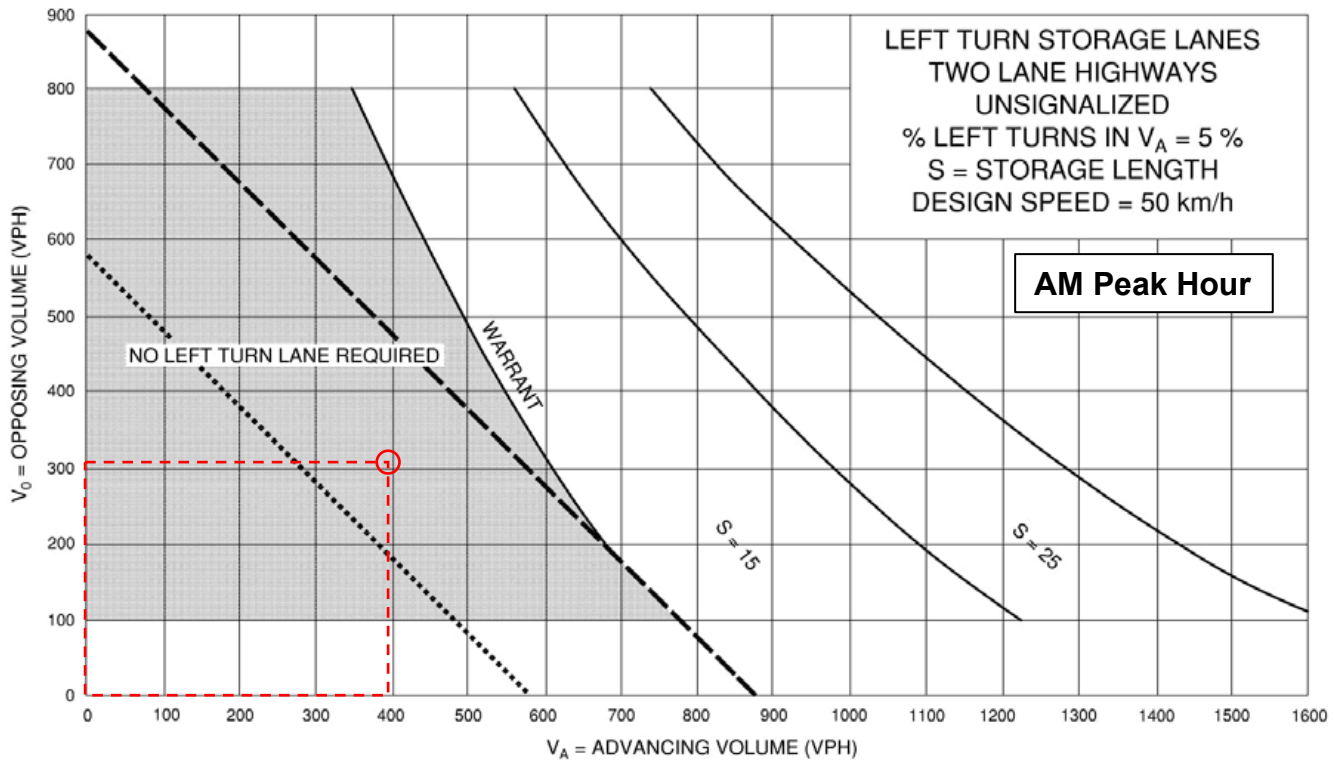
Eastbound Left-Turn Lane East Mill St (WR 18) & Colborne St 2026 Background



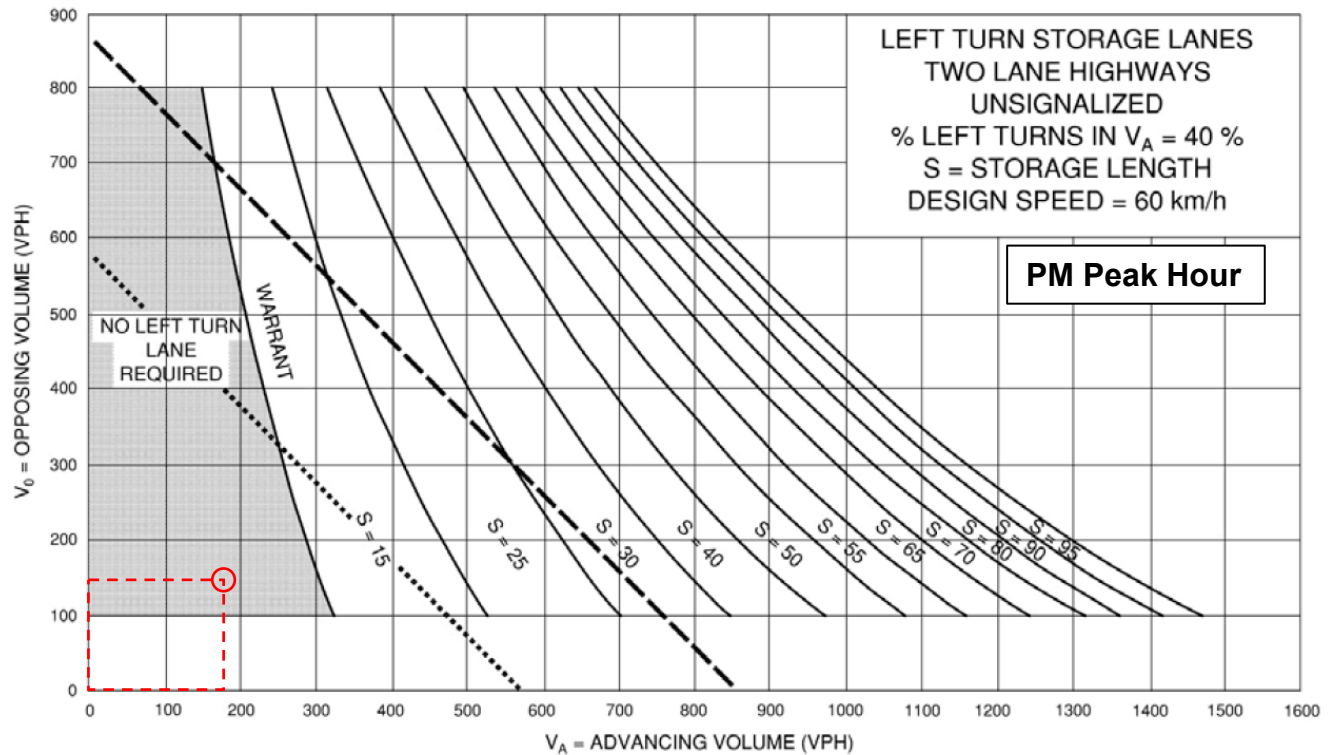
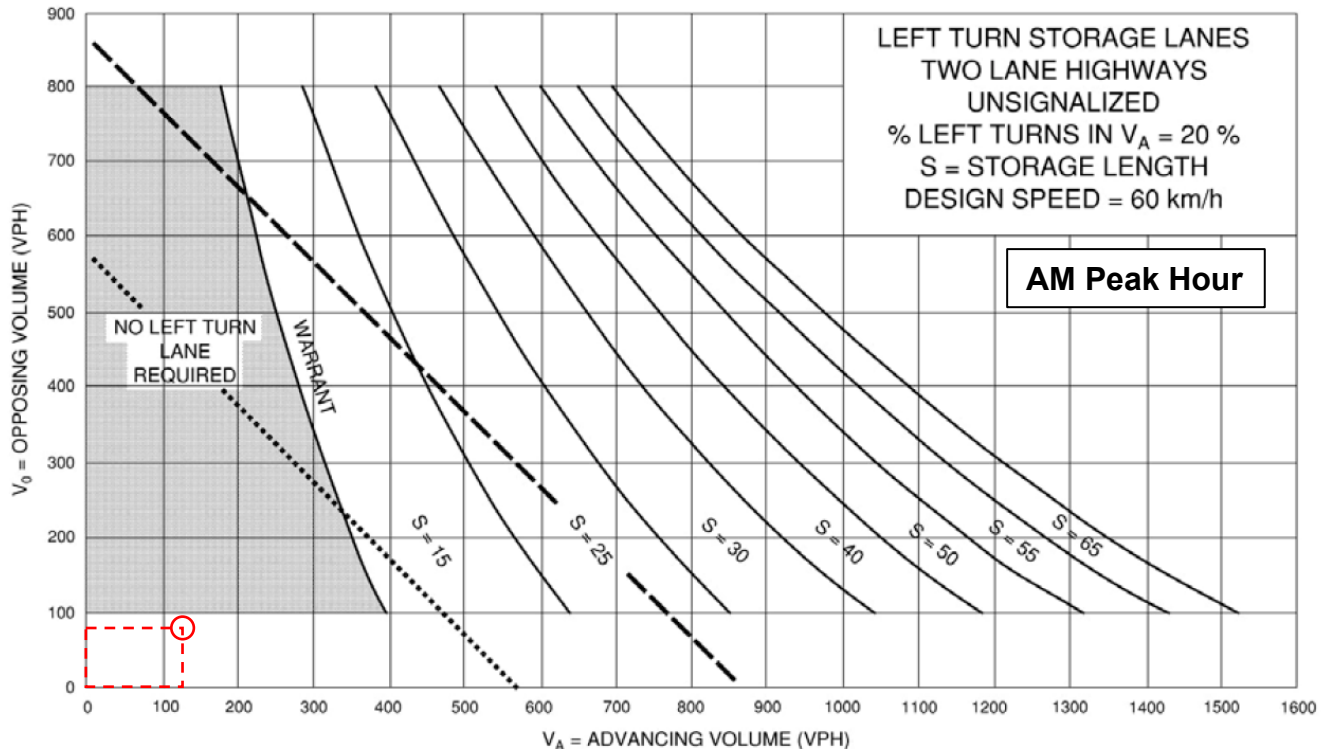
Eastbound Left-Turn Lane East Mill St (WR 18) & Colborne St 2026 Total



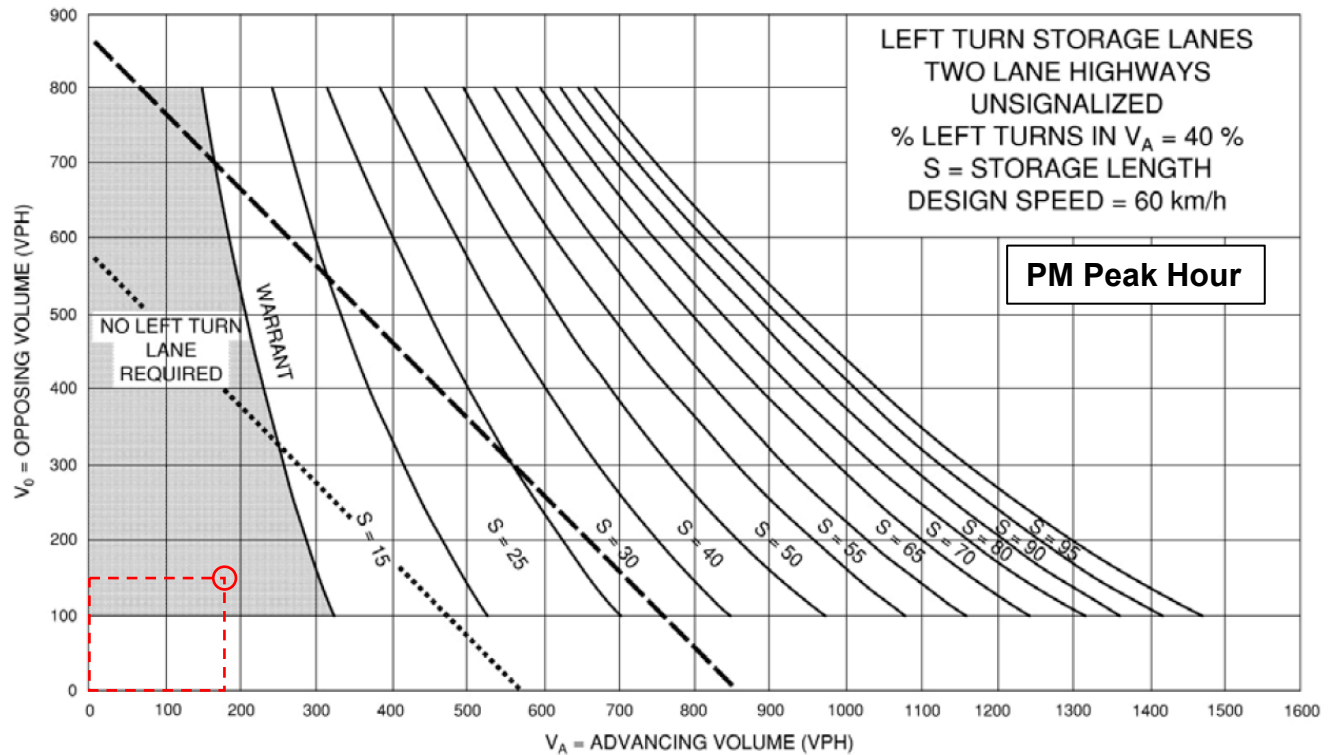
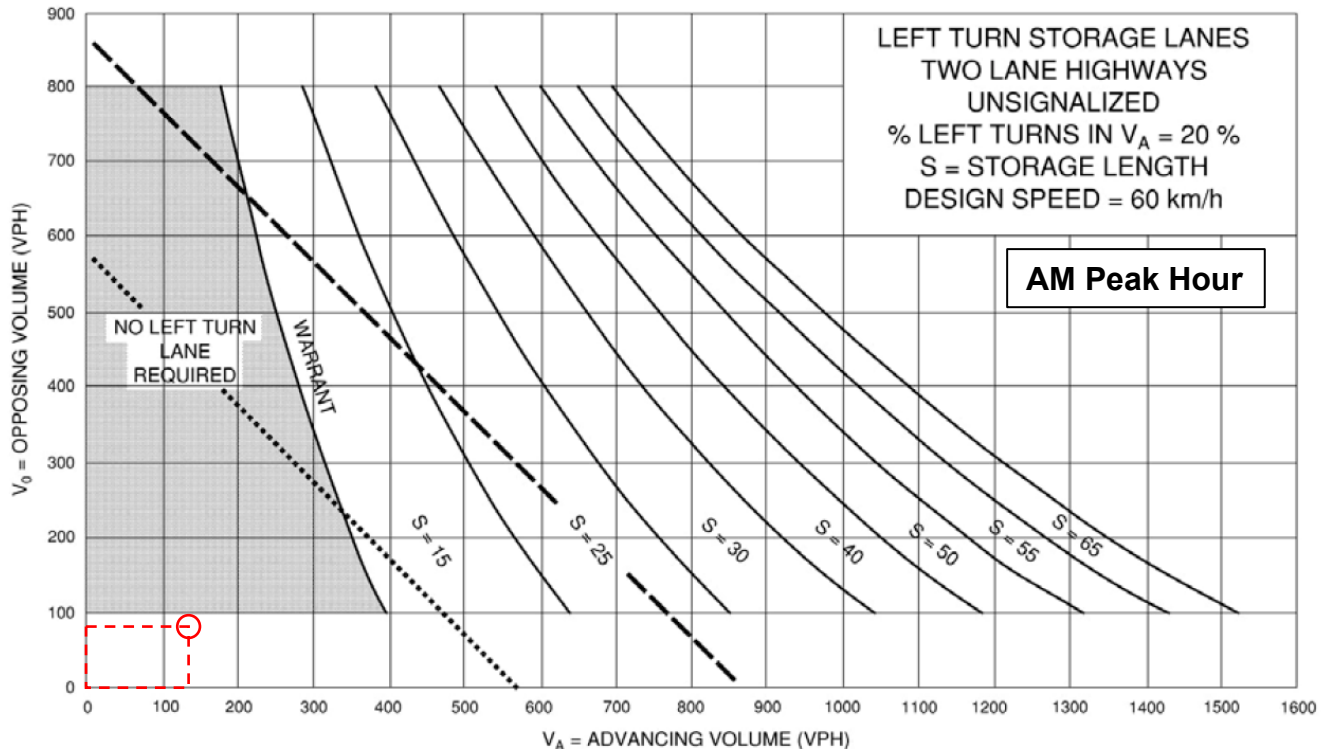
Eastbound Left-Turn Lane East Mill St (WR 18) & Colborne St 2031 Background



Eastbound Left-Turn Lane East Mill St (WR 18) & Colborne St 2031 Total



Northbound Left-Turn Lane Irvine Street & Street A 2026 Total



Northbound Left-Turn Lane Irvine Street & Street C 2031 Total

Appendix H

Alternative 2031 Total Scenario Operation Synchro Reports



Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Total (2031) - Alternative Scenario
AM Peak Hour

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	↕
Traffic Volume (vph)	9	279	471	34	9	6
Future Volume (vph)	9	279	471	34	9	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.991		0.944	
Flt Protected		0.998			0.971	
Satd. Flow (prot)	0	1860	1809	0	1742	0
Flt Permitted		0.998			0.971	
Satd. Flow (perm)	0	1860	1809	0	1742	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	10	303	512	37	10	7
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	313	549	0	17	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	36.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Total (2031) - Alternative Scenario
AM Peak Hour

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	↕
Traffic Volume (veh/h)	9	279	471	34	9	6
Future Volume (Veh/h)	9	279	471	34	9	6
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	10	303	512	37	10	7
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	551				856	532
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	551				856	532
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				97	99
cM capacity (veh/h)	1027				327	550
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	313	549	17			
Volume Left	10	0	10			
Volume Right	0	37	7			
eSH	1027	1700	393			
Volume to Capacity	0.01	0.32	0.04			
Queue Length 95th (m)	0.2	0.0	1.0			
Control Delay (s)	0.4	0.0	14.6			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	14.6			
Approach LOS			B			
Intersection Summary						
Average Delay			0.4			
Intersection Capacity Utilization	36.9%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031) - Alternative Scenario

AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (vph)	5	216	67	38	345	5	154	7	63	4	1	6
Future Volume (vph)	5	216	67	38	345	5	154	7	63	4	1	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.969			0.998			0.962			0.921	
Fit Protected		0.999			0.995			0.967			0.984	
Satd. Flow (prot)	0	1799	0	0	1858	0	0	1767	0	0	1722	0
Fit Permitted		0.999			0.995			0.967			0.984	
Satd. Flow (perm)	0	1799	0	0	1858	0	0	1767	0	0	1722	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			210.0			484.2	
Travel Time (s)		13.7			48.8			15.1			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	5	235	73	41	375	5	167	8	68	4	1	7
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	313	0	0	421	0	0	243	0	0	12	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	65.7% ICU Level of Service C
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031) - Alternative Scenario

AM Peak Hour

	↖	→	↘	↙	←	↖	↙	↑	↘	↙	↓	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Volume (veh/h)	5	216	67	38	345	5	154	7	63	4	1	6
Future Volume (Veh/h)	5	216	67	38	345	5	154	7	63	4	1	6
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	235	73	41	375	5	167	8	68	4	1	7
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	380			309			750	744	272	813	778	378
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	380			309			750	744	272	813	778	378
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			97			47	98	91	98	100	99
cM capacity (veh/h)	1190			1175			316	331	770	260	317	674
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	313	421	243	12								
Volume Left	5	41	167	4								
Volume Right	73	5	68	7								
cSH	1190	1175	379	415								
Volume to Capacity	0.00	0.03	0.64	0.03								
Queue Length 95th (m)	0.1	0.8	32.5	0.7								
Control Delay (s)	0.2	1.1	30.0	13.9								
Lane LOS	A	A	D	B								
Approach Delay (s)	0.2	1.1	30.0	13.9								
Approach LOS			D	B								

Intersection Summary

Average Delay	8.1
Intersection Capacity Utilization	65.7% ICU Level of Service C
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St/Keating

Total (2031) - Alternative Scenario
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	26	0	30	71	0	62	18	130	25	23	242	10
Future Volume (vph)	26	0	30	71	0	62	18	130	25	23	242	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.927			0.937			0.981			0.995	
Flt Protected		0.978			0.974			0.995			0.996	
Satd. Flow (prot)	0	1547	0	0	1700	0	0	1809	0	0	1863	0
Flt Permitted		0.978			0.974			0.995			0.996	
Satd. Flow (perm)	0	1547	0	0	1700	0	0	1809	0	0	1863	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		162.4			169.8			289.9			240.3	
Travel Time (s)		11.7			12.2			20.9			17.3	
Confl. Peds. (#/hr)							3					3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	21%	2%	2%	2%	0%	3%	2%	2%	0%	25%
Adj. Flow (vph)	28	0	33	77	0	67	20	141	27	25	263	11
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	61	0	0	144	0	0	188	0	0	299	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	35.3% ICU Level of Service A
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St/Keating

Total (2031) - Alternative Scenario
AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	26	0	30	71	0	62	18	130	25	23	242	10
Future Volume (Veh/h)	26	0	30	71	0	62	18	130	25	23	242	10
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	28	0	33	77	0	67	20	141	27	25	263	11
Pedestrians		3										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type							None				None	
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	583	530	272	546	522	154	277			168		
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	583	530	272	546	522	154	277			168		
tC, single (s)	7.1	6.5	6.4	7.1	6.5	6.2	4.1			4.1		
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.5	4.0	3.3	2.2			2.2		
p0 queue free %	93	100	95	82	100	92	98			98		
cM capacity (veh/h)	383	439	722	416	443	891	1294			1410		
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	61	144	188	299								
Volume Left	28	77	20	25								
Volume Right	33	67	27	11								
eSH	513	554	1294	1410								
Volume to Capacity	0.12	0.26	0.02	0.02								
Queue Length 95th (m)	3.1	7.9	0.4	0.4								
Control Delay (s)	13.0	13.8	1.0	0.8								
Lane LOS	B	B	A	A								
Approach Delay (s)	13.0	13.8	1.0	0.8								
Approach LOS	B	B										

Intersection Summary	
Average Delay	4.6
Intersection Capacity Utilization	35.3% ICU Level of Service A
Analysis Period (min)	15

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Total (2031) - Alternative Scenario
AM Peak Hour

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	37	1	1	27	5	18
Future Volume (vph)	37	1	1	27	5	18
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.997		0.892			
Fit Protected			0.998		0.990	
Satd. Flow (prot)	1592	0	0	1729	1565	0
Fit Permitted			0.998			
Satd. Flow (perm)	1592	0	0	1729	1565	0
Link Speed (k/h)	50		50			
Link Distance (m)	88.4		162.4		207.7	
Travel Time (s)	6.4		11.7			
Confl. Peds. (#/hr)	4		4		2	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	100%	0%	10%	0%	9%
Adj. Flow (vph)	40	1	1	29	5	20
Shared Lane Traffic (%)						
Lane Group Flow (vph)	41	0	0	30	25	0
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	14.9%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Total (2031) - Alternative Scenario
AM Peak Hour

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	37	1	1	27	5	18
Future Volume (Veh/h)	37	1	1	27	5	18
Sign Control	Free			Free Stop		
Grade	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	40	1	1	29	5	20
Pedestrians	2		1		4	
Lane Width (m)	3.6		3.6		3.6	
Walking Speed (m/s)	1.2		1.2		1.2	
Percent Blockage	0		0		0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume			45		78	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			45		78	
tC, single (s)			4.1		6.4	
tC, 2 stage (s)						
tF (s)			2.2		3.5	
p0 queue free %			100		99	
cM capacity (veh/h)			1571		925	
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	41	30	25			
Volume Left	0	1	5			
Volume Right	1	0	20			
eSH	1700	1571	984			
Volume to Capacity	0.02	0.00	0.03			
Queue Length 95th (m)	0.0	0.0	0.6			
Control Delay (s)	0.0	0.2	8.8			
Lane LOS	A		A			
Approach Delay (s)	0.0	0.2	8.8			
Approach LOS	A		A			
Intersection Summary						
Average Delay			2.4			
Intersection Capacity Utilization	14.9%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Total (2031) - Alternative Scenario
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	10	4	5	17	10	9	0	16	13	0	0
Future Volume (vph)	0	10	4	5	17	10	9	0	16	13	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.964			0.956			0.915				
Fit Protected					0.993			0.982				0.950
Satd. Flow (prot)	0	1416	0	0	1647	0	0	922	0	0	1805	0
Fit Permitted					0.993			0.982				0.950
Satd. Flow (perm)	0	1416	0	0	1647	0	0	922	0	0	1805	0
Link Speed (k/h)		50			50			50				50
Link Distance (m)		123.8			88.4			134.2				45.0
Travel Time (s)		8.9			6.4			9.7				3.2
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	11	4	5	18	11	10	0	17	14	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	15	0	0	34	0	0	27	0	0	14	0
Sign Control		Free			Free			Stop				Stop
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	18.5%				ICU Level of Service A							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Total (2031) - Alternative Scenario
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	10	4	5	17	10	9	0	16	13	0	0
Future Volume (Veh/h)	0	10	4	5	17	10	9	0	16	13	0	0
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	11	4	5	18	11	10	0	17	14	0	0
Pedestrians		1			4			9				5
Lane Width (m)		3.6			3.6			3.6				3.6
Walking Speed (m/s)		1.2			1.2			1.2				1.2
Percent Blockage		0			0			1				0
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	34			24			56	66	26	72	62	30
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	34			24			56	66	26	72	62	30
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			100			99	100	98	98	100	100
cM capacity (veh/h)	1584			1592			799	816	817	887	820	1046
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	15	34	27	14								
Volume Left	0	5	10	14								
Volume Right	4	11	17	0								
cSH	1584	1592	810	887								
Volume to Capacity	0.00	0.00	0.03	0.02								
Queue Length 95th (m)	0.0	0.1	0.8	0.4								
Control Delay (s)	0.0	1.1	9.6	9.1								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	1.1	9.6	9.1								
Approach LOS		A	A									
Intersection Summary												
Average Delay				4.7								
Intersection Capacity Utilization	18.5%			ICU Level of Service			A					
Analysis Period (min)	15											

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Total (2031) - Alternative Scenario
AM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	76	321	108	66	239	96
Future Volume (vph)	76	321	108	66	239	96
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.891	0.949				
Fit Protected	0.990				0.966	
Satd. Flow (prot)	1538	0	1731	0	0	1702
Fit Permitted	0.990					0.966
Satd. Flow (perm)	1538	0	1731	0	0	1702
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				4	4	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	13%	8%	3%	6%	7%	10%
Adj. Flow (vph)	83	349	117	72	260	104
Shared Lane Traffic (%)						
Lane Group Flow (vph)	432	0	189	0	0	364
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	62.6%		ICU Level of Service B			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Total (2031) - Alternative Scenario
AM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	76	321	108	66	239	96
Future Volume (Veh/h)	76	321	108	66	239	96
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	83	349	117	72	260	104
Pedestrians	4					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type		None			None	
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	781	157			193	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	781	157			193	
tC, single (s)	6.5	6.3			4.2	
tC, 2 stage (s)						
tF (s)	3.6	3.4			2.3	
p0 queue free %	70	60			81	
cM capacity (veh/h)	280	870			1346	
Direction, Lane #	WBL	NB 1	SB 1			
Volume Total	432	189	364			
Volume Left	83	0	260			
Volume Right	349	72	0			
eSH	619	1700	1346			
Volume to Capacity	0.70	0.11	0.19			
Queue Length 95th (m)	42.4	0.0	5.4			
Control Delay (s)	23.1	0.0	6.4			
Lane LOS	C		A			
Approach Delay (s)	23.1	0.0	6.4			
Approach LOS	C					
Intersection Summary						
Average Delay		12.5				
Intersection Capacity Utilization	62.6%		ICU Level of Service		B	
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Total (2031) - Alternative Scenario
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	25	112	9	14	264	54	6	103	13	79	273	89
Future Volume (vph)	25	112	9	14	264	54	6	103	13	79	273	89
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.992			0.978			0.986			0.973	
Flt Protected		0.992			0.998			0.997			0.991	
Satd. Flow (prot)	0	1719	0	0	1773	0	0	1425	0	0	1602	0
Flt Permitted		0.992			0.998			0.997			0.991	
Satd. Flow (perm)	0	1719	0	0	1773	0	0	1425	0	0	1602	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	57		1	1		57	4		50	50		4
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	6%	20%	33%	3%	5%	50%	32%	14%	4%	22%	0%
Adj. Flow (vph)	27	122	10	15	287	59	7	112	14	86	297	97
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	159	0	0	361	0	0	133	0	0	480	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	57.4%				ICU Level of Service B							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Total (2031) - Alternative Scenario
AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	25	112	9	14	264	54	6	103	13	79	273	89
Future Volume (vph)	25	112	9	14	264	54	6	103	13	79	273	89
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	122	10	15	287	59	7	112	14	86	297	97
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	159	361	133	480								
Volume Left (vph)	27	15	7	86								
Volume Right (vph)	10	59	14	97								
Hadj (s)	0.14	-0.01	0.48	0.16								
Departure Headway (s)	6.9	6.3	7.2	6.1								
Degree Utilization, x	0.31	0.63	0.27	0.82								
Capacity (veh/h)	466	529	439	573								
Control Delay (s)	13.0	19.4	12.8	30.6								
Approach Delay (s)	13.0	19.4	12.8	30.6								
Approach LOS	B	C	B	D								
Intersection Summary												
Delay				22.5								
Level of Service				C								
Intersection Capacity Utilization				57.4%				ICU Level of Service				B
Analysis Period (min)				15								

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Total (2031) - Alternative Scenario
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	57	378	1	1	258	54	1	4	0	159	0	138
Future Volume (vph)	57	378	1	1	258	54	1	4	0	159	0	138
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt				0.977							0.937	
Flt Protected		0.994						0.990			0.974	
Satd. Flow (prot)	0	1825	0	0	1797	0	0	1881	0	0	1734	0
Flt Permitted		0.994						0.990			0.974	
Satd. Flow (perm)	0	1825	0	0	1797	0	0	1881	0	0	1734	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)	42					42						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	4%	0%	0%	4%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	62	411	1	1	280	59	1	4	0	173	0	150
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	474	0	0	340	0	0	5	0	0	323	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	74.6%				ICU Level of Service D							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Total (2031) - Alternative Scenario
AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		↔			↔			↔				↔		
Traffic Volume (veh/h)	57	378	1	1	258	54	1	4	0	159	0	138		
Future Volume (Veh/h)	57	378	1	1	258	54	1	4	0	159	0	138		
Sign Control	Free			Free			Stop			Stop				
Grade	0%			0%			0%			0%				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	62	411	1	1	280	59	1	4	0	173	0	150		
Pedestrians												42		
Lane Width (m)												3.6		
Walking Speed (m/s)												1.2		
Percent Blockage												4		
Right turn flare (veh)														
Median type	None			None										
Median storage (veh)														
Upstream signal (m)														
pX, platoon unblocked														
vC, conflicting volume	381				412				997	918	412	891	890	352
vC1, stage 1 conf vol														
vC2, stage 2 conf vol														
vCu, unblocked vol	381				412				997	918	412	891	890	352
tC, single (s)	4.1				4.1				7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)														
tF (s)	2.2				2.2				3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	95				100				99	98	100	27	100	78
cM capacity (veh/h)	1147				1158				163	249	645	236	259	672
Direction, Lane #	EB 1	WB 1	NB 1	SB 1										
Volume Total	474	340	5	323										
Volume Left	62	1	1	173										
Volume Right	1	59	0	150										
eSH	1147	1158	225	338										
Volume to Capacity	0.05	0.00	0.02	0.96										
Queue Length 95th (m)	1.3	0.0	0.5	77.0										
Control Delay (s)	1.6	0.0	21.3	74.3										
Lane LOS	A	A	C	F										
Approach Delay (s)	1.6	0.0	21.3	74.3										
Approach LOS			C	F										
Intersection Summary														
Average Delay				21.8										
Intersection Capacity Utilization	74.6%			ICU Level of Service	D									
Analysis Period (min)	15													

Lanes, Volumes, Timings
201: Irvine St & Street A/Elora Sands

Total (2031) - Alternative Scenario
AM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	49	0	77	114	0	15	22	160	35	6	84	16
Future Volume (vph)	49	0	77	114	0	15	22	160	35	6	84	16
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.917			0.985			0.978			0.980	
Flt Protected		0.981			0.958			0.995			0.997	
Satd. Flow (prot)	0	1676	0	0	1758	0	0	1813	0	0	1820	0
Flt Permitted		0.981			0.958			0.995			0.997	
Satd. Flow (perm)	0	1676	0	0	1758	0	0	1813	0	0	1820	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		126.5			149.1			240.3			210.0	
Travel Time (s)		9.1			10.7			17.3			15.1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	53	0	84	124	0	16	24	174	38	7	91	17
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	137	0	0	140	0	0	236	0	0	115	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	38.5%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
201: Irvine St & Street A/Elora Sands

Total (2031) - Alternative Scenario
AM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	49	0	77	114	0	15	22	160	35	6	84	16
Future Volume (Veh/h)	49	0	77	114	0	15	22	160	35	6	84	16
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	53	0	84	124	0	16	24	174	38	7	91	17
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	370	374	100	438	363	193	108				212	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	370	374	100	438	363	193	108				212	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	91	100	91	74	100	98	98				99	
cM capacity (veh/h)	566	545	956	474	553	849	1483				1358	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	137	140	236	115								
Volume Left	53	124	24	7								
Volume Right	84	16	38	17								
cSH	755	499	1483	1358								
Volume to Capacity	0.18	0.28	0.02	0.01								
Queue Length 95th (m)	5.0	8.7	0.4	0.1								
Control Delay (s)	10.8	15.0	0.9	0.5								
Lane LOS	B	B	A	A								
Approach Delay (s)	10.8	15.0	0.9	0.5								
Approach LOS	B	B										

Intersection Summary	
Average Delay	6.1
Intersection Capacity Utilization	38.5%
ICU Level of Service A	
Analysis Period (min)	15

Lanes, Volumes, Timings
101: Woolwich St & Milford Cres

Total (2031) - Alternative Scenario
PM Peak Hour

Lane Group	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (vph)	5	506	330	16	9	2
Future Volume (vph)	5	506	330	16	9	2
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt			0.994		0.977	
Flt Protected					0.960	
Satd. Flow (prot)	0	1863	1815	0	1782	0
Flt Permitted					0.960	
Satd. Flow (perm)	0	1863	1815	0	1782	0
Link Speed (k/h)		40	40		50	
Link Distance (m)		407.4	152.7		138.9	
Travel Time (s)		36.7	13.7		10.0	
Confl. Peds. (#/hr)	2			2		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	4%	5%	0%	0%
Adj. Flow (vph)	5	550	359	17	10	2
Shared Lane Traffic (%)						
Lane Group Flow (vph)	0	555	376	0	12	0
Sign Control		Free	Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	40.6%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
101: Woolwich St & Milford Cres

Total (2031) - Alternative Scenario
PM Peak Hour

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	5	506	330	16	9	2
Future Volume (Veh/h)	5	506	330	16	9	2
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	550	359	17	10	2
Pedestrians					2	
Lane Width (m)					3.6	
Walking Speed (m/s)					1.2	
Percent Blockage					0	
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	378				930	370
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	378				930	370
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				97	100
cM capacity (veh/h)	1190				298	679
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	555	376	12			
Volume Left	5	0	10			
Volume Right	0	17	2			
eSH	1190	1700	328			
Volume to Capacity	0.00	0.22	0.04			
Queue Length 95th (m)	0.1	0.0	0.9			
Control Delay (s)	0.1	0.0	16.4			
Lane LOS	A		C			
Approach Delay (s)	0.1	0.0	16.4			
Approach LOS			C			
Intersection Summary						
Average Delay			0.3			
Intersection Capacity Utilization	40.6%		ICU Level of Service	A		
Analysis Period (min)	15					

Lanes, Volumes, Timings

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031) - Alternative Scenario

PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔				↔			↔	
Traffic Volume (vph)	4	322	189	49	211	4	127	7	53	4	2	7
Future Volume (vph)	4	322	189	49	211	4	127	7	53	4	2	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.950			0.998			0.962			0.923	
Flt Protected					0.991			0.967			0.986	
Satd. Flow (prot)	0	1772	0	0	1825	0	0	1767	0	0	1729	0
Flt Permitted					0.991			0.967			0.986	
Satd. Flow (perm)	0	1772	0	0	1825	0	0	1767	0	0	1729	0
Link Speed (k/h)		40			40			50			50	
Link Distance (m)		152.7			542.4			205.1			484.2	
Travel Time (s)		13.7			48.8			14.8			34.9	
Confl. Peds. (#/hr)			1	1								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	0%	16%	0%	0%	0%	0%	0%	0%	0%	0%
Adj. Flow (vph)	4	350	205	53	229	4	138	8	58	4	2	8
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	559	0	0	286	0	0	204	0	0	14	0
Sign Control		Free			Free			Stop			Stop	

Intersection Summary

Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	70.1%
ICU Level of Service	C
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis

102: Irvine St & Woolwich St/Nichol Rd 15

Total (2031) - Alternative Scenario

PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔				↔			↔	
Traffic Volume (veh/h)	4	322	189	49	211	4	127	7	53	4	2	7
Future Volume (Veh/h)	4	322	189	49	211	4	127	7	53	4	2	7
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	350	205	53	229	4	138	8	58	4	2	8
Pedestrians								1				
Lane Width (m)								3.6				
Walking Speed (m/s)								1.2				
Percent Blockage								0				
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	233			556			808	800	454	860	901	231
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	233			556			808	800	454	860	901	231
tC, single (s)	4.1			4.3			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.3			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	100			94			51	97	90	98	99	99
cM capacity (veh/h)	1346			947			284	301	610	236	263	813
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	559	286	204	14								
Volume Left	4	53	138	4								
Volume Right	205	4	58	8								
eSH	1346	947	335	407								
Volume to Capacity	0.00	0.06	0.61	0.03								
Queue Length 95th (m)	0.1	1.3	28.8	0.8								
Control Delay (s)	0.1	2.1	31.1	14.2								
Lane LOS	A	A	D	B								
Approach Delay (s)	0.1	2.1	31.1	14.2								
Approach LOS			D	B								

Intersection Summary

Average Delay	6.8
Intersection Capacity Utilization	70.1%
ICU Level of Service	C
Analysis Period (min)	15

Lanes, Volumes, Timings
103: Irvine St & Bricker St/Keating

Total (2031) - Alternative Scenario
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	16	0	17	47	0	42	21	283	86	76	184	29
Future Volume (vph)	16	0	17	47	0	42	21	283	86	76	184	29
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.931			0.936			0.970			0.986	
Flt Protected		0.976			0.974			0.997			0.987	
Satd. Flow (prot)	0	1558	0	0	1698	0	0	1791	0	0	1794	0
Flt Permitted		0.976			0.974			0.997			0.987	
Satd. Flow (perm)	0	1558	0	0	1698	0	0	1791	0	0	1794	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		162.4			231.9			289.9			245.2	
Travel Time (s)		11.7			16.7			20.9			17.7	
Confl. Peds. (#/hr)							3					3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	2%	21%	2%	2%	2%	0%	3%	2%	2%	0%	25%
Adj. Flow (vph)	17	0	18	51	0	46	23	308	93	83	200	32
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	35	0	0	97	0	0	424	0	0	315	0
Sign Control		Stop			Stop			Free			Free	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	53.7%					ICU Level of Service A						
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
103: Irvine St & Bricker St/Keating

Total (2031) - Alternative Scenario
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	16	0	17	47	0	42	21	283	86	76	184	29
Future Volume (Veh/h)	16	0	17	47	0	42	21	283	86	76	184	29
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	17	0	18	51	0	46	23	308	93	83	200	32
Pedestrians		3										
Lane Width (m)		3.6										
Walking Speed (m/s)		1.2										
Percent Blockage		0										
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	832	832	219	800	802	354	235				401	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	832	832	219	800	802	354	235				401	
tC, single (s)	7.1	6.5	6.4	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.5	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	93	100	98	81	100	93	98				93	
cM capacity (veh/h)	252	277	773	275	289	689	1341				1158	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	35	97	424	315								
Volume Left	17	51	23	83								
Volume Right	18	46	93	32								
eSH	386	385	1341	1158								
Volume to Capacity	0.09	0.25	0.02	0.07								
Queue Length 95th (m)	2.3	7.5	0.4	1.8								
Control Delay (s)	15.3	17.5	0.6	2.7								
Lane LOS	C	C	A	A								
Approach Delay (s)	15.3	17.5	0.6	2.7								
Approach LOS	C	C										
Intersection Summary												
Average Delay				3.8								
Intersection Capacity Utilization			53.7%				ICU Level of Service				A	
Analysis Period (min)				15								

Lanes, Volumes, Timings
104: Clegg Rd & Bricker St

Total (2031) - Alternative Scenario
PM Peak Hour

Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	25	7	10	41	1	9
Future Volume (vph)	25	7	10	41	1	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.969		0.877			
Fit Protected			0.990 0.995			
Satd. Flow (prot)	1354	0	0	1741	1533	0
Fit Permitted	0.990 0.995					
Satd. Flow (perm)	1354	0	0	1741	1533	0
Link Speed (k/h)	50		50			
Link Distance (m)	88.4		162.4		207.7	
Travel Time (s)	6.4		11.7 15.0			
Confl. Peds. (#/hr)	4		4		2 1	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	17%	100%	0%	10%	0%	9%
Adj. Flow (vph)	27	8	11	45	1	10
Shared Lane Traffic (%)						
Lane Group Flow (vph)	35	0	0	56	11	0
Sign Control	Free		Free		Stop	
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	19.7%		ICU Level of Service A			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
104: Clegg Rd & Bricker St

Total (2031) - Alternative Scenario
PM Peak Hour

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (veh/h)	25	7	10	41	1	9
Future Volume (Veh/h)	25	7	10	41	1	9
Sign Control	Free			Free Stop		
Grade	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	27	8	11	45	1	10
Pedestrians	2		1		4	
Lane Width (m)	3.6		3.6		3.6	
Walking Speed (m/s)	1.2		1.2		1.2	
Percent Blockage	0		0		0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume				39		104 36
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				39		104 36
tC, single (s)				4.1		6.4 6.3
tC, 2 stage (s)						
tF (s)				2.2		3.5 3.4
p0 queue free %				99		100 99
cM capacity (veh/h)				1579		888 1013
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	35	56	11			
Volume Left	0	11	1			
Volume Right	8	0	10			
sSH	1700	1579	1000			
Volume to Capacity	0.02	0.01	0.01			
Queue Length 95th (m)	0.0	0.2	0.3			
Control Delay (s)	0.0	1.5	8.6			
Lane LOS	A			A		
Approach Delay (s)	0.0	1.5	8.6			
Approach LOS	A					
Intersection Summary						
Average Delay				1.7		
Intersection Capacity Utilization	19.7%		ICU Level of Service		A	
Analysis Period (min)	15					

Lanes, Volumes, Timings
105: Marr Dr & Bricker St

Total (2031) - Alternative Scenario
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (vph)	0	17	12	9	18	15	11	0	6	9	0	0
Future Volume (vph)	0	17	12	9	18	15	11	0	6	9	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.943			0.953			0.950				
Flt Protected					0.989			0.969				0.950
Satd. Flow (prot)	0	1454	0	0	1661	0	0	1001	0	0	1805	0
Flt Permitted					0.989			0.969				0.950
Satd. Flow (perm)	0	1454	0	0	1661	0	0	1001	0	0	1805	0
Link Speed (k/h)		50			50			50				50
Link Distance (m)		123.8			88.4			134.2				45.0
Travel Time (s)		8.9			6.4			9.7				3.2
Confl. Peds. (#/hr)	5		9	9		5	1		4	4		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	40%	0%	0%	18%	0%	60%	0%	100%	0%	0%	0%
Adj. Flow (vph)	0	18	13	10	20	16	12	0	7	10	0	0
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	31	0	0	46	0	0	19	0	0	10	0
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	21.1%			ICU Level of Service A								
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
105: Marr Dr & Bricker St

Total (2031) - Alternative Scenario
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔				↔
Traffic Volume (veh/h)	0	17	12	9	18	15	11	0	6	9	0	0
Future Volume (Veh/h)	0	17	12	9	18	15	11	0	6	9	0	0
Sign Control	Free			Free			Stop			Stop		
Grade	0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	18	13	10	20	16	12	0	7	10	0	0
Pedestrians		1			4			9				5
Lane Width (m)	3.6			3.6			3.6			3.6		
Walking Speed (m/s)	1.2			1.2			1.2			1.2		
Percent Blockage	0			0			1			0		
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	41			40			82	94	38	88	93	34
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	41			40			82	94	38	88	93	34
tC, single (s)	4.1			4.1			7.7	6.5	7.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			4.0	4.0	4.2	3.5	4.0	3.3
p0 queue free %	100			99			98	100	99	99	100	100
cM capacity (veh/h)	1575			1571			765	785	804	875	787	1040
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	31	46	19	10								
Volume Left	0	10	12	10								
Volume Right	13	16	7	0								
eSH	1575	1571	779	875								
Volume to Capacity	0.00	0.01	0.02	0.01								
Queue Length 95th (m)	0.0	0.1	0.6	0.3								
Control Delay (s)	0.0	1.6	9.7	9.2								
Lane LOS		A	A	A								
Approach Delay (s)	0.0	1.6	9.7	9.2								
Approach LOS			A	A								
Intersection Summary												
Average Delay				3.3								
Intersection Capacity Utilization	21.1%			ICU Level of Service			A					
Analysis Period (min)	15											

Lanes, Volumes, Timings
106: Geddes St (WR18) & James St

Total (2031) - Alternative Scenario
PM Peak Hour

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	78	278	95	52	435	135
Future Volume (vph)	78	278	95	52	435	135
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt	0.895		0.952			
Flt Protected	0.989					0.963
Satd. Flow (prot)	1633	0	1777	0	0	1785
Flt Permitted	0.989					0.963
Satd. Flow (perm)	1633	0	1777	0	0	1785
Link Speed (k/h)	50		50			50
Link Distance (m)	111.8		38.3			111.4
Travel Time (s)	8.0		2.8			8.0
Confl. Peds. (#/hr)				5	5	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	3%	3%	0%	5%	3%	1%
Adj. Flow (vph)	85	302	103	57	473	147
Shared Lane Traffic (%)						
Lane Group Flow (vph)	387	0	160	0	0	620
Sign Control	Stop		Free			Free
Intersection Summary						
Area Type:	Other					
Control Type:	Unsignalized					
Intersection Capacity Utilization	71.8%		ICU Level of Service C			
Analysis Period (min)	15					

HCM Unsignalized Intersection Capacity Analysis
106: Geddes St (WR18) & James St

Total (2031) - Alternative Scenario
PM Peak Hour

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	78	278	95	52	435	135
Future Volume (Veh/h)	78	278	95	52	435	135
Sign Control	Stop		Free			Free
Grade	0%		0%			0%
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	85	302	103	57	473	147
Pedestrians	5					
Lane Width (m)	3.6					
Walking Speed (m/s)	1.2					
Percent Blockage	0					
Right turn flare (veh)						
Median type			None			None
Median storage (veh)						
Upstream signal (m)						
pX, platoon unblocked						
vC, conflicting volume	1230	136			165	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1230	136			165	
tC, single (s)	6.4	6.2			4.1	
tC, 2 stage (s)						
tF (s)	3.5	3.3			2.2	
p0 queue free %	34	67			66	
cM capacity (veh/h)	129	906			1401	
Direction, Lane #	WB 1	NB 1	SB 1			
Volume Total	387	160	620			
Volume Left	85	0	473			
Volume Right	302	57	0			
eSH	390	1700	1401			
Volume to Capacity	0.99	0.09	0.34			
Queue Length 95th (m)	90.3	0.0	11.5			
Control Delay (s)	76.9	0.0	7.5			
Lane LOS	F		A			
Approach Delay (s)	76.9	0.0	7.5			
Approach LOS	F					
Intersection Summary						
Average Delay			29.5			
Intersection Capacity Utilization	71.8%		ICU Level of Service	C		
Analysis Period (min)	15					

Lanes, Volumes, Timings
107: Irvine St & Colborne St

Total (2031) - Alternative Scenario
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	70	255	7	8	200	92	1	265	21	80	165	61
Future Volume (vph)	70	255	7	8	200	92	1	265	21	80	165	61
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.997			0.959			0.990			0.973	
Fit Protected		0.990			0.999						0.987	
Satd. Flow (prot)	0	1826	0	0	1796	0	0	1576	0	0	1543	0
Fit Permitted		0.990			0.999						0.987	
Satd. Flow (perm)	0	1826	0	0	1796	0	0	1576	0	0	1543	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		414.9			458.6			427.7			377.6	
Travel Time (s)		29.9			33.0			30.8			27.2	
Confl. Peds. (#/hr)	1		15	15		1			22	22		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	0%	3%	17%	0%	2%	0%	0%	21%	0%	6%	31%	0%
Adj. Flow (vph)	76	277	8	9	217	100	1	288	23	87	179	66
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	361	0	0	326	0	0	312	0	0	332	0
Sign Control		Stop			Stop			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	79.9%					ICU Level of Service D						
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
107: Irvine St & Colborne St

Total (2031) - Alternative Scenario
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Sign Control		Stop			Stop			Stop			Stop	
Traffic Volume (vph)	70	255	7	8	200	92	1	265	21	80	165	61
Future Volume (vph)	70	255	7	8	200	92	1	265	21	80	165	61
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	76	277	8	9	217	100	1	288	23	87	179	66
Direction, Lane #												
	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	361	326	312	332								
Volume Left (vph)	76	9	1	87								
Volume Right (vph)	8	100	23	66								
Hadj (s)	0.07	-0.16	0.29	0.24								
Departure Headway (s)	7.8	7.7	8.2	8.0								
Degree Utilization, x	0.78	0.70	0.71	0.74								
Capacity (veh/h)	436	429	409	417								
Control Delay (s)	33.3	26.7	28.6	30.8								
Approach Delay (s)	33.3	26.7	28.6	30.8								
Approach LOS	D	D	D	D								
Intersection Summary												
Delay				30.0								
Level of Service	D											
Intersection Capacity Utilization	79.9%				ICU Level of Service				D			
Analysis Period (min)	15											

Lanes, Volumes, Timings
108: East Mill St & Irvine St

Total (2031) - Alternative Scenario
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	189	239	0	0	294	97	0	0	0	87	0	94
Future Volume (vph)	189	239	0	0	294	97	0	0	0	87	0	94
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.967						0.930	
Flt Protected		0.978									0.976	
Satd. Flow (prot)	0	1801	0	0	1671	0	0	1900	0	0	1452	0
Flt Permitted		0.978									0.976	
Satd. Flow (perm)	0	1801	0	0	1671	0	0	1900	0	0	1452	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		497.2			520.7			62.2			427.7	
Travel Time (s)		35.8			37.5			4.5			30.8	
Confl. Peds. (#/hr)			54	54								
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	1%	0%	0%	5%	25%	0%	0%	0%	39%	0%	0%
Adj. Flow (vph)	205	260	0	0	320	105	0	0	0	95	0	102
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	465	0	0	425	0	0	0	0	197	0	
Sign Control		Free			Free			Stop			Stop	
Intersection Summary												
Area Type:	Other											
Control Type:	Unsignalized											
Intersection Capacity Utilization	65.0%				ICU Level of Service C							
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis
108: East Mill St & Irvine St

Total (2031) - Alternative Scenario
PM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR		
Lane Configurations		↔			↔			↔			↔			
Traffic Volume (veh/h)	189	239	0	0	294	97	0	0	0	87	0	94		
Future Volume (Veh/h)	189	239	0	0	294	97	0	0	0	87	0	94		
Sign Control	Free			Free			Stop			Stop				
Grade	0%			0%			0%			0%				
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		
Hourly flow rate (vph)	205	260	0	0	320	105	0	0	0	95	0	102		
Pedestrians												54		
Lane Width (m)												3.6		
Walking Speed (m/s)												1.2		
Percent Blockage												5		
Right turn flare (veh)														
Median type	None			None										
Median storage (veh)														
Upstream signal (m)														
pX, platoon unblocked														
vC, conflicting volume	425				314				1198	1149	314	1042	1096	372
vC1, stage 1 conf vol														
vC2, stage 2 conf vol														
vCu, unblocked vol	425				314				1198	1149	314	1042	1096	372
tC, single (s)	4.2				4.1				7.1	6.5	6.2	7.5	6.5	6.2
tC, 2 stage (s)														
tF (s)	2.3				2.2				3.5	4.0	3.3	3.9	4.0	3.3
p0 queue free %	82				100				100	100	100	35	100	85
cM capacity (veh/h)	1113				1201				110	156	698	147	168	678
Direction, Lane #	EB 1	WB 1	NB 1	SB 1										
Volume Total	465	425	0	197										
Volume Left	205	0	0	95										
Volume Right	0	105	0	102										
eSH	1113	1201	1700	247										
Volume to Capacity	0.18	0.00	0.61	0.80										
Queue Length 95th (m)	5.1	0.0	0.0	45.6										
Control Delay (s)	5.0	0.0	0.0	59.2										
Lane LOS	A		A	F										
Approach Delay (s)	5.0	0.0	0.0	59.2										
Approach LOS			A	F										
Intersection Summary														
Average Delay	12.9													
Intersection Capacity Utilization	65.0%				ICU Level of Service				C					
Analysis Period (min)	15													

Lanes, Volumes, Timings
201: Irvine St & Street A/Elora Sands

Total (2031) - Alternative Scenario
PM Peak Hour

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (vph)	35	0	48	78	0	10	80	142	119	20	163	57
Future Volume (vph)	35	0	48	78	0	10	80	142	119	20	163	57
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.922			0.985			0.953			0.968	
Flt Protected		0.979			0.958			0.988			0.996	
Satd. Flow (prot)	0	1681	0	0	1758	0	0	1754	0	0	1796	0
Flt Permitted		0.979			0.958			0.988			0.996	
Satd. Flow (perm)	0	1681	0	0	1758	0	0	1754	0	0	1796	0
Link Speed (k/h)		50			50			50			50	
Link Distance (m)		127.0			212.9			245.2			205.1	
Travel Time (s)		9.1			15.3			17.7			14.8	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	38	0	52	85	0	11	87	154	129	22	177	62
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	90	0	0	96	0	0	370	0	0	261	0
Sign Control		Stop			Stop			Free			Free	

Intersection Summary	
Area Type:	Other
Control Type:	Unsignalized
Intersection Capacity Utilization	51.7%
ICU Level of Service A	
Analysis Period (min)	15

HCM Unsignalized Intersection Capacity Analysis
201: Irvine St & Street A/Elora Sands

Total (2031) - Alternative Scenario
PM Peak Hour

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔			↔			↔			↔	
Traffic Volume (veh/h)	35	0	48	78	0	10	80	142	119	20	163	57
Future Volume (Veh/h)	35	0	48	78	0	10	80	142	119	20	163	57
Sign Control		Stop			Stop			Free			Free	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	38	0	52	85	0	11	87	154	129	22	177	62
Pedestrians												
Lane Width (m)												
Walking Speed (m/s)												
Percent Blockage												
Right turn flare (veh)												
Median type								None			None	
Median storage (veh)												
Upstream signal (m)												
pX, platoon unblocked												
vC, conflicting volume	656	709	208	696	676	218	239				283	
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	656	709	208	696	676	218	239				283	
tC, single (s)	7.1	6.5	6.2	7.1	6.5	6.2	4.1				4.1	
tC, 2 stage (s)												
tF (s)	3.5	4.0	3.3	3.5	4.0	3.3	2.2				2.2	
p0 queue free %	89	100	94	73	100	99	93				98	
cM capacity (veh/h)	351	330	832	313	345	821	1328				1279	
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total	90	96	370	261								
Volume Left	38	85	87	22								
Volume Right	52	11	129	62								
eSH	527	337	1328	1279								
Volume to Capacity	0.17	0.29	0.07	0.02								
Queue Length 95th (m)	4.6	8.8	1.6	0.4								
Control Delay (s)	13.2	19.9	2.3	0.8								
Lane LOS	B	C	A	A								
Approach Delay (s)	13.2	19.9	2.3	0.8								
Approach LOS	B	C										

Intersection Summary	
Average Delay	5.1
Intersection Capacity Utilization	51.7%
ICU Level of Service A	
Analysis Period (min)	15